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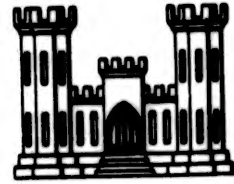
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DEPARTMENT OF THE ARMY
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Technical Report 1717-TR

AN ATTEMPT TO ISOLATE THE
BLAST PHENOMENA WHICH CONTRIBUTE MOST
TO THE EARTH-WALL FAILURE OF EXCAVATIONS

Project 8512-95-001

26 July 1962

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U S Army
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Accession No.
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Distributed by

The Commanding Officer
U. S. Army Engineer Research and Development Laboratories

Prepared by

James E. Stilwell and Andrew J. Romano, Sp/4, U. S. Army
Demolitions and Fortifications Branch
Military Department
U. S. Army Engineer Research and Development Laboratories
Fort Belvoir, Virginia

PREFACE

The test covered by this report was conducted under the authority of Project 8S12-95-001, with funds provided by the Defense Atomic Support Agency. A copy of the project card is included in Appendix A.

The test was performed and the report prepared by James E. Stilwell and Andrew J. Romano. The authors would like to express their appreciation to the Suffield Experimental Station personnel whose assistance and cooperation made this test possible and to the Ballistics Research Laboratories for blast instrumentation and photographic support.

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SUMMARY

This report covers one of the U. S. projects of the 100-ton high-explosive test at the Suffield Experimental Station on 3 August 1961. This project was designated US-5. The objectives were: (1) To obtain an insight into the blast phenomena which contribute most to the failure of earth walls of excavations and (2) to determine the vulnerability of the two-man foxhole to blast considering the soil type existing at the Canadian test site. To accomplish these objectives, 11 test excavations were exposed to the blast at overpressure ranges of 15 psi, 25 psi, and 30 psi.

The report concludes:

- a. Damage to foxholes at the 15-psi overpressure level indicates that the incident blast wave and its reflections in the excavation are primarily responsible for earth-wall damage.
- b. Damage to foxholes at the 25-psi overpressure level indicates that air-induced ground shock may be an important factor in earth-wall failure.
- c. In the soil existing at the Canadian test site, the standard two-man foxhole showed, qualitatively, a high resistance to collapse from peak overpressures ranging up to 30 psi.

AN ATTEMPT TO ISOLATE THE BLAST PHENOMENA WHICH CONTRIBUTE
MOST TO THE EARTH-WALL FAILURE OF EXCAVATIONS

I. INTRODUCTION

1. Subject. This report covers Project US-5 of the 100-ton high-explosive detonation which was conducted at the Suffield Experimental Station, Ralston, Alberta, Canada, on 3 August 1961. This project was sponsored by the Defense Atomic Support Agency and was conducted by the Nuclear Effects Section (formerly Special Projects Branch), Demolitions and Fortifications Branch, U. S. Army Engineer Research and Development Laboratories, Fort Belvoir, Virginia. The objectives of the project were: (a) To obtain an insight into the blast phenomena which contribute most to the failure of earth walls of excavations and (b) to determine the vulnerability of the two-man foxhole to blast, considering the soil type existing at the Canadian test site.

2. Background and Previous Investigation. In several nuclear tests, specific field fortifications were tested to determine their response in various overpressure regions and to determine the protection offered by these fortifications against thermal and nuclear radiation. In these previous tests, however, no attempt was made to determine exactly what blast phenomenon was primarily responsible for fortification failure. An attempt was made during the US-5 test to isolate the various blast phenomena and thus determine the factors that contribute most to earth-wall failure in excavations.

II. INVESTIGATION

3. Description of Test. The standard two-man foxhole was chosen as the basic excavation with which to accomplish the objectives of this test. Simple construction methods were used to vary the blast loading. Including the basic open foxhole configuration (loading not altered), five different designs (11 test excavations) were exposed to the blast from the 100-ton high-explosive detonation. Table I lists each design, its purpose, and the estimated peak over-pressures at which the designs were exposed. Drawings of the designs are presented in Figs. 1 through 5. All designs were exposed with the broadside to ground zero, as indicated in the figures. Figure 6 indicates the arrangement of the excavations at the test site.

Table I. Purposes of Designs and Overpressures at Which They Were Exposed

Design	Purpose	Approximate Overpressure (psi)
A	Basic foxhole configuration	30, 25, and 15
B	Vertical pressure magnification with ground shock	25 and 15
C	Pure ground shock	25 and 15
D	Minimize ground shock	25 and 15
E	Magnification of sudden drawdown and reflected pressure effects	25 and 15

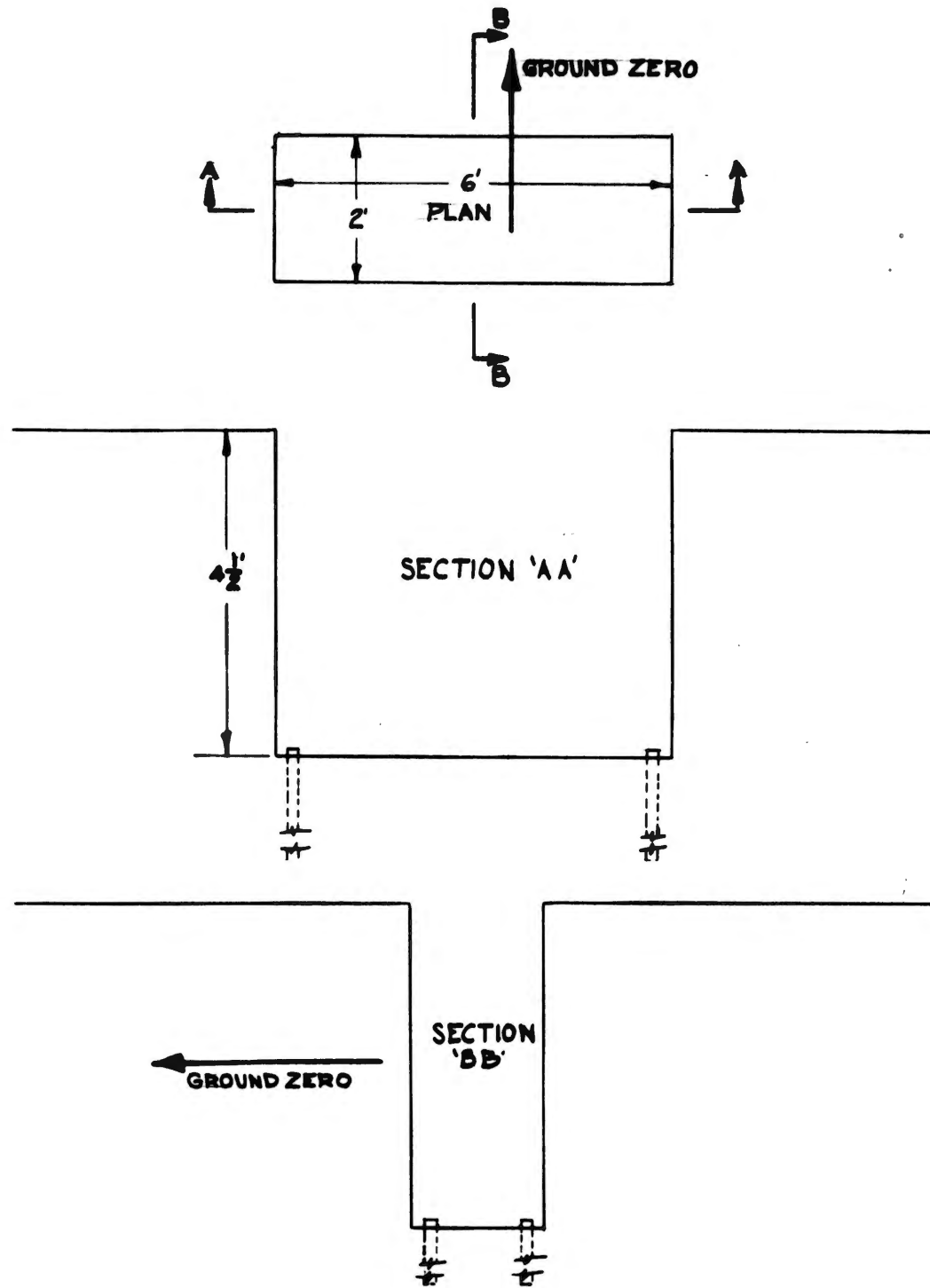
4. Field Procedure. The excavations were dug by hand, and the earth walls were trimmed as smooth as hand tools and soil conditions would permit.

Pre-shot and post-shot wall measurements were made in order to determine the amount of earth-wall sloughing or collapse. Horizontal measurements were made from a plumb line suspended from a straight-edged member positioned horizontally across the excavation. Stakes were driven in the bottom of each excavation at the corners to use as references to position the horizontal member. Measurements were made at 6-inch horizontal and vertical intervals along the walls of the excavations.

Photographs of the excavations were taken before and after the detonation. The end walls and sides were spray painted with yellow and white water-base paint, respectively, to provide better contrast, thus enhancing photographic detail.

After photographs of all the test units had been taken, the timber covers were placed on designs B, C, and D and anchored as shown in Figs. 3, 4, and 5. ●

5. Instrumentation. Ballistics Research Laboratory, self-recording, pressure-time gages were used to obtain measurements of peak overpressure. Two gages (one 360 feet and one 450 feet from ground zero) embedded flush with the ground surface recorded free-field pressure. A third gage was mounted on the bottom of unit 15C and held in place by sandbags.



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Fig. 1. Design A, standard two-man foxhole.

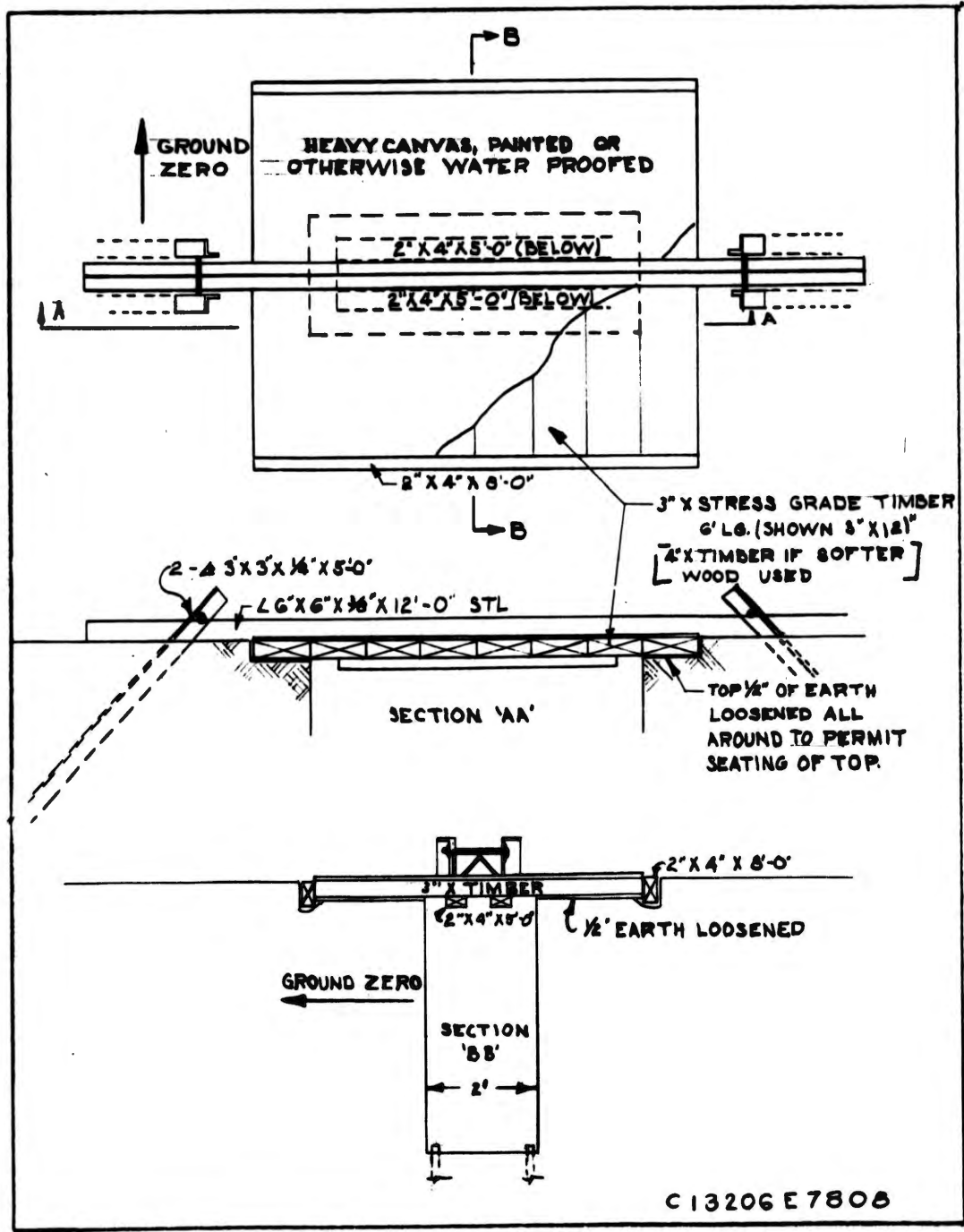


Fig. 2. Design B.

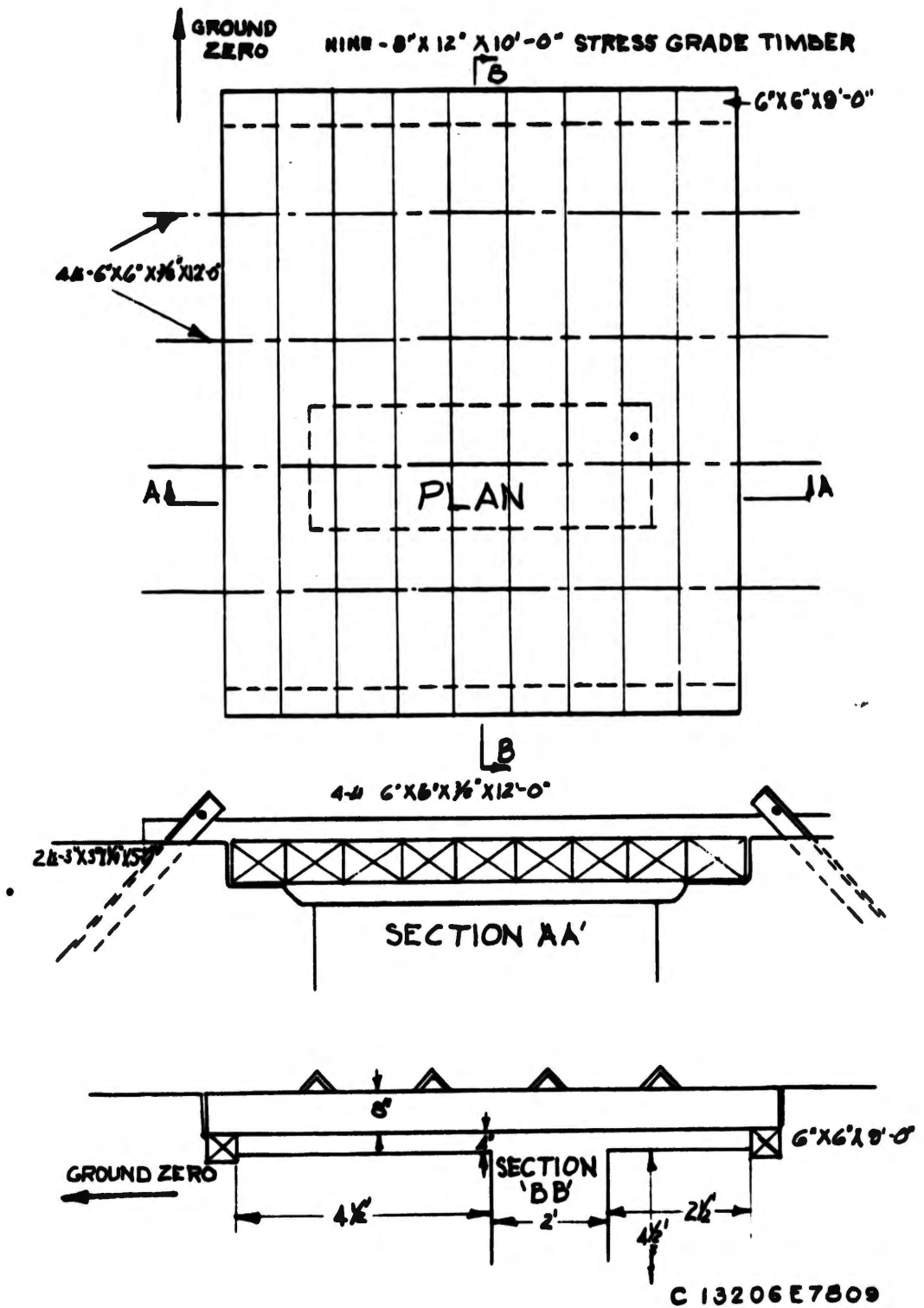
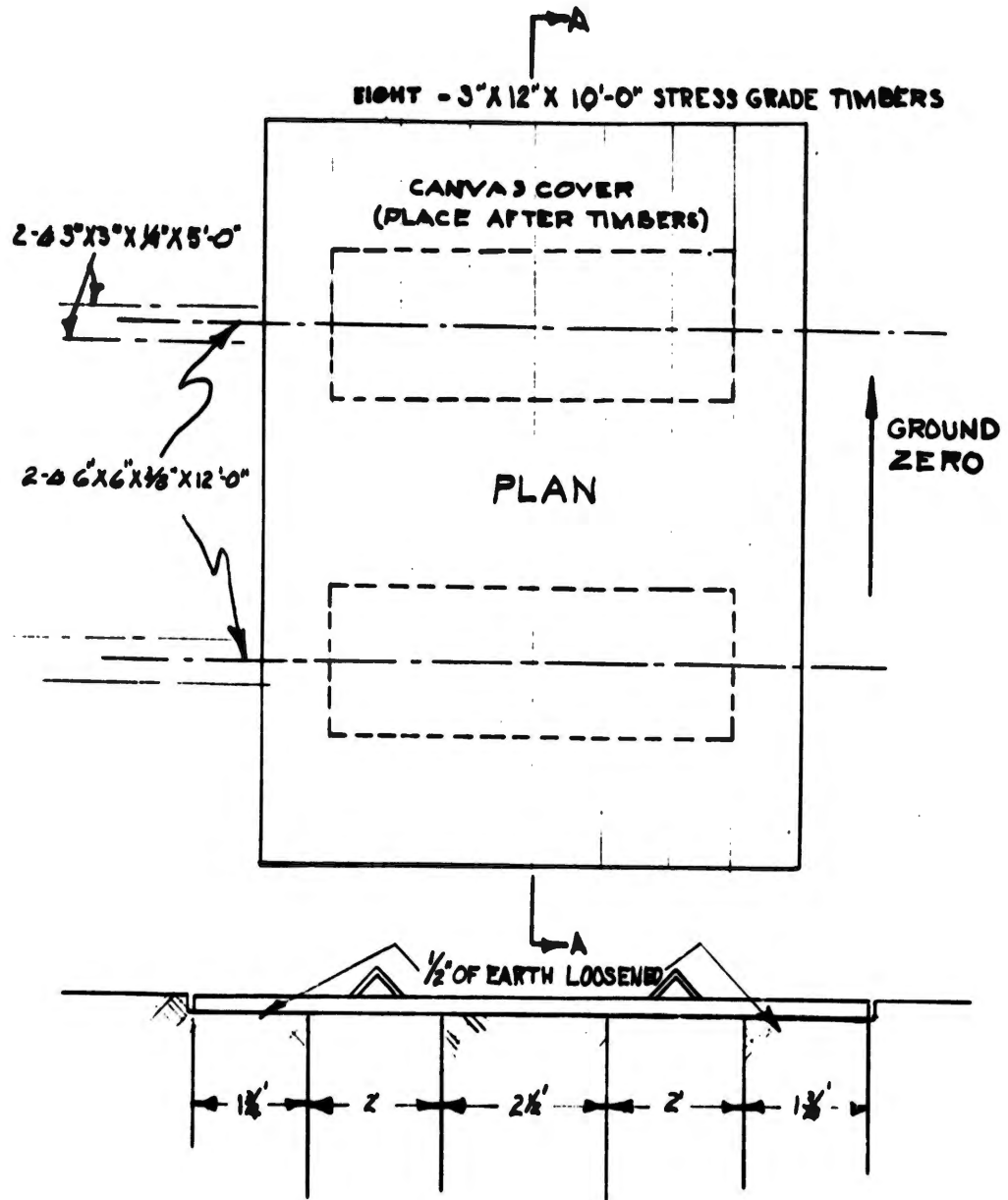
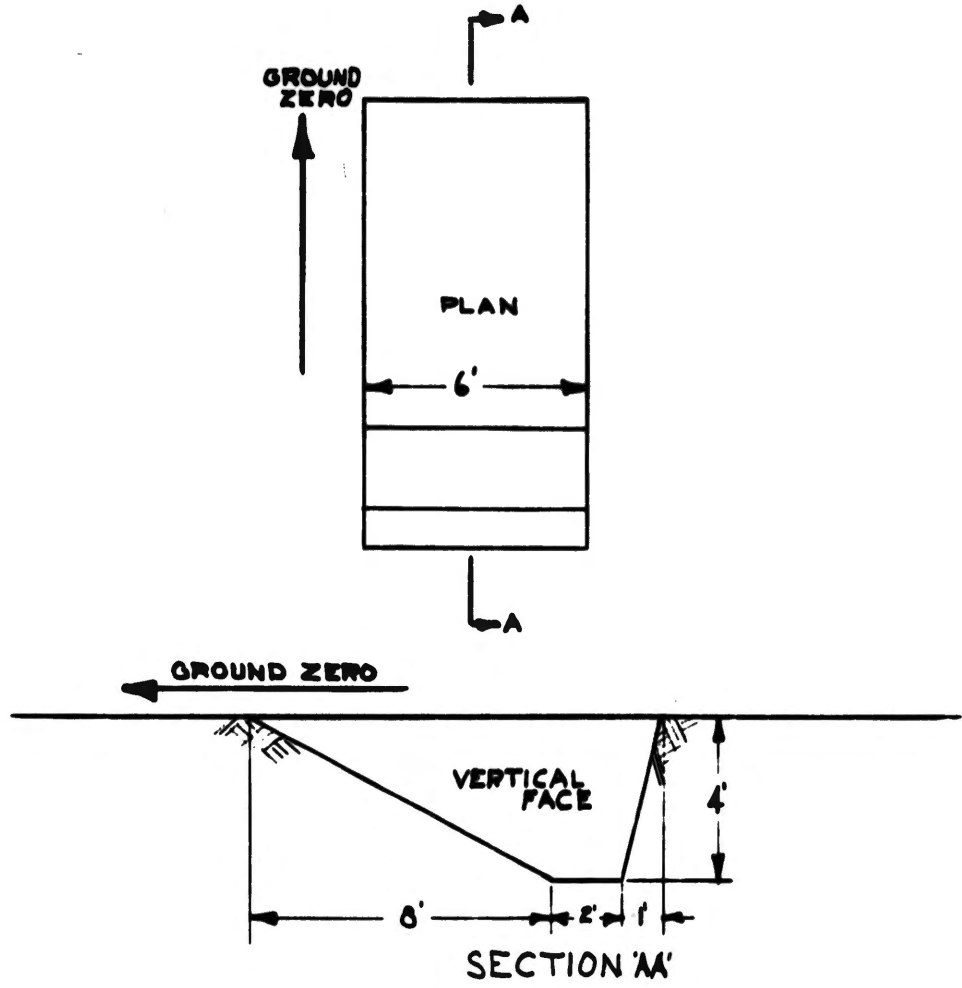


Fig. 3. Design C.



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Fig. 4. Design D.



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Fig. 5. Design E.

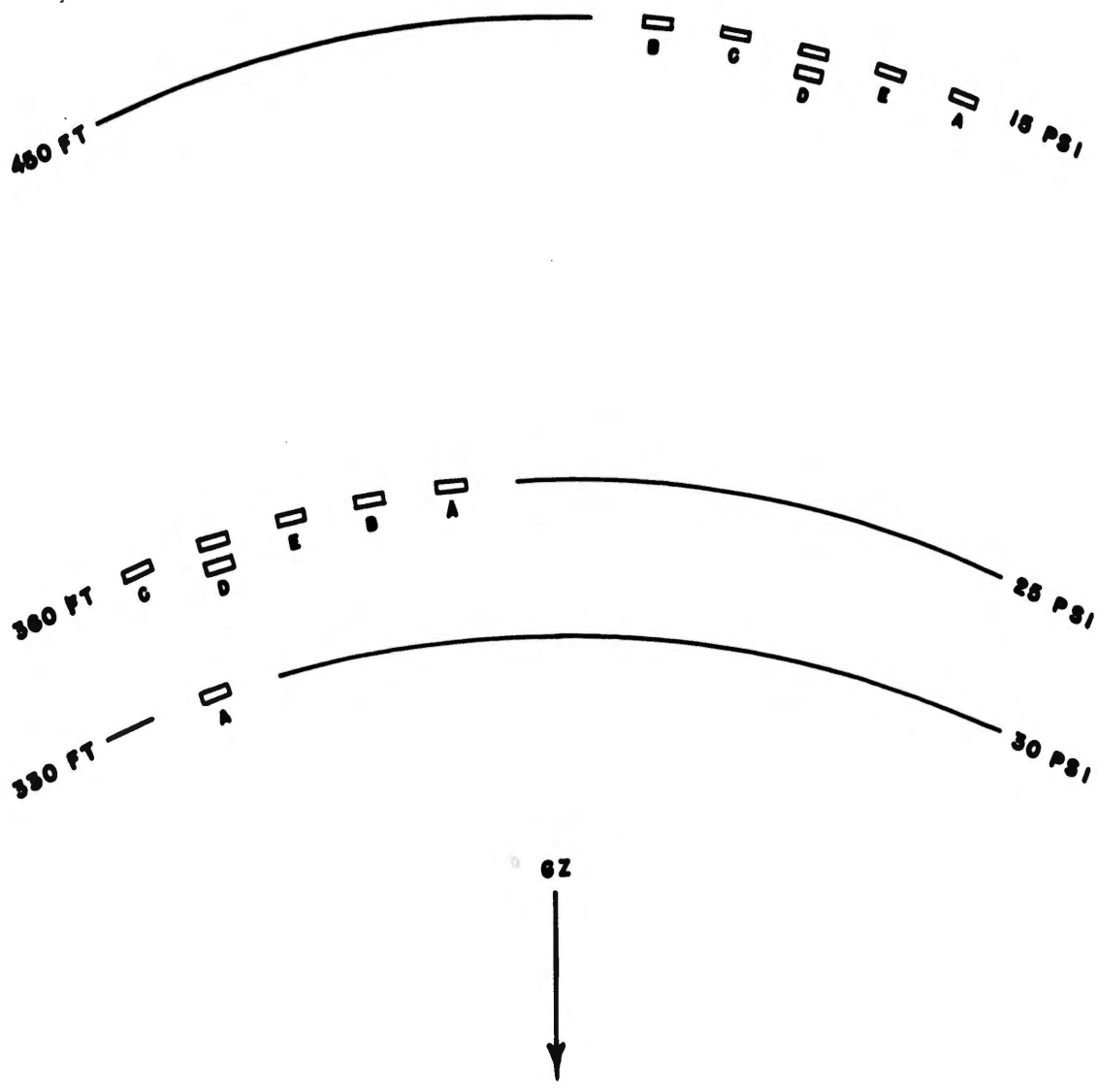


Fig. 6. Arrangement of excavations at test site.

6. Test Results. The peak overpressures recorded by the two surface gages and by the gage placed in unit 15C are presented in Table II.

Table II. Pressure Gage Data

Gage Distance from Ground Zero (ft)	Estimated Overpressure (psi)	Recorded Overpressure (psi)	Duration (ms)
330	30	32.5 ^(a)	--
360	25	27.0	116.2
450	15	17.4	124.5
450 ^(b)	--	2.5	Recorded peak pressure only

- (a) Value based on reading from pressure-distance curves furnished by Ballistics Research Laboratories.
 (b) Gage on bottom of unit 15C.

Of the 11 test units, only one (25C) was severely damaged. Damage to unit 25C consisted of sloughing of the front wall (wall nearer ground zero) and weakening of the rear wall, as shown by cracks at the surface.

Cracks appeared around all the test units located at the 30- and 25-psi ranges and around units 15A and 15E. Test units 15B, 15C, and 15D were almost undamaged, with no cracks forming around the holes at the surface.

Before the detonation, the 3/4-inch bolts (inserted through the ends of the 3-inch angle-irons used as anchor stakes) were in contact with the 6-inch angle-irons, holding them tightly against the timber covers. After the explosion, the bolts were no longer in contact with the angle-irons; a 1.5-inch gap between each bolt and angle-iron was observed in units 25B, 25C, and 25D, while a 0.7-inch gap was observed in units 15B, 15C, and 15D.

Table III contains descriptions of damage sustained by each unit. Figures 7 through 17 show the units before and after detonation.

Figures 18 through 21 show the amounts of deformation (in inches) which occurred in the walls of the foxholes as a result of the detonation. A negative figure indicates a bowing-in of the wall, while a positive figure indicates that the wall sloughed off

Table III. Description of Damage

Design	Damage	
	30 psi	15 psi
A	Front and rear walls bowed in toward center. Cracks around front and rear walls at surface, largest (2 in.) at center, extending 12 in. out. End walls relatively undamaged, only small vertical cracks. (Fig. 7.)	Front and rear walls bowed in toward center. Cracks around front and rear walls in pattern similar to 30A. Largest crack (1 in.) extending 15 in. from edge at center. No damage or signs of weakening of end walls. (Fig. 8.)
B	--	Approx. 1/2-in.-wide cracks at surface around front and rear walls approx. 9 in. from edge at center. No damage to end walls. (Fig. 10.)
C	--	End walls bowed slightly. Small cracks at surface around end walls approx. 6 in. from edge. Front wall sloughed at top to within 1 ft of bottom, rear wall severely weakened. Cracks around rear wall. (Fig. 12.)
D	--	Cracks around side walls of both foxholes, nearly parallel to side walls. Ends undamaged. (Fig. 14.)
E	--	Little sloughing of left wall. Cracks similar to other units at 25-psi range. Crack in Fig. 16 obscured by loose dirt thrown from crater.
		Very slight damage. Cracks similar to 30A and 25A but not as wide. End walls undamaged. (Fig. 9.)
		No damage to unit. No cracks observed. (Fig. 11.)
		No damage to unit. No cracks observed. (Fig. 13.)
		No damage to unit. No cracks observed. (Fig. 15.)
		Slight damage. Cracks similar to 25E. (Fig. 17.)

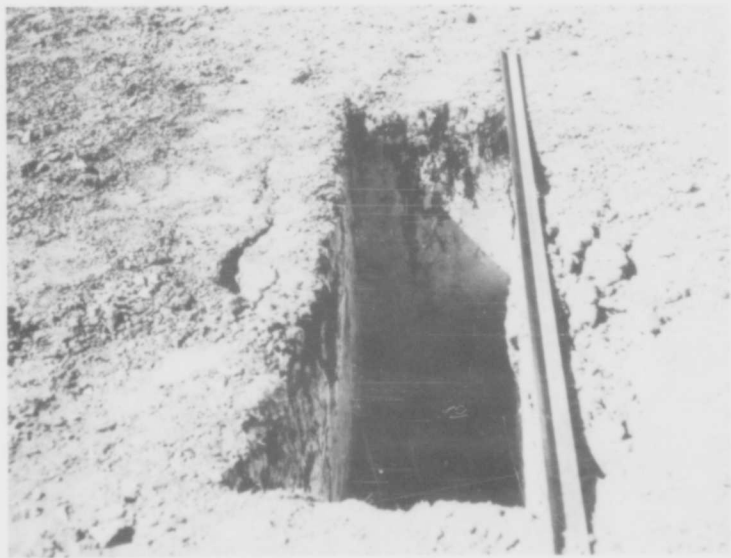


Fig. 7. Unit 30A. Top: pre-shot, ground zero toward top of photograph; bottom: post-shot, ground zero to left.

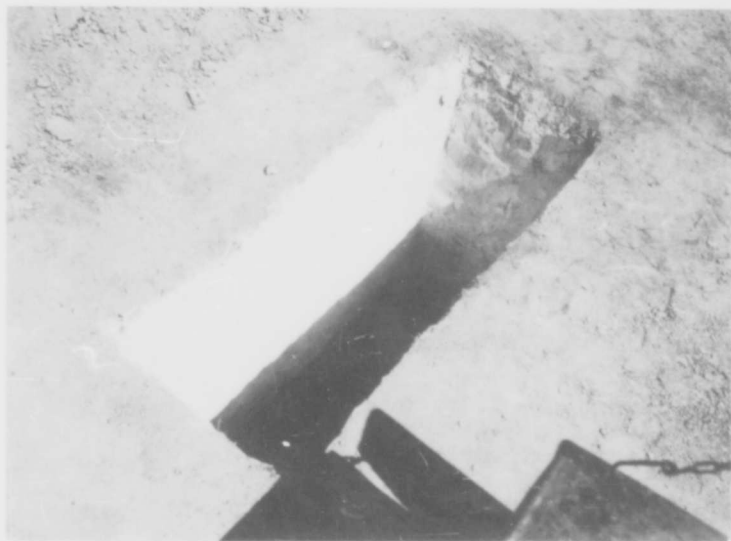


Fig. 8. Unit 25A. Top: pre-shot, ground zero toward right lower corner; bottom: post-shot, ground zero to left (note cracks around side walls and bow in walls).

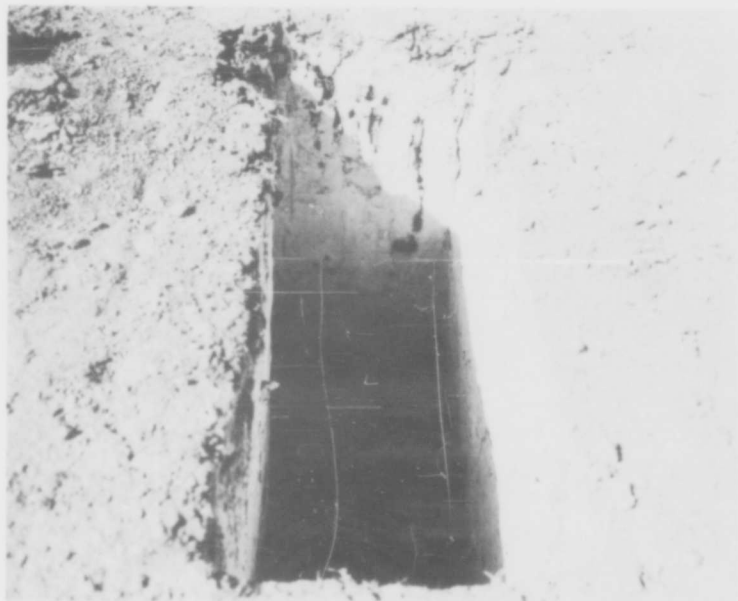
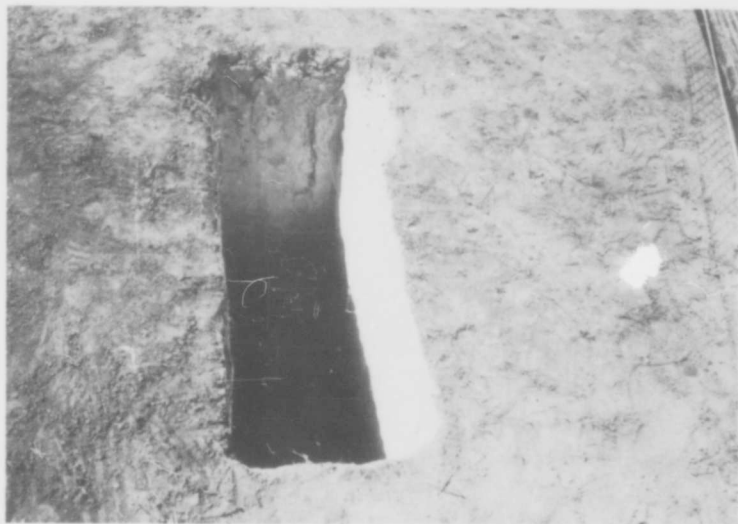


Fig. 9. Unit 15A. Top: pre-shot, ground zero to left;
bottom: post-shot, ground zero to left.



Fig. 10. Unit 25B. Top: pre-shot, ground zero to left;
bottom: post-shot, ground zero to left.



Fig. 11. Unit 15B. Top: pre-shot, ground zero indicated by arrow; bottom: post-shot, ground zero to left.

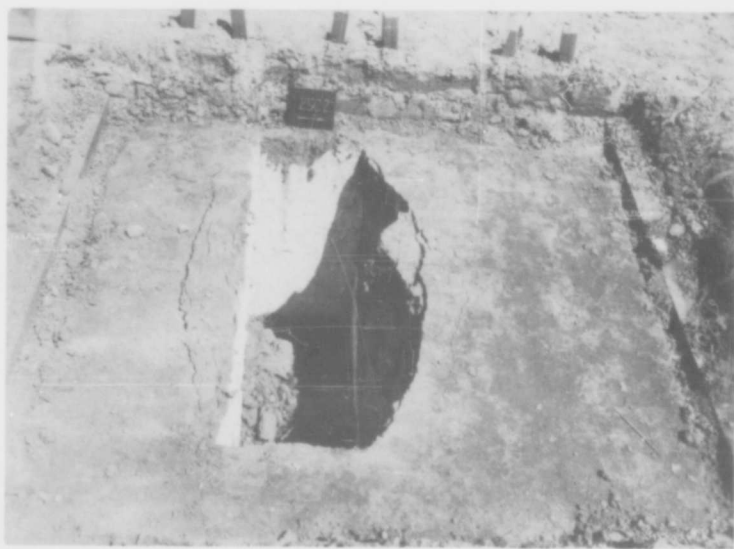
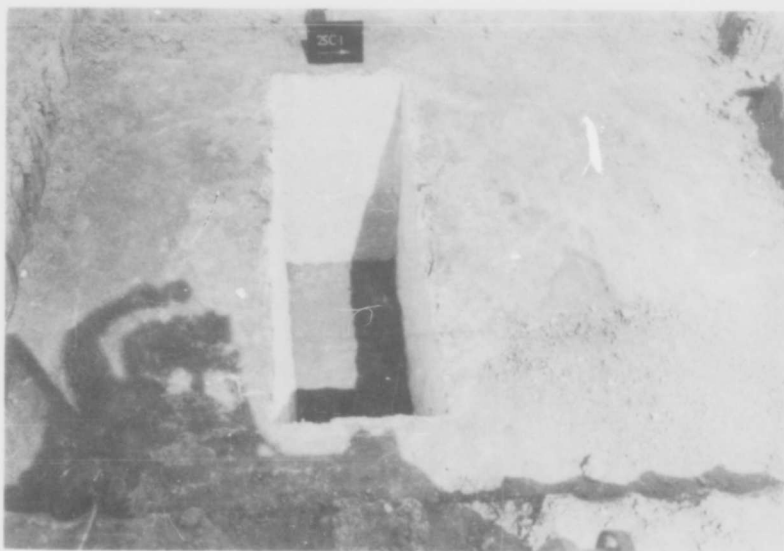


Fig. 12. Unit 25C. Top: pre-shot, ground zero indicated by arrow; bottom: post-shot, ground zero indicated by arrow (on right side of hole, note imprint made by one of the timbers, indicating cover struck ground when blast wave passed).



Fig. 13. Unit 15C. Top: pre-shot, ground zero indicated by arrow; bottom: post-shot, ground zero to left.

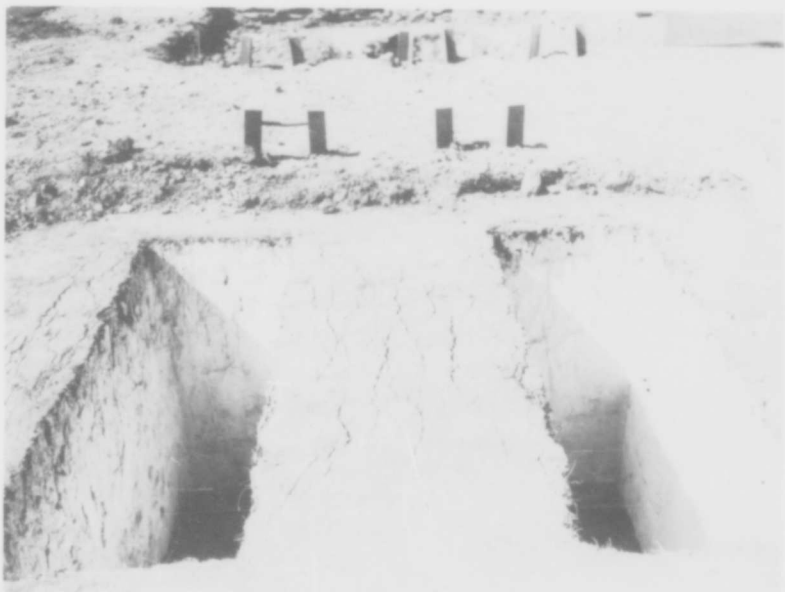
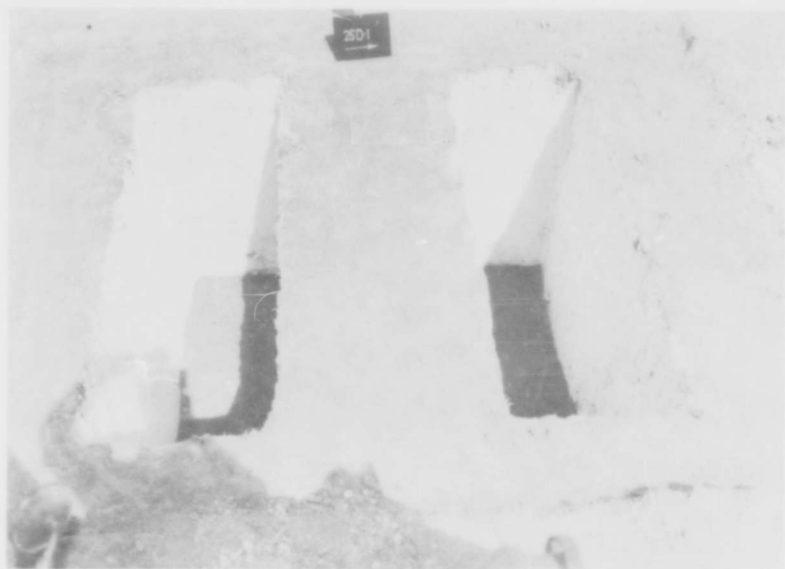


Fig. 14. Unit 25D. Top: pre-shot, ground zero indicated by arrow; bottom: post-shot, ground zero to left.



Fig. 15. Unit 15D. Top: pre-shot, ground zero indicated by arrow; bottom: post-shot, ground zero to left.

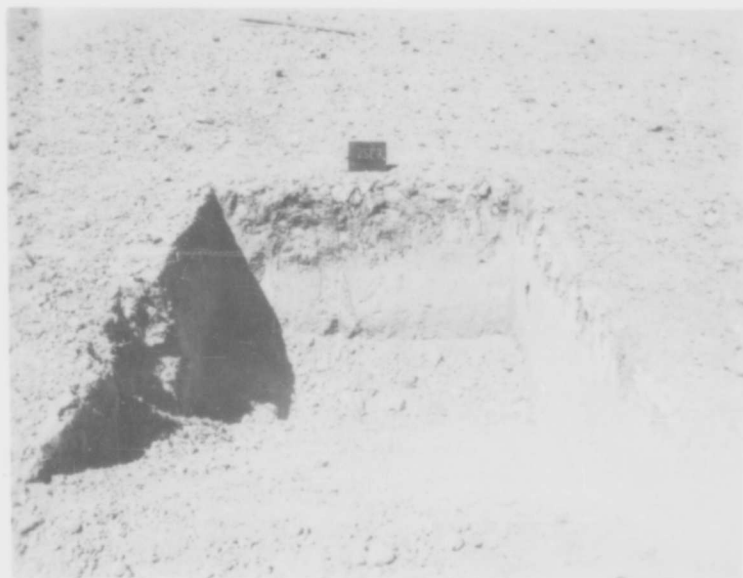
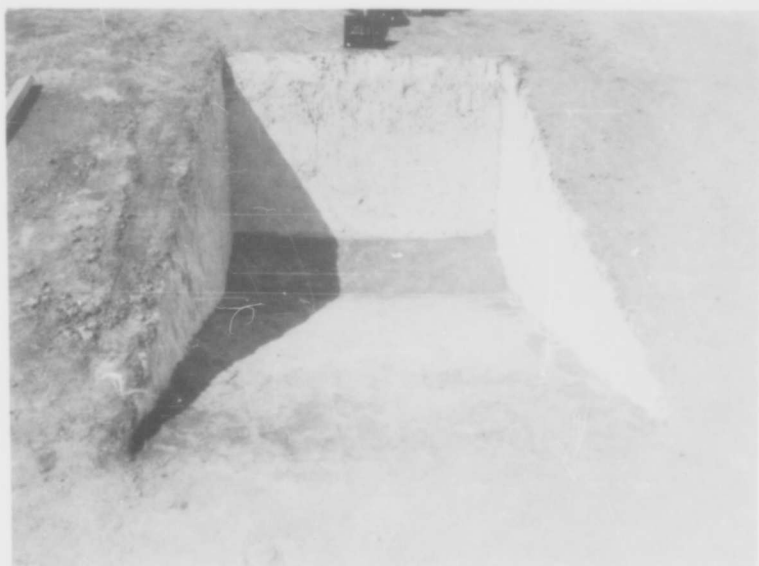


Fig. 16. Unit 25E. Top: pre-shot, ground zero toward bottom of photograph; bottom: post-shot (note sloughing of part of left side wall).



Fig. 17. Unit 15E. Top: pre-shot, ground zero toward bottom of photograph; bottom: post-shot (note cracks around side walls).

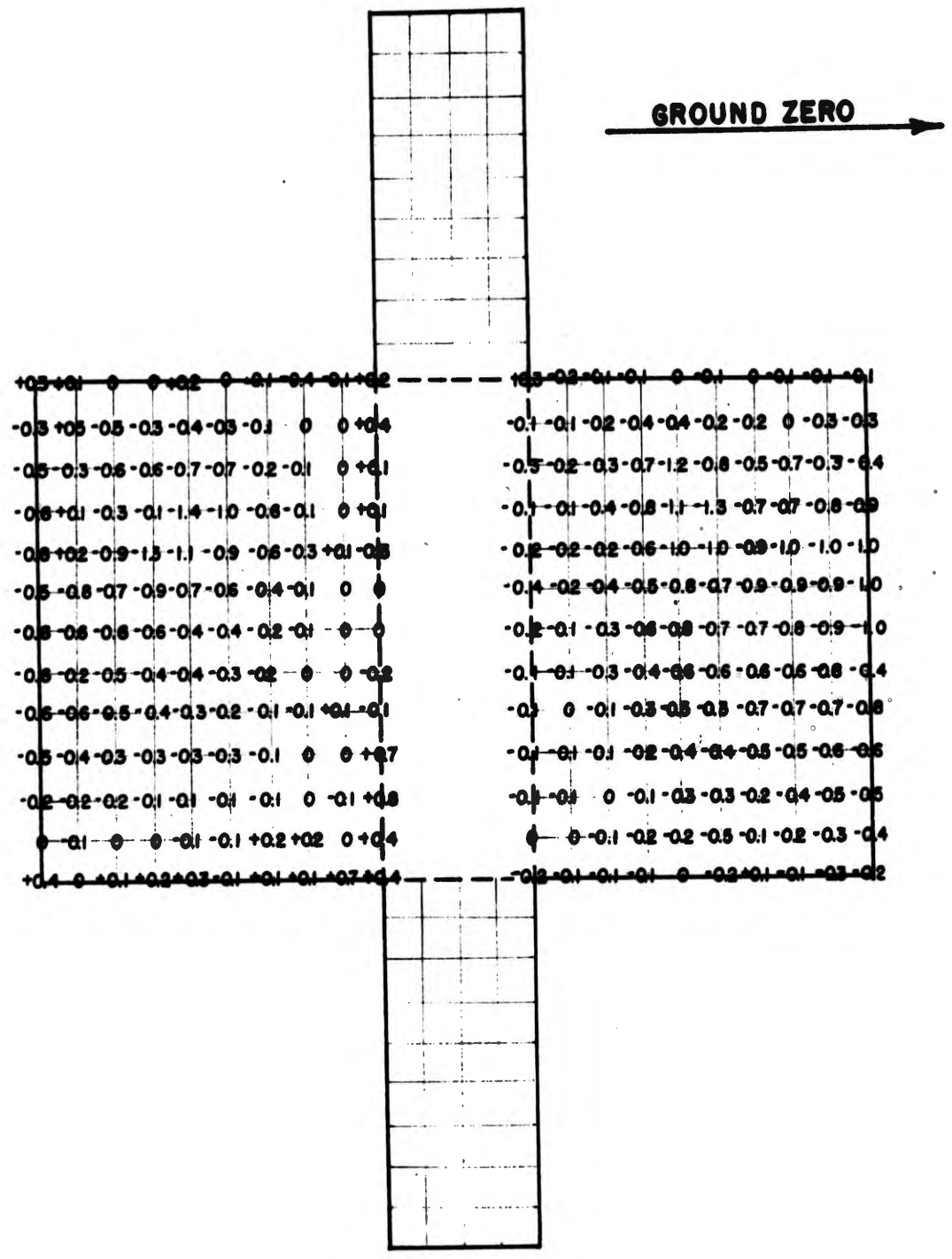


Fig. 19. Difference in pre-shot and post-shot wall measurements of unit 25A.

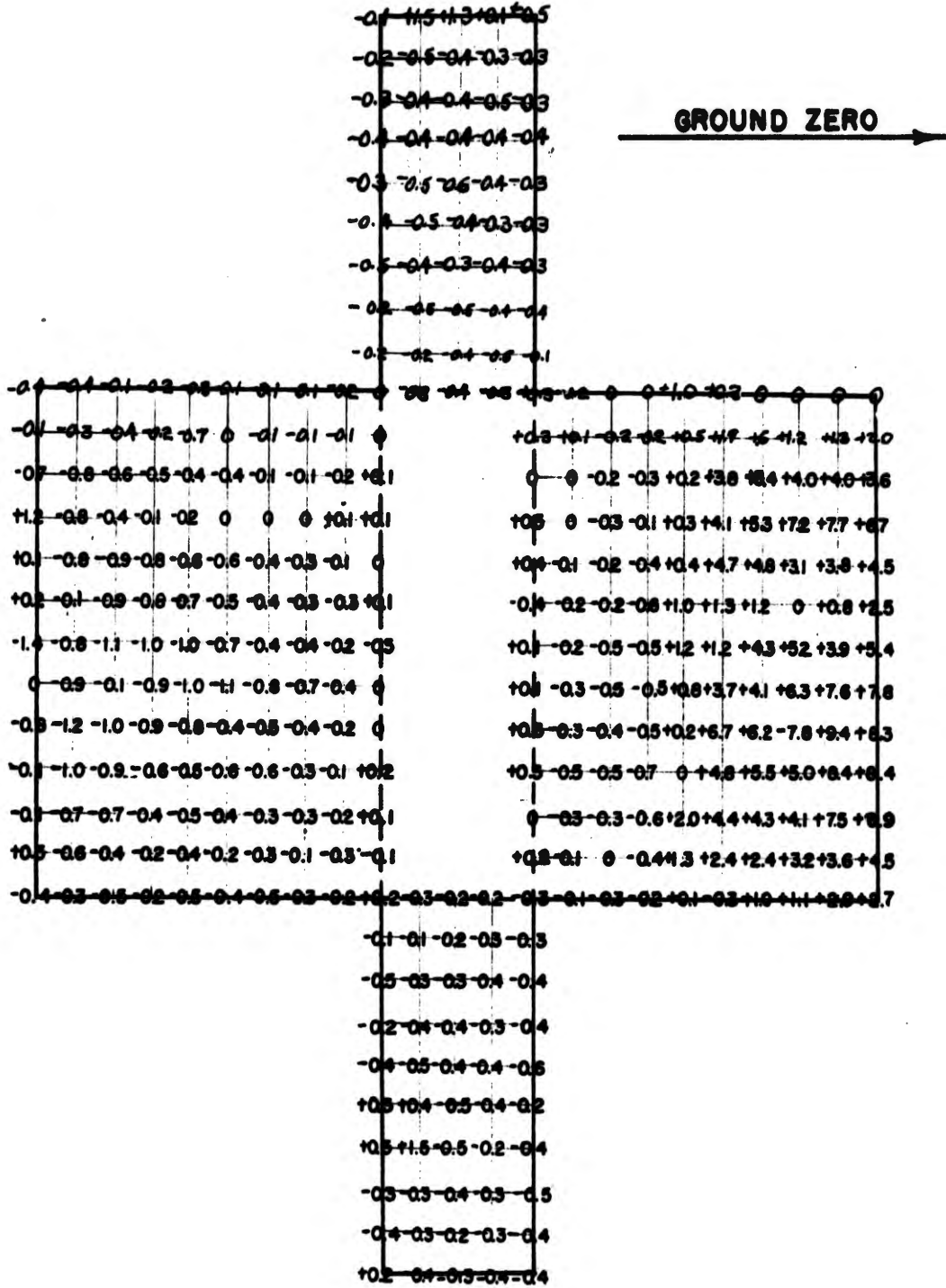


Fig. 20. Difference in pre-shot and post-shot measurements of unit 25C.

into the foxhole. Measurements of wall deformation were not made for some of the units because the magnitude of spalling was only slight; in the case of 25B, the walls bowed to a degree which prevented measurement.

III. DISCUSSION

7. Discussion. This test provided only some indication as to which blast phenomena contributed most to the failure of the walls of excavations. Additional units deployed at equal overpressures would be required to give more conclusive results. Also, the damage to the foxholes was not as severe as was expected.

This test did not evaluate blast damage to earth walls as a function of the variations of soil properties. The soil types and conditions were approximately the same at all of the 11 test units. Information about soil properties was available for the area in the vicinity of ground zero and is included in Appendices B and C.

At a distance of 450 feet from ground zero, the three covered excavations were unaffected, while the two open ones sustained cracks in the front and rear walls. This indicates that the incident air shock wave and its reflections in the cavity are primarily responsible for earth-wall damage.

At a distance of 360 feet from ground zero, the three covered excavations, as well as the two open ones, were cracked in the front and rear walls. Except for unit 25C, the damage to the covered excavations was less than that sustained by the open ones. The failure of unit 25C appears to have been caused by the cover timbers bending downward under the blast overpressure and striking the ground. This may have been due to settlement of the bearing timbers prior to the blast or to defective design of the cover itself. The front wall of the excavation was sloughed away, and the rear wall was severely weakened as shown in Fig. 12. The imprint of one of the 8- by 12-inch timbers can be seen where the cover struck the ground. It was noted that, of all the other test units 360 feet from ground zero which received noticeable damage, the wall nearer the blast was bowed in toward the center of the hole to a greater degree than was the opposite wall. This indicates that air-induced ground shock may be an important factor in earth-wall failure.

The front and rear walls of test unit 30A would have sloughed had it not been for the long roots of the prairie grass which retained the soil (Fig. 7).

IV. CONCLUSIONS

8. Conclusions. It is concluded:

a. Damage to the foxholes at the 15-psi overpressure level indicates that the incident blast wave and its reflections in the excavation are primarily responsible for earth-wall damage.

b. Damage to foxholes at the 25-psi overpressure level indicates that air-induced ground shock may be an important factor in earth-wall failure.

c. In the soil existing at the Canadian test site, the standard two-man foxhole showed, qualitatively, a high resistance to collapse from peak overpressures ranging up to 30 psi.

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APPENDICES

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APPENDIX A

AUTHORITY

Item Nr 1904
CETC Mtg 323

RDT & E PROJECT CARD		1. TYPE OF REPORT <input type="checkbox"/> REPLACES (No. & Date) 8-12-95-400, 31 Dec 59		<input type="checkbox"/> NEW <input type="checkbox"/> FINAL	REPORT CONTROL SYMBOL CICRD-1. (R2)
2. PROJECT TITLE Techniques and Materiel for Engineer Operation in ABC Warfare (U)			3. SECURITY OF PROJECT SECRET		4. PROJECT NO. 8812-95-001
7. BASIC FIELD OR SUBJECT Atomic Warfare			8. SUB FIELD OR SUBJECT SUB GROUP Effects of Nuclear Weapons		9. REPORT DATE 15 May 1961
10a. COGNIZANT AGENCY Corps of Engineers		11a. CONTRACTOR AND/OR GOVERNMENT LABORATORY U. S. Army Engineer Research and Development Laboratories, Fort Belvoir, Virginia		6. CATEGORY AR	
8. DIRECTING AGENCY Mil Sciences Div, R&D, OCE		9. REQUESTING AGENCY Office, Chief of Engineers		5. CONTRACT NUMBER -----	
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20. CDOG; 610; 1110; 1122a		17. PRIORITY 1A		17. BUDGET CODE 5000	
21. SPECIAL CODES		22. REQUIREMENT AND/OR JUSTIFICATION In any future conflict, Engineers will undoubtedly be required to operate in areas where nuclear weapons will be employed by both friendly and enemy forces. Thus, it is mandatory that a program of research and development be carried out to develop the techniques and materiel required to train and equip Engineer troops so that they may more effectively carry out their assigned missions. Some of the more important immediate requirements, as stated in CDOG, are as follows: a. Par 610b "Engineer units will exploit the application of . . . nuclear energy . . ." b. Par 610c(3) "Increased attention must be devoted to expedient and hasty type construction of field fortifications." c. Par 610c(4)(a) Barriers: CONFIDENTIAL d. Par 610c(4)(b) "Engineer troops or engineer equipment must assist . . . in breaching barriers of all kinds . . . (and) . . . to assist in decontamination of critical areas."			
DD FORM 613 FEB 60		PREVIOUS EDITIONS ARE OBSOLETE.		PAGE 1 OF 3 PAGES	

RDT & E PROJECT CARD CONTINUATION	REPORT DATE 15 May 1961	PROJECT NO. 8812-95-001
<p>22. Requirement and/or Justification (cont'd):</p>		
<p>e. Par 1110c(1)(c): SECRET</p>		
<p>f. Par 1110c(2)(f) and (g): SECRET</p>		
<p>g. Par 1122u: CDOG Study 61-1 <u>Army Requirements for Nuclear Weapons Effects Research</u> contains additional research requirements as follows: Annex B, par 2b(9); Annex D, par 3a(1); Annex D, par 3a(3).</p>		
<p>23. Brief of Project and Objective:</p>		
<p>a. Brief:</p>		
<p>The capability of nuclear weapons to move land masses and to destroy huge targets so revolutionizes Engineer activities, both in kind and in magnitude, that it is essential that accurate data on weapons effects be available to the Engineer planner. It is the aim of this project to study these effects in light of Engineer responsibilities:</p>		
<p>(1) Objectives:</p>		
<p>The principal objectives are:</p>		
<p>(a) Develop methods and techniques that will permit quick entry into, and effective operations in, areas devastated and contaminated by nuclear weapon detonations.</p>		
<p>(b) Devise methods of utilizing atomic demolitions for engineering purposes.</p>		
<p>(c) Develop means of protecting troops and materiel from nuclear weapons effects.</p>		
<p>(2) Military Characteristics: Not applicable.</p>		
<p>DD FORM 613c FEB 60</p> <p>REPLACES DD FORM 613-1, WHICH IS OBSOLETE.</p> <p>PAGE 2 OF 3 PAGES</p>		

RDT & E PROJECT CARD CONTINUATION	REPORT DATE 15 May 1961	PROJECT NO. 8S12-95-001
23. Brief of Project and Objective (Cont'd):		
b. Approach:		
<p>It is planned to approach this problem along concurrent theoretical and empirical lines. This will be accomplished by theoretical studies in physical sciences, analysis of existing test data, testing with high explosives, testing with radioactive isotopes, and participation in full scale nuclear tests whenever possible. Specific avenues of approach, as indicated on the attached task cards, are taken depending on the requirement to be met.</p>		
c. Tasks:		
<p>The following tasks will be carried out under this project:</p>		
8S12-95-001-01	Engineer Studies and Investigations	
8S12-95-001-02	Theory of Application of Atomic Demolition Munitions Effects	
8S12-95-001-03	Radiation Shielding Studies of Engineer Equipment and Field Protective Works	
8S12-95-001-04	Radioactive Barrier Breaching Methods and Equipment	
d. Other Information: None		
DD FORM 613c <small>1 FEB 60</small>		
<small>REPLACES DD FORM 613-1, WHICH IS OBSOLETE.</small>		
<small>PAGE 3 OF 3 PAGES</small>		

APPENDIX BPROPERTIES OF SOIL AT GROUND ZERO. 100-TON
TRIAL AT SUFFIELD EXPERIMENTAL STATION

Property	Result
Plastic Limit (%)	23.93
Liquid Limit (%)	45.95
Plasticity Index	22.02
Toughness Index	217.50
Shrinkage Limit (%)	14.05
Shrinkage Ratio	1.80
Shrinkage Index (%)	9.88
Specific Gravity	2.71
Percent Finer than No. 200 Sieve	98.86
Moisture Content (%)	13.32
Unified Soil Classification	CL

NOTE: Analysis run by Suffield Experimental Station personnel.

APPENDIX C

SOIL ANALYSES DATA FOR VARIOUS LOCATIONS OF TEST SITE

Distance From Ground Zero (ft)	Depth (ft)	Sample Description	Natural Water Content (%)
0	0	Clay (CL), very stiff, tan	12.1
0	1	Silty-clay (CH), very stiff, tan	13.4
0	2	Clay (CL), medium, tan	8.2
0	3	Clay (CL), medium, tan	7.7
0	4	Clay (CL), medium, tan	9.6
50	0	Silty-clay (CL), medium, tan	15.2
60	0	Silty-clay (CL), soft, tan	13.6
70	1	Silty-clay (CL), medium, tan	11.1
80	5	Silty-clay (CL), medium, tan	11.5

NOTE: Analysis run by Waterways Experiment Station.

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