

Strain Distribution Around Underground Openings

Strain Distribution Around Underground Openings

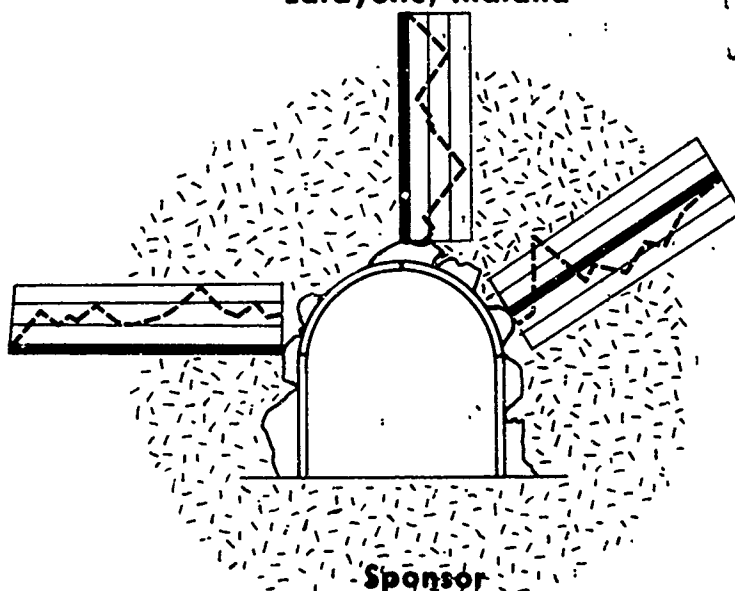
Technical Report No. 7

FINITE ELEMENT ANALYSIS OF JOINTED SYSTEMS

W. H. Perloff

**Soil and Rock Mechanics Area
School of Civil Engineering
Purdue University
Lafayette, Indiana**

18 1972



**ADVANCED RESEARCH PROJECTS AGENCY
DEPARTMENT OF DEFENSE**

MAY, 1972

DISTRIBUTION STATEMENT A
Approved for public release;
Distribution Unlimited

Reproduced by
**NATIONAL TECHNICAL
INFORMATION SERVICE**
U S Department of Commerce
Springfield VA 22151

Prepared For
OFFICE OF THE CHIEF OF ENGINEERS
DEPARTMENT OF THE ARMY
WASHINGTON, D.C.

152 R

Strain Distribution Around Underground Openings

Technical Report No. 7

FINITE ELEMENT ANALYSIS OF
JOINTED SYSTEMS

W. H. Perloff

Soil Rock Mechanics Area
School of Civil Engineering
Purdue University
Lafayette, Indiana

Contract No. DACA 73-68-C-0002(P002)

Sponsor

ADVANCED RESEARCH PROJECTS AGENCY
DEPARTMENT OF DEFENSE

May , 1972

Prepared For

OFFICE OF THE CHIEF OF ENGINEERS
DEPARTMENT OF THE ARMY
WASHINGTON, D.C.

Distribution of this document is unlimited

SUMMARY

This report describes progress on the analytical portion of project number DACA-73-68-C-002 for the period 1 April 1969 to 31 December , 1971.

The static SLAM finite element code was extended to include jointed systems with elastic-plastic mechanical characteristics satisfying a variety of possible yield criteria. The results indicated that

1. The two-dimensional static SLAM code for plane jointed systems could predict the response of such systems to imposed loadings when the system and loadings were properly characterized.
2. Predictions of displacements around excavations in natural jointed rock masses deviated from measured values. Reasons for this included both difficulty in correctly determining displacements in rock masses in the field as well as discrepancies between the simplified representation of natural conditions and the conditions themselves.
3. It was not practicable at the present time to carry out a three-dimensional finite element representation of excavations in natural jointed rock.

TABLE OF CONTENTS

	Page
SUMMARY	ii
TABLE OF CONTENTS	iii
LIST OF FIGURES	v
SECTION 1 - INTRODUCTION	1
1.1 - Introduction	1
1.2 - Outline of Progress	1
SECTION 2 - MODIFICATION OF STATIC SLAM CODE TO INCORPORATE JOINTS	2
2.1 - Review of Static SLAM Code	2
Revised solution procedure	2
2.2 - Behavior of Joints	4
2.3 - Joint Elements	6
2.4 - Constitutive Laws for Joint Elements	8
2.5 - Revised SLAM Code	10
2.6 - Interpolation Code for MPBX Displacements	10
SECTION 3 - RESULTS OF ANALYSES OF JOINTED SYSTEMS	11
3.1 - Introduction	11
3.2 - MRDL Jointed Model Tests	11
Analysis performed	12
Results	13
3.3 - Analysis of Straight Creek Pilot Bore	14
Description of the tunnel	14
Analysis performed	15
Results	16
SECTION 4 - CONSIDERATION OF THREE-DIMENSIONAL EFFECTS	19
SECTION 5 - CONCLUSIONS	21
REFERENCES	23
APPENDIX A - STATIC SLAM CODE FOR JOINTED SYSTEMS	A-1
A.1 - Code Description	A-1
A.2 - Data Deck Setup	A-1
A.3 - Listing of Code	A-9

TABLE OF CONTENTS (CONT'D)

	Page
APPENDIX B - INTERPOLATION CODE FOR DETERMINING MPBX DISPLACEMENTS	B-1
B.1 - Code Description	B-1
B.2 - Data Deck Setup	B-1
B.3 - Output	B-3
B.4 - Listing of Code	B-4

LIST OF FIGURES

Figure	Follows Page
2.1 Shear Stress-Shear Strain Relation for Two-Dimensional Mohr-Coulomb Material	5
2.2 Yield Criteria for Joint Elements	5
2.3 Rectangular Element	6
3.1 Schematic View of MRDL Jointed Block Model	11
3.2 Uniaxial Stress-Strain Relation for Vibrated Model Material	12
3.3 Mohr Envelopes for Ultimate Failure of Intact Vibrated Model Material Specimens from Triaxial and Direct Shear Tests	12
3.4 Mohr Envelope for Initial Slip along Joints from Triaxial and Direct shear Tests	12
3.5 Finite Element Mesh for MRDL Jointed Model	12
3.6 Comparison Between Displacement Along Joints Measured in MRDL Model That from Finite Element Analysis . .	13
3.7 Average Failure Envelope for Granite and Meta- morphitic Rocks at Straight Creek	15
3.8 Finite Element Mesh for Straight Creek Pilot Bore with Two Joint Systems	15
3.9 Comparison Between Measured and Predicted Dis- placements at Sta 114+53 at Straight Creek Pilot Bore.	16
A.1 Overlay Structure of SLAM Code	A-1
A.2 Coordinate Directions for Equivalent Roller Support .	A-8

SECTION 1 - INTRODUCTION

1.1 - Introduction

This report describes the analytical portion of project no. DACA-73-68-C-0002 for the period 1 April, 1969 to 31 December, 1971. As a part of the total effort to determine if the element method analysis will permit prediction of safe spans for existing and proposed underground openings, it was the objective of this portion of the project to extend the continuum model for underground openings, employing the finite element method to consideration of the rock mass as a two-dimensional jointed medium, in order to improve prediction of stresses around and displacement within underground openings. Three-dimensional effects were considered also.

1.2 - Outline of Progress

Accomplishment of this objective is described in the following sections. These sections consider the following areas of progress:

1. Modification of the existing continuum static SLAM Finite Element Code to incorporate direct consideration of joints and joint systems normal to the plane problem.
2. Comparison of analysis with experiments conducted on a small scale jointed model of a simulated rock system.
3. Results of analysis of an underground opening considering the influence of the observed joint systems, and preliminary comparisons with measured displacements.
4. Consideration of extension to three dimensions.

SECTION 2 - MODIFICATION OF STATIC SLAM CODE TO INCORPORATE JOINTS

2.1 - Review of Static SLAM Code

The static SLAM (Stresses In Layered Arbitrary Media) Code was developed for finite element analysis of large systems of continuous media. An outgrowth of the code for dynamic problems developed at IITRI by Costantino (1966, 1968) and Wachowski and Costantino (1966), its salient features are described by Perloff (1969).

The Static SLAM Code is characterized by a number of features which distinguish it from other available codes. These include:

1. The code contains an algorithm for renumbering the nodes so that the minimum band width of non-zero terms within the stiffness matrix results. This leads to an efficient operation, especially for large problems. Furthermore the user is able to number node points arbitrarily. Details are discussed by Wachowski and Costantino (1966).
2. A non-linear displacement field is assumed for rectangular elements, so that where rectangular elements can be incorporated in the geometry, fewer elements are required to represent the problem (Costantino, 1966).
3. The constitutive laws used in the code are contained within a material "catalog" and new constitutive relations can be added without modifying the basic code.

Revised solution procedure

Two revisions have been incorporated in the solution procedure for the code to reduce computer time:

1. The initial elastic solution is obtained by direct elimination of the node point equilibrium equations, rather than by iteration as in the earlier version of the code (Perloff, 1969).
2. An over-relaxation factor is incorporated in the iterative procedure for determining the node point displacements when a structure or continuum is behaving in a non-linear fashion. When it is determined that the yield point has been exceeded in one or more elements, the applied boundary loadings and displacements are reduced until all elements are acting in the elastic range. The remaining nonlinear part of the solution is carried out in a series of small steps, by increasing the applied loads or displacements in increment until the final loading condition is reached. At each increment the node point displacements and loads are determined and added to those from the previous increment. For each nonlinear increment the initial trial solution for the iteration is the displacement field obtained from the previous increment. For the first nonlinear increment, the elastic solution is used as the initial trial solution.

The system equilibrium equations for the nonlinear increment at each node are:

$$[K]\{\Delta U\} = \{\Delta R\} + \{\Delta R^N\} \quad (2.1)$$

in which $[K]$ is the stiffness matrix of the continuum composed of the assembled elements, calculated by adding the stiffnesses of all elements in the system, $\{\Delta U\}$ are the node point displacements, $\{\Delta R\}$ are the applied node point loads and $\{\Delta R^N\}$ are the incremental nonlinear correction terms in the applied node point loads. The error at each

stage of the iteration process is then

$$\{\Delta\}^i = [K]\{\Delta U\}^i - \{\Delta R\} - \{\Delta R^N\} \quad (2.2)$$

in which the superscript i indicates the i^{th} iteration. The displacement increment for the $(i+1)$ increment is then

$$\{\Delta U\}^{i+1} = \{\Delta U\}^i - \alpha \left\{ \frac{\Delta}{K_m} \right\}^i \quad (2.3)$$

where K_m is the main diagonal stiffness at node m , and α is the over-relaxation factor. The iteration process is carried out until the specified allowable error is reached at each node.

2.2 - Behavior of Joints

Most natural rock contains more or less planar surfaces across which the rock has separated at some time in the past. Such defects, called joints, commonly occur as approximately parallel multiple surfaces spaced from fractions of an inch to many feet apart. Systems of joints frequently intersect so that a large rock mass may contain many such families at various spacings and orientations. It is generally recognized that the mechanical behavior of masses of rock is influenced strongly by the presence of such joint systems, along with other geologic defects; and this has been demonstrated by field observations and laboratory experiments (Obert, 1967; Rosenblad, 1971).

Joints may be clean surfaces of separation, or they may be filled with a variety of materials. Sometimes joints contain precipitates, such as calcite or chlorite, which may have a strength approximately the same as that of the natural rock and which may serve as cementing agents to impart tensile resistance normal to the joint. Other filling materials such as clays, lead to joints which are much weaker than the intact rock. In the

case of unfilled joints, the rock on either side of the joint is frequently altered to a weaker, or less stiff form by chemical and/or mechanical action.

Natural joint surfaces are rarely smooth. Even when they are approximately planar, they contain asperities which impart roughness to the joint. The role of these asperities in the shearing resistance along joints is a function of the magnitude of the pressure normal to the joints (Patton, 1966).

Because of the approximately planar nature of most joint systems it is useful to describe the mechanical behavior of joints in terms of stresses and displacements normal to and parallel to the joint surface.

The relationship between the average shear stress applied to a joint and the shear displacement, or strain, corresponding to a given normal pressure can be idealized as shown in Figure 2.1. That is, the joint deforms in a more or less linear way until the yield, or peak strength is reached. Further displacement occurs at a shear stress magnitude equal to that of the residual strength. The residual strength is usually equal to or less than the peak value.

The magnitude of the peak strength has been commonly described in terms of the normal stress on the joint by the conventional two-dimensional Mohr-Coulomb criterion

$$\tau_f = c + \sigma_f \tan \phi \quad (2.4)$$

in which τ_f is the shear stress on the joint at failure, c is the magnitude of the peak stress at zero normal pressure on the joint, σ_f is the normal pressure acting on the joint, and $\tan \phi$ is the slope of the shear strength envelope illustrated in Figure 2.2a. Recent evidence, (Patton, 1966; Rosenblad, 1971) suggests that a bilinear relation for the peak stress,

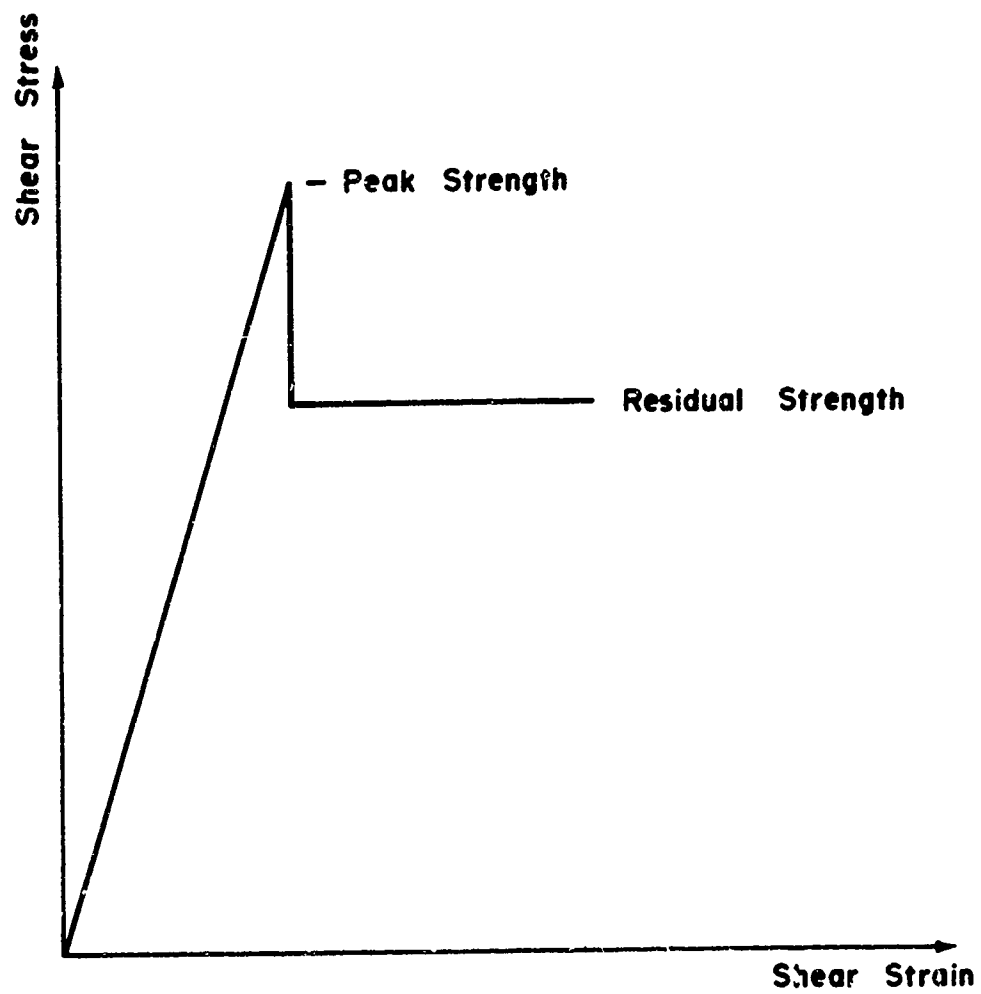
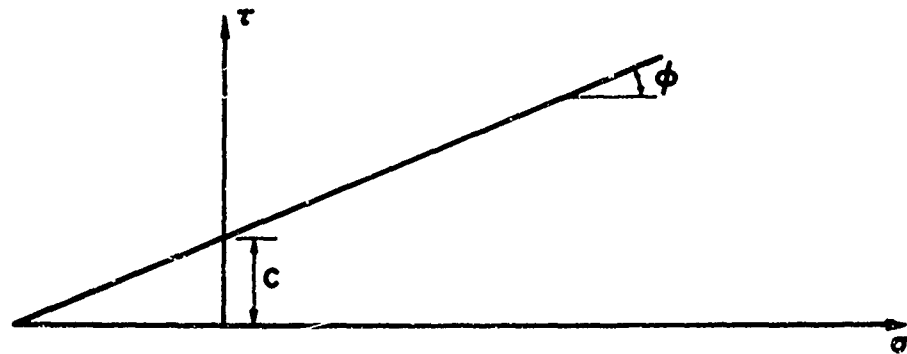
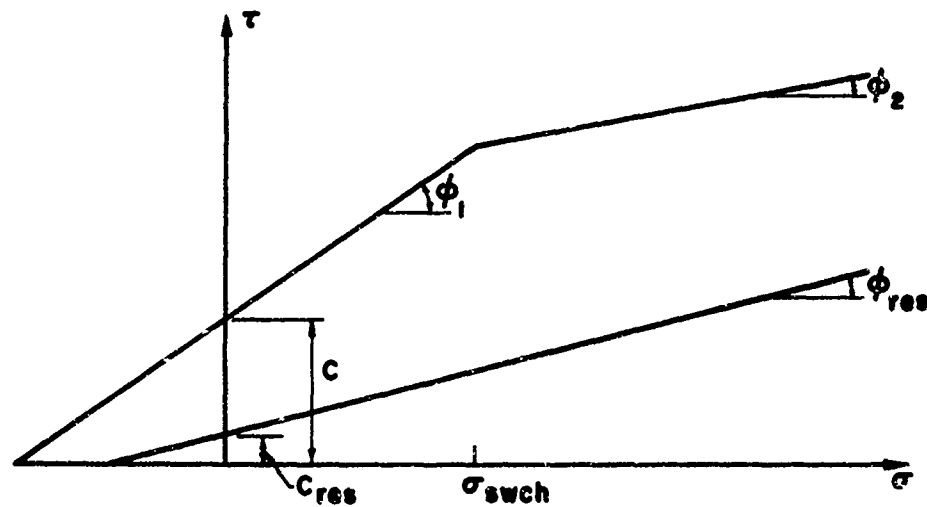


Figure 2.1 - Shear Stress - Shear Strain Relation for Two - Dimensional Mohr - Coulomb Material



a) - Linear Mohr - Coulomb Criterion for Peak Strength



b) - Bilinear Mohr - Coulomb Criterion for Peak Strength
with Lower Residual Strength

Figure 2.2 - Yield Criteria for Joint Elements

as illustrated in Figure 2.2b is more appropriate.

$$\tau_f = c + \sigma_f \tan \phi_1, \quad \sigma_f \leq \sigma_{swch} \quad (2.5a)$$

$$\tau_f = c + \sigma_{swch} \tan \phi_1 + (\sigma_f - \sigma_{swch}) \tan \phi_2, \quad \sigma_f > \sigma_{swch} \quad (2.5b)$$

in which σ_{swch} is the normal stress at which the bilinear failure envelope changes slope.

The residual shear strength relation, also shown in Figure 2.2b is

$$\tau_{f_{res}} = c_{res} + \sigma_f \tan \phi_{res} \quad (2.6)$$

in which the subscript (res) denotes the residual shear strength parameters.

While Figure 2.1 and Equations 2.5 and 2.6 constitute a somewhat idealized depiction of the observed behavior of natural and artificially created joints, the difficulty of testing insitu joint behavior and the variability of results (Goodman, 1969) suggest that the above description is sufficiently detailed at the present time.

2.3 - Joint Elements

In the Static SLAM Code joints are characterized as rectangular elements of zero thickness. This is illustrated in Figure 2.3 which shows a rectangular element of length a and width b in the plane of the page. The joint is described by such an element in which the dimension b approaches zero so that node points i and l have the same coordinates, and node points j and k have the same coordinates. An elongate joint is then made up of a series of such joint elements to which suitable elastic-plastic properties have been assigned.

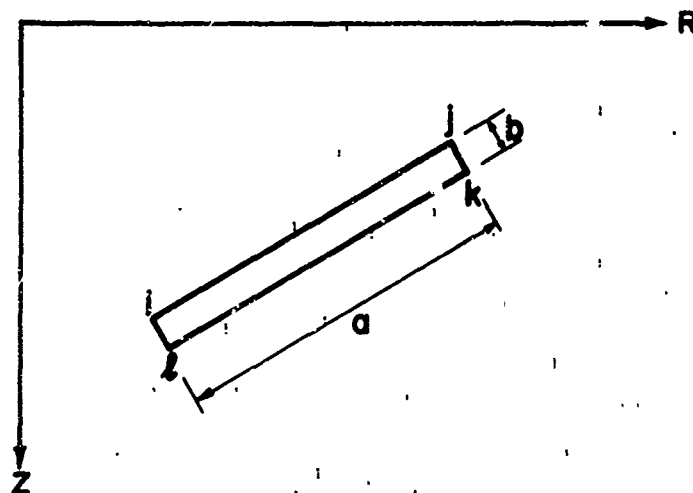


Figure 2.3 - Rectangular Element

The stiffness matrix for the joint elements is derived by determining the stiffness for a rectangular joint in terms of the width b and then allowing b to approach zero in the limit. That is, an equivalent strain, $\{\epsilon'\}$ is defined as

$$\{\epsilon'\} = b\{\epsilon\} \quad (2.7)$$

in which $\{\epsilon\}$ is the appropriate strain vector. The stresses, $\{\sigma\}$ are

$$\{\sigma\} = [C'] \{\epsilon'\} \quad (2.8)$$

where

$$[C'] = \frac{1}{b} [C] \quad (2.9)$$

and $[C]$ is the matrix of elastic constants.

Imposing a set of virtual nodal displacements $\{\delta u\}$, the equivalent strain is related to the virtual node point displacements by

$$\{\delta \epsilon'\} = b[A] \{\delta u\} = [A'] \{\delta u\} \quad (2.10)$$

where the matrix $[A]$ is determined from the definition of the strain components and the assumed displacement field for the element. The internal strain energy δW_i developed by these displacements is

$$\delta W_i = \frac{1}{b} \int_V \{\delta \epsilon'\}^T \{\sigma\} dV \quad (2.11)$$

in which the superscript T denotes the transpose of the matrix and the integration is taken over the volume V of the element. The corresponding external work done by the node point resisting forces during the virtual displacement δW_e is

$$\delta W_e = \{\delta u\}^T \{S\} \quad (2.12)$$

in which $\{S\}$ is the vector of node point forces for the element.

Invoking the principal of virtual work, expressions 2.11 and 2.12 are equated. Substituting Equation 2.10 into the result yields

$$\begin{aligned}\{\delta u\}^T \{S\} &= \frac{1}{b} \int_V \{\delta \epsilon'\}^T \sigma \, dV \\ &= \frac{1}{b} \int_V \{\delta u\}^T [A']^T \sigma \, dV\end{aligned}\quad (2.13)$$

Or, solving for the node point forces,

$$\begin{aligned}\{S\} &= \frac{1}{b} \int_V [A']^T \sigma \, dV \\ &= \frac{1}{b} \int_V [A']^T [C'] [A'] \, dV \{u\}\end{aligned}\quad (2.14)$$

This can be written

$$\{S\} = [k] \{u\} \quad (2.15)$$

in which the stiffness matrix $[k]$ is

$$[k] = \int_V \frac{1}{b} [A']^T [C] \frac{1}{b} [A'] \, dV \quad (2.16)$$

When the rectangular element is a joint element, the stiffness is then

$$[k]_{\text{joint}} = \lim_{b \rightarrow 0} [k] \quad (2.17)$$

The individual terms of a stiffness matrix which are preserved are determined by substituting the appropriate element integrals as given by Costantino and Wachowski (1966).

2.4 - Constitutive Laws for Joint Elements

Three constitutive laws are provided in the SLAM Code material catalog for the description of the mechanical behavior of the joint elements.

They are:

1. An elastic-plastic material obeying the Von Mises yield criterion and the Prandtl-Reuss flow equations. This model, which incorporates strain hardening effects, is described in detail by Perloff (1969) and Costantino (1968). The constitutive relation can be employed for regular elements as well as joint elements.
2. An elastic-plastic material obeying the Drucker and Prager (1952) three-dimensional extension of the Mohr-Coulomb criterion. This relation which can also be viewed as an extended Von Mises yield criterion is also described in the earlier report (Perloff, 1969) and by Costantino (1968). Although usable for both joint elements and regular elements, this constitutive relation is probably applicable to joints only when they are filled.
3. An elastic-plastic material which obeys a two-dimensional bilinear Mohr-Coulomb yield criterion described in Equations 2.5, and depicted graphically in Figure 2.2b. Post-yield behavior is governed by the residual strength parameters as indicated in Equation 2.6 and Figure 2.2b. An option is also provided in the SLAM code to require that the joint is incapable of withstanding tension normal to the joint surface. In Figure 2.2b this would correspond to a case in which the failure envelopes would be vertical along the τ axis. Such a case corresponds to a clean unhealed joint.

Plastic strains, for the post-yield condition, are calculated as,

$$\{\epsilon^P\} = \{\epsilon^T\} - \{\epsilon^E\} \quad (2.18)$$

in which $\{\epsilon^T\}$ are the total computed strains determined from the

node point displacements, and $\{\epsilon^E\}$ are the elastic strains determined from

$$\{\epsilon^E\} = [C]^{-1} \{\sigma\} \quad (2.19)$$

This constitutive relation is applicable to joint elements only.

These constitutive laws permit consideration of a wide range of types of joint behavior. The nature of the material catalog in the SLAM code also allows for relatively straightforward incorporation of additional constitutive relations, such as those involving time-dependent behavior.

2.5 - Revised SLAM Code

The current version of the Static SLAM Code, containing the revisions described above and incorporating consideration of joint elements is listed in Appendix A. The form of the data input required is given at the beginning of Appendix A and is indicated by comment cards within the code itself.

2.6 - Interpolation Code for MPBX Displacements

To assist in comparing the results of the analysis with displacement measurements along MPBX lines, an auxiliary code has been developed to compute displacements along these lines. The node point displacements determined by the SLAM code are used as input to the interpolation code for MPBX displacements. The input node point coordinates and displacements may be in either magnetic tape or punched card form.

Details of data input and a listing of the code are given in Appendix B.

SECTION 3. RESULTS OF ANALYSES OF JOINTED SYSTEMS

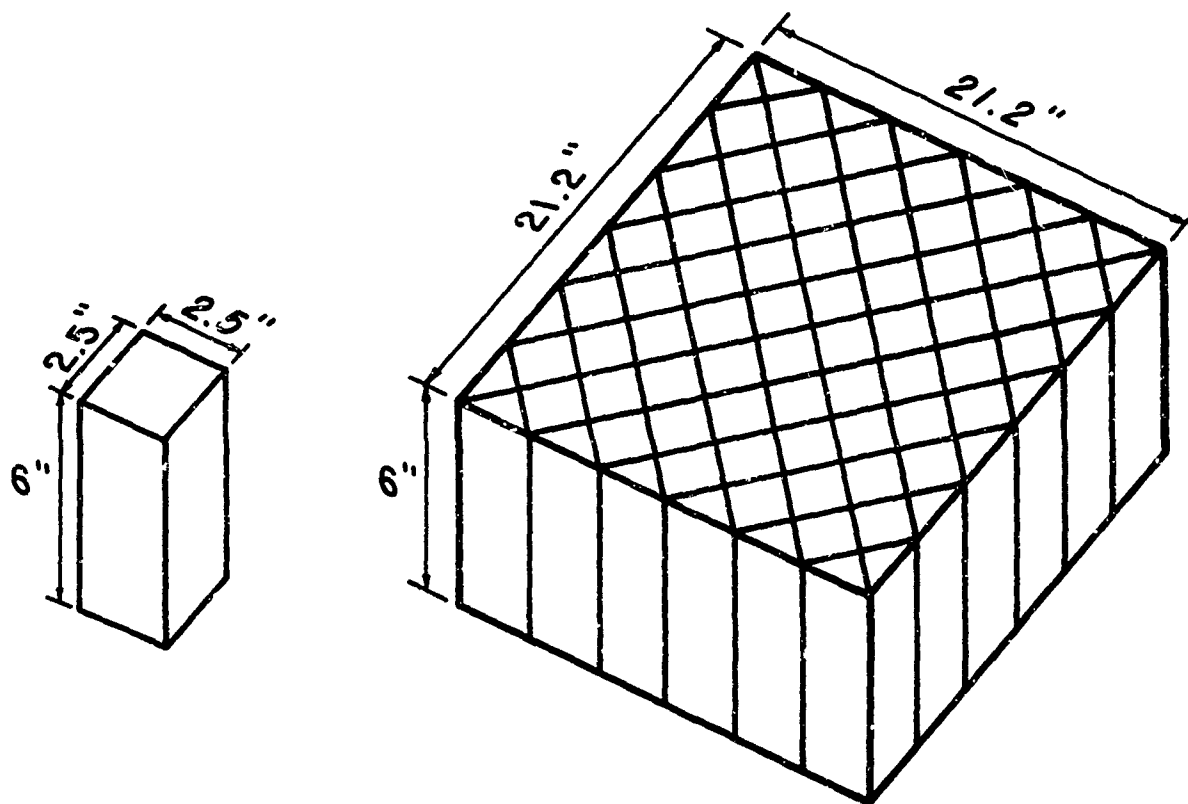
3.1 - Introduction

Two types of jointed systems were selected for comparative analyses to indicate the degree to which the behavior of the system could be predicted by the static SLAM code incorporating joint elements. The first of these was a series of model tests conducted on a mass of simulated rock blocks arranged to provide two families of intersecting joints. This model was developed at the Missouri River Division Laboratory (MRDL) of the U. S. Army Corps of Engineers (Rosenblad, 1971).

The second case considered is a typical section at the Straight Creek Pilot bore in which at least two families of intersecting joints were found intersecting the tunnel. These cases are discussed individually below.

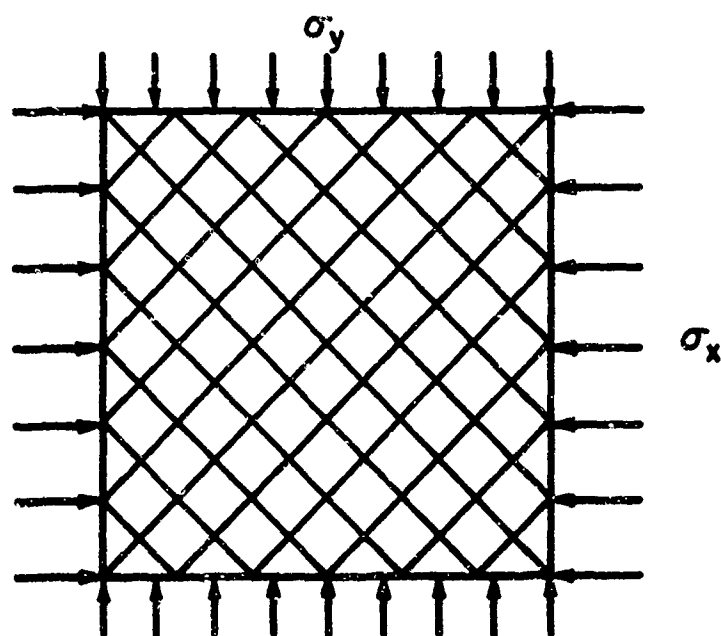
3.2 - MRDL Jointed Model Tests

The MRDL Jointed Block model is illustrated schematically in Figure 3.1. It consists of a series of blocks, square or rectangular in cross-section grouped together to form a body intersected by sets of parallel joints normal to one plane. In Figure 3.1a a typical square section block is illustrated. The square blocks are grouped as shown in Figure 3.1b with triangular blocks where required in order to form a larger mass which is square in cross-section. The model is loaded in the horizontal plane as indicated schematically in Figure 3.1c. Details of the apparatus construction, development and operation are given by Rosenblad (1971). The individual blocks are fabricated by molding using a model material consisting of sand, gypsum cement and water vibrated in a mold. The development of the model material resulted from an extensive experimentation program conducted by Rosenblad (1971) for this purpose.



a) - Typical Block

b) - Blocks Combined to Form Model



c) - Schematic Diagram of Applied Stresses Shown in Plan View

Figure 3.1- Schematic View of MRDL Jointed Block Model

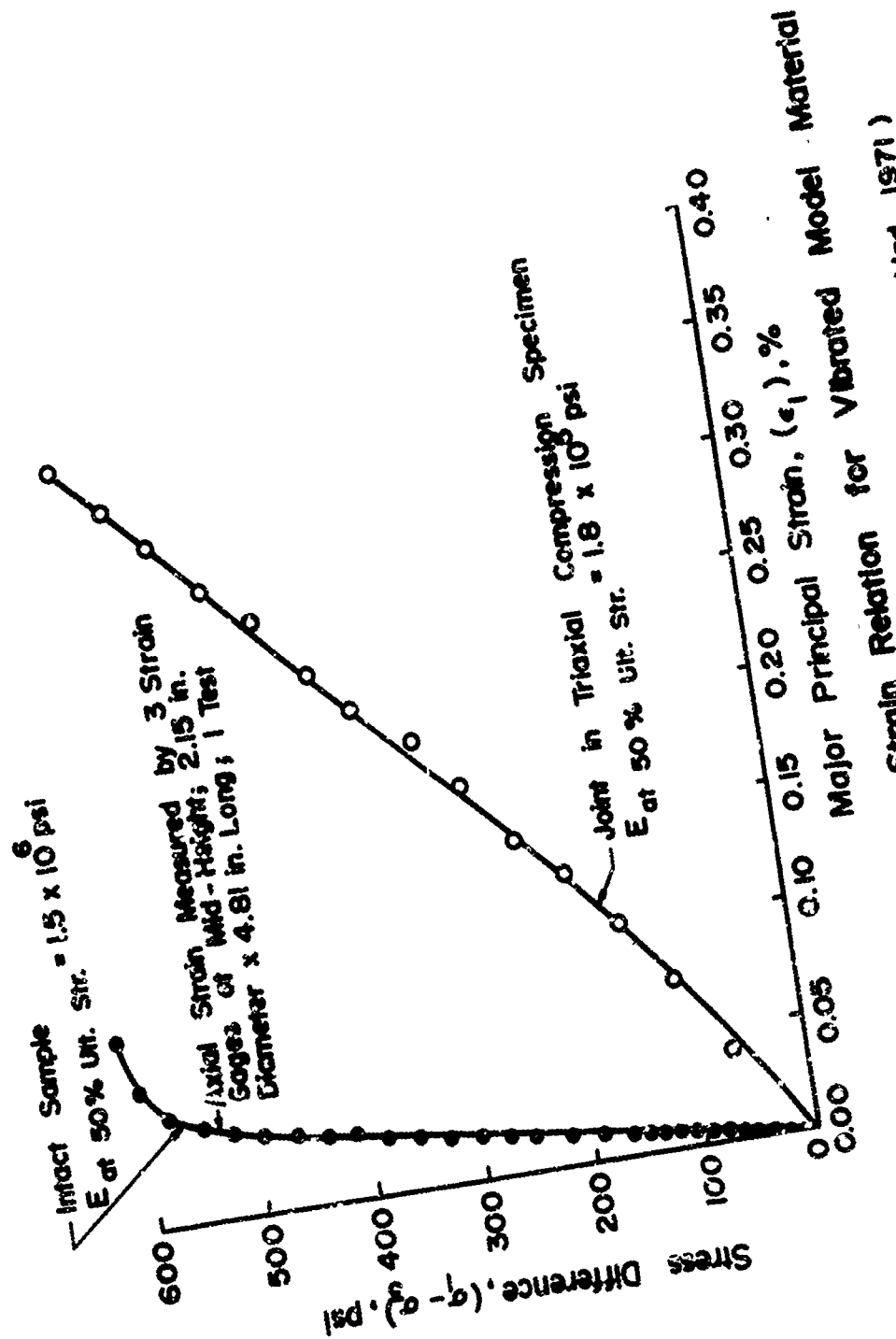
A typical uniaxial stress-strain curve for an intact cylindrical specimen of the vibrated model material is shown in Figure 3.2. The axial strain data were obtained from strain gages mounted in the central portion of the test sections. Rosenblad reports significant differences between the relation obtained from such strain gage measurements and those from gross measurements from the specimen. He attributes this discrepancy to end restraint effects. The importance of such effects in interpreting test results has been investigated in an earlier report (Perloff, 1969), and by Perloff and Pombo (1969). The Mohr envelopes for peak points on the stress-strain curves for the intact model material, obtained from both direct shear and triaxial compression tests, are shown in Figure 3.3.

The effect of a joint oriented at 45° to the axis of a triaxial compression specimen on the stress-deformation behavior of the model material is shown in Figure 3.2. This curve is for a triaxial compression test in which the confining pressure was 500 psi. However, the equivalent Young's modulus at 50 percent peak strength was of similar magnitude for lower confining pressures. Analysis of a single-jointed specimen indicated that the results in Figure 3.2 corresponded to a joint modulus of 1.8×10^5 psi.

Mohr envelopes for the joints between the blocks as obtained from triaxial compression and direct shear tests are shown in Figure 3.4. As might be expected, the joints exhibit no cohesive components of shearing resistance, and have a bilinear failure envelope.

Analysis performed

The analysis was carried out for a two-dimensional jointed model in which the blocks were assumed square. The finite element mesh used is shown in Figure 3.5. The mesh corresponds to one-quarter of the model and



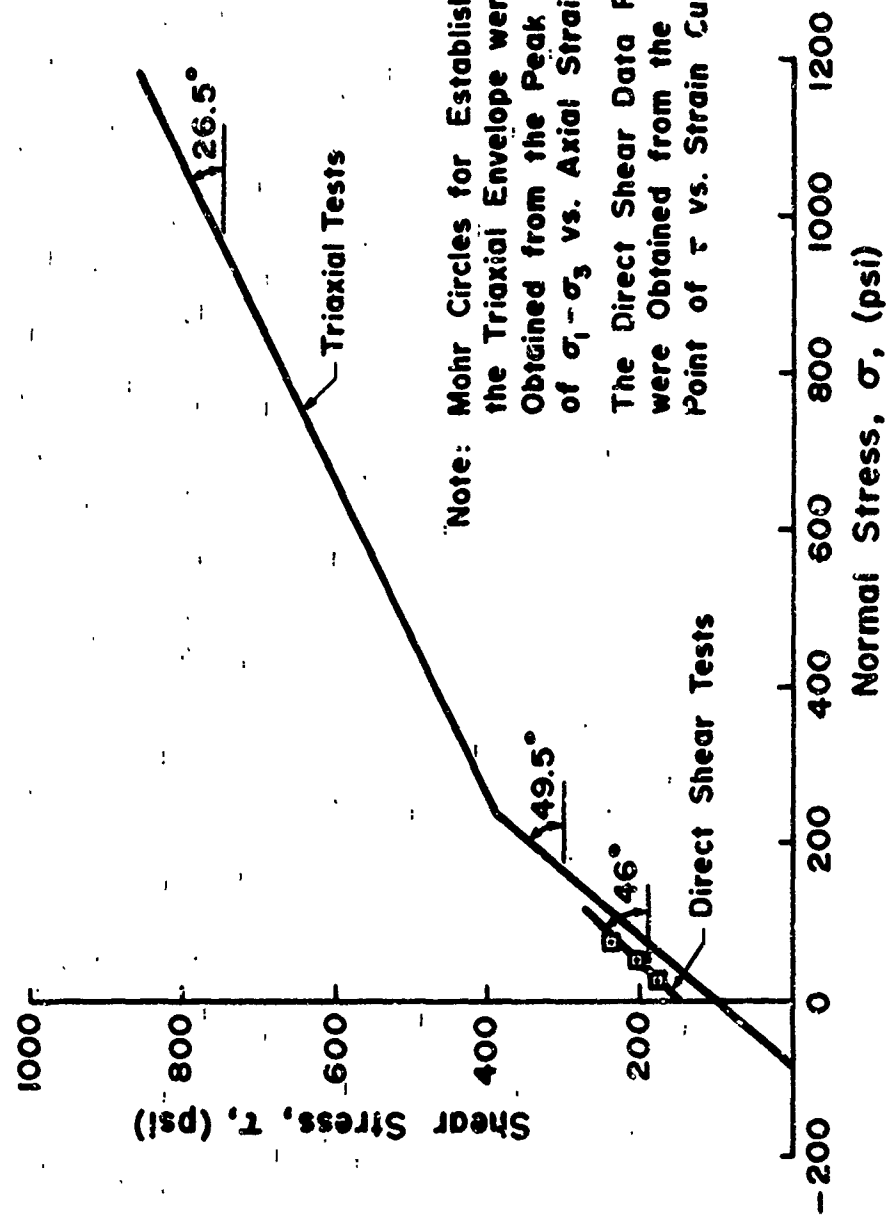


Figure 3.3 - Mohr Envelopes for Ultimate Failure of Intact Vibrated Model Material Specimens from Triaxial and Direct Shear Tests
(from Rosenblad, 1971)

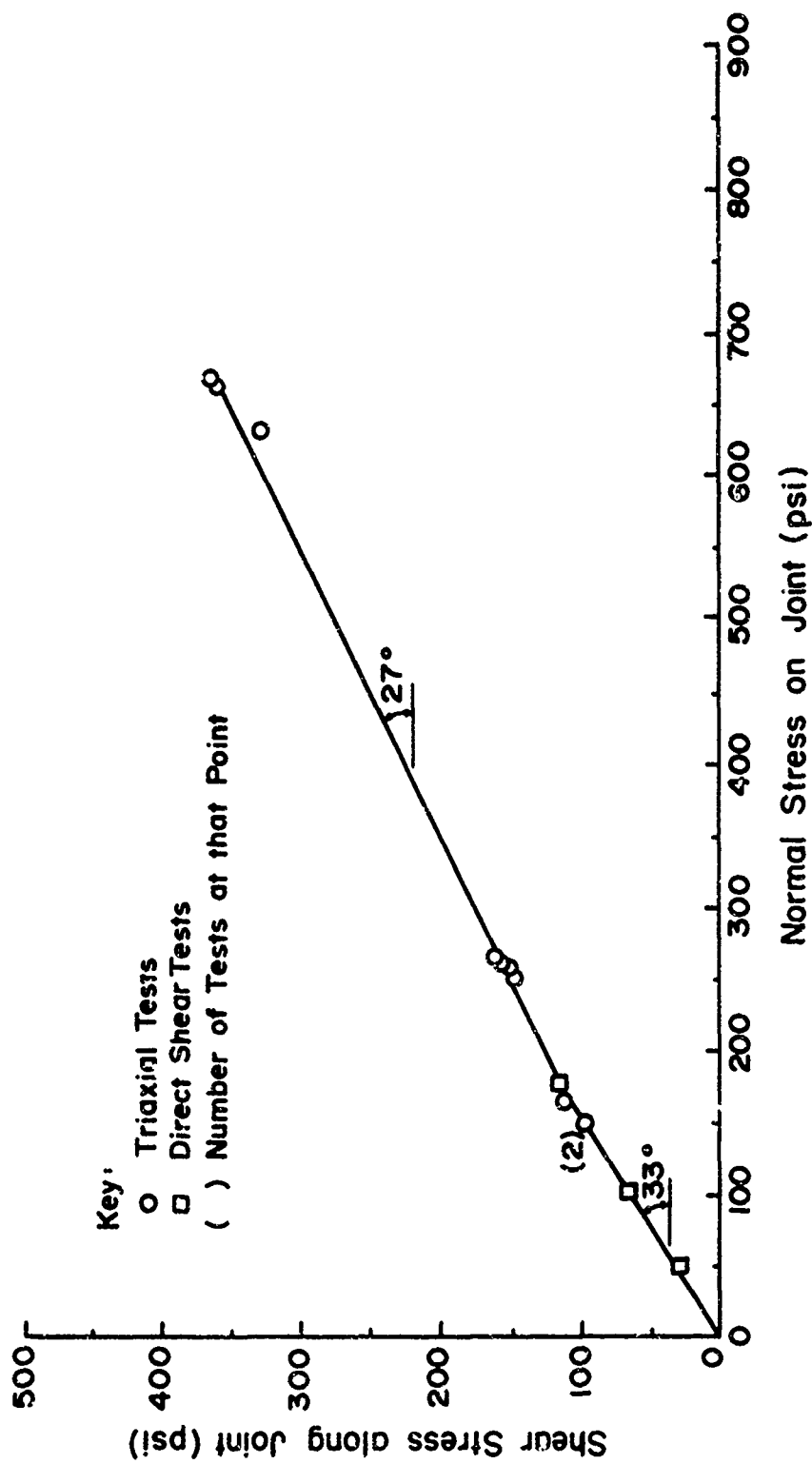


Figure 3.4 - Mohr Envelope for Initial Slip along Joints from Triaxial and Direct Shear Tests

(from Rosenblad , 1971)

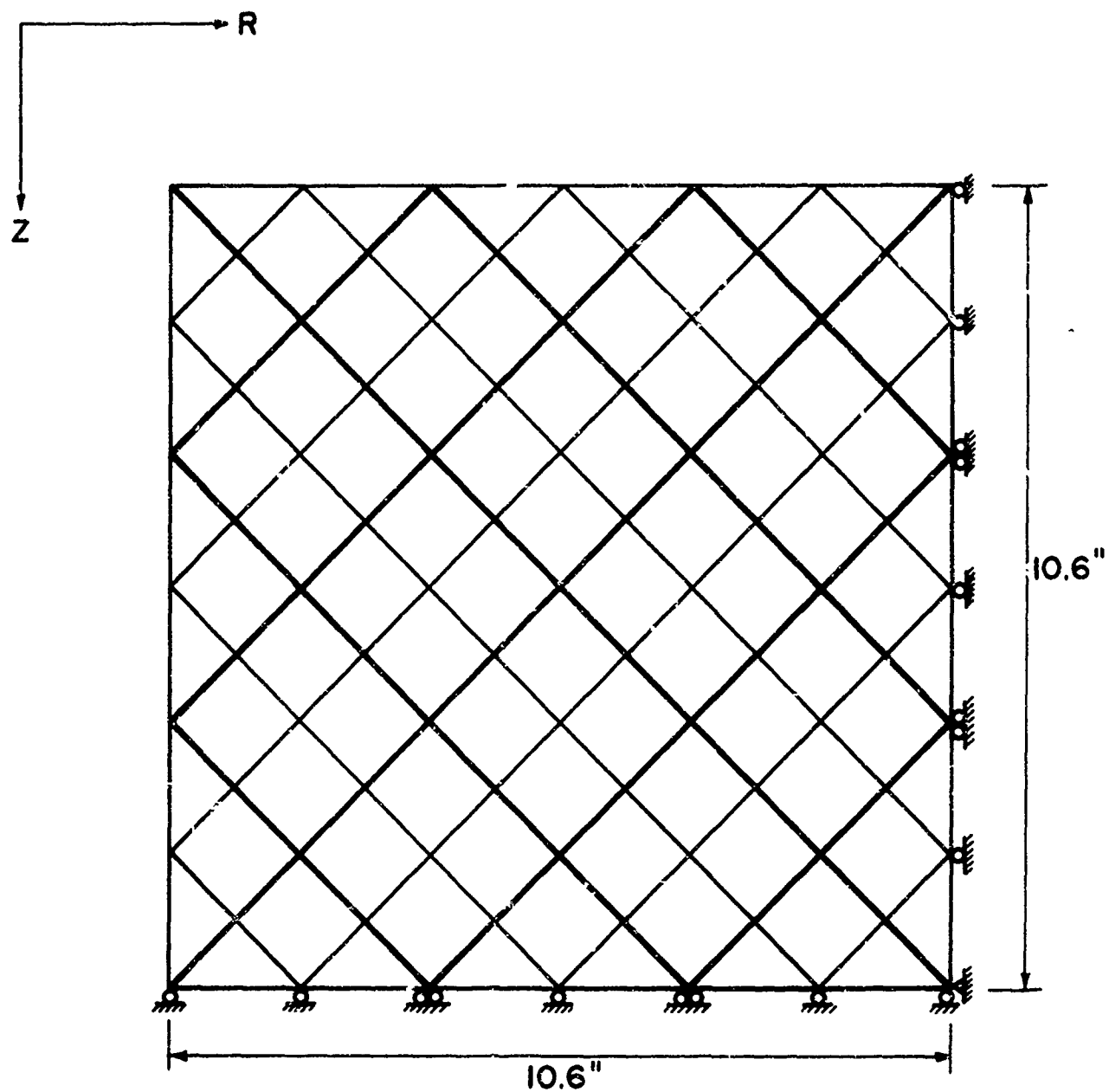


Figure 3.5 - Finite Element Mesh for MRD₁ Jointed Model

consists of 180 nodes and 156 elements. Each of the heavy lines indicates a joint element between intact blocks. The light lines are boundaries of elements forming the intact blocks. The material parameters used in the analysis were obtained from the test data presented by Rosenblad (1971) and shown in Figures 3.2-3.4:

Intact Blocks - Elastic-plastic material obeying the three-dimensional generalized Mohr-Coulomb yield criterion:

$$E = 1.5 \times 10^6 \text{ psi}$$

$$\nu = 0.230$$

$$c = 100 \text{ psi}$$

$$\phi = 49.5^\circ \text{ } (\sigma_f \leq 230 \text{ psi})$$

Joints - Elastic-plastic "material" obeying the two-dimensional bilinear Mohr-Coulomb yield criterion. Post-yield behavior is governed by residual strength parameters which are the same as those producing initial yield:

$$E = 1.8 \times 10^5 \text{ psi}$$

$$\nu = 0.230$$

$$c = 0$$

$$\phi_1 = 33^\circ \text{ } (\sigma_f \leq 175 \text{ psi})$$

$$\phi_2 = 27^\circ \text{ } (\sigma_f > 175 \text{ psi})$$

The imposed loading used in the analysis was $\sigma_y = \sigma_2 = 25 \text{ psi}$, $\sigma_z = \sigma_1$ increasing to a maximum of 275 psi.

Results

Results of an analysis of the jointed rock model are shown as the dashed line in Figure 3.6. The line shows the relative displacement on either side of a joint, parallel to the joint, as a function of the major principal

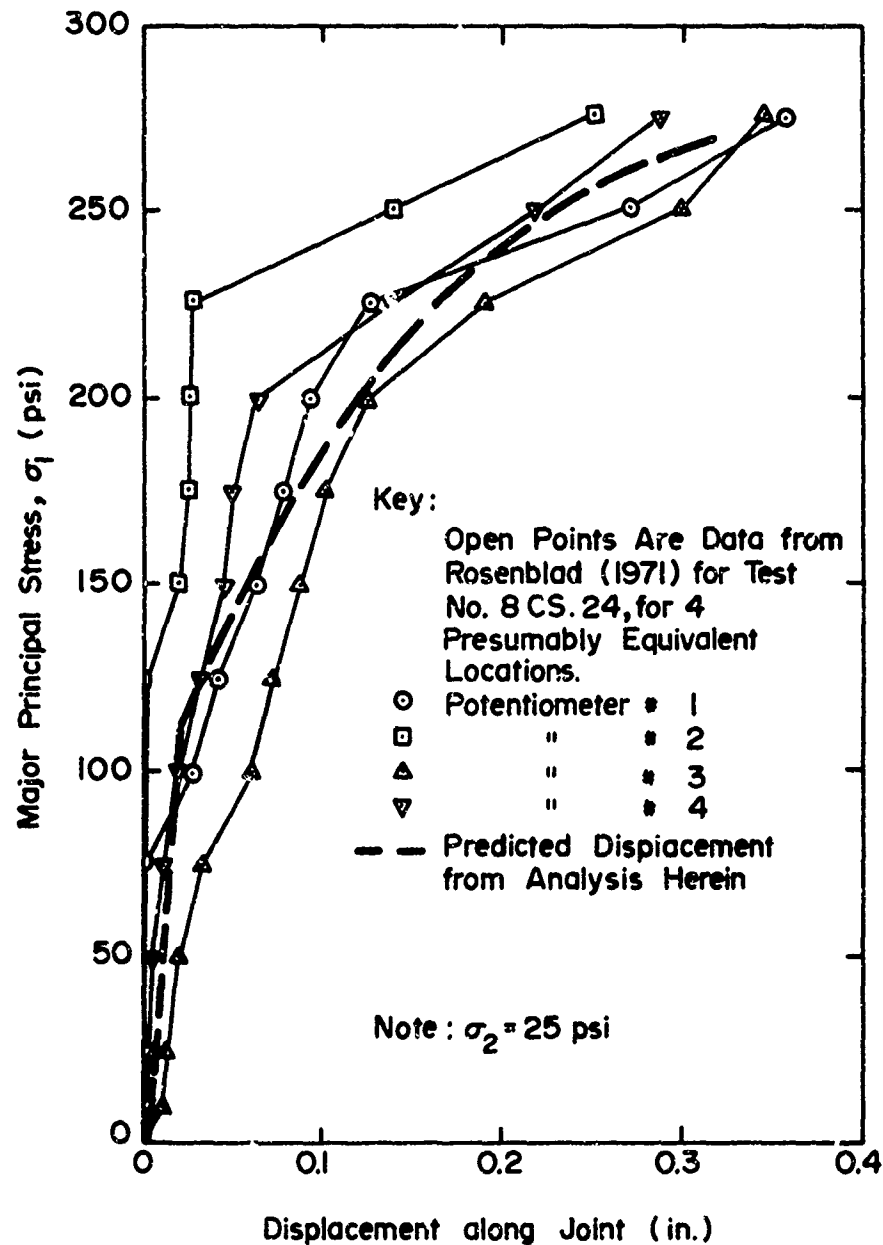


Figure 3.6 - Comparison Between Displacement along Joints Measured in MRDL Model Study and that Predicted by Finite Element Analysis.

stress. The curve shown corresponds to any joint because of the symmetry of the test.

Measured results for four presumably equivalent joints are also shown in Figure 3.6. The scatter in the experimental results probably arises from rotation of individual blocks, and consequent nonuniform distribution of frictional forces between the blocks, due to minor eccentricity in the jack loading system. Nonetheless, the ability of the analysis to predict the observed displacements is evident.

On the basis of these results it was concluded that the SLAM code was capable of describing the behavior of jointed systems which satisfied the following criteria:

1. The geometric arrangement of joints and intact elements can be completely described in terms of a two-dimensional system.
2. The mechanical behavior of the intact materials and individual joints can be characterized by one of the constitutive relations incorporated in the material catalogue of the code.
3. The imposed loads and displacements are known.

3.3 - Analysis of Straight Creek Pilot Bore

Description of the tunnel

The Straight Creek Pilot Bore is located about 55 miles west of Denver on the proposed highway I-70. About 75 percent of the rock in the pilot bore is fine to medium grained granite (Brown, 1970). The remainder of the rock consists of metasediments that include a variety of materials. The tunnel is transected by the Loveland Pass fault zone, which contains numerous shear zones of diverse orientations. The rock is jointed, and joint surfaces are

commonly coated with chlorite and/or calcite.

The section chosen for analysis, Sta. 114+53, was composed of predominantly granitic rock with two major joint systems oriented approximately 38 and 52 degrees from the horizontal on a plane normal to the tunnel axis. The joint spacing observed at the tunnel wall averaged one to three feet, but was quite variable. At this location the tunnel is 250 feet below the ground surface.

Mechanical characteristics of the rock were determined by Robinson and Lee (1965). Their test results, illustrated by the Mohr circles and solid failure envelope in Figure 3.7, indicate that the intact rock obeys a Mohr-Coulomb failure criterion. Tests on samples with chlorite and calcite filled joints, in which the failure occurred along the joints, indicate peak strength behavior of the joints shown as the dashed line in Figure 3.7. Once the initial failure takes place however, it seems reasonable that the cohesive resistance diminishes to zero.

Analysis performed

The two-dimensional jointed finite element mesh used to represent the problem is shown in Figure 3.8. The heavy lines are joint elements, light lines indicate boundaries of intact elements. In the vicinity of the tunnel, joint spacing was three feet. The spacing was increased at increasing distance from the tunnel, Figure 3.8, so that there were less than 1600 nodes. As in the case of the continuum analysis (Perloff, 1969), the problem is solved in two stages. The displacements resulting from the tunnel construction were determined as the difference between the displacements of the mass without the tunnel, and those with the tunnel (shaded in Figures 3.8) removed. The mesh without the tunnel consisted of 1507 nodes and 1385

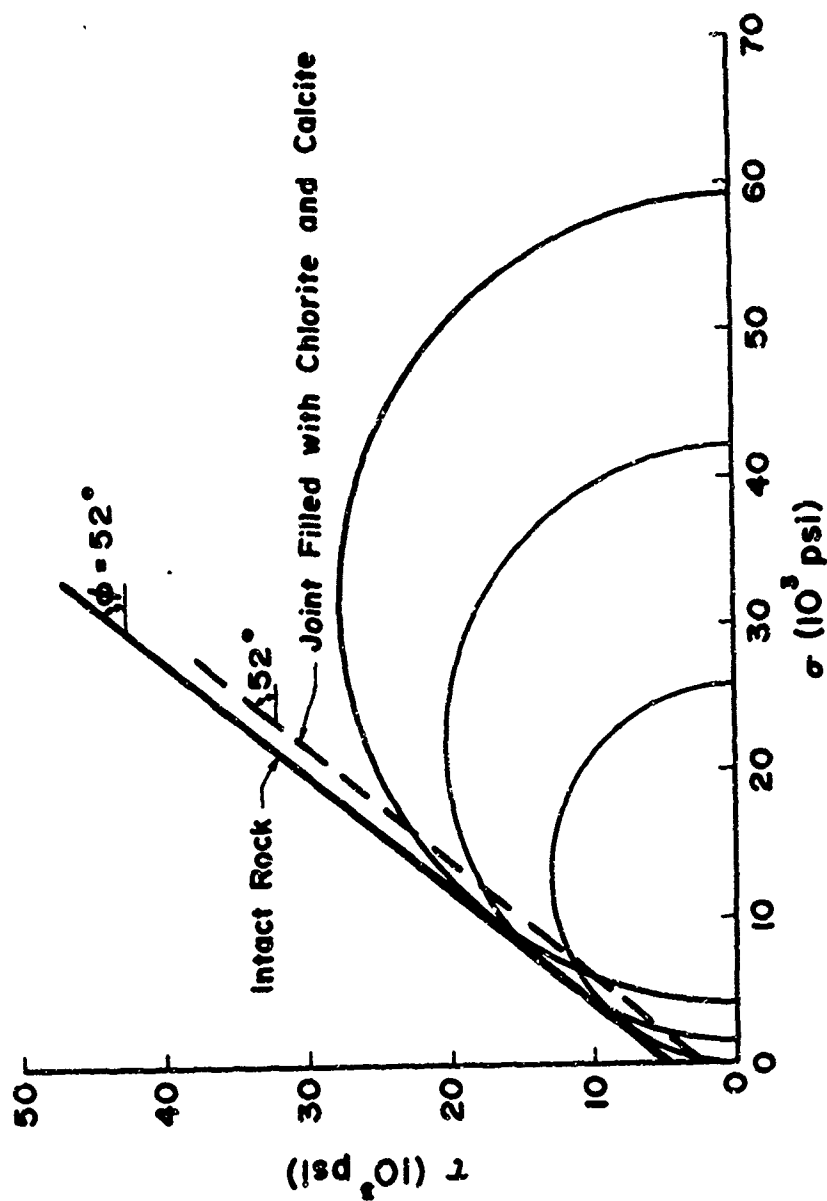


Figure 3.7 - Average Failure Envelope for Granite and Metamorphic
 Rocks at Straight Creek
 (Data from Robinson and Lee, 1965)

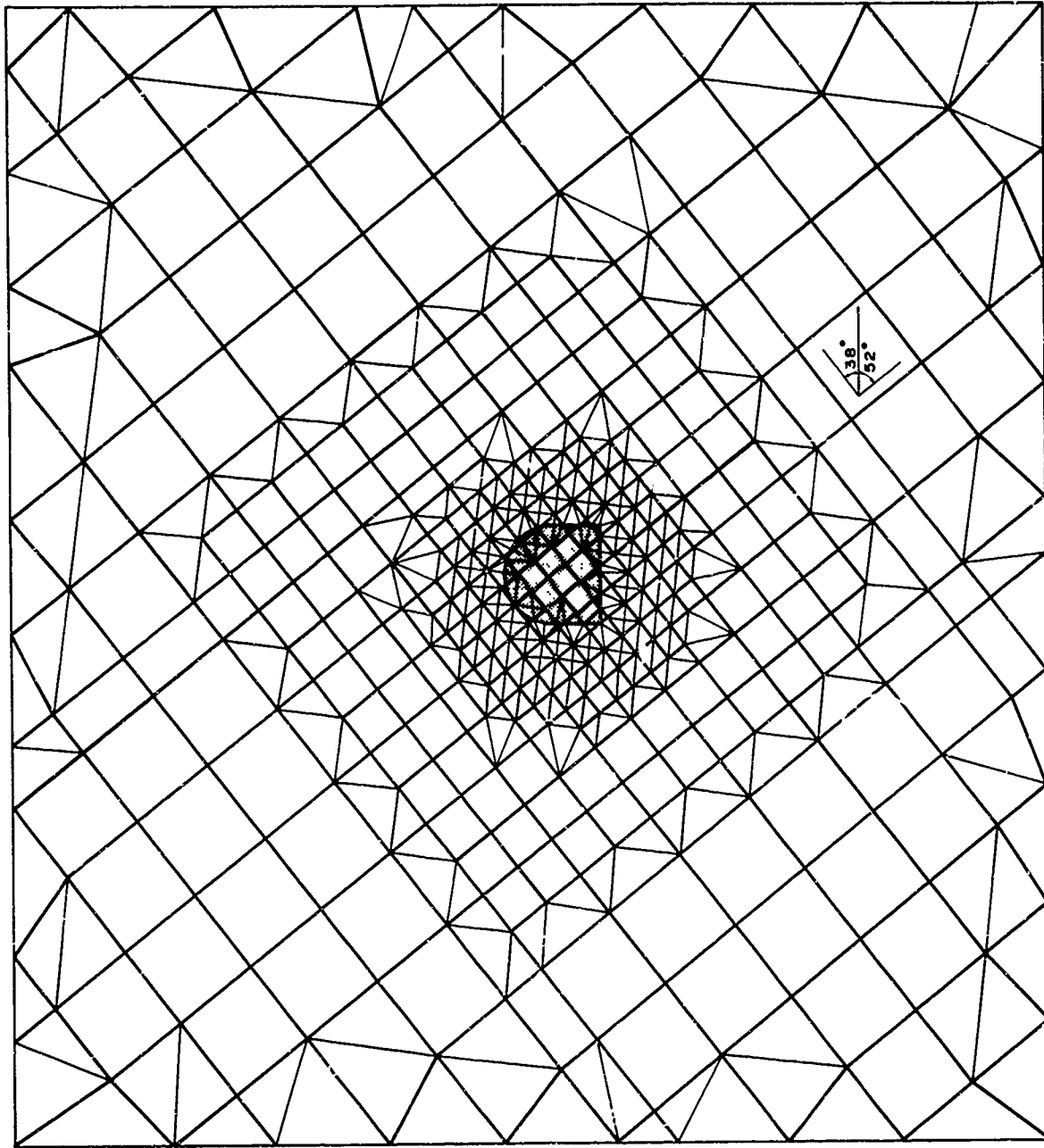


Figure 3.8 - Finite Element Mesh for Straight Creek Pilot Bore
With Two Joint Systems

elements; that with the tunnel contained 1444 nodes and 1300 elements.

The material parameters used in the analysis were:

Intact rock - Elastic-plastic material obeying the three-dimensional generalized Mohr-Coulomb yield criterion:

$$E = 8.98 \times 10^6 \text{ psi}$$

$$\nu = 0.243$$

$$c = 4500 \text{ psi}$$

$$\phi = 52^\circ$$

Joints - Elastic-plastic material obeying the two-dimensional Mohr-Coulomb yield criterion. Post-yield behavior is cohesionless in nature as discussed above:

$$E = 8.98 \times 10^5 \text{ psi}$$

$$\nu = 0.243$$

$$c = 2500 \text{ psi}$$

$$\phi_1 \phi_2 = 52^\circ$$

$$c_{\text{res}} = 0$$

$$\phi_{\text{res}} = 52^\circ$$

Imposed loads were due solely to gravity. That is, the material weight acted on all elements shown. In addition, a uniform vertical loading of 208 psi was applied to the upper boundary of the mesh to account for the overburden above the mesh.

Results

Results of the analysis and their relation to field measurements are shown graphically in Figure 3.9. This figure indicates the cross-section of the tunnel and the three MPBX's located at Sta. 114 + 53.

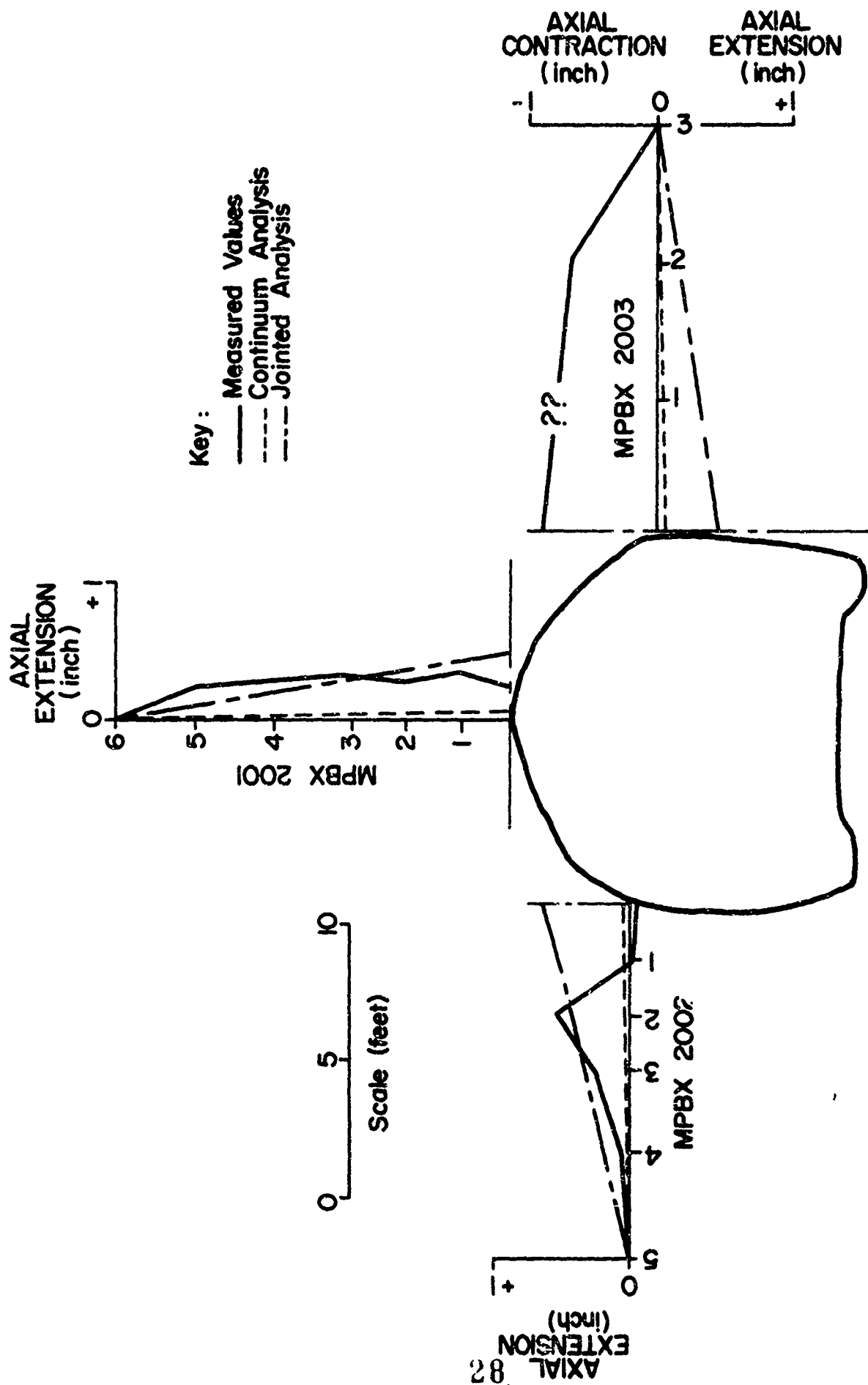


Figure 3.9 - Comparison Between Measured and Predicted Displacement at Sta 114+53 at Straight Creek Pilot Bore.

Superimposed on the diagrams of the MPBX's are the measured displacements along the MPBX axis, values calculated by the analysis for jointed systems given herein and, for comparison, the results from the continuum elastic analysis (Perloff, 1969). The analytical results for the jointed model are closer to the measured values at MPBX's 2001 and 2002 than those for the continuum model. The calculations predict a movement in MPBX 2003 which opposite in direction to that measured. Furthermore the irregular movements recorded for MPBX 2002 near the tunnel face are not predicted by the analysis. Several possible reasons for these discrepancies can be identified. Among these are:

1. Incorrect measurement of rock movement based on MPBX data.

This could arise from at least two sources:

- a. Displacements are likely to occur immediately upon excavation. Because the MPBX is installed only after excavation, important components of displacement, not necessarily in the same direction as subsequently measured values may be lost.
- b. Anchor slip may occur leading to spurious relative displacement values between individual anchors. If the anchor most remote from the tunnel slips, the whole displacement axis is translated.

2. The specific orientation and spacing of the joints in the vicinity of the MPBX's is not known but only estimated for simplified representation. This can affect predicted displacements markedly in the region near the tunnel where stress relief is the greatest.

3. The mechanical characteristics of the joints may be incorrectly described for at least two reasons:
 - a. The elastic parameters for the joint materials were estimated on the basis of the measured relative moduli in the MRDL tests. Actual data on this point were not available.
 - b. The joints were assumed to be unfailed, i. e., peak strength parameters were initially applicable. If the excavation process produces temporary joint separation, for example during blasting, then residual strength parameters may be more applicable.
4. Initial stress conditions are important both to the magnitude of elastic deformations as well as to the onset of yielding as the tunnel material is removed. This important point was discussed in an earlier report (Perloff, 1969) in more detail. Unfortunately, no proven means for reliably measuring the initial stress state (prior to excavation) is available at the present time.
5. The three-dimensional nature of the problem, especially the jointing and faulting undoubtedly has some influence. This point is discussed further below.

On the basis of these observations it was concluded that a more accurate prediction of tunnel behavior based on presently available input information would be fortuitous.

SECTION 4 - CONSIDERATION OF THREE-DIMENSIONAL EFFECTS

Three-dimensional effects are undoubtedly important in the response of a rock mass to the opening of an excavation within the mass. These effects arise from at least three sources:

1. The three-dimensional nature of the tunnel geometry produces a corresponding set of stresses and deformations in the rock medium. Even in the case of a long tunnel, the geometry is decidedly three-dimensional at the end of the tunnel where construction is occurring, as well as in the vicinity of the portals.
2. The preexisting stratigraphic features, especially joints and faults, are likely to interact with the tunnel in a fashion which requires a three-dimensional framework for a realistic representation.
3. The failure criterion appropriate to the materials involved most probably involves the complete stress (or strain) field at a point. Therefore the response characteristics of the materials themselves are three-dimensional in nature.

In spite of these features, however, it was found to be impractical to incorporate three-dimensional effects into the jointed system analysis at the present time. Reasons for this included:

1. Although three-dimensional effects are likely to be significant, there are many situations (including, probably, the Straight Creek Pilot Bore) where the

other discrepancies between the real conditions and those identified are much more important. It is likely that the most important of these are the joint spacing and mechanical characteristics. Thus, only a marginal gain in accuracy seemed likely as the result of incorporating these effects at this time.

2. Even a simplified representation of the major joint systems in a two-dimensional framework required approximately 1500 nodes and 1400 elements. The SLAM code capable of managing this size problem uses approximately 53,000 words of central memory storage and approximately 15 minutes of computation time on the CDC 6500 computer. Expanding the program to three-dimensions without imposing some symmetry requirements would lead to an inordinately large problem (see for example, Corum and Krishnamurthy, 1969).

Consequently it was concluded that three-dimensional considerations could not be profitably incorporated into the analysis at the present time.

SECTION 5 - CONCLUSIONS

Based on the results described in this report for analyses of jointed systems and measurement of performance of those systems, the following conclusions have been drawn:

1. The static SLAN code for plane jointed systems described herein can predict the response of such systems to imposed loadings when the following conditions are satisfied:
 - a. The geometric arrangement of joints and intact elements can be completely described in terms of a two dimensional system.
 - b. The mechanical behavior of the intact materials and individual joints can be characterized by one of the constitutive relations incorporated in the material catalogue of the code.
 - c. The imposed loads and displacements are known.
2. Predictions of displacements of excavations in a natural jointed rock mass are likely to differ from measured values. The discrepancy may arise from numerous sources including:
 - a. Errors inherent in the measurements themselves.
 - b. Insufficient knowledge of the spacing and orientation of the joints and faults in the zone of interest.
 - c. Incorrect assessment of the mechanical characteristics of the joints or joint-filling materials.

- d. Inadequate knowledge of insitu stress conditions prior to excavation.
 - e. Three dimensional effects which cannot be incorporated in a plane analysis. This effect is likely to be less important than others mentioned above.
3. Incorporation of a three-dimensional finite-element representation of jointed systems into the analysis is not practicable at this time due both to the consequent requirement for computer storage and time, and to the limited benefit likely to be gained from such an undertaking.

REFERENCES

- Brown, L. A., (1969) "Data Bank: Underground Openings Rock Deformation Measurement-Abstracts", Technical Report No. 3, Purdue University School of Civil Engineering Project on "Strain Distribution Around Underground Openings".
- Corum, J. M. and Krishnamurthy, N., (1969) "A Three-Dimensional Finite Element Analysis of a Prestressed Concrete Reactor Vessel Model", Application of Finite Element Methods in Civil Engineering, ASCE, New York, p. 63.
- Costantino, C. J., (1966), "Automated Design of Advanced Hardened Facilities Study", IIT Research Institute, BSB TR-66-256.
- Drucker, D. C. and Prager, W. (1952), "Soil Mechanics and Plastic Analysis or Limited Design", Quarterly of Applied Mathematics, Vol. 10, No. 2, July 1952.
- Goodman, R. E., (1969) "The Deformability of Joints", Determination of the Insitu Modulus of Deformation of Rock, ASTM STP 477.
- Judd, W. R. and Perloff, W. H., (1971), "Comparison between Predicted and Measured Displacements", Technical Report No. 5, Purdue University School of Civil Engineering Project on "Strain Distribution Around Underground Openings".
- Obert, L., "Determination of Stress in Rock - A State of the Art Report", ASTM STP 429, ASTM, 1967.
- Patton, F. D., (1966), "Multiple Mode of Shear Failure in Rock", Proceedings, First Congress of International Society of Rock Mechanics, V. 1, p. 509.
- Robinson, C. S. and Lee, F. T., (1965), "Preliminary Report on the Engineering Geology of the Straight Creek Tunnel Pilot Bore, Clear Creek and Summit Counties, Colorado", Prepared in Co-operation with the Colorado Department of Highways.
- Rosenblad, J. L., (1971), "Geomechanical Model Study of the Failure Modes of Jointed Rock Masses", Technical Report MRD 1-171, Missouri River Division USCE, Omaha.
- Wachowski, A. and Costantino, C. J., (1966), "Automated Design of Advanced Hardened Facilities Study", BSD TR-66-256, Vol. II, Norton AFB, Calif.

APPENDIX A - STATIC SLAM CODE FOR JOINTED SYSTEMS

A.1 - Code Description

The Static SLAM Code is written entirely in FORTRAN IV and makes use of the overlay features of FORTRAN IV to optimize usage of the high speed core. The code consists of a main program, twelve overlays and 27 separate subroutines. The overlay structure and subroutines are shown schematically in Figure A.1.

The code uses 11 tape drives for immediate storage of data and output. The logical numbers for these tapes are 1, 2, 3, 4, 8, 9, 10, 11, 12, 14, 15. In addition, I/O is handled by tape 5 for input and 6 output. The solution is stored on tapes 3 or 12, and 15. The code is presently operational on the CDC 6500, using the Purdue MACE operating system.

A.2 - Data Deck Setup

The following description of the data deck setup assumes that, in general, all numbers are right-oriented in their fields. Inclusion of the decimal point in floating point (real) numbers overrides the right-orientation requirement. Generally all integer data are entered in 5-column fields while all floating point data are entered in 10-column fields.

Data entered in card groups 1.1 to 6.2 are read in overlay LNK1A. Data entered in group 7.1 to 9.3 are read in overlay LNK1G. The remaining data are read in overlay LNK2.

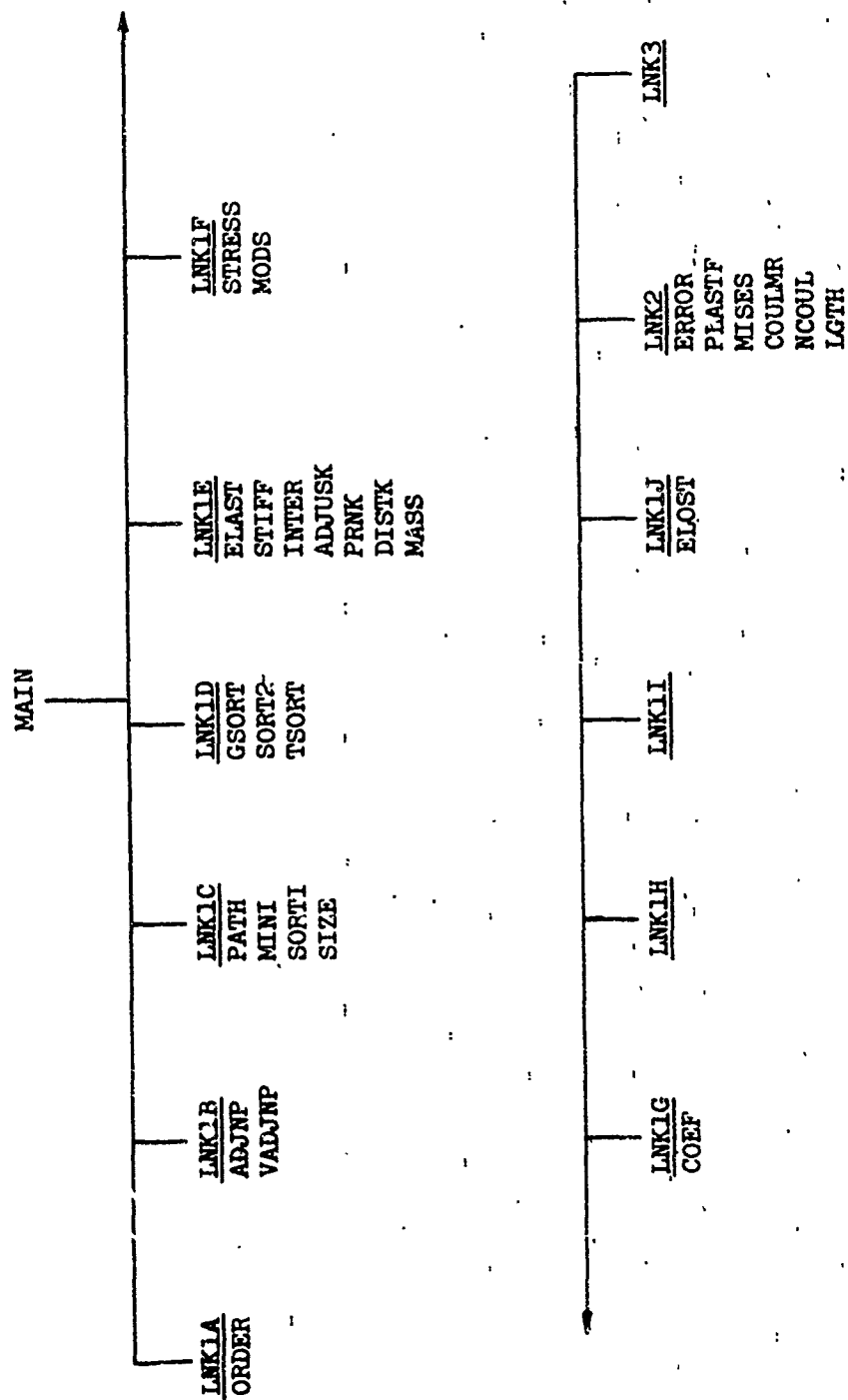


Figure A.1 - Overlay Structure of SLAM Code

<u>CARD</u>	<u>VARIABLE</u>	<u>FORMAT</u>
1.1	ANAME	(18A4)
	ANAME = Problem Descriptor to be printed as output, up to 72 characters	
2.1	NUMNP, NUMEL, ISTRES, IMPBX, IPRINT	(515)
	NUMNP = Number of node points ($\leq$ 1600)	
	NUMEL = Number of elements	
	ISTRES = Counter to describe stress condition,	
	= 0, axisymmetric problem,	
	= 1, plane strain problem,	
	= 2, plane stress problem.	
	IMPBX = Counter for storage on magnetic tape (logical number 15) for use in interpolation code (Appendix B) for determining displacement along MPBX lines,	
	= 0, data are not stored on tape 15	
	= 1, data are stored on tape 15 for subsequent use.	
	IPRINT = Counter for intermediate printout,	
	= 0, no intermediate printout other than input data,	
	= 1, print adjacency table and input data,	
	= 2, print stiffness table and input data,	
	= 3, print stress table and input data,	
	= 4, print mass vector and input data,	
	= 5, print load tables and input data,	
	= 6, print results of elimination solution and input data,	
	= 7, print stresses in plastic elements and input data,	
	=99, print all above tables.	

<u>CARD</u>	<u>VARIABLE</u>	<u>FORMAT</u>
3.1	N, R, Z, ITYPE, THETA	(I5,2E10.4, I10,E10.4)
	N = Node point number,	
	R = Radial (horizontal) coordinate (ft), increasing to the right,	
	Z = Vertical coordinate (ft), increasing down,	
	ITYPE = Counter for support conditions,	
	= 0, free node,	
	= 1, fixed in one direction,	
	= 2, fixed in both directions	
	THETA = Angle (in degrees) of roller support measured positive clockwise from the horizontal, for ITYPE = 1 only.	
	Note: Card 3.1 repeated NUMNP times.	
4.1	NZONES	(I5)
	NZONES = Number of different materials (≤ 20)	
4.2	IZ, ANAME	(I5,I8A4)
	IZ = Material or zone number	
	ANAME = Material or zone descriptor to be printed as output, up to 72 characters.	
4.3	IELAST, IPLAST, WGT, E1, E2, E3, E4, E5	(2I5,E10.0,5E10.0)
	IELAST = Type of linear material behavior,	
	= 1, isotropic elastic material,	
	= 2, transversely anisotropic elastic material,	
	= 3, linear compressible fluid.	
	IPLAST = Counter to describe nonlinear behavior,	
	= 0, elastic or linear material,	
	= 1, Mises (Prandtl-Reuss) elastic-plastic material,	
	= 2, Elastic-Plastic material with generalized three-dimensional Mohr-Coulomb yield criterion.	

CARD VARIABLE

FORMAT

= 3, Elastic-Plastic material with two-dimensional Mohr-Coulomb yield criterion. Applicable for joint elements only.

Note: if IPIAST = 1, 2 or 3, IELAST must equal 1 (isotropic elasticity)

WGT = Unit weight of material (pcf)

E1 to E5 = Elastic Property data.

If IELAST = 1,

E1 = Young's Modulus (psi),

E2 = Poisson's Ratio

E3 to E5 are neglected.

If IELAST = 2,

E1 = E_x (psi),

E2 = E_z (psi),

E3 = E_{rz} (psi),

E4 = G (psi),

E5 = $E_{r\theta}$ (psi).

If IELAST = 3,

E1 = bulk modulus, and

E2 to E5 are neglected.

4.4 NOYILD

(I5)

NOYILD = Number of nonlinear segments of effective stress-strain curve of elastic-Mises plastic material (≤ 10)

4.5 (SSTAR(I), I = 1, NOYILD)

(7E10.4)

SSTAR = Stress (psi) at beginning of non-linear segment, up to 7 per card.

4.6 (HSTAR(I), I = 1, NOYIELD)

(7E10.4)

HSTAR = Slope (psi) of nonlinear segment, up to 7 per card.

Note: Cards 4.4, 4.5, and 4.6 omitted if IPLAST $\neq 1$.

<u>CARD</u>	<u>VARIABLE</u>	<u>FORMAT</u>
4.7	COHESN, FRCTAN	(2E10.4)
	COHESN = Value of cohesion (psi) for generalized three-dimensional Mohr-Coulomb material, as determined from standard triaxial compression test. FRCTAN = Corresponding friction angle (in degrees) <u>Note:</u> Card 4.7 omitted if IPLAST not equal to 2.	
4.8	COHESN, FRCTN1, FRCTN2, SNSWCH	(4E10.4)
	COHESN = Value of peak cohesion (psi) for two-dimensional Mohr-Coulomb material (applicable for joint elements only). FRCTN1 = Peak friction angle (in degrees) for normal stress $\leq$ SNSWCH. FRCTN2 = Peak friction angle (in degrees) for normal stress greater than SNSWCH. SNSWCH = Normal pressure (psi) on joint at which slope angle of bilinear peak yield envelope changes from FRCTN1 to FRCTN2.	
4.9	JTENSN, IRESID	(2I5)
	JTENSN = Counter indicating tensile resistance across joint = 0, Joint material can withstand no tension normal to joint, = 1, Joint material can resist tension normal to joint up to magnitude $C/\tan(\text{FRCTN1})$. IRESID = Counter indicating whether residual shear strength along joint is less than peak value. = 0, Residual shear strength along joint = the peak value, = 1, Residual shear strength along joint is less than peak value	
4.10	CRESID, FRESID	(2E10.4)
	CRESID = Residual (post-peak) cohesion (psi) FRESID = Residual (post-peak) friction angle (in degrees), $\leq$ FRCTN2. <u>Note:</u> If IRESID = 0, card 4.10 is omitted. <u>Note:</u> Card group 4.2 to 4.10 repeated NZONES times.	

<u>CARD</u>	<u>VARIABLE</u>	<u>FORMAT</u>
5.1	NUME, IZONE, NPI, NPJ, NPK, NPL, NCRACK	(7I5)
	NUME = Element number,	
	IZONE = Material zone number in which element is located,	
	NPI to NPL = Node numbers comprising element. If NPL = 0, element is considered triangle. If element is a joint element, NPI must be either node with smallest R-coordinate. If the element is vertical, NPI must be either node with the smallest z-coordinate. NPJ, NPK, NPL must be nodes given in <u>clockwise</u> order around the element. For all other element types, there is no restriction on ordering of nodes.	
	NCRACK = Counter to identify joint elements, = 0, regular triangular or rectangular element, = 1, rectangular joint element of zero thickness. Thus two nodes will have the same coordinate, and the other two nodes will have the same coordinates.	
	<u>Note:</u> Card 5.1 repeated NUMEL times.	
6.1	NUMST	(15)
	NUMST = Number of start nodes for renumbering scheme (< 80)	
6.2	(NE RT(I), I = 1, NUMST)	(14I5)
	NSTART = Start node numbers, 14 per card.	
7.1	NLINES	(15)
	NLINES = Number of surfaces along which applied pressure acts.	
7.2	LOADNP, ANAME	(15,18A4)
	LOADNP = Number of node points that are loaded by pressure on one surface (≤ 100).	
	ANAME = Pressure descriptor to be printed as output, up to 72 characters.	
7.3	NPLOAD, PRESSU, PRESSW	(15,2E10.0)
	NPLOAD = Node number of node to which pressure is applied.	
	PRESSU = Horizontal pressure (psi) applied to loaded surface at node number NPLOAD, positive in positive R-direction (to the right).	

CARD VARIABLE

FORMAT

PRESSW = Vertical pressure (psi) applied to loaded surface at node number NPLOAD, positive in positive z-direction (down).

Note: Card 7.3 repeated LOADNP times. Loaded node numbers, NPLOAD, are in consecutive order along pressure surface such that in moving from the first to the last the pressures are applied on the left hand side of the surface, and the boundary element is to the right of the surface.

Note: Card group 7.2 through 7.3 repeated NLines times. If NLines = 0, they are omitted.

8.1 NLines (I5)

NLines = Number of clusters of nodes to which concentrated loads are applied.

8.2 LOADNO, ANAME (I5,18A4)

LOADNP = Number of nodes in cluster (≤ 100)

ANAME = Concentrated load cluster descriptor to be printed as output, up to 72 characters.

8.3 NPLOAD, PLOADU, PLOADW (I5,2E10.0)

NPLOAD = Node number of node to which pressure is applied.

PLOADU = Horizontal force (lbs.) applied to node number NPLOAD, positive in positive R-direction (to the right).

PLOADW = Vertical force (lbs.) applied to node number NPLOAD, positive in positive Z-direction (down).

Note: Card 8.3 repeated LOADNP times.

Note: Card group 8.2 through 8.3 repeated NLines times. If NLines = 0, they are omitted.

9.1 NLines (I5)

NLines = Number of clusters of nodes for which displacements are specified.

9.2 LDISP, ANAME (I5,18A4)

LDISP = Number of nodes in displacement cluster.

ANAME = Displacement cluster descriptor to be printed as output, up to 72 characters.

CARD VARIABLE

FORMAT

9.3 NPDISP, UDISP, WDISP

(I5,2E10.0)

NPDISP = Node number of node for which displacements are specified.

UDISP = u - displacement (inches)¹

WDISP = w - displacement (inches)¹

Note: Card 9.3 is repeated LDISP times.

Note: Card group 9.2 through 9.3 repeated NLines times.
If NLines = 0, they are omitted.

10.1 ITMAX, ERRMAX, NFAC, KTAPE, ICONTU, OVERLX

(I5,E10.0,3I5,E10.0)

ITMAX = Maximum number of iterations to be used for each solution increment.

ERRMAX = Maximum allowable error (lbs) in force computations.

NFAC = Number of increments to be used in nonlinear solution to go from loads at which stresses reach the elastic limit to the actual loads.

KTAPE = Counter for use of tapes for storage of node point data,
= 0, uses two K tapes (normal usage),
= 1, uses only 1 K tape.

ICONTU = 0 will continue complete solution if ITMAX is reached in any load increment without convergence to within the allowable error, (ERRMAX).

OVERLX = Over-relaxation factor to reduce required number of iterations (usual values 1.2 - 1.8).

1. When ITYPE = 1 on card 3.1, only one displacement component is specified at the particular node point. This is equivalent to a roller support, as shown in Figure A.2, in which the roller is free to move along a line oriented at the angle θ (also input on card 3.1) with respect to the horizontal. The angle θ is considered positive when measured clockwise from the horizontal, and defines a new set of coordinates u, w as shown in Figure A.2. The direction in which the roller is free to move is defined as the $\bar{u}$ direction. The direction in which the displacement is specified is defined as the $\bar{w}$ direction. Thus in the case of the equivalent roller support node point, WDISP is the specified displacement in the $\bar{w}$ direction, and UDISP is ignored. Similarly in the output, the result given for u is the displacement in the $\bar{u}$ direction and w is the specified input displacement in the $\bar{w}$ direction.

Forces at nodes with equivalent roller supports are specified in the usual horizontal and vertical coordinate directions.

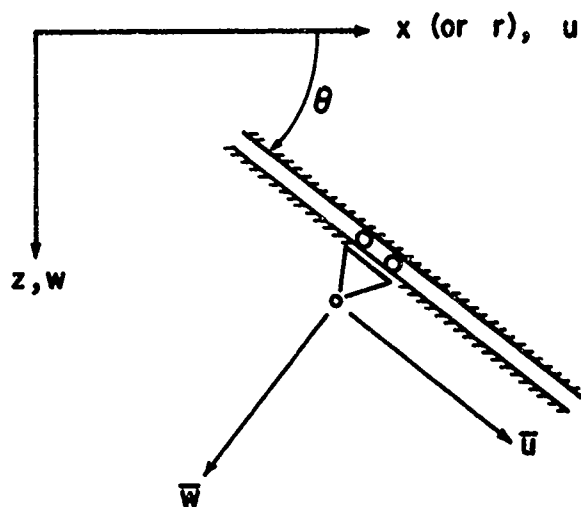


Figure A.2- Coordinate Directions for Equivalent Roller Support

A.3 - Listing of Code

```

OVERLAY(MOHAN,0,0)
PROGRAM SLAMT INPUT, OUTPUT, TAPE5=INPUT, TAPE6=OUTPUT,
1 TAPE1, TAPE2, TAPE3, TAPE4, TAPE8, TAPE9, TAPE10, TAPE11, TAPE12,
2 TAPE14, TAPE15, TAPE16)

```

```

COMMON MAXNP, MXCLS, MXADJP, MXZONE, MXNPB, NTONES, MXPELB, NUMNP,
1 NUMEL, ISTRES, NUMPEL, NUMELP, PERIOD, NMKCLS, FACTOR, ALAMB,
2 KTAPE, KRJN, IPRINT, NUMST, MXSTRT, IELAST(20), IPLAST(20),
3 WGT(20), NSTART(79), EI(5, 20), IPELTP, INT, NPRCDS, IMPBX

```

```

MXCLS=80
MXNPB=350
MAXNP=1600
MXADJP=8
MXZONE=20
MXSTRT=79
MXPELB=24
KRUN=0

```

```

MCHAN=5HMOFAN

```

```

CALL OVERLAY(MOHAN, 1, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 2, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 3, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 4, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 5, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 6, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 7, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 8, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 9, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 10, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 11, 0, 6HRECALL)

```

```

CALL OVERLAY(MOHAN, 12, 0, 6HRECALL)

```

```

STOP
END

```

```

C
C
C
OVERLAY(MOHAN,1,0)
PROGRAM LNK1A
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPELB,NUMNP,
1      NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2      KTAPE,KRJN,IPRINT,NUMST,MXSTRT,IELAST(20),IPLAST(20),
3      WGT(20),NSTART(79),EI(5,20),IPELTP,INT,NPRCDS,IMPBX
C
      DIMENSION R(1600),Z(1600),ITYPE(1600),THETA(1600)
C
C
      DIMENSION ANAME(18),SSTAR(10),HSTAR(10)
C
C**** SYSTEM DATA
      MOHAN=5HMOHAN
C
      READ(5,100) ANAME,NUMNP,NUMEL,ISTRES,IMPBX,IPRINT
100  FORMAT(18A4/14I5)
C
C      ANAME = PROBLEM TITLE
C      NUMNP = NO. OF NODE POINTS
C      NUMEL = NO. OF ELEMENTS
C      ISTRES=0  AXISYMMETRIC PROBLEM
C              =1  PLANE STRAIN PROBLEM
C              =2  PLANE STRESS PROBLEM
C      IPRINT=0  ONLY ECHO PRINT INPUT DATA
C              =1  ONLY PRINT ADJACENCY TABLE
C              =2  ONLY PRINT STIFFNESS TABLE
C              =3  ONLY PRINT STRESS TABLE
C              =4  ONLY PRINT MASS VECTOR
C              =5  ONLY PRINT LOAD TABLES
C              =6  ONLY PRINT RESULTS OF ELIMINATION
C              =7  ONLY PRINT STRESSES IN PLASTIC ELEMENTS
C              =99 PRINT ALL ABOVE TABLES
C
      WRITE(6,101) ANAME,NUMNP,NUMEL,IPRINT
101  FORMAT('1H',18A4//20H NO. OF NODE POINTS=,I5/
1      20H NO. OF ELEMENTS      =,I5/
2      20H IPRINT                =,I5)
      IF(ISTRES.NE.0) GO TO 103
      WRITE(6,104) ISTRES
104  FORMAT(20H ISTRES              =,I5,3X,20HAXISYMMETRIC PROBLEM)
      GO TO 102
103  IF(ISTRES.NE.1) GO TO 105
      WRITE(6,106) ISTRES
106  FORMAT(20H ISTRES              =,I5,3X,20HPLANE STRAIN PROBLEM)
      GO TO 102
105  IF(ISTRES.NE.2) GO TO 107
      WRITE(6,108) ISTRES
108  FORMAT(20H ISTRES              =,I5,3X,20HPLANE STRESS PROBLEM)
      GO TO 102
107  WRITE(6,109) ISTRES
109  FORMAT(20H ISTRES              =,I5,3X,20HERROR IN ISTRES DATA)
      CALL EXIT
C
C**** READ NODE POINT DATA AND STORE ON TAPE 14
C

```

```

102 PI=3.1415927

      REWIND 14
      WRITE(6,110)
110 FORMAT(1H1,15HNODE POINT DATA//12X,7HNODE PT,4X,4HTYPE,13X,
15HTHETA,15X,6HRADIUS,14X,5HDEPTH/14X,3HNO.,21X,9H(DEGREES),
214X,4H(FT),16X,4H(FT) //)
C
      DO 111 I=1,NJMNP
111 READ(5,112) NPN,R(NPN),Z(NPN),ITYPE(NPN),THETA(NPN)
112 FORMAT(15,2E10.4,110,E10.4)
C
C      NPN      =NODE POINT NO.
C      R        =RADIUS (FT)
C      Z        =DEPTH (FT)
C      ITYPE    =0 FREE NODE
C              =1 FIXED NODE IN ONE DIRECTION (EXCEPT Z-AXIS)
C              =2 FIXED NODE IN TWO DIRECTIONS(INCLUDING Z-AXIS)
C              =3 FREE NODE ON Z-AXIS (NOT NECESSARY TO SPECIFY)
C      THETA    =CLOCKWISE ANGLE OF ROLLER DIRECTION FROM
C              R-AXIS (DEGREES)
C*****
      IF (IMPBX.NE.1) GO TO 15
      REWIND 15
      WRITE(15)NUMNP,(R(I),Z(I),I=1,NUMNP)
C*****
C
15  DC 16 NPN=1,NJMNP
      IF(R(NPN).NE.0.0) GO TO 17
      IF(ITYPE(NPN).EQ.2) GO TO 17
      IF(ISTRES.NE.0) GO TO 17
      ITYPE(NPN)=3
17  WRITE(6,113) NPN,ITYPE(NPN),THETA(NPN),R(NPN),Z(NPN)
113 FORMAT(116.19,5X,1PSE20.7)
      R(NPN)=R(NPN)*12.0
      Z(NPN)=Z(NPN)*12.0
      THETA(NPN)=THETA(NPN)*PI/180.
      WRITE(14) NPN,R(NPN),Z(NPN),ITYPE(NPN),THETA(NPN)
16  CONTINUE
C
C**** READ ZONE PROPERTY DATA AND STORE ON TAPE 14
C
      READ(5,114) NZONES
114 FORMAT(14I5)
      WRITE(6,118)NZONES
118 FORMAT(1H1,18HZONE PROPERTY DATA/14H NO. OF ZONES=,I5)
      WRITE(14) NZONES
      DO 1 I=1,NZONES
      READ(5,119) IZ,ANAME
119 FORMAT(15,18A4)
C
C      NZONES=NO. OF MATERIAL ZONES
C      IZ     =ZONE NUMBER
C      ANAME  =ZONE DESCRIPTOR
C
      WRITE(6,120) IZ,ANAME
120 FORMAT(//13H ZONE NUMBER=,15,2X,18A4)
C
      READ(5,121) IELAST(IZ),IPLAST(IZ),WGT(IZ),(EI(J,IZ),J=1,5)
121 FORMAT(2I5,F10.0,5E10.0)

```

```

C
C      WGT      =UNIT WEIGHT (LB/FT3)
C      E1       =ELASTIC MODULUS (PSI) FOR ISOTROPIC MATERIAL
C      E2       =POISSON'S RATIO FOR ISOTROPIC MATERIAL
C      E3,4,5=PARAMETERS FOR ANISOTROPIC ELASTICITY
C      IELAST=1, ISOTROPIC ELASTICITY
C              =2, ANISOTROPIC ELASTICITY
C              =3, COMPRESSIBLE FLUID
C      IPLAST=0, LINEAR MATERIAL
C              =1, VON MISES PLASTICITY
C              =2, THREE-DIMEN. MOHR-COULOMB MATERIAL
C              =3, TWO-DIMEN. MOHR-COULOMB MATERIAL, JOINT ELEMENTS ONLY
C
C      WRITE(6,122) IELAST(IZ),IPLAST(IZ),WGT(IZ)
122  FORMAT(10X,20HIELAST      =,I5/
1      10X,20HIPLAST      =,I5/
2      10X,20HUNIT WEIGHT      =,1PE15.5,2X,3HPCF)
C
C      IF((IELAST(IZ).EQ.0).OR.(IELAST(IZ).GT.3)) GO TO 400
C      IF(IELAST(IZ).GT.1) GO TO 123
C      WRITE(6,124) EI(1,IZ),EI(2,IZ)
124  FORMAT(10X,20HELASTIC MODULUS      =,1PE15.5,2X,3HPSI/
1      10X,20HPOISSON,S RATIO      =,1PE15.5)
C      GO TO 500
123  IF(IELAST(IZ).GT.2) GO TO 125
C      WRITE(6,137) EI(1,IZ),EI(2,IZ),EI(3,IZ),EI(4,IZ),EI(5,IZ)
137  FORMAT(10X,20HE1      =,1PE15.5/
1      10X,20HE2      =,1PE15.5/
2      10X,20HE3      =,1PE15.5/
3      10X,20HE4      =,1PE15.5/
4      10X,20HE5      =,1PE15.5)
C      GO TO 500
125  WRITE(6,126) EI(1,IZ)
126  FORMAT(10X,20HBJLK MODULUS      =,1PE15.5,2X,3HPSI)
C      GO TO 500
C
400  WRITE(6,401)
401  FORMAT(19H ERROR IN ZONE DATA)
C      CALL EXIT
C
500  WRITE(14) IZ,IELAST(IZ),IPLAST(IZ),WGT(IZ),(EI(J,IZ),J=1,5)
C
C      IF(IPLAST(IZ).GT.3) GO TO 400
C      IF(IPLAST(IZ).EQ.0) GO TO 1
C      IF(IPLAST(IZ).GT.1) GO TO 200
C
C      MISES MATERIAL DATA
C
C      IF(IELAST(IZ).NE.1) GO TO 400
C      READ(5,114) NOYILD
C      READ(5,127) (SSTAR(J),J=1,NOYILD)
C      READ(5,127) (HSTAR(J),J=1,NOYILD)
127  FORMAT(7E10.4)
C
C      NOYILD=NO. OF NONELASTIC STRAIGHT LINE SEGMENTS ON
C      UNIAxIAL STRESS-STRAIN CURVE
C      SSTAR =STRESS AT BEGINNING OF SEGMENT (PSI)
C      HSTAR =SLOPE OF SEGMENT (PSI)
C
C      WRITE(6,128) NOYILD

```

```

128 FCRMAT(10X,20HNO. OF PL. SEGMENTS=,15)
WRITE(6,139) SSTAR(1)
139 FCRMAT(10X,20HSTRESS AT START =,1PE15.5,2X,3HP SI)
IF(NYILD.EQ.1) GO TO 141
WRITE(6,140) (SSTAR(J),J=2,NYILD)
140 FCRMAT(29X,1H=,1PE15.5,2X,3HP SI)
141 WRITE(6,142) HSTAR(1)
142 FCRMAT(10X,20HSLOPE OF EL. CURVE =,1PE15.5,2X,3HP SI)
IF(NYILD.EQ.1) GO TO 143
WRITE(6,140) (HSTAR(J),J=2,NYILD)
143 DO 129 J=1,NYILD
IF(HSTAR(J).GE.EI(1,IZ)) GO TO 400
129 HSTAR(J)=EI(1,IZ)*HSTAR(J)/(EI(1,IZ)-HSTAR(J))
WRITE(6,144) HSTAR(1)
144 FCRMAT(10X,20HSLOPE OF PL. CURVE =,1PE15.5,2X,3HP SI)
IF(NYILD.EQ.1) GO TO 145
WRITE(6,140) (HSTAR(J),J=2,NYILD)
145 WRITE(14) NYILD,(SSTAR(J),J=1,NYILD),(HSTAR(J),J=1,NYILD)
GO TO 1

```

C
C
C

THREE-DIMENSIONAL MOHR-COULOMB MATERIAL

```

200 IF(IPLAST(IZ).GT.2) GO TO 300
IF(IELAST(IZ).NE.1) GO TO 400
READ(5,127) COHESN,FRCTAN

```

C
C
C
C

COHESN=SOIL COHESION (PSI) FROM TRIAXIAL TEST
FRCTAN=FRICTION ANGLE (DEGREES)

```

WRITE(6,201) COHESN,FRCTAN
201 FCRMAT(10X,20HCOHESION, TRIAXIAL =,1PE15.5,2X,3HP SI /
1 10X,20HFRICITION ANGLE =,1PE15.5,2X,7HDEGREES)
FRCTAN=FRCTAN*PI/180.
ALPHA=(2./SQRT(3.))* SIN(FRCTAN)/(3.-SIN(FRCTAN))
CAPPA=(6./SQRT(3.))*COHESN*COS(FRCTAN)/(3.-SIN(FRCTAN))
WRITE(6,202) ALPHA,CAPPA
202 FCRMAT(10X,20HYIELD COEF, ALPHA =,1PE15.5/
1 10X,20HYIELD COEF, K =,1PE15.5,2X,3HP SI)
COSTH=SQRT(ALPHA**2./((ALPHA**2.+(1./6.)))
IF((ISTRES.EQ.2).AND.(CAPPA.EQ.0.0)) GO TO 400
WRITE(14) ALPHA,CAPPA,COSTH
GO TO 1

```

C
C
C

TWO-DIMEN. MOHR-COULOMB MATERIAL FOR JOINTS ONLY

```

300 READ(5,127) COHESN,FRCTN1,FRCTN2,SNSWCH

```

C
C
C
C
C
C
C
C
C

COHESN=COHESIVE COMPONENT OF PEAK STRENGTH (PSI)
FRCTN1=INITIAL PHI ANGLE FOR BILINEAR FAILURE ENVELOPE (DEGREES)
FRCTN2=PHI ANGLE FOR BILINEAR FAILURE ENVELOPE WHEN NORMAL
STRESS GREATER THAN SNSWCH (DEGREES)
SNSWCH=NORMAL STRESS AT WHICH BILINEAR FAILURE
ENVELOPE CHANGES SLOPE

```

WRITE(6,301) COHESN,FRCTN1,FRCTN2,SNSWCH
301 FCRMAT(10X,20HCOHESION =,1PE15.5,2X,3HP SI /
1 10X,20HINITIAL PHI ANGLE =,1PE15.5,2X,7HDEGREES /
2 10X,20HSECOND BILINEAR PHI =,1PE15.5,2X,7HDEGREES /
3 10X,17HNORMAL STRESS FOR
4 12X,17HBREAK IN BILINEAR /

```



```

5      12X,15HFAILURE ENVELOPE =,1PE15.5,3HP$1)
      SNSWCH=-SIGN CH
C
      READ(5,114) JTENSN, IRESID
C
      JTENSN=0, JOINT MATERIAL CAN WITHSTAND NO TENSION NORMAL TO JOINT
      =1, JOINT MATERIAL CAN WITHSTAND TENSION NORMAL TO JOINT
      UP TO MAGNITUDE COHESN/TAN(FRCTN1)
C
      IRESID=0, RESIDUAL SHEAR STRENGTH AFTER YIELD=PEAK SHEAR STRENGTH
      1, RESIDUAL SHEAR STRENGTH LESS THAN PEAK
C
      IF(JTENSN.EQ.0) WRITE(6,305)
305    FORMAT(10X,20HJOINT MATERIAL TAKES/
1       12X,17HNO TENSION NORMAL/
2       12X,8HTO JOINT)
C
      IF(IRESID.EQ.1) GO TO 310
      WRITE(6,307)
307    FORMAT(10X,14HRESIDUAL SHEAR/
1       12X,15HSTRENGTH = PEAK)
      CRESID = COHESN
      FRESID = FRCTN1
      GO TO 320
C
310    READ (5,127) CRESID, FRESID
C
      CRESID = RESIDUAL COHESION (PSI)
      FRESID = RESIDUAL PHI ANGLE (DEGREES)
C
      WRITE(6,315) CRESID, FRESID
315    FORMAT(10X,20HRESIDUAL COHESION =,1PE15.5,2X,3HP$1 /
1       10X,20HRESIDUAL PHI ANGLE =,1PE15.5,2X,7HDEGREES)
C
320    FRCTN1=FRCTN1*PI/180.
      FRCTN2=FRCTN2*PI/180.
      FRESID=FRESID*PI/180.
      MYIELD = 0
      WRITE(14)COHESN,FRCTN1,FRCTN2,SNSWCH,CRESID,
1       FRESID,MYIELD,IRESID,JTENSN
      GO TO 1
C
1    CONTINUE
C
C**** READ ELEMENT DATA, REORDER ELEMENT NODES, OUT ON TAPE 1
C    PLASTIC ELEMENTS ON TAPE 14
C
      REWIND 1
      NUMPEL=0
      WRITE(6,131)
131    FORMAT(11H,12HELEMENT DATA//12X,7HELEMENT,5X,4HZONE,6X,3HNPI,
17X,3HNPJ,7X,3HNPK,7X,3HNPL,7X,6HNCRAK/14X,3HNO. 8X,3HNO.//)
C*****
      IF(IMPBX.EQ.1)WRITE(15)NUMEL
C*****
C
      DO 7 M=1,NUMEL
      NPL=0
      READ(5,114) NJME, IZONE, NPI, VPJ, NPK, VPL, NCRACK

```

```

C
C     NUME = ELEMENT NUMBER
C     IZONE = ZONE NUMBER
C     NP = NODE POINT NUMBERS
C     NCRACK=0 REGULAR ELEMENT, =1 CRACK MODEL
C
C     CALL ORDER(NPI,NPJ,NPK,NPL,R,Z,ISTRES,KASE,MAXNP,NCRACK)
C     WRITE(1) NUME,IZONE,KASE,NPI,NPJ,NPK,NPL,NCRACK
C     WRITE(6,132) NUME,IZONE,NPI,NPJ,NPK,NPL,NCRACK
132  FORMAT(116,111,3X,16,4X,16,4X,16,4X,16,4X,16)
C*****
C     IF(IMPBX.EQ.1)WRITE(15)NUME,NPI,NPJ,NPK,NPL
C*****
C
C     IF(IPLAST(IZONE).EQ.0) GO TO 7
C     NUMPEL=NUMPEL+1
C     IF(NPL.NE.0) GO TO 133
C     ITL=0
C     THL=0.0
C     RL=0.0
C     ZL=0.0
C     GO TO 134
133  ITL=ITYPE(NPL)
C     THL=THETA(NPL)
C     RL=R(NPL)
C     ZL=Z(NPL)
134  WRITE(14) NUME,IZONE,IPLAST(IZONE),NPI,NPJ,NPK,NPL,NCRACK,
C     1ITYPE(NPI),ITYPE(NPJ),ITYPE(NPK),ITL,
C     2THETA(NPI),THETA(NPJ),THETA(NPK),THL,
C     3 R(NPI), R(NPJ), R(NPK),RL,
C     4 Z(NPI), Z(NPJ), Z(NPK),ZL
C
C     7 CONTINUE
C
C     WRITE(6,150) NUMPEL
150  FORMAT(11H1,26HNO. OF NONLINEAR ELEMENTS=,I5)
C
C**** STARTING NODE DATA FOR PATH ROUTINE
C
C     READ(5,114) NUMST
C     READ(5,114) (NSTART(I),I=1,NUMST)
C     NUMST = NO. OF STARTING NODES(LT.100)
C     NSTART=STARTING NODE NUMBERS
C
C     WRITE(6,135) NUMST
135  FORMAT(11H1,18HSTARTING NODE DATA//20H NO. OF START NODES=,I5//)
C     WRITE(6,136) (NSTART(I),I=1,NUMST)
136  FORMAT(22H STARTING NODE NUMBERS/(15X,10I7))
C
C**** AT THIS TIME, TAPE 14 HAS ORIGINAL NODE POINT DATA
C     ZONE DATA
C     PLASTIC ELEMENT DATA, ORIGINAL NODE CROD
C     TAPE 1 HAS ALL ORIGINAL ELEMENT DATA
C
C     REWIND 14
C     REWIND 1
C     RETURN
C     END

```

```

C
SUBROUTINE ORDER(NPI,NPJ,NPK,NPL,R,Z,ISTRES,KASE,MAXNP,NCRAK)
DIMENSION R(MAXNP),Z(MAXNP)
C
C*** ORDER NODE POINT LETTERING FOR ELEMENT AND DEFINE CASE
C
C      R      =RADIAL    COORDINATE OF NODE POINT
C      Z      =VERTICAL COORDINATE OF NODE POINT
C      KASE   =1, GENERAL TRIANGLE
C             =2, TRIANGLE, ONE NODE ON Z-AXIS
C             =3, TRIANGLE, TWO NODES ON Z-AXIS
C             =4, GENERAL RECTANGLE
C             =5, RECTANGLE, ONE NODE ON Z-AXIS
C             =6, RECTANGLE, TWO NODES ON Z-AXIS
C      ISTRES=0, AXISYMMETRIC PROBLEM
C             =1, PLANE STRAIN PROBLEM
C             =2, PLANE STRESS PROBLEM
C
      IF(NCRAK.EQ.0) GO TO 20
      A=(R(NPJ)-R(NPI))**2+(Z(NPJ)-Z(NPI))**2
      IF(A.GT.0.0) GO TO 21
      I=NPJ
      J=NPK
      K=NPL
      L=NPI
      NPI=I
      NPJ=J
      NPK=K
      NPL=L
21 CONTINUE
      KASE=4
      IF(R(NPI).NE.0.0) RETURN
      IF(ISTRES.EQ.0) KASE=6
      RETURN
20 CONTINUE
      NI=NPI
      NJ=NPJ
      NK=NPK
      NL=NPL
      IF(R(NI).LT.R(NJ)) GO TO 1
      IF(R(NI).NE.R(NJ)) GO TO 2
      IF(Z(NI).LT.Z(NJ)) GO TO 1
2  NI=NPJ
   NJ=NPI
   NPI=NI
   NPJ=NJ
1  IF(R(NI).LT.R(NK)) GO TO 3
   IF(R(NI).NE.R(NK)) GO TO 4
   IF(Z(NI).LT.Z(NK)) GO TO 3
4  NI=NPK
   NK=NPI
   NPI=NI
   NPK=NK
3  IF(NPL.EQ.0) GO TO 5
   IF(R(NI).LT.R(NL)) GO TO 5
   IF(R(NI).NE.R(NL)) GO TO 6
   IF(Z(NI).LT.Z(NL)) GO TO 5
6  NI=NPL
   NL=NPI
   NPI=NI

```

NPL=NL

C
C
C NODE POINT I IN PROPER LOCATION (CLOSEST TO ORIGIN)

5 AJ=R(NJ)-R(NI)
BJ=Z(NJ)-Z(NI)
AK=R(NK)-R(NI)
BK=Z(NK)-Z(NI)
HJK=AJ*BK-AK*BJ

IF(NPL.NE.0) GO TO 7

C
C
C TRIANGULAR ELEMENT

IF(HJK.GT.0.) GO TO 8

NPK=NJ

NPJ=NK

8 IF(R(NPI).EQ.0.) GO TO 9

10 KASE=1

RETURN

9 IF(ISTRES.NE.0) GO TO 10

IF(R(NPK).EQ.0.) GO TO 11

KASE=2

RETURN

11 KASE=3

RETURN

C
C
C RECTANGULAR ELEMENT

7 AL=R(NL)-R(NI)

BL=Z(NL)-Z(NI)

HJL=AJ*BL-AL*BJ

HKL=AK*BL-AL*BK

IF(HJK.GT.0.) GO TO 12

IF(HJL.GT.0.) GO TO 13

IF(HKL.GT.0.) GO TO 14

NPJ=NL

NPL=NJ

GO TO 15

14 NPJ=NK

NPK=NL

NPL=NJ

GO TO 15

13 NPJ=NK

NPK=NJ

GO TO 15

12 IF(HJL.GT.0.) GO TO 16

NPJ=NL

NPK=NJ

NPL=NK

GO TO 15

16 IF(HKL.GT.0.) GO TO 15

NPK=NL

NPL=NK

15 IF(R(NPI).EQ.0.) GO TO 17

18 KASE=4

RETURN

17 IF(ISTRES.NE.0) GO TO 18

IF(R(NPL).EQ.0.) GO TO 19

KASE=5

RETURN

```

19 KASE=6
   RETURN
   END

```

```

C
C
C

```

```

OVERLAY(MOHAN,2,0 )
PROGRAM LNK1B
COMMON MAXNP, MXCLS, MXADJP, MXZONE, MXNPB, NZONES, MXPELB, NUMNP,
1  NUMEL, ISTRS, NUMPEL, NUMELP, PERIOD, NMKCLS, FACTOR, ALAMB,
2  KTAPE, KRJN, IPRINT, NUMST, MXSTRT, IELAST(20), IPLAST(20),
3  WGT(20), NSTART(79), EI(5,20), IPELTP, INT, NPRCDS, IMPBX

```

```

C

```

```

DIMENSION NPADJ(1600,8), NADJNP(1600), NADJEL(1600)
MOHAN=5HMOHAN
DO 5 I=1, NUMNP
  NADJNP(I)=0
  NADJEL(I)=0
  DO 5 J=1, MXADJP
5  NPADJ(I,J)=0

```

```

C

```

```

REWIND 1
DO 7 M=1, NUMEL
  READ(1) NUME, IZONE, KASE, NPI, NPJ, NPK, NPL, NCRACK
7  CALL ADJNP(MXADJP, MAXNP, NUMNP, NPADJ, NADJEL, NUME, NPI, NPJ, NPK, NPL)
  CALL VADJNP(MXADJP, NADJNP, MAXNP, NUMNP, NPADJ)
  REWIND 1

```

```

C

```

```

REWIND 8
WRITE(8) (NADJNP(I), NADJEL(I), (NPADJ(I,J), J=1, MXADJP), I=1, NUMNP)
REWIND 8

```

```

C

```

```

IF((IPRINT.NE.1).AND.(IPRINT.NE.99))RETURN

```

```

C

```

```

WRITE(6,1)
1  FORMAT(1H1,38HTABLE OF ORIGINAL ADJACENT NODE POINTS//
2  4X,4HNODE,13X,6HNO. OF,4X,6HNO. OF,27X,2CHADJACENT NODE POINTS/
3  4X,5HPOINT,11X,9HADJ. PTS.,1X,9HADJ. ELS.,5X,1H1,9X,1H2,9X,1H3,
4  9X,1H4,9X,1H5,9X,1H9,9X,1H7,9X,1H8//)
DO 2 I=1, NUMNP
2  WRITE(6,3) I, NADJNP(I), NADJEL(I), (NPADJ(I,J), J=1, MXADJP)
3  FORMAT(18,8X,2I10,8I10)

```

```

C

```

```

RETURN
END

```

```

C

```

```

C

```

```

C

```

```

SUBROUTINE ADJNP(MXADJP, MAXNP, NUMNP, NPADJ, NADJEL, NUME,
1  NPI, NPJ, NPK, NPL)
  DIMENSION NPADJ(MAXNP, MXADJP), NADJEL(MAXNP), NA(4)

```

```

C

```

```

C*** FORM TABLE OF ADJACENT NODAL POINTS

```

```

C

```

```

C

```

```

C

```

```

C

```

```

C

```

```

C

```

```

C

```

```

  MXADJP=MAX. NO. OF ADJACENT NODAL POINTS ALLOWED
  MAXNP =MAX. NO. OF NODE POINTS
  NUMNP =NO. OF NODE POINTS
  NPADJ =ADJACENT NODE POINT NUMBER
  NADJEL=NUMBER OF ADJACENT ELEMENTS AT EACH NODE POINT

```

```

C      NPI   =ELEMENT NODE POINT I
C      NPJ   =ELEMENT NODE POINT J
C      NPK   =ELEMENT NODE POINT K
C      NPL   =ELEMENT NODE POINT L, IF 0, TRIANGULAR ELEMENT
C      NUME  =ELEMENT NUMBER BEING CONSIDERED
C      NOTE- TABLE ASSJMED TO BE ALREADY ZEROED OUT
C
      NA(1)=NPI
      NA(2)=NPJ
      NA(3)=NPK
      NA(4)=NPL
      ICOUNT=1
9      NPNUM=NA(1)
      NADJEL(NPNUM)=NADJEL(NPNUM)+1
      MX=NA(2)
      JCOUNT=1
5      DO 1 I=1,MXADJP
         J=I
         IF(NPADJ(NPNUM,I).EQ.MX) GO TO 2
         IF(NPADJ(NPNUM,I).EQ.0) GO TO 3
1      CONTINUE
      WRITE(6,10) NUME,NPNUM,MX,(NPADJ(NPNUM,I),I=1,MXADJP)
      CALL EXIT
C
3      NPADJ(NPNUM,J)=MX
2      JCOUNT=JCOUNT+1
      IF(JCOUNT.GT.3) GO TO 4
      IF(JCOUNT.GT.2) GO TO 102
      MX=NA(3)
      GO TO 5
102     MX=NA(4)
      IF(MX.EQ.0) GO TO 4
      GO TO 5
C
4      GO TO (6,7,8,103),ICOUNT
6      ICOUNT=2
      NA(1)=NPJ
      NA(2)=NPK
      NA(3)=NPI
      NA(4)=NPL
      GO TO 9
C
7      ICOUNT=3
      NA(1)=NPK
      NA(2)=NPI
      NA(3)=NPJ
      NA(4)=NPL
      GO TO 9
C
8      ICOUNT=4
      NA(1)=NPL
      IF(NA(1).EQ.0) GO TO 103
      NA(2)=NPI
      NA(3)=NPJ
      NA(4)=NPK
      GO TO 9
C
101    FORMAT(1H1,43HERROR IN FORMING ADJACENT NODAL POINT ARRAY/
121H ELEMENT NJMBER      =,15/21H NODE POINT NUMBER      =,15/
221H AD-ACENT NODE POINT=,15//15H NPADJ(NPNUM,I)/(21X,15))

```

57

```

C
103 RETURN
C
END
C
C
C
SUBROUTINE VADJNP(MXADJP,NADJNP,MAXNP,NUMNP,NPADJ)
DIMENSION NADJNP(MAXNP),VPADJ(MAXNP,MXADJP)
C
C**** FORM VECTOR INDICATING THE NUMBER OF ADJACENT NODE POINTS
C      AT EACH NODE POINT
C
C      MXADJP=MAX. NO. OF ADJACENT NODE POINTS ALLOWED
C      NADJNP=NO. OF ADJACENT NODE POINTS AT EACH NODE POINT
C      MAXNP=MAX. NO. OF NODE POINTS
C
C      NUMNP=NO. OF NODE POINTS
C      NPADJ=ADJACENT NODE POINT NUMBER
C
      DO 12 M=1,NUMNP
      DO 10 I=1,MXADJP
      J=I
      IF (NPADJ(M,I).EQ.0) GO TO 11
10  CONTINUE
      NADJNP(M)=MXADJP
      GO TO 12
11  NADJNP(M)=J-1
12  CONTINUE
      RETURN
C
END
C
C
C
OVERLAY(MOHAN,3,0)
PROGRAM LNK1C
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPELB,NUMNP,
1  NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2  KTAPE,KRJN,IPRINT,NUMST,MXSTRT,IELAST(20),IPLAST(20),
3  WGT(20),NSTART(79),EI(5,20),IPELTP,INT,NPRCDS,IMPBX
C
DIMENSION NPADJ(1600,8),NADJNP(1600),NADJEL(1600),NPTN(1600),
1  INPTP(1600),IGP(80),S(1600),NPLOW(80),NPHIGH(80),NPOLT(80),
2  NUMCP(80)
C
EQUIVALENCE (NADJEL(1),NPLOW),(NADJEL(81),NPHIGH),(NADJEL(161),
1  INPOLT),(NADJEL(241),NUMCP)
C
MOHAN=5HMOHAN
C
REWIND 8
READ(8) (NADJNP(I),NADJEL(I),(NPADJ(I,J),J=1,MXADJP),I=1,NUMNP)
REWIND 8
C
MAXBD=0
DO 5 I=1,NUMNP
NUM=NADJNP(I)
DO 5 J=1,NUM

```

```

      NP=NPADJ(I,J)
      NBAN=IABS(NP-I)
      IF(NBAN.LE.MAXBD) GO TO 6
      MAXBD=NBAN
6 CONTINUE
C
      CALL PATH(MAXNP,NUMNP,NUMST,VSTART,NPTN,NPTP,MXADJP,NADJNP,NPADJ,
      IIGP,NJMGP)
C
      WRITE(6,8) NUMGP
8 FORMAT(1H1,23H NO. OF BASIC SEGMENTS=,I5//
      123H PARTITION NEW NODE NO.//)
      WRITE(6,9) (I,IIGP(I),I=1,NUMGP)
9 FORMAT(2X,I5,10X,I5)
C
      WRITE(6,10) MAXBD
10 FORMAT(1H1/26H ORIGINAL HALF BAND WIDTH=,I5)
C
      MAXBD=0
      DO 11 I=1,NUMNP
      NPNEW=NPTP(I)
      NUM=NADJNP(I)
      DO 11 J=1,NUM
      MP=NPADJ(I,J)
      MPNEW=NPTP(MP)
      NBAN=IABS(NPNEW-MPNEW)
      IF(NBAN.LE.MAXBD) GO TO 11
      MAXBD=NBAN
11 CONTINUE
C
      WRITE(6,12) MAXBD
12 FORMAT(26H NEW HALF BAND WIDTH=,I5)
C
      CALL MIN1(MAXNP,NUMNP,NADJNP,MXADJP,NPADJ,NPTN,NPTP,S,MAXBD)
C
      IF(MAXBD.LT.MXNPB) GO TO 14
      WRITE(6,13)
13 FORMAT(//20H BANDWIDTH TOO LARGE)
      CALL EXIT
C
14 DO 15 I=1,NUMNP
      KN=NADJNP(I)
      DO 15 J=1,KN
      KT=NPADJ(I,J)
15 NPADJ(I,J)=NPTP(KT)
C
      DO 1 I=1,NUMNP
      KP=NPTN(I)
1 WRITE(8) I,NADJNP(KP),NADJEL(KP),(NPADJ(KP,J),J=1,MXADJP)
      REWIND 8
      DO 2 I=1,NUMNP
2 READ(8) I,NADJNP(I),NADJEL(I),(NPADJ(I,J),J=1,MXADJP)
C
      IF((IPRINT.NE.1).AND.(IPRINT.NE.99)) GO TO 19
      WRITE(6,16)
16 FORMAT(1H1,38H TABLE OF NEW ADJACENT NODE POINTS//
      14X,4HNEW,3X,4HOLD,6X,6HNO. OF,4X,6HNO. OF,27X,
      220HADJ+CENT NODE POINTS/4X,4HNODE,3X,4HNODE,5X,9HADJ. PTS.,
      31X,9HADJ. ELS.,5X,1H1,9X,1H2,9X,1H3,
      4 9X,1H4,9X,1H5,9X,1H6,9X,1H7,9X,1H8//)

```



```

      DC 17 I=1,NUMNP
      KP=NPTN(I)
17  WRITE(6,18) I,KP,NADJNP(I),NADJEL(I),(NPADJ(I,J),J=1,MXADJP)
18  FORMAT(2I8,2T10,8T10)
      GC TO 26

C
19  IF(IPRINT.EQ.0) GO TO 26
      WRITE(6,20)
20  FORMAT(1H1,21H NEW NUMBERING SCHEME//)
      DC 21 I=1,NUMNP,10
      IF((NUMNP-I).LT.10) IDUM=NUMNP
      IF((NUMNP-I).GE.10) IDJM=I+9
23  WRITE(6,24) (J,J=I,IDUM)
24  FORMAT(/20H ORIGINAL NODES      =,10I8)
21  WRITE(6,25) (NPTP(J),J=I,IDUM)
25  FORMAT(20H NEW      NODES      =,10I8)
26  CONTINUE

C
      CALL SIZE(MXCLS,NUMCLS,NPLOW,NPHIGH,VPOUT,NUMCP,NUMNP,MXADJP,
1 MXNPB,NADJNP,NPADJ,MAXNP)

C
      WRITE(8) NUMCLS,(NPLOW(I),NPHIGH(I),VPOUT(I),NUMCP(I),
1 I=1,NUMCLS)
      WRITE(8) (NPTN(I),NPTP(I),I=1,NUMNP)
      REWIND 8

C

      RETURN
      END

C
C
C
      SUBROUTINE PATH(MAXNP,NUMNP,NUMST,NSTART,NPTN,NPTP,MXADJP,
1 NADJNP,NPADJ,IGP,NUMGP)

C
      DIMENSION NSTART(NUMST),NPTN(MAXNP),NPTP(MAXNP),NADJNP(MAXNP),
1 NPADJ(MAXNP,MXADJP),IGP(1)

C
C      NUMST = NO. OF START NODES
C      NSTART=STARTING NODE NUMBERS
C      NPTN  = OLD NODE NOS. IN NEW ORDER
C      NPTP  = NEW NODE NOS. IN OLD ORDER
C      IGP   = LAST NODE IN PARTITION
C      NUMGP = NO. OF PARTITIONS

C
      KOUNT=0
      IN=1
      DC 1 I=1,NUMNP
      NPTN(I)=0
1  NPTP(I)=0

C
      DC 2 I=1,NUMST
      NP=NSTART(I)
      NPTP(NP)=IN
      KOUNT=KOUNT+1
2  NPTN(KOUNT)=NP

C
      IGP(IN)=KOUNT

C
4  DO 7 I=1,NUMNP

```

60

```

      IF(NPTP(I).NE.IN) GO TO 7
      NUM=NADJNP(I)
      DO 3 J=1,NUM
      NP=NPADJ(I,J)
      IF(NPTP(NP).NE.J) GO TO 3
      NPTP(NP)=IN+1
      KOUNT=KOJNT+1
      NPTN(KOUNT)=NP
      IF(KOUNT.EQ.NUMNP) GO TO 5
3 CONTINUE
7 CONTINUE

C
      IN=IN+1
      IGP(IN)=KOUNT
      GO TO 4

C
5 IGP(IN+1)=KOJNT
  NUMGP=IN+1

C
      DO 6 I=1,NUMNP
      NPOLD=NPTN(I)
6 NPTP(NPOLD)=I

C
      RETURN
      END

C
C
C
      SUBROUTINE MINIMAX(MAXNP,NUMNP,VADJNP,MXADJP,NPADJ,NPTN,NPTP,S,
1 MAXBDP)

C
      DIMENSION NADJNP(MAXNP),NPADJ(MAXNP,MXADJP),NPTN(MAXNP),
1 NPTP(MAXNP),S(MAXNP)

C
      S      =VECTOR OF WEIGHTING FACTORS
      MAXBDP=MAX. PREVIOUS BANDWIDTH
      MAXBC =MAX. BANDWIDTH

C
      COMPUTE WEIGHTING FACTORS FOR OLD ORDER

C
3 DO 1 I=1,NUMNP
  S(I)=FLCAT(I)
  NPOLD=NPTN(I)
  NUM=NADJNP(NPOLD)
  DO 2 J=1,NUM
  NADJ=NPADJ(NPOLD,J)
  NPNEW=NPTP(NADJ)
2 S(I)=S(I)+FLOAT(NPNEW)
1 S(I)=S(I)/FLOAT(NUM+1)

C
      SORT S VECTOR AND REORDER NUDES

C
      CALL SORT1(S,NPTN,MAXNP,NUMNP,1,1,1,1)

C
      C-MPUTE BANDWIDTH OF NEW ORDER

C
      MAXBD=0
      DO 14 I=1,NUMNP
      NPOLD=NPTN(I)
      NUM=NADJNP(NPOLD)

```

```

DC 10 J=1,NUM
NADJ=NPADJ(NPOLD,J)
DC 11 K=1,NUMNP
KK=K
IF(NADJ.EQ.NPTN(K)) GO TO 15
11 CCNTINUE
WRITE(6,100) I,NPOLD,(NPTN(L),L=1,NUMNP)
100 FORMAT(1H1,13HERROR IN MINI//10X,2I10//10X,1C110)
CALL EXIT
15 NPNEW=KK
NBAN=IABS(NPNEW-I)
IF(MAXBD.LT.NBAN) MAXBD=NBAN
10 CONTINUE
14 CONTINUE
C
WRITE(5,6T MAXBD
6 FCRMAT(26H NEW HALF BAND WIDTH=,15)
C
IF(MAXBDP.LE.MAXBD) GO TO 12
C
DC 5 I=1,NUMNP
NPOLD=NPTN(I)
5 NPTN(NPOLD)=I
MAXBDP=MAXBD
GO TO 3
C
12 DO 16 I=1,NUMNP
NPNEW=NPTN(I)
16 NPTN(NPNEW)=I
RETURN
END
C
C
C
SUBROUTINE SORT1(IARRAY,JARRAY,MXRCDS,NRECDS,
1IWRDS,JWRDS,IKEY,ISWT)
C
C DIMENSION IARRAY(MXRCDS,IWRDS),JARRAY(MXRCDS,JWRDS)
C
C IARRAY=ARRAY TO BE SORTED
C JARRAY=ASSOCIATED ARRAY THAT MAY BE SORTED AS IARRAY
C MXRCDS=MAX. NO. OF RECORDS IN ARRAYS
C NRECDS=NO. OF RECORDS TO BE SORTED
C IWRDS =WORDS PER RECORD IN IARRAY
C JWRDS =WORDS PER RECORD FOR JARRAY
C IKEY =LOCATION IN IARRAY RECORD OF SORT WORD
C ISWT =0 ONLY SORT IARRAY
C =1 ALSO SORT JARRAY
C
C LOGICAL CHECK
M=NRECDS-1
C
1 CHECK=.FALSE.
C
DC 5 I=1,2
C
DC 2 J=1,M,2
C
IF(IARRAY(J,IKEY).LE.IARRAY(J+1,IKEY)) GO TO 2
C

```

```

      DO 3 K=1,IWRDS
      ITEMP=IARRAY(J,K)
      IARRAY(J,K)=IARRAY(J+1,K)
3     IARRAY(J+1,K)=ITEMP
C
      IF(ISWT.EQ.0) GO TO 5
      DO 4 K=1,JWRDS
      JTEMP=JARRAY(J,K)
      JARRAY(J,K)=JARRAY(J+1,K)
4     JARRAY(J+1,K)=JTEMP
C
      5 CHECK=.TRUE.
C
      2 CCNTINUE
      6 CCNTINUE
C
      IF(CHECK) GO TO 1
      RETURN
      END
C
C
C
      SUBROUTINE SIZE(MXCLS,NUMCLS,NPLOW,NPHIGH,NPOUT,NUMCP,NUMNP,
1 MXADJP,MXNPB,NADJNP,NPADJ,MAXNP)
C
      DIMENSION NPLOW(MXCLS),NPHIGH(MXCLS),NPOUT(MXCLS),NUMCP(MXCLS)
C
      DIMENSION NADJNP(MAXNP),NPADJ(MAXNP,MXADJP)
C
      DO 1 I=1,MXCLS
      NPLOW(I)=0
      NPHIGH(I)=0
      NPOUT(I)=0
      1 NUMCP(I)=0
C
      ICOUNT=1
      NPOUT(1)=0
      NPLOW(1)=1
C
      NP=1
C
      8 MPLRG=0
      MPSML=NUMNP
      NPLRG=0
      NPSML=NUMNP
C
      7 NUM=NADJNP(NP)
      LPLRG=0
      LPSML=NUMNP
      DO 2 J=1,NUM
      NPNUM=NPADJ(NP,J)
      IF(NPNUM.GT.NPLRG) NPLRG=NPNUM
      IF(NPNUM.GT.LPLRG) LPLRG=NPNUM
      IF(NPNUM.LT.NPSML) NPSML=NPNUM
      IF(NPNUM.LT.LPSML) LPSML=NPNUM
      2 CONTINUE
C
      IF((LPLRG-LPSML+1).LE.MXNPB) GO TO 3
C
      IDUMMY=LPLRG-LPSML+1

```

```

WRITE(6,103) NP, IDUMMY
103 FORMAT(1H1,13HERROR IN SIZE/29H BAND WIDTH TOO LARGE AT NODE,
115/12H BAND WIDTH=,15)
CALL EXIT

C
3 IF((NPLRG-NPSML+1).GT.MXNPB) GO TO 4
C
IF(MPLRG.LT.NPLRG) MPLRG=NPLRG
IF(MPSML.GT.NPSML) MPSML=NPSML
IF(NP.GE.NUMNP) GO TO 5
NP=NP+1
GO TO 7

C
5 NPOUT(ICOUNT)=MPSML-1
NUMCP(ICOUNT)=MPLRG
NPHIGH(ICOUNT)=NP
6 NUMCLS=ICOUNT
C
DO 9 I=1,NUMCLS
IF(NPOUT(I).GE.NPLOW(I)) NPOUT(I)=NPLOW(I)-1
IF(NUMCP(I).LT.NPHIGH(I)) NUMCP(I)=NPHIGH(I)
9 CONTINUE
C
RETURN
C
4 NPOUT(ICOUNT)=MPSML-1
NUMCP(ICOUNT)=MPLRG
NPHIGH(ICOUNT)=NP-1
ICOUNT=ICOUNT+1
IF(ICOUNT.LE.MXCLS) GO TO 101
WRITE(6,102) NP,NPHIGH(ICOUNT),NUMNP,ICOUNT
102 FORMAT(1H1,25HTOO MANY CLUSTERS IN SIZE//
110X,4I10)
CALL EXIT

C
101 NPLOW(ICOUNT)=NPHIGH(ICOUNT-1)+1
GO TO 8
C
C
C
C
END
C
C
C
OVERLAY(MOHAN,4,0)
PROGRAM LNK1D
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPELB,NUMNP,
1 NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2 KTAPE,KRJN,IPRINT,NUMST,MXSTRT,IELAST(20),IPLAST(20),
3 WGT(20),NSTART(79),EI(5,20),IPELTP,INT,NPRCOS,IMPBX
C
DIMENSION NPTP(1600),NPTN(1600)
DIMENSION NPLOW(80),NPHIGH(80),NPOUT(80),NUMCP(80),NELCLS(80)
1,NMPCLS(80)
DIMENSION TMP(15000),NTMP(15000),SS(10),HS(10)
C
EQUIVALENCE (TMP,NTMP)
MOHAN=5HMOHAN
REWIND 1

```

```

REWIND 3
REWIND 8
DO 1 I=1,NUMNP
1 READ(8) I,NTMP(I),NTMP(I),(NTMP(J),J=1,MXADJP)
  READ(8) NUMCLS,(NPLOW(I),NPHIGH(I),NPOUT(I),NUMCP(I),
  1I=1,NUMCLS)
  READ(8) (NPTN(I),NPTP(I),I=1,NUMNP)
REWIND 8

```

C

```

DO 27 NN=1,NUMEL
  READ(1) N,IZ,KASE,I,J,K,L,NC
  I=NPTP(I)
  J=NPTP(J)
  K=NPTP(K)
  IF(L.EQ.0) GO TO 29
  L=NPTP(L)
  KEY=MINO(I,J,K,L)
  GO TO 27
29 KEY=MINO(I,J,K)
27 WRITE(3) KEY,N,IZ,KASE,I,J,K,L,NC
  MXRCDS=15000/9
  NWRDS=9
  CALL GSCRT(NTMP,NUMEL,NWRDS,1,MXRCDS,3,1,4,12)

```

C

```

REWIND 14
REWIND 3
DO 30 N=1,NUMNP
  READ(14) I,R,D,IT,TH
  NP=NPTP(I)
30 WRITE(3) NP,R,D,IT,TH
  MXRCDS=15000/5
  NWRDS=5
  CALL GSCRT(NTMP,NUMNP,NWRDS,1,MXRCDS,3,4,10,12)

```

C

```

DO 31 I=1,NUMNP
31 READ(4) N,R,D,IT,TH
  READ(14) NZONES
  WRITE(4) NZONES
  DO 32 NN=1,NZONES
    READ(14) I,IE,IP,w,(EI(J),J=1,5)
    WRITE(4) I,IE,IP,w,(EI(J),J=1,5)
    IF(IP.EQ.0) GO TO 32
    IF(IP.GT.1) GO TO 33
    READ(14) N,(SS(J),J=1,N),(HS(J),J=1,N)
    WRITE(4) N,(SS(J),J=1,N),(HS(J),J=1,N)
    GO TO 32
33 IF(IP.GT.2) GO TO 48
    READ(14) A,B,C
    WRITE(4) A,B,C
    GO TO 32
48 IF(IP.GT.3) GO TO 34
    READ(14) A,B,C,D,E,F,MYIELC,IRESID,JTENSIN
    WRITE(4) A,B,C,D,E,F,MYIELC,IRESID,JTENSIN
    GO TO 32
34 WRITE(6,35) IP,N
35 FORMAT(1H1,21HERROR IN IELC, IPLAST=,15,9H FOR ZONE,15)
  CALL EXIT
32 CONTINUE

```

C

```

DO 40 I=1,MXCLS

```

```

40 NELCLS(I)=0
   NUMELP=0
   IF (NUMPEL.EQ.0) GO TO 39
   REWIND 3
   DO 36 NN=1,NUMPEL
   READ(14) N,IZ,IP,I,J,K,L,NC,ITI,ITJ,ITK,ITL,THI,THJ,THK,THL,
1 RI,RJ,RK,RL,ZI,ZJ,ZK,ZL
   I=NPTP(I)
   J=NPTP(J)
   K=NPTP(K)
   IF (L.EQ.0) GO TO 37
   L=NPTP(L)
37 DO 41 JJ=1,NUMCLS
   IF ((I.GE.NPLOW(JJ)).AND.(I.LE.NPHIGH(JJ))) GO TO 42
   IF ((J.GE.NPLOW(JJ)).AND.(J.LE.NPHIGH(JJ))) GO TO 42
   IF ((K.GE.NPLOW(JJ)).AND.(K.LE.NPHIGH(JJ))) GO TO 42
   IF ((L.GE.NPLOW(JJ)).AND.(L.LE.NPHIGH(JJ))) GO TO 42
   GO TO 41
42 WRITE(3) JJ,N,IZ,IP,I,J,K,L,NC,ITI,ITJ,ITK,ITL,THI,THJ,THK,
1 THL,RI,RJ,RK,RL,ZI,ZJ,ZK,ZL
   NUMELP=NUMELP+1
   NELCLS(JJ)=NELCLS(JJ)+1
41 CONTINUE
36 CONTINUE
   REWIND 3
   REWIND 14
   MXRCDS=15000/25
   NWRDS=25
   CALL GSORT(NTMP,NUMELP,NWRDS,1,MXRCDS,3,14,10,12)
   REWIND 14
   DO 38 NN=1,NUMELP
   READ(14) JJ,N,IZ,IP,I,J,K,L,NC,ITI,ITJ,ITK,ITL,
1 THI,THJ,THK,THL,RI,RJ,RK,RL,ZI,ZJ,ZK,ZL
38 WRITE(4) JJ,N,IZ,IP,I,J,K,L,NC,ITI,ITJ,ITK,ITL,
1 THI,THJ,THK,THL,RI,RJ,RK,RL,ZI,ZJ,ZK,ZL
39 REWIND 14
   REWIND 4
   DO 47 IC=1,NJMCLS
   KC=0
   IF ((NELCLSTIC)/MXPELB)*MXPELB.LT.NELCLS(IC) KC=1
   IF (NELCLS(IC).EQ.0) KC=0
   NMPCLS(IC)=NELCLS(IC)/MXPELB+KC
47 CONTINUE
   DO 43 I=1,NJMNP
43 READ(8) I,NTMP(I),NTMP(I),(NTMP(J),J=1,MXADJP)
   WRITE(8) NUMCLS,(NPLOW(I),NPHIGH(I),NPOUT(I),NUMCP(I),NELCLS(I),
1 NMPCLS(I),I=1,NJMCLS)
   WRITE(8) (NPTN(I),NPTP(I),I=1,NUMNP)
   REWIND 8
   WRITE(6,44) NJMCLS
44 FORMAT(1H1,10HCLUSTERING//10X,16HNO. OF CLUSTERS=,I5//
110X,5HNPLOW,5X,6HNPHIGH,4X,5HNPOUT,5X,5HNUMCP,5X,6HNELCLS,5X,
26HNMPCLS//)
   WRITE(6,45) (NPLOW(I),NPHIGH(I),NPOUT(I),NUMCP(I),NELCLS(I),
1 NMPCLS(I),I=1,NJMCLS)
45 FORMAT(5X,6.10)
C
   WRITE(6,46) NUMELP
C
46 FORMAT(//35H NO. OF NONLINEAR ELEMENTS ON TAPE=,I5)

```

RETURN
END

SUBROUTINE GSORT(IARRAY,NRCDS,NWRDS,NKEY,MXRCDS,
1 INTAPE, IOUTAP, INT1, INT2)

DIMENSION IARRAY(MXRCDS,NWRDS)

IARRAY=BUFFER STORAGE REGION

NRCDS =NO. OF RECORDS IN ARRAY TO BE SORTED

NWRDS =NO. OF WORDS PER RECORD

NKEY =LOCATION OF WORD TO BE SORTED ON

MXRCDS=MAX. SIZE OF BUFFER REGION AVAILABLE (INPUT)

INTAPE=INPUT TAPE WITH ORIGINAL DATA

IOUTAP=OUTPUT TAPE WITH REORDERED DATA

INT =INTERMEDIATE TAPES

REWIND INTAPE

REWIND IOUTAP

REWIND INT1

REWIND INT2

IF(NRCDS.GT.MXRCDS) GO TO 1

C**** INTERNAL SORT ONLY REQUIRED

DO 2 I=1,NRCDS

2 READ(INTAPE) (IARRAY(I,J),J=1,NWRDS)

CALL SORT2(IARRAY,IARRAY,MXRCDS,NRCDS,NWRDS,NWRDS,NKEY,0)

DO 3 J=1,NRCDS

3 WRITE(IOUTAP) (IARRAY(I,J),J=1,NWRDS)

REWIND INTAPE

REWIND IOUTAP

RETURN

C**** TAPE SORT ROUTINE REQUIRED

1 IXRCDS=(MXRCDS/4)

CALL TSORT(IARRAY,NRCDS,IXRCDS,NWRDS,NKEY,

1 INTAPE, IOUTAP, INT1, INT2)

RETURN

END

SUBROUTINE SORT2(IARRAY,JARRAY,MXRCDS,NRECDS,
1 IWRDS, JWRDS, IKEY, ISWT)

DIMENSION IARRAY(MXRCDS,IWRDS),JARRAY(MXRCDS,JWRDS)

IARRAY=ARRAY TO BE SORTED

JARRAY=ASSOCIATED ARRAY THAT MAY BE SORTED AS IARRAY

MXRCDS=MAX. NO. OF RECORDS IN ARRAYS

NRECDS=NO. OF RECORDS TO BE SORTED

IWRDS =WORDS PER RECORD FOR IARRAY

JWRDS =WORDS PER RECORD FOR JARRAY

IKEY =LOCATION IN IARRAY RECORD OF SORT WORD

ISWT =0 ONLY SORT IARRAY

=1 ALSO SORT JARRAY


```

C
LOGICAL CHECK
M=NRECD5-1
C
1 CHECK=.FALSE.
C
DO 6 I=1,2
C
DO 2 J=1,M,2
C
IF(IARRAY(J,IKEY).LE.IARRAY(J+1,IKEY)) GO TO 2
C
DO 3 K=1,IWRDS
ITEMP=IARRAY(J,K)
IARRAY(J,K)=IARRAY(J+1,K)
3 IARRAY(J+1,K)=ITEMP
C
IF(ISWT.EQ.0) GO TO 5
DO 4 K=1,IWRDS
JTEMP=JARRAY(J,K)
JARRAY(J,K)=JARRAY(J+1,K)
4 JARRAY(J+1,K)=JTEMP
C
5 CHECK=.TRUE.
C
2 CONTINUE
6 CONTINUE
C
IF(CHECK) GO TO 1
RETURN
END
C
C
C
SUBROUTINE TSORT(IARRAY,NRCDS,IXRCD5,NWRDS,NKEY,
1INTAPE,IOUTAP,INT1,INT2)
C
DIMENSION IARRAY(IXRCD5,NWRDS,4),CHECK(2),ISWT(2),IOUT(2),JNUM(2)
C
LOGICAL CHECK
C
C**** READ INTAPE, SORT GROUPS, AND SPLIT ONTO INT1 AND INT2
C
IRCD5=0
ISWC=0
JNUM(1)=0
JNUM(2)=0
KTAPE=INT1
3 IF((IRCD5+IXRCD5).LE.NRCDS) KN=IXRCD5
IF((IRCD5+IXRCD5).GT.NRCDS) KN=NRCDS-IRCD5
DO 1 I=1,KN
1 READ(INTAPE) (IARRAY(I,J,1),J=1,NWRDS)
IRCD5=IRCD5+KN
CALL SORT2(IARRAY,IARRAY,IXRCD5,KN,NWRDS,NWRDS,NKEY,0)
WRITE(KTAPE) KN,((IARRAY(I,J,1),J=1,NWRDS),I=1,KN)
IF(ISWC.EQ.0) GO TO 2
ISWC=0
JNUM(2)=JNUM(2)+1
KTAPE=INT1
GO TO 4

```

```

2 ISWC=1
  JNUM(1)=JNUM(1)+1
  KTAPE=INT2
4 IF(IRCDS.LT.NRCDS) GO TO 3

```

```

C
C**** SORT RECORD CLUSTERS ON INT1 AND INT2
C

```

```

  REWIND INTAPE
  REWIND IOUTAP
  REWIND INT1
  REWIND INT2
  DC 5 I=1,2

```

```

5 ISWT(I)=0
  KP1=INT1
  KP2=INTAPE
  LP1=INT2
  LP2=IOUTAP
  ICOUNT=2

```

```

C
C

```

```

900 I=1

```

```

C

```

```

  CHECK(1)=.FALSE.
  CHECK(2)=.FALSE.

```

```

C

```

```

902 IF(ICOUNT.EQ.1) GO TO 800
  ICOUNT=1
  GO TO 901

```

```

800 ICOUNT=2

```

```

C

```

```

901 DC 100 NC=1,2

```

```

C

```

```

  IF(ICOUNT.EQ.2) GO TO 6

```

```

C

```

```

  IF(NC.EQ.2) GO TO 7
  KT1=KP1
  KT2=KP2
  GO TO 8

```

```

C

```

```

7 KT1=LP1
  KT2=LP2
  GO TO 8

```

```

C

```

```

6 IF(NC.EQ.2) GO TO 9
  KT1=KP2
  KT2=KP1
  GO TO 8

```

```

C

```

```

9 KT1=LP2
  KT2=LP1

```

```

C

```

```

8 IF(ISWT(NC).EQ.1) GO TO 100

```

```

C

```

```

C**** READ FIRST TWO SORTED CLUSTERS
C

```

```

  J=1
  READ(KT1) KN1,((IARRAY(K,L,1),L=1,NWRDS),K=1,KN1)
  IF(I.EQ.1) GO TO 200
  WRITE(KT2) KN1,((IARRAY(K,L,1),L=1,NWRDS),K=1,KN1)
  J=2

```

```

      READ(KT1) KN1,((IARRAY(K,L,1),L=1,NWRDS),K=1,KN1)
200  J=J+1
      READ(KT1) KN2,((IARRAY(K,L,2),L=1,NWRDS),K=1,KN2)
C
C**** SORT THE CLUSTERS
C
201  K=1
      L=1
      M=1
      M2=KN1+KN2
C
207  IF(IARRAY(K,NKEY,1).LE.IARRAY(L,NKEY,2)) GO TO 202
      CHECK(NC)=.TRUE.
      IF(M.GT.IXRCDS) GO TO 203
      M1=M
      IS=3
      GO TO 204
203  M1=M-IXRCDS
      IS=4
204  DO 205 N=1,NWRDS
205  IARRAY(M1,N,IS)=IARRAY(L,N,2)
      M=M+1
      IF(M.GT.M2) GO TO 300
      L=L+1
      IF(L.LE.KN2) GO TO 207
      DO 208 KM=K,KN1
      IF(M.GT.IXRCDS) GO TO 209
      M1=M
      IS=3
      GO TO 210
209  M1=M-IXRCDS
      IS=4
210  DO 211 N=1,NWRDS
211  IARRAY(M1,N,IS)=IARRAY(KM,N,1)
208  M=M+1
      GO TO 300
C
202  IF(M.GT.IXRCDS) GO TO 212
      M1=M
      IS=3
      GO TO 213
212  M1=M-IXRCDS
      IS=4
213  DO 214 N=1,NWRDS
214  IARRAY(M1,N,IS)=IARRAY(K,N,1)
      M=M+1
      IF(M.GT.M2) GO TO 300
      K=K+1
      IF(K.LE.KN1) GO TO 207
      DO 215 LM=L,KN2
      IF(M.GT.IXRCDS) GO TO 216
      M1=M
      IS=3
      GO TO 217
216  M1=M-IXRCDS
      IS=4
217  DO 218 N=1,NWRDS
218  IARRAY(M1,N,IS)=IARRAY(LM,N,2)
215  M=M+1
      GO TO 300

```

70

```

C
C**** WRITE TWO MERGED ARRAYS ONTO 2ND TAPE
C
300 WRITE(KT2) KN1,((IARRAY(K,L,3),L=1,NWRDS),K=1,KN1)
    WRITE(KT2) KN2,((IARRAY(K,L,4),L=1,NWRDS),K=1,KN2)
C
    IF(J.GE.(JNUM(NC)-1)) GO TO 219
    READ(KT1) KN1,((IARRAY(K,L,1),L=1,NWRDS),K=1,KN1)
    READ(KT1) KN2,((IARRAY(K,L,2),L=1,NWRDS),K=1,KN2)
    J=J+2
    GO TO 201
C
219 ICUT(NC)=KT2
    IF(J.EQ.JNUM(NC)) GO TO 220
    READ(KT1) KN1,((IARRAY(K,L,1),L=1,NWRDS),K=1,KN1)
    WRITE(KT2) KN1,((IARRAY(K,L,1),L=1,NWRDS),K=1,KN1)
220 REWIND KT1
    REWIND KT2
C
100 CONTINUE
C
    I=I+1
C
    IF(JNUM(1).EQ.2) ISWT(1)=1
    IF(JNUM(2).EQ.2) ISWT(2)=1
C
    IF(I.EQ.2) GO TO 902
C
    IF(.NOT.CHECK(1)) ISWT(1)=1
    IF(.NOT.CHECK(2)) ISWT(2)=1
    IF((ISWT(1).EQ.1).AND.(ISWT(2).EQ.1)) GO TO 101
    GO TO 900
C
C**** MUST NOW MERGE THE TWO ORDERED TAPES
C
101 KP1=IOUT(1)
    KP2=IOUT(2)
    REWIND INTAPE
    REWIND IOUTAP
    REWIND INT1
    REWIND INT2
    IF(KP1.NE.INTAPE) GO TO 103
    IF(KP2.NE.IOUTAP) GO TO 102
106 INTER=INT1
    GO TO 400
102 INTER=ICUTAP
    GO TO 400
103 IF(KP1.NE.IOUTAP) GO TO 105
    IF(KP2.NE.INTAPE) GO TO 104
    GO TO 106
104 INTER=INTAPE
    GO TO 400
105 IF(KP1.NE.INT1) GO TO 107
    IF(KP2.NE.INTAPE) GO TO 108
    GO TO 102
108 IF(KP2.NE.IOUTAP) GO TO 102
    INTER=INTAPE
    GO TO 400
107 IF(KP2.NE.IOUTAP) GO TO 102
    GO TO 106

```

```

C
C   FIRST TAPE IS KP1 , HAS JNUM(1) CLUSTERS
C   SECOND TAPE IS KP2, HAS JNUM(2) CLUSTERS
C   MERGED TAPE IS INTER
C
400 J1=1
    J2=1
405 READ(KP1) KN1,((IARRAY(I,J,1),J=1,NWRDS),I=1,KN1)
    READ(KP2) KN2,((IARRAY(I,J,2),J=1,NWRDS),I=1,KN2)
C
    K=1
    L=1
402 IF(IARRAY(K,NKEY,1).LE.IARRAY(L,NKEY,2)) GO TO 401
    WRITE(INTER) (IARRAY(L,N,2),N=1,NWRDS)
    L=L+1
    IF(L.LE.KN2) GO TO 402
    DO 403 KN=K,KN1
403 WRITE(INTER) (IARRAY(KN,N,1),N=1,NWRDS)
    GO TO 500
401 WRITE(INTER) (IARRAY(K,N,1),N=1,NWRDS)
    K=K+1
    IF(K.LE.KN1) GO TO 402
    DO 404 LN=L,KN2
404 WRITE(INTER) (IARRAY(LN,N,2),N=1,NWRDS)
    GO TO 500
C
C
500 J1=J1+1
    J2=J2+1
    IF((J1.LE.JNUM(1)).AND.(J2.LE.JNUM(2))) GO TO 405
    IF((J1.GT.JNUM(1)).AND.(J2.GT.JNUM(2))) GO TO 600
C
    IF(J2.GT.JNUM(2)) GO TO 501
503 READ(KP2) KN2,((IARRAY(I,J,2),J=1,NWRDS),I=1,KN2)
    DO 502 I=1,KN2
502 WRITE(INTER) (IARRAY(I,N,2),N=1,NWRDS)
    J2=J2+1
    IF(J2.GT.JNUM(2)) GO TO 600
    GO TO 503
C
501 READ(KP1) KN1,((IARRAY(I,J,1),J=1,NWRDS),I=1,KN1)
    DO 504 I=1,KN1
504 WRITE(INTER) (IARRAY(I,N,1),N=1,NWRDS)
    J1=J1+1
    IF(J1.GT.JNUM(1)) GO TO 600
    GO TO 501
C
600 REWIND KP1
    REWIND KP2
    REWIND INTER
    IF(INTER.EQ.IOUTAP) RETURN
    DO 601 I=1,NRCLS
    READ(INTER)(IARRAY(I,J,1),J=1,NWRDS)
601 WRITE(ICUTAP) (IARRAY(I,J,1),J=1,NWRDS)
    REWIND INTER
    REWIND IOUTAP
    RETURN
C
C
C

```

C
C
C

C

C

C

C

C

1

C

C

C

C

C

C

C

C

C

5

C

C

C

1

C

```

      ICOUNT=0
4  READ(1)  KEY, NUME, IZONE, KASE, NTI, NTJ, NTK, NTL, NCRACK
      ICOUNT= ICOUNT +1

```

```

      LNP=MAX0(NTI,NTJ,NTK,NTL)
C
      IF((LNP-NPCUT).GT.MXNPB) GO TO 100
C
      SUFFICIENT ROOM IN BUFFER REGION
C
6     NPI=NTI-NPCUT
      NPJ=NTJ-NPCUT
      NPK=NTK-NPCUT
      IF(NTL.EQ.0) NPL=0
      IF(NTL.NE.0) NPL=NTL-NPCUT
      S1=0.0
      C1=0.0
      IE=IELAST(IZONE)
      A1=EI(1,IZCNE)
      A2=EI(2,IZCNE)
      A3=EI(3,IZCNE)
      A4=EI(4,IZCNE)
      A5=EI(5,IZCNE)
      RHO=WGT(IZCNE)/(386.4*1728.)
      CALL ELAST(IE,ISTRES,A1,A2,A3,A4,A5,C,NUME)
      CALL STIFFTKASE,NPI,NPJ,NPK,NPL,NUME,MXNPB,ISTRES,C,R,Z,CK,AINT,
      IS1,C1,NCRAK)
      CALL ADJUSK(MXNPB,CK,ITYPE,THETA,NPI,NPJ,NPK,NPL)
      CALL DISTK(MXNPB,MXADJP,CK,SNPUU,SNPUW,SNPWW,SADUU,SADLU,SADWL,
      ISADWW,NPI,NPJ,NPK,NPL,NPACJ,NPCUT)
      IF(NCRAK.EQ.1) GO TO 5
      CALL MASS(MXNPB,RHO,R,Z,AINT,XMASS,S1,C1,NPI,NPJ,NPK,NPL,ISTRES)
5     WRITE(12) KEY,NUME,IZONE,NTI,NTJ,NTK,NTL,NCRAK,
      1((C(I,J),I=1,4),J=1,4),KASE,S1,C1
C
      IF(ICOUNT.LT.NUMEL) GO TO 4
      KEND=1
      KEY=NUMNP+I
C
C**** INSUFFICIENT ROOM IN BUFFER REGION
C
100    NUMCP=KEY-1
      NUMNPB=NUMCP-NPCUT
C
      PRINT STIFFNESS TABLES
C
113   CALL PRNK(MXNPB,MXADJP,NADJNP,NPADJ,NADJEL,IPRINT,SNPUU,
      1SNPUW,SNPWW,SADJU,SADUW,SADWU,SADWW,THE TA,ITYPE,XMASS,NPCUT,
      2NUMNPB)
C
      WRITE STIFFNESS TABLES ONTO TAPE 10 WITH MASS VECTOR
C
      DO 101 I=1,NUMNPB
101   WRITE(10) I ,NADJNP(I),ITYPE(I),THETA(I),XMASS(I),SNPUU(I),
      1SNPUW(I),SNPWW(I),(NPADJ(I,J),SADUU(I,J),SADUW(I,J),SADWU(I,J),
      2SADWW(I,J),J=1,MXADJP)
C
      IF(KEND.EQ.1) RETURN
      NPR=MXNPB-NUMNPB
      GO TO 902
C
      ZERO REMAINING BUFFER AREA
C
107   KX=NPR+1

```

```

      ISWTCH=2
      GC TO 900
C
C      READ IN REMAINING NODE POINT AND ADJACENCY DATA TO FILL IN BUFFER
C
108 IF((NUMNP-NJMCP).LT.MXNPB) KVP=NUMNP-VUMCP-NPR
      IF((NUMNP-NJMCP).GE.MXNPB) KVP=MXNPB-VPR
      DO 109 I=1,KNP
        L=NPR+I
        READ(8) NPN,NADJNP(L),NADJEL(L),(NPADJ(L,J),J=1,MXADJP)
        READ(4) NPN,R(L),Z(L),ITYPE(L),THETA(L)
109 CONTINUE
      NPOUT=NUMCP
      GO TO 6
C
C**** TRANSFER PART OF NODE BUFFER REGION
C
902 DC 903 K=1,NPR
      L=NUMNPB+K
      NADJNP(K)=NADJNP(L)
      NADJEL(K)=NADJEL(L)
      ITYPE(K)=ITYPE(L)
      THETA(K)=THETA(L)
      R(K)=R(L)
      Z(K)=Z(L)
      XMASS(K)=XMASS(L)
      SNPUU(K)=SNPUU(L)
      SNPUW(K)=SNPUW(L)
      SNPWW(K)=SNPWW(L)
      DC 903 J=1,MXADJP
      NPADJ(K,J)=NPADJ(L,J)
      SADUU(K,J)=SADUU(L,J)
      SADUW(K,J)=SADUW(L,J)
      SADWW(K,J)=SADWW(L,J)
903 SADWW(K,J)=SADWW(L,J)
C
      GO TO 107
C
C**** ZER OUT BUFFER REGION SECTION ASSOCIATED WITH NODE DATA
C
900 DC 901 L=KX,MXNPB
      NADJNP(L)=0
      NADJEL(L)=0
      ITYPE(L)=0
      THETA(L)=0.0
      R(L)=0.0
      Z(L)=0.0
      XMASS(L)=0.0
      SNPUU(L)=0.0
      SNPUW(L)=0.0
      SNPWW(L)=0.0
      DC 901 J=1,MXADJP
      NPADJ(L,J)=0
      SADUU(L,J)=0.0
      SADUW(L,J)=0.0
      SADWW(L,J)=0.0
901 SADWW(L,J)=0.0
C
      GO TO (2,108),ISWTCH
C

```



```

END
SUBROUTINE ELAST(IELAST,ISTRES,E1,E2,E3,E4,E5,C,NUME)
C
  DIMENSION C(4,4)
C
C**** FORM STRESS-STRAIN MATRIX
C
  DO 1 I=1,4
    DO 1 J=1,4
      1 C(I,J)=0.0
C
  IF(IELAST.NE.1) GO TO 20
C
C**** ISOTROPIC ELASTIC MATERIAL
C
  IF(ISTRES.EQ.2) GO TO 4
C
  AXISYMMETRIC OR PLANE STRAIN PROBLEM
C
  EBAR=E1/((1.+E2)*(1.-2.*E2))
  C(1,1)=EBAR*(1.-E2)
  C(1,2)=EBAR*E2
  C(1,3)=C(1,2)
  C(2,1)=C(1,2)
  C(2,2)=C(1,1)
  C(2,3)=C(1,2)
  C(3,1)=C(1,2)
  C(3,2)=C(1,2)
  C(3,3)=C(1,1)
  C(4,4)=EBAR*(1.-2.*E2)/2.
  RETURN
C
C  PLANE STRESS PROBLEM
C
  4 EBAR=E1/(1.-E2*E2)
  C(1,1)=EBAR
  C(3,1)=EBAR*E2
  C(1,3)=C(3,1)
  C(3,3)=C(1,1)
  C(4,4)=EBAR*(1.-E2)/2.
  RETURN
C
C**** ANISOTROPIC ELASTIC MATERIAL
C
  20 IF(IELAST.NE.2) GO TO 30
C
  IF(ISTRES.EQ.2) GO TO 2
C
  C(1,1)=E1
  C(1,2)=E1-2.*E5
  C(1,3)=E3
  C(2,1)=C(1,2)
  C(2,2)=C(1,1)
  C(2,3)=C(1,3)
  C(3,1)=C(1,3)
  C(3,2)=C(2,3)
  C(3,3)=E2
  C(4,4)=E4
  RETURN
C

```

```

2  C(1,1)=2.*E5*(E1-2.*E5)/E1
   C(1,3)=2.*E3*E5/E1
   C(3,1)=C(1,3)
   C(3,3)=E2-E3**2/E1
   C(4,4)=E4
   RETURN
C
21 WRITE(6,3) IELAST,NUME,ISTRES
3  FORMAT(1H1/31H ERROR IN ELASTIC CONSTANT DATA/
113H IELAST      =,15/13H ELEMENT NO.=,15/
213H ISTRES      =,15)
   CALL EXIT
C
30 IF(IELAST.NE.3) GO TO 21
C
C**** COMPRESSIBLE FLUID
C
   IF(ISTRES. .2) GO TO 21
C
   DO 31 I=1,3
   DO 31 J=1,3
31  C(I,J)=E1
C
   RETURN
   END
C
C
C
C
   SUBROUTINE STIFF(KASE,NPI,NPJ,NPK,NPL,NUME,MAXNP,ISTRES,C,R,Z,
1  CK,AI,S1,C1,NCRACK)
C
   DIMENSION C(4,4),R(MAXNP),Z(MAXNP),CK(8,8),AI(23),D(8,8),G(8,8),
1  IVEC(8)
C
C**** COMPUTE ELEMENT STIFFNESS MATRIX
C
   KASE  =1 GENERAL TRIANGLE
C        =2 NCDE I ON Z-AXIS
C        =3 NCDES I,K ON Z-AXIS
C        =4 GENERAL RECTANGLE
C        =5 NCDE I ON Z-AXIS
C        =6 NCDES I,L ON Z-AXIS
C        C  =ELASTIC MODULI MATRIX
C        R  =RADIAL COORDINATE OF NODE POINTS
C        Z  =VERTICAL COORDINATE OF NODE POINTS
C        CK  =STIFFNESS MATRIX
C        AI  =INTEGRALS FOR COMPUTING K AND M
C        NUME =ELEMENT NUMBER
C
C
C
   DO 1 I=1,8
   DO 1 J=1,8
   D(I,J)=0.0
   G(I,J)=0.0
1  CK(I,J)=0.0
   IF(NCPACK.EQ.1) GO TO 300
   CALL INTER(KASE,NPI,NPJ,NPK,NPL,ISTRES,R,Z,AI,S1,C1,MAXNP)
C
   IF(NPL.NE.0) GO TO 300
C

```

C TRIANGULAR ELEMENTS
C

```

AJ=R(NPJ)-R(NPI)
AK=R(NPK)-R(NPI)
BJ=Z(NPJ)-Z(NPI)
BK=Z(NPK)-Z(NPI)
H=AJ*BK-AK*BJ
B=BJ-BK
A=AJ-AK
IF(KASE.NE.1) GO TO 2
D(1,1)=1.0
D(2,1)=B/H
D(3,1)=-A/H
2 D(4,2)=1.0
D(5,2)=B/H
D(6,2)=-A/H
D(2,3)=BK/H
D(3,3)=-AK/H
D(5,4)=D(2,3)
D(6,4)=D(3,3)
IF(KASE.EQ.3) GO TO 3
D(2,5)=-BJ/H
D(3,5)=AJ/H
3 D(5,6)=-BJ/H
D(6,6)=AJ/H
NCRD=6

```

C

```

IF(KASE.NE.1) GO TO 4
IF(ISTRES.NE.0) GO TO 4
G(1,1)=C(2,2)*AI(5)
G(2,1)=C(1,2)*AI(1)+C(2,2)*AI(7)
G(3,1)=C(2,2)*AI(6)
G(5,1)=C(2,3)*AI(1)
4 G(2,2)=C(1,1)*AI(4)+2.*C(1,2)*AI(7)+C(2,2)*AI(10)
IF(KASE.EQ.3) GO TO 5
G(3,2)=C(1,2)*AI(2)+C(2,2)*AI(9)
G(3,3)=C(2,2)*AI(8)+C(4,4)*AI(4)
G(5,3)=C(4,4)*AI(4)
G(6,3)=C(2,3)*AI(2)
5 G(5,2)=C(1,3)*AI(4)+C(2,3)*AI(3)
G(5,5)=C(4,4)*AI(4)
G(6,6)=C(3,3)*AI(4)
GO TO 301

```

C RECTANGULAR ELEMENTS
C

```

300 AJ=R(NPJ)-R(NPI)
BJ=Z(NPJ)-Z(NPI)
A=SQRT(AJ*AJ+BJ*BJ)
AL=R(NPL)-R(NPI)
BL=Z(NPL)-Z(NPI)
B=SQRT(AL*AL+BL*BL)
H=A*B
IF(NCRACK.EQ.0) GO TO 9
AL=A
A=1.0
B=1.0
H=1.0
9 IF(KASE.NE.4) GO TO 6
D(1,1)=1.0

```

```

D(2,1)=-8/H
D(3,1)=1./H
D(4,1)=-A/H
6 D(5,2)=1.0
D(6,2)=-8/H
D(7,2)=1./H
D(8,2)=-A/H
D(2,3)=-D(6,2)
D(3,3)=-D(7,2)
D(6,4)=D(2,3)
D(7,4)=D(3,3)
D(3,5)=D(7,2)
D(7,6)=D(7,2)
IF(KASE.EQ.6) GO TO 7
D(3,7)=D(3,3)
D(4,7)=-D(8,2)
7 D(7,8)=D(7,4)
D(8,9)=-D(8,2)
NCRD=8
IF(NCRACK.EQ.1) GO TO 10

```

C

```

IF(KASE.NE.4) GO TO 8
IF(ISTRES.NE.0) GO TO 8
G(1,1)=C(2,2)*AI(5)
G(2,1)=C(1,2)*C1*AI(1)+C(2,2)*AI(7)
G(3,1)=C(1,2)*(C1*AI(2)+S1*AI(3))+C(2,2)*AI(9)
G(4,1)=C(1,2)*S1*AI(1)+C(2,2)*AI(6)
G(6,1)=-C(2,3)*S1*AI(1)
G(7,1)=C(2,3)*(C1*AI(3)-S1*AI(2))
G(8,1)=C(2,3)*(C1*AI(1))

```

C

```

8 DUM1=C1*AI(14)+S1*AI(13)
DUM2=C1*AI(13)-S1*AI(14)
DUM3=C1*C1*AI(12)+2.*S1*C1*AI(15)+S1*S1*AI(11)
DUM4=S1*S1*AI(12)-2.*S1*C1*AI(15)+C1*C1*AI(11)
G(2,2)=C1*(C(1,1)*C1*AI(4)+2.*C(1,2)*AI(3))+C(2,2)*AI(10)
1+C(4,4)*S1*S1*AI(4)
G(3,2)=C(1,1)*C1*DUM1+C(1,2)*(2.*C1*AI(16)+S1*AI(18))
1+C(2,2)*AI(17)-C(4,4)*S1*DUM2
G(7,2)=C(1,3)*C1*DUM2+C(2,3)*(C1*AI(18)-S1*AI(16))
1-C(4,4)*S1*DUM1
G(8,2)=AI(4)*(C(1,3)*C1*C1-C(4,4)*S1*S1)+C(2,3)*C1*AI(3)
G(3,3)=C(1,1)*DUM3+2.*C(1,2)*(C1*AI(20)+S1*AI(23))
1+C(2,2)*AI(21)+C(4,4)*DUM4
G(6,3)=-C(1,3)*S1*DUM1-C(2,3)*S1*AI(16)+C(4,4)*C1*DUM2
G(7,3)=(C(1,3)+C(4,4))*S1*C1*(AI(11)+AI(12))+AI(15)*(C1*C1-S1*S1)
1+C(2,3)*(C1*AI(23)-S1*AI(20))
G(8,3)=C(1,3)*C1*DUM1+C(2,3)*C1*AI(16)+C(4,4)*S1*DUM2
G(5,5)=(C(3,3)*S1*S1+C(4,4)*C1*C1)*AI(4)
G(7,5)=-C(3,3)*S1*DUM2+C(4,4)*C1*DUM1
G(7,7)=C(3,3)*DUM4+C(4,4)*DUM3
G(8,7)=C(3,3)*C1*DUM2+C(4,4)*S1*DUM1
G(8,8)=(C(3,3)*C1*C1+C(4,4)*S1*S1)*AI(4)
IF(KASE.EQ.6) GO TO 301
G(4,2)=(C(1,1)-C(4,4))*S1*C1*AI(4)+C(1,2)*(C1*AI(2)+S1*AI(3))
1+C(2,2)*AI(9)
G(5,2)=-C(1,3)+C(4,4)*S1*C1*AI(4)-C(2,3)*S1*AI(3)
G(4,3)=C(1,1)*S1*DUM1+C(1,2)*(C1*AI(19)+2.*S1*AI(16))
1+C(2,2)*AI(22)+C(4,4)*C1*DUM2
G(4,4)=C(1,1)*S1*S1*AI(4)+2.*C(1,2)*S1*AI(2)+C(2,2)*AI(8)

```

```

1+C(4,4)*C1*C1*AI(4)
G(5,4)=AI(4)*(C(4,4)*C1*C1-C(1,3)*S1*S1)-C(2,3)*S1*AI(2)
G(7,4)=C(1,3)*S1*C1*2+C(2,3)*(C1*AI(16)-S1*AI(19))+C(4,4)*C1*DUM1
G(8,4)=(C(1,3)+C(4,4))*S1*C1*AI(4)+C(2,3)*C1*AI(2)
G(8,5)=S1*C1*(C(4,4)-C(3,3))*AI(4)

```

```

C
301 DC 201 I=2,NCRD
    K=I-1
    DC 201 J=1,K
201 G(I,J)=C(I,J)

```

```

C
C
    DC 51 J=1,NCRD
    DC 51 L=1,NCRD
    VEC(L)=0.0
    DC 51 K=1,NCRD
51 VEC(L)=VEC(L)+G(L,K)*D(K,J)
    DC 51 I=1,NCRD
    CK(I,J)=0.0
    DC 51 L=1,NCRD
51 CK(I,J)=CK(I,J)+D(L,I)*VEC(L)
C
    RETURN
C

```

```

10 C1=AJ/AL
    S1=-BJ/AL
    H1=C(1,1)*S1*S1+C(4,4)*C1*C1
    H2=(C(1,3)+C(4,4))*S1*C1
    H3=C(3,3)*C1*C1+C(4,4)*S1*S1
    IF(ISTRES.EQ.0) GO TO 11
    G(3,3)=AL*H1/3.
    G(4,3)=AL*H1/2.
    G(7,3)=AL*H2/3.
    G(8,3)=AL*H2/2.
    G(4,4)=3.*G(3,3)
    G(7,4)=G(8,3)
    G(8,4)=2.*G(7,4)
    G(7,7)=AL*H3/3.
    G(8,7)=AL*H3/2.
    G(8,8)=2.*G(8,7)
    GO TO 301
11 RI=R(NP1)
    IF(ABS(S1).GT.0.01) GO TO 12
    G(3,3)=AL*H1/3.
    IF(KASE.EQ.6) GO TO 14
    G(4,3)=AL*H1*(RI/2.+AL/3.)
    G(4,4)=AL*H1*(RI+AL/2.)
14 G(7,7)=AL*RI*H3/3.
    G(8,7)=AL*H3*(RI/2.+AL/3.)
    G(8,8)=AL*H3*(RI+AL/2.)
    GO TO 301
12 IF(ABS(C1).GT.0.01) GO TO 13
    G(3,3)=AL*H1*(RI/3.+AL/4.)
    G(4,3)=AL*RI*H1/2.
    G(4,4)=AL*RI*H1
    G(7,7)=AL*H3*(RI/3.+AL/4.)
    G(8,7)=AL*RI*H3/2.
    G(8,8)=2.*G(8,7)
    GO TO 301
13 R12=RI+AL*C1

```

```

H4=(AL**3)*(RI/3.+AL*C1/4.)
H5=(-AL**3/(10.*S1))*(3.*RI+2.*AL*C1)-AL*AL*RI**2/(30.*S1*S1*
1 C1)+AL*RI**3*(1.-RI/RIST)/(30.*S1*S1*C1*C1)+AL**3*(1C.*RI**2+15.
2 *AL*RI*C1+6.*AL**2*C1*C1)/(30.*RI*ST*S1*S1)
H6=AL**2*(RI/2.+AL*C1/3.)
H7=-AL**2*(5.*RI+3.*AL*C1)/(12.*S1*S1)-AL*RI**2*(1.-RI/RIST)/
1 (12.*S1*S1*C1)+AL**2*(6.*RI**2+8.*AL*RI*C1+3.*AL**2*C1*C1)/
2 (12.*S1*S1*RI*ST)
H8=AL*(RI+AL*C1/2.)
G(3,3)=H1*H4+C(2,2)*H5
G(4,3)=H1*H6+C(2,2)*H7
G(7,3)=H2*H4
G(8,3)=H2*H6
G(4,4)=H1*H8
G(7,4)=H2*H6
G(8,4)=H2*H8
G(7,7)=H3*H4
G(8,7)=H3*H6
G(8,8)=H3*H8
GC TO 301

```

END

SUBROUTINE INTER(KASE,NPI,NPJ,NPK,NPL,ISTRES,R,Z,AI,S1,C1,MAXNP)

C**** COMPUTE ELEMENT INTEGRALS
 DIMENSION AI(23),R(MAXNP),Z(MAXNP)

DO 1 I=1,23
 1 AI(I)=0.

IF(NPL.NE.0) GO TO 300

TRIANGULAR ELEMENTS

AJ=R(NPJ)-R(NPI)
 AK=R(NPK)-R(NPI)
 BJ=Z(NPJ)-Z(NPI)
 BK=Z(NPK)-Z(NPI)
 H=AJ*BK-AK*BJ
 B=BJ-BK
 A=AJ-AK
 RI=R(NPI)

IF(ISTRES.EQ.0) GO TO 2
 AI(4)=H/2.
 AI(13)=H*(AJ+AK)/6.
 AI(14)=H*(BJ+BK)/6.
 RETURN

AI(1)=H/2.
 AI(2)=H*(BJ+BK)/6.
 AI(3)=H*(AJ+AK)/6.
 AI(4)=RI*AI(1)+AI(3)
 IF(KASE.EQ.1) GO TO 3
 AI(5)=AI(2)
 AI(6)=AI(4)

C

```

3  ICOUNT=1
   RA=RI
   RB=R(NPJ)
   C=BJ/AJ
   D=0.
   DUM=-1.
   IF(C.EQ.0.) GO TO 100
101 IF(KASE.EQ.1) GO TO 102
   IF(KASE.NE.2) GO TO 104
   IF(RA.NE.0.) GO TO 102
   FO=ALOG(RB)
   GO TO 104
102 FC=ALOG(RB/RA)
104 DUM1=RB-RA
   DUM2=RB*RB-RA*RA
   DUM3=RB*RB*RB-RA*RA*RA
   DUM4=DJM2*(RB*RB+RA*RA)
   IF(KASE.EQ.3) GO TO 103
   F1=DUM1-RI*FO
   F2=DUM2/2.-2.*RI*DUM1+RI*RI*FO
   F3=DUM3/3.-1.5*RI*DUM2+3.*RI*RI*DUM1-RI*RI*RI*FO
103 GO=DUM2/2.
   G1=DUM3/3.-RI*DJM2/2.
   G2=DUM4/4.-2.*RI*DJM3/3.+RI*RI*DUM2/2.

```

C

```

   IF(KASE.NE.1) GO TO 105
   AI(5)=AI(5)+DJM*(C*F1+D*FO)
   AI(6)=AI(6)+DJM*(C*C*F2/2.+C*D*F1+D*D*FO/2.)
   AI(7)=AI(7)+DUM*(C*F2+D*F1)
   AI(9)=AI(9)+DJM*(C*C*F3/2.+C*D*F2+D*D*F1/2.)
   AI(10)=AI(10)+DJM*(C*F3+C*F2)
   GO TO 106
105 IF(KASE.NE.2) GO TO 107
106 AI(8)=AI(8)+DJM*(C*C*C*F3/3.+C*C*D*F2+C*D*D*F1+D*D*D*FO/3.)
107 AI(13)=AI(13)+DJM*(C*G2+D*G1)
   AI(14)=AI(14)+DJM*(C*C*G2/2.+C*D*G1+D*D*GO/2.)

```

C

```

100 GO TO(201,202,203), ICOJNT
201 ICOUNT=2
   IF(A.EQ.0.) GO TO 100
   RB=R(NPJ)
   RA=R(NPK)
   C=B/A
   D=H/A
   DUM=+1.
   GO TO 101
202 ICOUNT=3
   IF(AK.EQ.0.) GO TO 100
   IF(BK.EQ.0.) GO TO 100
   RB=R(NPK)
   RA=RT
   C=BK/AK
   D=0.
   DUM=+1.
   GO TO 101
203 RETURN

```

C

C

C

RECTANGULAR ELEMENTS

```

300  AJ=R(NPJ)-R(NPI)
      BJ=Z(NPJ)-Z(NPI)
      A=SQRT(AJ*AJ+BJ*BJ)
      AL=R(NPL)-R(NPI)
      BL=Z(NPL)-Z(NPI)
      B=SQRT(AL*AL+BL*BL)
      H=A*B
      S1=-BJ/A
      C1=AJ/A
      RI=R(NPI)
      IF(S1.GT..01) GO TO 301
      S1=0.
      C1=1.
      GC TO 302
301  IF(C1.GT..01) GO TO 302
      S1=1.
      C1=0.
302  S2=S1*S1
      S3=S2*S1
      C2=C1*C1
      C3=C2*C1

```

```

C
      IF(ISTRES.EQ.0) GO TO 303
      AI(4)=H
      AI(11)=A*A*H/3.
      AI(12)=B*B*H/3.
      AI(13)=A*H/2.
      AI(14)=B*H/2.
      AI(15)=H*H/4.
      RETURN

```

```

C
303  AI(1)=H
      AI(2)=B*H/2.
      AI(3)=A*H/2.
      AI(4)=RI*AI(1)+C1*AI(3)+S1*AI(2)
      AI(18)=A*A*H/3.
      AI(19)=B*B*H/3.
      AI(20)=B*H*H/6.
      AI(23)=A*H*H/6.
      AI(16)=H*H/4.
      AI(15)=RI*AI(16)+C1*AI(23)+S1*AI(20)
      AI(14)=RI*AI(2)+C1*AI(16)+S1*AI(19)
      AI(13)=RI*AI(3)+C1*AI(18)+S1*AI(16)
      AI(12)=RI*AI(19)+C1*AI(20)+S1*B**3*H/4.
      AI(11)=RI*AI(18)+S1*AI(23)+C1*A**3*H/4.
      IF(KASE.NE.6) GO TO 304
      AI(2)=0.
      AI(10)=AI(4)
      AI(17)=AI(14)
      AI(19)=0.
      AI(21)=AI(12)
      RETURN

```

```

C
304  A2=A*A
      A3=A2*A
      B2=B*B
      B3=B2*B
      RI2=RI*RI
      RI3=RI2*RI
      IF(S1.NE.0.) GO TO 305

```



```

D1=ALOG(1.+A/RI)
AI(5)=B*D1
AI(6)=B2*D1/2.
AI(7)=H-B*RI*D1
AI(8)=B3*D1/3.
AI(9)=B2*(A-RI*D1)/2.
AI(10)=B*(A2/2.-A*RI+RI2*D1)
AI(11)=B2*RI2*D1/2.-B*H*RI/2.+H*H/4.
AI(21)=B3*(RI2*D1+A2/2.-A*RI)/3.
AI(22)=B2*(H-B*RI*D1)/3.
RETURN

```

C

```

305 IF(C1.NE.0.) GO TO 306
D1=ALOG(1.+B/RI)
AI(5)=A*D1
AI(6)=H-A*RI*D1
AI(7)=A2*D1/2.
AI(8)=A*RI2*D1- *RI+B*H/2.
AI(9)=A2*(B-RI*D1)/2.
AI(10)=A3*C1/3.
AI(11)=A2*H/3.-A3*RI*C1/3.
AI(21)=A3*(RI2*D1+B2/2.-B*RI)/3.
AI(22)=H*H/4.-A*H*RI/2.+A2*RI2*D1/2.
RETURN

```

C

```

306 D1=ALOG(1.-H*S1*C1/((RI+B*S1)*(RI+A*C1)))
D2=ALOG(1.+A*C1/(RI+B*S1))
D3=ALOG(1.+B*S1/(RI+A*C1))
AI(5)=(RI*D1+B*D2*S1+A*D3*C1)/(S1*C1)
AI(6)=(B2*D2*S2-RI2*D1-A*C1*D3*(2.*RI+A*C1)+H*S1*C1)/(2.*S2*C1)
AI(7)=(A2*D3*C2-RI2*D1-B*S1*D2*(2.*RI+B*S1)+H*S1*C1)/(2.*S1*C2)
AI(8)=(B*H*S2*C1/6.-2.*H*RI*S1*C1/3.-A*H*S1*C2/3.+RI3*D1/3.
+ B3*S3*D2/3.+C1*A*(RI2+A*RI*C1+A2*C2/3.)*D3)/(S3*C1)
AI(9)=(A*H*S1*C2/3.+B*H*S2*C1/3.+H*RI*S1*C1/6.+RI3*D1/6.
- S2*(B2*RI/2.+B3*S1/3.)*D2-C2*(A2*RI/2.+A3*C1/3.)*D3)/(S2*C2)
AI(10)=(A*H*S1*C2/6.-2.*H*RI*S1*C1/3.-B*H*S2*C1/3.+RI3*D1/3.
+ A3*C3*D3/3.+S1*B*(RI2+B*RI*S1+B2*S2/3.)*D2)/(S1*C3)
AI(11)=(A2*H*S1*C3/4.-RI*RI3*D1/12.+S2*B2*(RI2/2.+2.*H*RI*S1/3.
+ B2*S2/4.)*D2-C3*A3*(RI/3.+A*C1/4.)*D3-S1*C1*H*(RI2/12.+5.*B*RI
2*S1/12.+B2*S2/4.)*S1*C2*H*(A*RI/12.+H*S1/8.))/(S2*C3)
AI(21)=-H*(A3*C1/S2+B3*S1/C2)/5.+H*H*(A/S1+B/C1)/10.-RI2*H*(B/C1
+ A/S1)/(30.*S1*C1)-.3*RI*H*(A2/S2+B2/C2)+RI3*H/(30.*S2*C2)
+ RI*H*H/(20.*S1*C1)+RI2*RI3*D1/(30.*S3*C3)+B3*D2*(RI2/3.+B*RI*S1
3/2.+B2*S2/5.)/C3+A3*D3*(RI2/3.+A*RI*C1/2.+A2*C2/5.)/S3
AI(22)=H*H/(8.*S1)-5.*A*H*RI/(12.*S2)-A2*H*C1/(4.*S2)-H*RI2/(12.*
S2*C1)+B*H*RI/(12.*S1*C1)+B2*H/(4.*C1)+A2*D3*(RI2/2.+2.*A*RI*C1
2/3.+A2*C2/4.)/S3-RI*RI3*D1/(12.*S3*C2)-B3*D2*(RI/3.+B*S1/4.)/C2

```

C

RETURN

C

END

C

C

C

C

SUBROUTINE ADJUSK(MAXNP,BK,ITYPE,THETA,NPI,NPJ,NPK,NPL)

C

DIMENSION BK(8,8),BKBAR(8,8),C(8,8),VEC(8)

DIMENSION ITYPE(MAXNP),THETA(MAXNP)

C

NCRD=8

```

      IF(NPL.EQ.0) NORD=6
C
      ISWTC=0
      IF(ITYPE(NPI).NE.1) GO TO 3
      MX=1
      NP=NPI
      ICOUNT=1
      GO TO 6
3    IF(ITYPE(NPJ).NE.1) GO TO 4
      MX=3
      NP=NPJ
      ICOUNT=2
      GO TO 6
4    IF(ITYPE(NPK).NE.1) GO TO 5
      MX=5
      NP=NPK
      ICOUNT=3
      GO TO 6
5    IF(NPL.EQ.0) GO TO 7
      IF(ITYPE(NPL).NE.1) GO TO 7
      MX=7
      NP=NPL
      ICOUNT=4
C
6    IF(ISWTC.EQ.1) GO TO 8
      ISWTC=1
      DO 1 I=1,NCRD
      DO 1 J=1,NCRD
      IF(I.NE.J) GO TO 2
      C(I,J)=1.0
      GO TO 1
2    C(I,J)=0.0
      BKBAR(I,J)=0.0
C
8    NX=MX+1
      C(MX,MX)=COS(THETA(NP))
      C(NX,MX)=SIN(THETA(NP))
      C(MX,NX)=-C(NX,MX)
      C(NX,NX)=C(MX,MX)
      GO TO (3,4,5,9), ICOUNT
7    IF(ISWTC.EQ.0) RETURN
C
9    DO 51 J=1,NORD
      DO 50 L=1,NORD
      VEC(L)=0.0
      DO 50 K=1,NORD
50    VEC(L)=VEC(L)+BK(L,K)*C(K,J)
      DO 51 I=1,NORD
      BKBAR(I,J)=0.0
      DO 51 L=1,NORD
51    BKBAR(I,J)=BKBAR(I,J)+C(L,I)*VEC(L)
C
      DO 52 I=1,NORD
      DO 52 J=1,NORD
52    BK(I,J)=BKBAR(I,J)
C
      RETURN
      END
C
C

```

```

C      SUBROUTINE DISTK(MAXNP,MXADJP,BK,SNPUU,SNPUW,SNPWW,SADUU,
1SADUW,SADWU,SADWW,NPI,NPJ,NPK,NPL,NPADJ,NPOUT)
C
C      DIMENSION BK(8,8),SNPUU(MAXNP),SNPUW(MAXNP),SADUU(MAXNP,MXADJP),
1SADUW(MAXNP,MXADJP),SNPWW(MAXNP),SADWU(MAXNP,MXADJP),
2SADWW(MAXNP,MXADJP),NPADJ(MAXNP,MXADJP)
C
C**** DISTRIBUTE ELEMENT STIFFNESS TO NODE POINT STIFFNESS
C
C      BK      =ELEMENT STIFFNESS, 6*6 FOR TRIANGLE, 8*8 FOR RECT
C      SNPUU =NODE PT. STIFF, U-DIRECTION, U-DISPL.
C      SNPUW =NODE PT. STIFF, U-DIRECTION, W-DISPL.
C      SNPWW =NODE PT. STIFF, W-DIRECTION, W-DISPL.
C      SADUU =ADJ. PT. STIFF, U-DIRECTION, U-DISPL.
C      SADUW =ADJ. PT. STIFF, U-DIRECTION, W-DISPL.
C      SADWU =ADJ. PT. STIFF, W-DIRECTION, U-DISPL.
C      SADWW =ADJ. PT. STIFF, W-DIRECTION, W-DISPL.
C
C      ICOUNT=1
1  GO TO (2,3,4,5,6),ICOUNT
C
C      2 NI=NPI
C      NJ=NPJ
C      NK=NPK
C      NL=NPL
C      MX=1
C      LX=3
C      LY=5
C      LZ=7
C      GO TO 100
C
C      3 NI=NPJ
C      NJ=NPI
C      NK=NPK
C      NL=NPL
C      MX=3
C      LX=1
C      LY=5
C      LZ=7
C      GO TO 100
C
C      4 NI=NPK
C      NJ=NPI
C      NK=NPJ
C      NL=NPL
C      MX=5
C      LX=1
C      LY=3
C      LZ=7
C      GO TO 100
C
C      5 IF(NPL.EQ.0) GO TO 6
C      NI=NPL
C      NJ=NPI
C      NK=NPJ
C      NL=NPK
C      MX=7
C      LX=1
C      LY=3

```

LZ=5
GC TO 100

C

6 RETURN

C

100 SNPUU(NI)=SNPUU(NI)+BK(MX,MX)
 SNPUW(NI)=SNPUW(NI)+BK(MX,MX+1)
 SNPWW(NI)=SNPWW(NI)+BK(MX+1,MX+1)
 DO 101 I=1,MXADJP
 J=I
 IF((NPADJ(NI,I)-NPOUT).EQ.NJ) GO TO 102
 101 CONTINUE
 205 NPI=NPI+NPCJT
 NPJ=NPJ+NPCJT
 NPK=NPK+NPCJT
 IF(NPL.EQ.0) GO TO 204
 NPL=NPL+NPCJT
 204 WRITE(6,203) NPI,NPJ,NPK,NPL,NI,NJ,NK,NL,NPOUT,
 1(NPADJ(NI,I),I=1,MXADJP)
 203 FORMAT(1H1/32H ERROR IN STIFFNESS DISTRIBUTION//
 1 13H NPI =,15/13H NPJ =,15/13H NPK =,15/
 213H NPL =,15/10X,515/10X,815)
 CALL EXIT

C

102 SADUU(NI,J)=SADJJ(NI,J)+BK(MX,LX)
 SADUW(NI,J)=SADJW(NI,J)+BK(MX,LX+1)
 SADWU(NI,J)=SADWJ(NI,J)+BK(MX+1,LX)
 SADWW(NI,J)=SADWW(NI,J)+BK(MX+1,LX+1)

C

DO 103 I=1,MXADJP
 J=I
 IF((NPADJ(NI,I)-NPOUT).EQ.NK) GO TO 104
 103 CONTINUE
 GC TO 205
 104 SADUU(NI,J)=SADJJ(NI,J)+BK(MX,LY)
 SADUW(NI,J)=SADJW(NI,J)+BK(MX,LY+1)
 SADWU(NI,J)=SADWJ(NI,J)+BK(MX+1,LY)
 SADWW(NI,J)=SADWW(NI,J)+BK(MX+1,LY+1)

C

IF(NL.EQ.0) GO TO 105
 DO 106 I=1,MXADJP
 J=I
 IF((NPADJ(NI,I)-NPOUT).EQ.NL) GO TO 107
 106 CONTINUE
 GO TO 205
 107 SADUU(NI,J)=SADUU(NI,J)+BK(MX,LZ)
 SADUW(NI,J)=SADJW(NI,J)+BK(MX,LZ+1)
 SADWU(NI,J)=SADWJ(NI,J)+BK(MX+1,LZ)
 SADWW(NI,J)=SADWW(NI,J)+BK(MX+1,LZ+1)
 105 ICOUNT=ICOUNT+1
 GO TO 1

C

C

END

C

C

SUBROUTINE MASS(MAXNP,RHO,R,Z,AI,XMASS,S1,C1,NPI,NPJ,NPK,NPL,
 11STRES)

C

```
DIMENSION R(MAXNP),Z(MAXNP),XMASS(MAXNP),AI(23)
```

```
C
C**** COMPUTE AND DISTRIBUTE MASS POINTS
```

```
C
C   GRAV=386.4
```

```
C
C   IF(NPL.NE.0) GO TO 2
```

```
C
C   TRIANGULAR ELEMENT
```

```
C
C   AJ=R(NPJ)-R(NPI)
C   AK=R(NPK)-R(NPI)
C   BJ=Z(NPJ)-Z(NPI)
C   BK=Z(NPK)-Z(NPI)
C   H=AJ*BK-AK*BJ
C   B=BJ-BK
C   A=AJ-AK
```

```
C
C   DUM1=RHC/H*GRAV
C   AMI=DUM1*(F*AI(4)+B*AI(13)-A*AI(14))
C   AMJ=DUM1*(BK*AI(13)-AK*AI(14))
C   AMK=DUM1*(AJ*AI(14)-BJ*AI(13))
C   GC TO 3
```

```
C
C   RECTANGULAR ELEMENT
```

```
2  C   AJ=R(NPJ)-R(NPI)
C   BJ=Z(NPJ)-Z(NPI)
C   A=SQRT(AJ*AJ+BJ*BJ)
C   AL=R(NPL)-R(NPI)
C   BL=Z(NPL)-Z(NPI)
C   B=SQRT(AL*AL+BL*BL)
```

```
C
C   IF(ISTRES.EQ.0) GO TO 4
```

```
C
C   AMI=RHO*AI(4)/4.*GRAV
C   AMJ=AMI
C   AMK=AMI
C   AML=AMI
C   GC TO 3
```

```
C
C   4  DUM1=RHC/((1.+S1+C1)*GRAV
C   AMI=DUM1*((S1+C1)*AI(4)-S1*AI(14)/B-C1*AI(13)/A)
C   AMJ=DUM1*(AI(4)-(1.+C1)*AI(14)/B+C1*AI(13)/A)
C   AMK=DUM1*(-AI(4)+(1.+C1)*AI(14)/B+(1.+S1)*AI(13)/A)
C   AML=DUM1*(AI(4)+S1*AI(14)/B-(1.+S1)*AI(13)/A)
```

```
C
C   3  XMASS(NPI)=XMASS(NPI)+AMI
C   XMASS(NPJ)=XMASS(NPJ)+AMJ
C   XMASS(NPK)=XMASS(NPK)+AMK
C   IF(NPL.EQ.0) RETURN
C   XMASS(NPL)=XMASS(NPL)+AML
C   RETURN
```

```
C
C   END
```

```
C
C   SUBROUTINE PRNK(MAXNP,MXADJP,NADJNP,VPAJ,NADJEL,IPRINT,SNPLU,
```

```
1 SNPUW,SNPWW,SADJU,SADUW,SADWU,SADWW,THETA,ITYPE,XMASS,NPOUT,
2 NUMNP)
```

```
    DIMENSION NADJNP(MAXNP),NPADJ(MAXNP,MXADJP),NADJEL(MAXNP),
    1 XMASS(MAXNP),          SNPUU(MAXNP),SNPUW(MAXNP),SNPWW(MAXNP),
    2 SADUU(MAXNP,MXADJP),SADUW(MAXNP,MXADJP),SADWU(MAXNP,MXADJP),
    3 SADWW(MAXNP,MXADJP),THETA(MAXNP),ITYPE(MAXNP)
```

```
    IF((IPRINT.NE.2).AND.(IPRINT.NE.99))GO TO 15
```

```
    WRITE(6,1)
    1 FORMAT(1H1,16HSTIFFNESS TABLES//6H  NODE,8X,6H SNPUU ,8X,
    16H SNPUW ,8X,6H SNPWW //)
    DO 2 I=1,NUMNP
    K=I+NPOUT
    2 WRITE(6,3) K,SNPUU(I),SNPUW(I),SNPWW(I)
    3 FORMAT(15,3X,1P8E14.4)
```

```
    WRITE(6,4)
    4 FORMAT(1H1,18HADJACENT STIFFNESS//)
    WRITE(6,5)
    5 FORMAT(6H  NODE,10X,5HSADUU//)
    DO 6 I=1,NUMNP
    NUM=NADJNP(I)
    K=I+NPOUT
    6 WRITE(6,3) K,(SADUU(I,J),J=1,NUM)
```

```
    WRITE(6,4)
    WRITE(6,7)
    7 FORMAT(6H  NODE,10X,5HSADUW//)
    DO 8 I=1,NUMNP
    NUM=NADJNP(I)
    K=I+NPOUT
    8 WRITE(6,3) K,(SADUW(I,J),J=1,NUM)
```

```
    WRITE(6,4)
    WRITE(6,9)
    9 FORMAT(6H  NODE,10X,5HSADWU//)
    DO 10 I=1,NUMNP
    NUM=NADJNP(I)
    K=I+NPOUT
    10 WRITE(6,3) K,(SADWU(I,J),J=1,NUM)
```

```
    WRITE(6,4)
    WRITE(6,11)
    11 FORMAT(6H  NODE,10X,5HSADWW//)
    DO 12 I=1,NUMNP
    NUM=NADJNP(I)
    K=I+NPOUT
    12 WRITE(6,3) K,(SADWW(I,J),J=1,NUM)
```

```
    15 IF((IPRINT.NE.4).AND.(IPRINT.NE.99))RETURN
    WRITE(6,13)
    13 FORMAT(1H1,23HMASS VECTOR, LB SEC2/IV//6H  NODE//)
    DO 14 I=1,NUMNP,8
    L=I+NPC
    NUM=
    IF( .GT. NUMNP) NUM=NUMNP
    14 WRITE(6,3) L,(XMASS(J),J=1,NUM)
```

```

      RETURN
C
C
      END
C
C
C
      OVERLAY(MOHAN,6,0      )
      PROGRAM LNKIF
      COMMON MAXNP, MXCLS, MXADJP, MXZONE, MXNPB, NZONES, MXPELB, NUMNP,
1         NUMEL, ISTRES, NUMPEL, NUMELP, PERIOD, NMKCLS, FACTOR, ALAMB,
2         KTAPE, KRJN, IPRINT, NUMST, MXSTRT, IELAST(20), IPLAST(20),
3         WGT(20), NSTART(79), ET(5,20), IPELTP, INT, NPRCDS, IMPBX
C
      DIMENSION C(4,4)
      DIMENSION NADJNP(350), NPADJ(350,8), NADJEL(350)
      DIMENSION R(350), Z(350), ITYPE(350), THETA(350)
C
      DIMENSION STNPU(4,350), STNPW(4,350), STADU(4,350,8),
      1STADW(4,350,8)
C
C**** INITIALIZE
      MOHAN=5HMOHAN
      REWIND 4
      REWIND 8
      REWIND 12
      REWIND 3
C
      KEND=0
      NPOUT=0
      NUMCP=0
      NUMNPB=0
      NPR=MXNPB
      KX=1
C
C**** ZERO OUT BUFFER REGION
C
      1 ISWCH=1
      GO TO 900
C
C**** READ IN FIRST RECORDS
C
      2 IF (NUMNP.LT.MXNPB) NUMNPB=NUMNP
      IF (NUMNP.GE.MXNPB) NUMNPB=MXNPB
      DO 3 I=1,NUMNPB
      READ(8) NPN,NADJNP(I),NADJEL(I),(NPADJ(I,J),J=1,MXADJP)
      READ(4) NPN,R(I),Z(I),ITYPE(I),THETA(I)
      3 CONTINUE
C
      ICOUNT=0
      4 READ(12) KEY,NUME,IZONE,NTI,NTJ,NTK,NTL,NCRACK,
      1((C(I,J),I=1,4),J=1,4),KASE,SI,CI
      ICOUNT=ICOUNT+1
      LNP=MAX0(NTI,NTJ,NTK,NTL)
C
      IF ((LNP-NPOUT).GT.MXNPB) GO TO 100
C
C      SUFFICIENT ROOM IN BUFFER REGION
C
      6 NPI=NTI-NPOUT

```

```

NPJ=NTJ-NPCJT
NPK=NTK-NPCJT
IF(NTL.EQ.0) NPL=0
IF(NTL.NE.0) NPL=NTL-NPOUT
CALL STRESS(MXNPB,MXADJP,C,NPI,NPJ,NPK,NPL,R,Z,KASE,NUME,NPADJ,
1 STNPU,STNPW,STADU,STADW,ISTRES,S1,C1,NPOUT,ITYPE,THETA,NCRACK)
C
IF(ICOUNT.LT.NUMFL) GO TO 4
KEND=1
KEY=NUMAP+1
C
C**** INSUFFICIENT ROOM IN BUFFER REGION
C
100 NUMCP=KEY-1
NUMNPB=NUMCP-NPOUT
C
C MODIFY STRESS TABLES
113 CALL MODS(MXNPB,MXADJP,NADJNP,NPADJ,NADJEL,STNPU,STNPW,
1 STADU,STADW,IPRINT,THETA,ITYPE,NPOUT,NUMNPB)
C WRITE STRESS TABLES ONTO TAPE 3
C
DC 101 I=1,NUMNPB
101 WRITE(3) I,NADJNP(I),NADJEL(I),(NPADJ(I,J),J=1,MXADJP),(STNPU(K,I),
1 STNPW(K,I),K=1,4),((STADU(K,I,J),STADW(K,I,J),K=1,4),J=1,MXADJP)
IF(KEND.EQ.1) RETURN
IF(KEND.EQ.1) RETURN
C
C MOVE UP INCOMPLETED NODES IN BUFFER REGION
NPR=MXNPB-NJMNBP
GO TO 902
C ZERO REMAINING BUFFER AREA
107 KX=NPR+1
WHICH=2
TO 'CJ
C AS N REMAINING NODE POINT AND ADJACENCY DATA TO FILL IN BUFFER
C
1 IF((NUMNP-NJMCP).LT.MXNPB) KNP=NUMNP-NUMC JPR
IF((NUMNP-NJMCP).GE.MXNPB) KNP=MXNPB-NPR
DC 109 I=1,KNP
L=NPR+I
READ(8) NPN,NADJNP(L),NADJEL(L),(NPADJ(L,J),J=1,MXADJP)
READ(4) NPN,R(L),Z(L),ITYPE(L),THETA(L)
109 CONTINUE
NPOUT=NUMCP
GO TO 6
C**** TRANSFER PART OF NODE BUFFER REGION
C
902 DC 903 K=1,NPR
L=NUMNPB+K
NADJNP(K)=NADJNP(L)
NADJEL(K)=NADJEL(L)
ITYPE(K)=ITYPE(L)
THETA(K)=THETA(L)
R(K)=R(L)
Z(K)=Z(L)
DC 904 J=1,4
STNPU(J,K)=STNPU(J,L)
904 STNPW(J,K)=STNPW(J,L)
DC 903 J=1,MXADJP
NPADJ(K,J)=NPADJ(L,J)

```



```

      DC 903 I=1,4
      STADU(I,K,J)=STADU(I,L,J)
903  STADW(I,K,J)=STADW(I,L,J)
C
      GO TO 107
C
C**** ZERO OUT BUFFER REGION SECTION
C
900  DC 901 L=KX,MXNPB
      NADJNP(L)=0
      NADJEL(L)=0
      ITYPE(L)=0
      THETA(L)=0.0
      R(L)=0.0
      Z(L)=0.0
      DC 905 J=1,4
      STNPU(J,L)=0.0
905  STNPW(J,L)=0.0
      DC 901 J=1,MXADJP
      NPADJ(L,J)=0
      DO 901 I=1,4
      STADU(I,L,J)=0.0
901  STADW(I,L,J)=0.0
C
      GO TO (2,108),ISWTC
C
C
C
C
C
      SUBROUTINE STRESS(MAXNP,MXADJP,C,NPI,NPJ,NPK,NPL,RA,ZA,KASE,NUME,
      INPADJ,STNPU,STNPW,STADU,STADW,ISTRES,S1,C1,NPOLT,ITYPE,THETA,
      2NCRACK)
C
      DIMENSION C(4,4),RA(MAXNP),ZA(MAXNP),NPADJ(MAXNP,MXADJP),S(4,8),
      1STNPU(4,MAXNP),STNPW(4,MAXNP),STADU(4,MAXNP,MXADJP),
      2STADW(4,MAXNP,MXADJP),ITYPE(MAXNP),THETA(MAXNP)
C
      DIMENSION SBAR(4,8),CBAR(8,8)
C
C**** COMPUTE NODE POINT STRESS-DISPLACEMENT RELATIONS
C
C
C      C      =STRESS-STRAIN MATRIX
C      S      =STRESS-DISPLACEMENT MATRIX FOR ELEMENT
C      ST(1) =RADIAL      STRESS, SIGMA R
C      ST(2) =MERIDIONAL STRESS, SIGMA THETA
C      ST(3) =VEPTICAL   STRESS, SIGMA Z
C      ST(4) =SHEAR      STRESS, TAU
C
C      ST(I) =STNPJ(I)*J+STNPW(I)*W+SUM(STADU(I)*U+STADW(I)*W)
C
      DO 1 I=1,4
      DO 1 J=1,8
1  S(I,J)=0.0
C
      RR=RA(NPI)
      R=0.
      RP=0.

```

Z=0.
ZP=0.
ICOUNT=1

IF(NPL.NE.0) GO TO 200

TRIANGULAR ELEMENTS

AJ=RA(NPJ)-RA(NPI)
AK=RA(NPK)-RA(NPI)
BJ=ZA(NPJ)-7A(NPI)
BK=ZA(NPK)-7A(NPI)
H=AJ*BK-AK*BJ
B=BJ-BK
A=AJ-AK

25 IF(KASE.NE.1) GO TO 26
IF(ISTRES.NE.0) GO TO 27
AOR=1./RR
RCR=K/RR
ZCR=Z/RR
GO TO 28
27 IF(ICOUNT.NE.1) GO TO 100
ACR=0.0
RCR=0.0
ZOR=0.0
28 DUM1=B/H
DUM2=AOR+(B*RCR-A*ZOR)/H
DO 35 I=1,3
35 S(I,1)=C(I,1)*DUM1+C(I,2)*DUM2
S(4,1)=-C(4,4)*A/H
GO TO 29
26 RCR=1.0
IF(KASE.NE.2) GO TO 30
IF(ICOUNT.NE.1) GO TO 31
ZOR=0.0
GO TO 29
31 ZOR=Z/RR
GO TO 29
30 ZCR=0.0
IF(ICOUNT.NE.1) GO TO 100
29 DO 36 I=1,3
36 S(I,2)=-C(I,3)*A/H
S(4,2)=C(4,4)*B/H
DUM1=BK/H
DUM2=(BK*RCR-AK*ZOR)/H
DO 37 I=1,3
37 S(I,3)=C(I,1)*DUM1+C(I,2)*DUM2
DUM1=AK/H
S(4,3)=-C(4,4)*DUM1
DO 38 I=1,3
38 S(I,4)=-C(I,3)*DUM1
S(4,4)=C(4,4)*BK/H
DUM1=AJ/H
DO 39 I=1,3
39 S(I,6)=C(I,3)*DUM1
S(4,6)=-C(4,4)*BJ/H
IF(KASE.EQ.3) GO TO 300
DUM1=-BJ/H
DUM2=(AJ*ZCR-BJ*RCR)/H

```

      DC 40 I=1,3
40  S(I,5)=C(I,1)*DJM1+C(I,2)*DUM2
      S(4,5)=C(4,4)*AJ/H
      GO TO 300

```

C
C
C

RECTANGULAR ELEMENT

```

200  AJ=RA(NPJ)-RA(NPI)
      BJ=ZA(NPJ)-ZA(NPI)
      A=SQRT(AJ*AJ+BJ*BJ)
      AL=RA(NPL)-RA(NPI)
      BL=ZA(NPL)-ZA(NPI)
      B=SQRT(AL*AL+BL*BL)
      H=A*B

```

C

```

50  IF(KASE.NE.4) GO TO 51
      IF(ISTRES.NE.0) GO TO 52
      AOR=1./RR
      ROR=RP/RR
      ZOR=ZP/RR
      IF(NCRACK.EQ.0) GO TO 53
      H=A
      AOR=0.0
      B=0.0
      GO TO 53
52  AOR=0.0
      ROR=0.0
      ZOR=0.0
      IF(NCRACK.EQ.0) GO TO 53
      H=A
      B=0.0
53  DUM1=(RP*S1+ZP*C1-A*S1-B*C1)/H
      DUM2=AOR+(ROR*ZP-B*ROR-A*ZOR)/H
      DO 65 I=1,3
65  S(I,1)=C(I,1)*DJM1+C(I,2)*DUM2
      S(4,1)=C(4,4)*(B*S1-A*C1+RP*C1-ZP*S1)/H
      GO TO 54
51  IF(KASE.EQ.5) GO TO 73
      ROR=1.
      ZOR=0.
      IF(NCRACK.EQ.0) GO TO 54
      H=A
      AOR=0.0
      B=0.0
      GO TO 54
73  IF(ICOUNT.NE.1) GO TO 74
      ROR=C1
      ZOR=S1
      GO TO 54
74  ROR=RP/RR
      ZOR=ZP/RR
54  DUM1=(B*S1-A*C1+RP*C1-ZP*S1)/H
      DO 66 I=1,3
66  S(I,2)=C(I,3)*DJM1
      S(4,2)=C(4,4)*(RP*S1+ZP*C1-B*C1-A*S1)/H
      DUM1=(B*C1-RP*S1-ZP*C1)/H
      DUM2=(B*ROR-ROR*ZP)/H
      DO 67 I=1,3
67  S(I,3)=C(I,1)*DJM1+C(I,2)*DUM2
      DUM1=(-B*S1-RP*C1+ZP*S1)/H

```

```

S(4,3)=C(4,4)*DJM1
DO 68 I=1,3
68 S(I,4)=C(I,3)*DJM1
S(4,4)=C(4,4)*(B*C1-RP*S1-ZP*C1)/H
DUM1=(RP*S1+ZP*C1)/H
DUM2=ROR*ZP/H
DO 59 I=1,3
69 S(I,5)=C(I,1)*DJM1+C(I,2)*DUM2
DUM1=(RP*C1-ZP*S1)/H
S(4,5)=C(4,4)*DJM1
DO 70 I=1,3
70 S(I,6)=C(I,3)*DJM1
S(4,6)=C(4,4)*(ZP*C1+RP*S1)/H
DUM1=(A*C1-RP*C1+ZP*S1)/H
DO 71 I=1,3
71 S(I,8)=C(I,3)*DJM1
S(4,8)=C(4,4)*(A*S1-RP*S1-ZP*C1)/H
IF(KASE.EQ.6) GO TO 300
S(4,7)=C(4,4)*DJM1
DUM1=(A*S1-RP*S1-ZP*C1)/H
DUM2=(A*ZOR-ROR*ZP)/H
DO 72 I=1,3
72 S(I,7)=C(I,1)*DJM1+C(I,2)*DUM2

```

C
C
C

MODIFY S-MATRIX

```

300 ISWTC=0
IF(ITYPE(NPI).NE.1) GO TO 303
MX=1
NP=NPI
KCOUNT=1
GO TO 306
303 IF(ITYPE(NPJ).NE.1) GO TO 304
MX=3
NP=NPJ
KCOUNT=2
GO TO 306
304 IF(ITYPE(NPK).NE.1) GO TO 305
MX=5
NP=NPK
KCOUNT=3
GO TO 306
305 IF(NPL.EQ.0) GO TO 307
IF(ITYPE(NPL).NE.1) GO TO 307
MX=7
NP=NPL
KCOUNT=4

```

C

```

306 IF(ISWTC.EQ.1) GO TO 308
ISWTC=1
DO 301 I=1,8
DO 301 J=1,8
IF(I.NE.J) GO TO 302
CBAR(I,J)=1.0
GO TO 301
302 CBAR(I,J)=0.0
301 CONTINUE
DO 312 I=1,4
DO 312 J=1,8
312 SBAR(I,J)=0.0

```

```

C
308 NX=MX+1
   CBAR(MX,MX)=COS(THETA(NP))
   CBAR(NX,MX)=SIN(THETA(NP))
   CBAR(MX,NX)=-CBAR(NX,MX)
   CBAR(NX,NX)=CBAR(MX,MX)
   GO TO (303,304,305,309),KCOUNT
307 IF(ISWTCH.EQ.0) GO TO 100
C
309 DO 310 I=1,4
   DO 310 J=1,8
   SBAR(I,J)=0.0
   DO 310 K=1,8
310 SBAR(I,J)=SBAR(I,J)+S(I,K)*CBAR(K,J)
C
   DO 311 I=1,4
   DO 311 J=1,8
311 S(I,J)=SBAR(I,J)
C
C   DISTRIBUTE S-MATRIX
C
100 IF(ICOUNT.NE.1) GO TO 101
   MI=1
   MJ=3
   MK=5
   ML=7
   NI=NPI
   NJ=NPJ
   NK=NPK
   NL=NPL
   GO TO 103
C
101 IF(ICOUNT.NE.2) GO TO 102
   MI=3
   MJ=1
   MK=5
   ML=7
   NI=NPJ
   NJ=NPI
   NK=NPK
   NL=NPL
   GO TO 103
C
102 IF(ICOUNT.NE.3) GO TO 107
   MI=5
   MJ=1
   MK=3
   ML=7
   NI=NPK
   NJ=NPI
   NK=NPJ
   NL=NPL
   GO TO 103
107 MI=7
   MJ=1
   MK=3
   ML=5
   NI=NPL
   NJ=NPI
   NK=NPJ

```

NL=NPK

C

```

103 DO 114 I=1,4
    STNPU(I,NI)=STNPJ(I,NI)+S(I,M1)
114 STNPW(I,NI)=STNPW(I,NI)+S(I,M1+1)
    N=1
155 GO TO (150,151,152,153),N
150 NN=NJ
    MN=MJ
    GO TO 154
151 NN=NK
    MN=MK
    GO TO 154
152 IF(NPL.EQ.0) GO TO 153
    NN=NL
    MN=ML

```

C

```

154 DO 104 K=1,MXADJP
    J=K
    IF((NPADJ(NI,K)-NPQUT).EQ.NN) GO TO 105
104 CONTINUE
109 WRITE(6,106) NUNE,NI,N,NPQUT
106 FORMAT(1H1/30H ERROR IN STRSS, STATEMENT 104/
    113H ELEMENT NO.=,15/13H NODE POINT =,15/13H N
    213H NPQUT      =,15)
    CALL EXIT
105 DO 115 I=1,4
    STADU(I,NI,J)=STADJ(I,NI,J)+S(I,MN)
115 STADW(I,NI,J)=STACW(I,NI,J)+S(I,MN+1)
    N=N+1
    GO TO 155

```

=,15/

C

```

153 GO TO (110,111,112,113),ICOUNT
110 ICOUNT=2
    RR=RA(NPJ)
    R=AJ
    Z=BJ
    RP=A
    ZP=0.0
    GO TO 120
111 ICOUNT=3
    RR=RA(NPK)
    R=AK
    Z=BK
    RP=A
    ZP=B
120 IF(NPL.EQ.0) GO TO 25
    GO TO 50
112 IF(NPL.EQ.0) RETURN
    ICOUNT=4
    RR=RA(NPL)
    RP=0.0
    ZP=B
    GO TO 50
113 RETURN
    END

```

C
C
C

SUBROUTINE MUDD(MAXNP,MXADJP,NADJNP,NPADJ,NADJEL,STNPU,STNPW,

```

1STADU,STADW,IPRINT,THETA,ITYPE,NPOUT,NUMNP)
  DIMENSION NADJNP(MAXNP),NPADJ(MAXNP,MXADJP),NADJEL(MAXNP),
1STNPU(4,MAXNP),STNPW(4,MAXNP),STADU(4,MAXNP,MXADJP),
2STADW(4,MAXNP,MXADJP),THETA(MAXNP),ITYPE(MAXNP)
  DO 2 I=1,NUMNP
    DUM=NADJEL(I)
    DO 1 K=1,4
      STNPU(K,I)=STNPJ(K,I)/DUM
1  STNPW(K,I)=STNPW(K,I)/DUM
    NUM=NADJNP(I)
    DO 2 J=1,NUM
      DO 2 K=1,4
        STADU(K,I,J)=STADU(K,I,J)/DUM
2  STADW(K,I,J)=STADW(K,I,J)/DUM
    IF((IPRINT.NE.3).AND.(IPRINT.NE.99))RETURN
    WRITE(6,3)
3  FORMAT(1H1,13HSTRESS TABLES//6H  NODE,8X,6HSTRNPU,8X,6HSTRNPW,
18X,6HSTTNPW,8X,6HSTTNPW,8X,6HSTZNPW,8X,6HSTZNPW,8X,6HSTSNPU,8X,
26HSTSNPW//)
    DC 4 I=1,NUMNP
    L=I+NPOUT
4  WRITE(6,5) L,(STNPU(K,I),STNPW(K,I),K=1,4)
5  FORMAT(15,3X,1P8E14.4)
    WRITE(6,6)
6  FORMAT(1H1,22HADJACENT STRESS TABLES//6H  NODE,10X,6HSTRADU/
116X,6HSTRADW /16X,6HSTTADU/16X,6HSTTADW/16X,6HSTZADU/
216X,6HSTZADW/16X,6HSTSADU/16X,6HSTSADW//)
    DC 7 I=1,NUMNP
    L=I+NPOUT
    WRITE(6,5) L
    NUM=NADJNP(I)
    DO 9 K=1,4
      WRITE(6,8) (STADJ(K,I,J),J=1,NUM)
9  WRITE(6,8) (STADW(K,I,J),J=1,NUM)
8  FORMAT(8X,1P8E14.4)
7  CONTINUE

```

C

RETURN

C

C

END

C

C

C

```

OVERLAY(MOHAN,7,0)
PROGRAM LNKIG

```

```

COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPCL3,NUMNP,
1  NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2  KTAPE,KRJN,IPRINT,NUMST,MXSTRT,IELAST(20),IPLAST(20),
3  WGT(20),NSTART(79),EI(5,20),IPELTP,INT,NPRCDS,IMPBX
  DIMENSION FAJ(1600),FAW(1600),R(1600),Z(1600),ITYPE(1600),
1THETAT(1600),NPTP(1600),ANAME(18),CPRESS(100,3),PRESSU(100),
2PRESSW(100),NPLOAD(100),U(1600),W(1600),VPADJ(8),SADU(8),
3SADUW(8),SADWJ(8),SADWW(8),PLOADU(100),PLOADW(100),UDISP(100),
4WDISP(100),NPDISP(100)
  EQUIVALENCE(U,R),(W,Z),(PLOADU,PRESSU,UDISP),
1  (PLOADW,PRESSW,WDISP),(NPDISP,NPLOAD)
  MOHAN=5HMOHAN
  REWIND 8
  DC 6 I=1,NUMNP

```

```

6 READ(8) J,R(1),R(1),(R(K),K=1,MXADJP)
  READ(8) NJ,(NPLOAD(I),NPLOAD(I),NPLOAD(I),NPLOAD(I),NPLOAD(I),
  1 NPLOAD(I),I=1,NJ)
  READ(8) (NU,NPTP(I),I=1,NUMNP)
  REWIND 8
  WRITE(6,2)
2 FORMAT(1H1,21HAPPLIED PRESSURE DATA//)
  READ(5,3) NLINES
3 FORMAT(14I5)
  WRITE(6,4) NLINES
4 FORMAT(26H NO. OF PRESSURE SURFACES=,I5//)
  REWIND 4
  DO 5 I=1,NUMNP
    FAU(I)=0.0
    FAW(I)=0.0
5 READ(4) N,R(1),Z(1),ITYPE(1),THETA(1)
  REWIND 4

```

C

```

  IF(NLINES.EQ.0) GO TO 20
  DO 19 ILINE=1,NLINES
    READ(5,8) LOADNP,ANAME
8 FORMAT(15,18A4)
    WRITE(6,9) ANAME,LOADNP,ILINE
9 FORMAT(//18A4/20H NO. OF NODE POINTS=,I5,12H ON SURFACE,I5//)
    WRITE(6,11)
11 FORMAT(5H NODE,10X,6HPRESSU,14X,6HPRESSW/16X,3HPSI,17X,3HPSI//)
    DO 7 I=1,LOADNP
      READ(5,10) NPLOAD(I),PRESSU(I),PRESSW(I)
10 FORMAT(15,2E10.0)
      7 WRITE(6,12) NPLOAD(I),PRESSU(I),PRESSW(I)
12 FORMAT(15,1P2E20.5)
      DO 13 I=1,LOADNP
        NP=NPLOAD(I)
        NPLOAD(I)=NPTP(NP)
        CALL COEF(MAXNP,NUMNP,LOADNP,NPLOAD,CPRESS,R,Z,ISTRESS)
        DO 14 I=1,LOADNP

```

C*****

```

  IF(I.EQ.1) GO TO 16
  DUMU=PRESSU(I-1)*CPRESS(I,1)
  DUMW=PRESSW(I-1)*CPRESS(I,1)
  GO TO 15
16 DUMU=0.0
  DUMW=0.0
15 DUMU=DUMU+PRESSU(I)*CPRESS(I,2)
  DUMW=DUMW+PRESSW(I)*CPRESS(I,2)
  IF(I.EQ.LOADNP) GO TO 17
  DUMU=DUMU+PRESSU(I+1)*CPRESS(I,3)
  DUMW=DUMW+PRESSW(I+1)*CPRESS(I,3)

```

C*****

```

17 NP=NPLOAD(I)
  FAU(NP)=FAU(NP)+DUMU
  FAW(NP)=FAW(NP)+DUMW
14 CONTINUE
19 CONTINUE

```

C

```

20 WRITE(6,21)
21 FORMAT(1H1,22HCONCENTRATED LOAD DATA//)
  READ(5,3) NLINES
  WRITE(6,22) NLINES
22 FORMAT(22H NO. OF LOAD CLUSTERS=,I5//)

```



```

C      IF(NLINES.EQ.0) GO TO 26
C
      DO 28 ILINE=1,NLINES
      READ(5,8) LOADNP, ANAME
      WRITE(6,23) ANAME,LOADNP, ILINE
23     FORMAT(//18A4/14H NO. OF NODES=,15,21H IN LOAD CLUSTER NO.,15//)
      WRITE(6,24)
24     FORMAT(5H NODE,10X,5HLOADU,15X,5HLOADW/10X,3HLBS,17X,3HLBS//)
      DO 18 I=1,LOADNP
      READ(5,10) NPLOAD(I), PLOADU(I), PLOADW(I)
18     WRITE(6,12) NPLOAD(I), PLOADU(I), PLOADW(I)
C
      DO 25 I=1,LOADNP
      NP=NPLOAD(I)
      PLOADU(I)=NPLOAD(I)
      PLOADW(I)=NPLOAD(I)
      FAU(NP) = PLOADU(I) + PLOADU(I)
25     FAW(NP) = FAW(NP) + PLOADW(I)
C
28     CONTINUE
26     DO 27 I=1,NUMNP
      IF(IATYPE(I).NE.1) GO TO 27
      DUMU=FAU(I)*COS(THETA(I))+FAW(I)*SIN(THETA(I))
      DUMW=-FAU(I)*SIN(THETA(I))+FAW(I)*COS(THETA(I))
      FAU(I)=DUMU
      FAW(I)=DUMW
27     CONTINUE
C
      DO 29 I=1,NUMNP
      U(I)=0.0
29     W(I)=0.0
C
      WRITE(6,30)
30     FORMAT(11H1,17HDISPLACEMENT DATA//)
      READ(5,3) NLINES
      WRITE(6,31) NLINES
31     FORMAT(30H NO. OF DISPLACEMENT CLUSTERS=,15//)
C
      IF(NLINES.EQ.0) GO TO 37
      DO 32 ILINE=1,NLINES
      READ(5,8) LDISP, ANAME
      WRITE(6,33) ANAME, LDISP, ILINE
33     FORMAT(//18A4/20H NO. OF NODE POINTS=,15,11H ON CLUSTER,15//)
      WRITE(6,34)
34     FORMAT(5H NODE,10X,6HUDISPL,14X,6HWDISPL/17X,3HIN.,17X,3HIN.//)
      DO 35 I=1,LDISP
      READ(5,10) NPDISP(I), UDISP(I), WDISP(I)
35     WRITE(6,12) NPDISP(I), UDISP(I), WDISP(I)
      DO 36 I=1,LDISP
      NP = NPDISP(I)
      NPDISP(I) = NPTP(NP)
      NP=NPDISP(I)
      U(NP)=UDISP(I)
36     W(NP)=WDISP(I)
32     CONTINUE
C
37     REWIND 13
      REWIND 14
      DO 38 I=1,NUMNP

```

100

```

READ(10) N,NADJNP, ITYPE(1), THETA(1), XMASS, SNPLU, SNPUW, SNPWW,
1(NPADJ(J), SADUJ(J), SADJW(J), SADWU(J), SADWW(J), J=1, MXADJP)
FAW(1)=FAW(1)+XMASS
38 WRITE(14)1,NADJNP, ITYPE(1), THETA(1), XMASS, SNPLU, SNPUW, SNPWW,
1(NPADJ(J), SADUJ(J), SADJW(J), SADWU(J), SADWW(J), J=1, MXADJP),
2FAU(1), FAW(1), J(1), W(1)
REWIND 10
REWIND 14
IF((IPRINT.NE.5).AND.(IPRINT.NE.99))RETURN
WRITE(6,101)
101 FORMAT(1H1,32HINPUT LOAD AND DISPLACEMENT DATA//
15X,3HNEW,12X,6HJDISPL,14X,6HWDISPL,14X,5HLOADU,15X,5HLOADW/
25X,4HNOCE,12X,4H(IN),16X,4H(IV),15X,5H(LBS),15X,5H(LBS)//)
DC 102 I=1,NJMNP
102 WRITE(6,103) I,J(1),W(1),FAU(1),FAW(1)
103 FCRMAT(18,2X,1P4E20.5)
RETURN

```

C
C
C

```

END
SUBROUTINE COEF(MAXNP,NUMNP,LOADNP,NPLOAD,CPRESS,R,Z,ISTRES)
DIMENSION NPLOAD(LOADNP),CPRESS(100,3),R(MAXNP),Z(MAXNP)
DC 1 I=1,LOADNP
DC 1 J=1,3
1 CPRESS(I,J)=0.0

```

C

```

NUM=LOADNP-1
DC 4 I=1,NUM
NP=NPLOAD(I)
NP1=NPLCAD(I+1)
AJ=R(NP1)-R(NP)
BJ=Z(NP1)-Z(NP)
AL=SQRT(AJ*AJ+BJ*BJ)
IF(ISTRES.NE.0) GO TO 2
C1=AL*(3.*R(NP)+R(NP1))/12.
C2=AL*(R(NP)+R(NP1))/12.
C3=AL*(R(NP)+3.*R(NP1))/12.
GO TO 3
2 C1=AL/3.
C2=AL/6.
C3=C1
3 CPRESS(I,2)=CPRESS(I,2)+C1
CPRESS(I,3)=C2
M=I+1
CPRESS(M,2)=C3
CPRESS(M,1)=C2
4 CCNT INJE
RETURN
END

```

C
C
C

```

OVERLAY(MOHAN,10,0)
PROGRAM LNK1H
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPEL3,NUMNP,
1 NUMEL,ISTRES,NUMPFL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2 KTAPE,KRJN,IPRINT,NUMST,MXSTRT,IELAST(20),LAST(20),
3 WGT(20),NSTART(79),EI(5,20),IPELTP,INT,NPRGDS
DIMENSION NADJNP(400),ITYPE(400),SNPUU(400),SNPUW(400),

```

```

1SNPUW(400),SNPWW(400),NPACJ(400,16),SADUU(400,16),
2SADUW(400,16),SADWJ(400,16),SADWW(400,16),FAU(400),
3FAW(400),J(400),W(400)
DIMENSION UN(1600),WN(1600)
MCHAN=5HMOHAN

```

```

C*****

```

```

WRITE(6,1000)

```

```

1000 FORMAT(* ELIMINATION SOLUTION HAS STARTED*)

```

```

C*****

```

```

REWIND 13

```

```

REWIND 14

```

```

NXADJP=2*MXADJP

```

```

NPOUT=0

```

```

NUMCP=0

```

```

KX=1

```

```

GO TO 900

```

```

1 DO 50 N=1,NUMNPB

```

```

IDUM=NACJNP(N)

```

```

IF(IDUM.EQ.0) GO TO 60

```

```

DO 51 J=1,IDJM

```

```

IF(NPADJ(N,J).GT.(NPOUT+NUMNPB)) GO TO 59

```

```

51 CONTINUE

```

```

C

```

```

60 IF(ITYPE(N).EQ.0) GO TO 50

```

```

IF(IDUM.EQ.0) GO TO 61

```

```

DO 52 J=1,IDJM

```

```

NADJ=NPADJ(N,J)

```

```

NRDJ=NADJ-NPOUT

```

```

JDUM=NACJNP(NRDJ)

```

```

DO 53 K=1,JDJM

```

```

KK=K

```

```

IF(NPADJ(NRDJ,K).EQ.(N+NPOUT)) GO TO 55

```

```

53 CONTINUE

```

```

WRITE(6,54)

```

```

54 FORMAT(1H1,26HERROR IN LIF, STATEMENT 54//)

```

```

NCDE=N

```

```

GO TO 909

```

```

55 IF(ITYPE(N).EQ.1) GO TO 56

```

```

FAU(NRDJ)=FAJ(NRDJ)-SADUU(NRDJ,KK)*U(N)

```

```

FAW(NRDJ)=FAW(NRDJ)-SADWU(NRDJ,KK)*U(N)

```

```

SADUU(NRDJ,KK)=0.0

```

```

SADWU(NRDJ,KK)=0.0

```

```

SADUU(N,J)=0.0

```

```

SADUW(N,J)=0.0

```

```

56 IF(ITYPE(N).EQ.3) GO TO 52

```

```

FAU(NRDJ)=FAJ(NRDJ)-SADUW(NRDJ,KK)*W(N)

```

```

FAW(NRDJ)=FAW(NRDJ)-SADWW(NRDJ,KK)*W(N)

```

```

SADUW(NRDJ,KK)=0.0

```

```

SADWW(NRDJ,KK)=0.0

```

```

SADWU(N,J)=0.0

```

```

SADWW(N,J)=0.0

```

```

52 CONTINUE

```

```

61 IF(ITYPE(N).GT.1) GO TO 57

```

```

FAW(N)=W(N)

```

```

FAU(N)=FAU(N)-SNPUW(N)*W(N)

```

```

SNPWW(N)=1.0

```

```

SNPUW(N)=0.0

```

```

SNPWU(N)=0.0

```

```

GO TO 50

```

```

57 IF(ITYPE(N).GT.2) GO TO 58

```

```

FAU(N)=U(N)
FAW(N)=W(N)
SNPUU(N)=1.0
SNPWW(N)=1.0
SNPUW(N)=0.0
SNPWU(N)=0.0
GC TO 50
58 FAU(N)=U(N)
   FAW(N)=FAW(N)-SNPWU(N)*U(N)
   SNPUU(N)=1.0
   SNPUW(N)=0.0
   SNPWU(N)=0.0
50 CONTINUE
59 DC 3 N=1, NUMNPB
   NN=N
   JDUM=N+NPOUT
   IF(JDJM.EQ.NJMNP) GO TO 14
   IDUM=NADJNP(N)
   DC 4 J=1, IDJM
   IF(NPADJ(N,J).GT.(NPOUT+NUMNPB)) GO TO 43
4 CONTINUE
   NRMCP=NRMCP+1
   DC 5 J=1, IDJM
   NADJ=NPADJ(N,J)
   NRDJ=NADJ-NPOUT
   DC 6 K=J, IDJM
   IF(NPADJ(N,K).EQ.NADJ) GO TO 6
   DO 7 L=1, NXADJP
   LL=L
   LA=LL
   IF(NPADJ(NRDJ,L).EQ.NPADJ(N,K)) GO TO 6
   IF(NPADJ(NRDJ,L).EQ.0) GO TO 9
7 CONTINUE
   IF(NADJ.LT.300) GO TO 6
   WRITE(6,8)
8 FORMAT(1H1,25HERROR IN LIFE, STATEMENT 8//)
   NCDE=N
   GO TO 909
9 NPADJ(NRDJ,LL)=NPADJ(N,K)
   NADJNP(NRDJ)=NADJNP(NRDJ)+1
   NBDJ=NPADJ(N,K)-NPOUT
   DC 10 L=1, NXADJP
   LL=L
   IF(NPADJ(NBDJ,L).EQ.0) GO TO 12
10 CONTINUE
   NPADJ(NRDJ,LA)=0
   NADJNP(NRDJ)=NADJNP(NRDJ)-1
   IF(NADJ.LE.300) GO TO 6
   WRITE(6,11)
11 FORMAT(1H1,26HERROR IN LIFE, STATEMENT 11//)
   NCDE=N
   GO TO 909
12 NPADJ(NBDJ,LL)=NADJ
   NADJNP(NBDJ)=NADJNP(NBDJ)+1
6 CONTINUE
5 CONTINUE
29 DO 20 J=1, IDJM
   NADJ=NPADJ(N,J)
   NRDJ=NADJ-NPOUT
   JDUM=NADJNP(NRDJ)

```

3

```

DC 21 K=1, JDUM
KK=K
IF (NPADJ(NRDJ,K).EQ.(N+NPOUT)) GO TO 23
21 CONTINUE
IF (NADJ.LE.300) GO TO 20
WRITE(6,22)
22 FORMAT(1H1,26HERROR IN L1F, STATEMENT 22//)
NODE=N
GO TO 909
23 SMULU=SADUU(NRDJ, KK)
SMULW=SADWU(NRDJ, KK)
SNPUU(NRDJ)=SNPJJ(NRDJ)-SMULU*SADUU(V,J)/SNPUU(N)
SNPUW(NRDJ)=SNPJW(NRDJ)-SMULU*SADUW(V,J)/SNPUU(N)
SNPWU(NRDJ)=SNPWJ(NRDJ)-SMULW*SADUU(V,J)/SNPUU(N)
SNPWW(NRDJ)=SNPWJ(NRDJ)-SMULW*SADUW(V,J)/SNPUU(N)
DC 24 K=1, JDUM
IF (NPADJ(NRDJ,K).NE.(N+NPOUT)) GO TO 25
SADUW(NRDJ,K)=SADUW(NRDJ,K)-SMULU*SNPUW(V)/SNPLU(N)
SADWW(NRDJ,K)=SADWW(NRDJ,K)-SMULW*SNPUW(V)/SNPLU(N)
SADUU(NRDJ,K)=0.0
SADWU(NRDJ,K)=0.0
GO TO 24
25 DC 26 L=1, IDJM
LL=L
IF (NPADJ(N,L).EQ.NPADJ(NRDJ,K)) GO TO 27
26 CONTINUE
GO TO 24
27 SADUU(NRDJ,KT)=SADUU(NRDJ,K)-SMULU*SADUU(N,LL)/SNPLU(N)
SADUW(NRDJ,K)=SADUW(NRDJ,K)-SMULU*SADUW(N,LL)/SNPLU(N)
SADWU(NRDJ,K)=SADWU(NRDJ,K)-SMULW*SADUU(N,LL)/SNPLU(N)
SADWW(NRDJ,K)=SADWW(NRDJ,K)-SMULW*SADUW(N,LL)/SNPLU(N)
24 CONTINUE
FAU(NRDJ)=FAJ(NRDJ)-SMJLU*FAU(V)/SNPUU(N)
FAW(NRDJ)=FAW(NRDJ)-SMJLW*FAU(N)/SNPUU(N)
SADWU(N,J)=SADWJ(N,J)-SNPWU(V)*SADUU(N,J)/SNPUU(N)
SADWW(N,J)=SADWW(N,J)-SNPWU(V)*SADUW(N,J)/SNPUU(N)
20 CONTINUE
C
14 FAW(N)=FAW(N)-SNPWJ(N)*FAU(V)/SNPUU(V)
SNPWW(N)=SNPWJ(N)-SNPWU(N)*SNPUW(N)/SNPUU(N)
SNPUW(N)=SNPJW(N)/SNPUJ(N)
FAU(N)=FAJ(N)/SNPUJ(N)
JDUM=N+NPOUT
IF (JDUM.EQ.NUMNP) GO TO 15
DO 28 J=1, IDJM
SADUU(N,J)=SADJJ(N,J)/SNPUU(V)
28 SADUW(N,J)=SADUJ(N,J)/SNPUU(V)
C
30 DC 31 J=1, IDJM
NADJ=NPADJ(N,J)
NRDJ=NADJ-IPOUT
JDUM=NADJNP(NRDJ)
DC 32 K=1, JDJM
KK=K
IF (NPADJ(NRDJ,K).EQ.(N+NPOUT)) GO TO 34
32 CONTINUE
IF (NADJ.LE.300) GO TO 31
WRITE(6,33)
33 FORMAT(1H1,26HERROR IN L1F, STATEMENT 33//)
NODE=N

```

```

GC TO 909
34 SMULU=SADUW(NRDJ, KK)
   SMULW=SADWW(NRDJ, KK)
   SNPUU(NRDJ)=SNPJU(NRDJ)-SMULU*SADWU(V, J)/SNPWW(N)
   SNPUW(NRDJ)=SNPJW(NRDJ)-SMULU*SADWW(V, J)/SNPWW(N)
   SNPWU(NRDJ)=SNPWJ(NRDJ)-SMULW*SADWU(V, J)/SNPWW(N)
   SNPWW(NRDJ)=SNPWJ(NRDJ)-SMULW*SADWW(V, J)/SNPWW(N)
DO 35 K=1, JDUM
   IF(NPADJ(NRDJ, K).EQ.(N+NPOUT)) GO TO 35
DO 36 L=1, IDJM
   LL=L
   IF(NPADJ(N, L).EQ.NPADJ(NRDJ, K)) GO TO 37
36 CONTINUE
   GO TO 35
37 SADUU(NRDJ, K)=SADUU(NRDJ, K)-SMULU*SADWU(N, LL)/SNPWW(N)
   SADUW(NRDJ, K)=SADUW(NRDJ, K)-SMULU*SADWW(V, LL)/SNPWW(N)
   SADWU(NRDJ, K)=SADWU(NRDJ, K)-SMULW*SADWU(N, LL)/SNPWW(N)
   SADWW(NRDJ, K)=SADWW(NRDJ, K)-SMULW*SADWW(V, LL)/SNPWW(N)
38 CONTINUE
   FAU(NRDJ)=FAU(NRDJ)-SMJLU*FAU(N)/SNPWW(N)
   FAW(NRDJ)=FAW(NRDJ)-SMJLW*FAW(N)/SNPWW(N)
31 CONTINUE
   FAN(N)=FAW(N)/SNPWW(N)
DO 38 J=1, IDJM
   SADWU(N, J)=SADWJ(N, J)/SNPWW(V)
38 SADWW(N, J)=SADWW(N, J)/SNPWW(V)
2 DO 40 J=1, IDJM
   NADJ=NPADJ(N, J)
   NRDJ=NACJ-NPOUT
   ISW=0
   JDUM=NADJNP(NRDJ)
DO 41 K=1, JDUM
   IF(ISW.EQ.1) GO TO 42
   IF(NPADJ(NRDJ, K).NE.(N+NPOUT)) GO TO 41
   ISW=1
42 IF(K.EQ.JDUM) GO TO 39
   NPADJ(NRDJ, K)=NPADJ(NRDJ, K+1)
   SADUU(NRDJ, K)=SADUU(NRDJ, K+1)
   SADUW(NRDJ, K)=SADUW(NRDJ, K+1)
   SADWU(NRDJ, K)=SADWJ(NRDJ, K+1)
   SADWW(NRDJ, K)=SADWW(NRDJ, K+1)
   GO TO 41
39 NPADJ(NRDJ, K)=0
   SADUU(NRDJ, K)=0.0
   SADUW(NRDJ, K)=0.0
   SADWU(NRDJ, K)=0.0
   SADWW(NRDJ, K)=0.0
41 CONTINUE
   NADJNP(NRDJ)=NADJNP(NRDJ)-1
40 CONTINUE
C
   GC TO 3
C
15 W(N)=FAW(N)/SNPWW(N)
   U(N)=FAU(N)-SNPJW(N)*W(N)
   GO TO 800
3 CONTINUE
C
43 WRITE(10) NPOUT, NRMCP, ( ITYPE(I), FAU(I), FAW(I), SNPUW(I),
1 NADJNP(I), (SADJ(I, J), SADUW(I, J), SADWU(I, J), SADWW(I, J),

```

```

2NPADJ(I,J),J=1,NXADJP),I=1,NRMCP)
NUMCP=NUMCP+NRMCPC
NPOUT=NUMCP
NPR=NUMNPB-NRMCP
GO TO 902
44 KX=NPR+1
GO TO 900
45 NODESR=NUMNP-NUMCP
IF(NODESR.LE.MXNPB) NUMNPB=NODESR
IF(NODESR.GT.MXNPB) NUMNPB=MXNPB
GO TO 904
46 NRMCP=0
GO TO 1
C
900 DO 901 I=KX,MXNPB
NADJNP(I)=0
ITYPE(I)=0
SNPUU(I)=0.0
SNPUW(I)=0.0
SNPWU(I)=0.0
SNPWW(I)=0.0
FAU(I)=0.0
FAW(I)=0.0
U(I)=0.0
W(I)=0.0
DO 901 J=1,NXADJP
NPADJ(I,J)=0
SADUU(I,J)=0.0
SADUW(I,J)=0.0
SADWU(I,J)=0.0
901 SADWW(I,J)=0.0
GO TO 45
902 DO 903 L=1,NPR
K=NRMCPC+L
NADJNP(L)=NADJNP(K)
ITYPE(L)=ITYPE(K)
SNPUU(L)=SNPUU(K)
SNPUW(L)=SNPUW(K)
SNPWU(L)=SNPWU(K)
SNPWW(L)=SNPWW(K)
FAU(L)=FAU(K)
FAW(L)=FAW(K)
U(L)=U(K)
W(L)=W(K)
DO 903 J=1,NXADJP
NPADJ(L,J)=NPADJ(K,J)
SADUU(L,J)=SADUU(K,J)
SADUW(L,J)=SADUW(K,J)
SADWU(L,J)=SADWU(K,J)
903 SADWW(L,J)=SADWW(K,J)
GO TO 44
904 CONTINUE
DO 905 I=KX,NMNPB
READ(14) N,NADJNP(I),ITYPE(I),THETA,XMASS,SNPUU(I),SNPUW(I),
1SNPWU(I),NPADJ(I,J),SADUU(I,J),SADUW(I,J),SADWU(I,J),
2J=1,MXADJP),FAU(I),FAW(I),U(I),W(I)
905 CONTINUE
DO 913 I=KX,NUMNPB
SNPWU(I)=SNPWU(I)
913 CONTINUE

```

```

GO TO 46
909 WRITE(6,910) NPOUT,NOCE,NADJ
    WRITE(6,911) (NPADJ(NODE,K),K=1,NXADJP)
    WRITE(6,911) (NPADJ(NRDJ,K),K=1,NXADJP)
910 FORMAT(7H NPOUT=,15/7H NOCE =,15/7H NADJ =,15/7H NPADJ=)
911 FORMAT(16I5)
    CALL EXIT
800 DO 801 N=1,NUMNP
    UN(N)=0.0
801 WN(N)=0.0
    UN(NUMNP)=U(NN)
    WN(NUMNP)=W(NN)
C
    IF(NPOUT.EQ.0) GO TO 807
    BACKSPACE 10
807 CONTINUE
805 DC 802 I=1,NRMCP
    J=NRMCP+1-I
    N=J+NPOUT
    WN(N)=FAW(J)
    IDUM=NADJNP(J)
    DO 803 K=1,IDUM
    NADJ=NPADJ(J,K)
803 WN(N)=WN(N)-SADWJ(J,K)*UN(NADJ)-SADWW(J,K)*WN(NADJ)
    UN(N)=FAU(J)-SNPUW(J)*WN(N)
    DO 804 K=1,IDUM
    NADJ=NPADJ(J,K)
804 UN(N)=UN(N)-SADJJ(J,K)*UN(NADJ)-SADUW(J,K)*WN(NADJ)
C
802 CONTINUE
C
    IF(N.EQ.1) GO TO 806
C
    READ(10) NPOUT,NRMCP,(ITYPE(I),FAU(I),FAW(I),SNPUW(I),
1 NADJNP(I),(SADJJ(I,J),SADUW(I,J),SADWU(I,J),SADWW(I,J),
2 NPADJ(I,J),J=1,NXADJP),I=1,NRMCP)
    IF(NPOUT.EQ.0) GO TO 805
    BACKSPACE 10
    BACKSPACE 10
    GO TO 805
C
806 REWIND 10
    REWIND 14
C
    DC 808 I=1,NUMNP
C
    READ(14) N,NTDJNP,JTYPE,THETA,XMASS,STNPUU,STNPUW,STNPHW,
1 (NPADJ(1,J),SADUU(1,J),SADUW(1,J),SADWU(1,J),SADWW(1,J),
2 J=1,MXADJP),FU,FW,UDUM,WCUW
C
808 WRITE(10) N,NTDJNP,JTYPE,THETA,XMASS,STNPLU,STNPUW,STNPHW,FU,FW,
1 (NPADJ(1,J),SADUU(1,J),SADUW(1,J),SADWW(1,J),J=1,MXADJP)
C
    REWIND 8
    REWIND 10
    REWIND 14
C
    DC 809 I=1,NUMNP
809 READ(8) J,FAU(1),FAU(1),(FAU(K),K=1,MXADJP)
    READ(8) NU,(ITYPE(1),ITYPE(1),ITYPE(1),ITYPE(1),ITYPE(1),

```



```

1 ITYPE(I), I=1, NJ)
  READ(8) (NJ, NU, I=1, NUMNP)
  WRITE(8) (UN(I), WN(I), I=1, NUMNP)
C
  REWIND 8
  IF((IPRINT.NE.6).AND.(IPRINT.NE.99))RETURN
  WRITE(6,810)
810 FORMAT(1H1,22HRESULTS OF ELIMINATION//
  15X,3HNEW,12X,6HJDISPL,14X,6HWDISPL/
  25X,4HNOCE,12X,4H(IN),16X,4H(IN)//)
  DO 811 I=1, NUMNP
811 WRITE(6,812) I, JN(I), WN(I)
812 FORMAT(18,2X,1P2E20.5)
  RETURN
  END
C
C
C
  OVERLAY(MOHAN,11,0 )
  PROGRAM LNK11
  COMMON MAXNP, MXCLS, MXADJP, MXZONE, MXNPB, NZONES, MXPELB, NUMNP,
1      NJMEL, ISTRES, NUMPEL, NUMELP, PERIOD, NMKCLS, FACTOR, ALAMB,
2      KTAPE, KKJN, IPRINT, NUMST, MXSTRT, IELAST(20), IPLAST(20),
3      WGT(20), NSTART(79), EI(5,20), IPELTP, INT, NPRCDS, IMPBX
C
  DIMENSION NADJNP(400), ITYPE(400), THETA(400), XMASS(400),
1SNPUU(400), SNPUW(400), SNPWW(400), NPADJ(400,8), SADLU(400,8),
2SADUW(400,8), SADWW(400,8), NADJEL(400)
C
C
  DIMENSION STNPU(4,400), STNPW(4,400), STADU(4,400,1),
1STADW(4,400,8)
  DIMENSION NPLOW(80), NPHIGH(80), NPOUT(80), NUMCP(80),
1NELCLS(80), NMPCLS(80), NPTN(1600), FAU(400), FAW(400)
C
  EQUIVALENCE (STNPU(1), ITYPE), (STNPU(401), THETA),
1(STNPU(801), XMASS), (STNPU(1201), SNPUU), (STNPW(1), SNPUW),
2(STNPW(401), SNPWW), (STADU(1), SADUU), (STADU(3201), SADUW),
3(STADU(6401), SADWW), (STADW(1), NPTN), (STNPW(801), FAU),
4(STNPW(1201), FAW)
C
  MOHAN=5HMOHAN
  REWIND 8
  DO 1 I=1, NUMNP
1 READ(8) N, NPLOW(1), NPLOW(1), (NPLOW(J), J=1, MXADJP)
  READ(8) NJMCLS, (NPLOW(I), NPHIGH(I), NPOUT(I), NUMCP(I),
1NELCLS(I), NMPCLS(I), I=1, NUMCLS)
  REWIND 10
  REWIND 14
  IC=1
  NLOW=NPCUT(IC)+1
  NHGH=NUMCP(IC)
104 DO 101 L=NLOW, NHGH
101 READ(10) N, NADJNP(L), ITYPE(L), THETA(L), XMASS(L), SNPUU(L),
1SNPUW(L), SNPWW(L), FAU(L), FAW(L), (NPADJ(L,J), SADUU(L,J), SADUW(L,J),
2SADWW(L,J), J=1, MXADJP)
  NUMNPB=NUMCP(IC)-NPOUT(IC)
  WRITE(14) NPLOW(IC), NPHIGH(IC), NPOUT(IC), NUMCP(IC), NELCLS(IC),
1NMPCLS(IC), NJMNPB, (NADJNP(I), ITYPE(I), THETA(I), XMASS(I), SNPUU(I),
2SNPUW(I), SNPWW(I), FAU(I), FAW(I), (NPADJ(I,J), SADUU(I,J), SADUW(I,J),

```

```

3SADWW(I,J),J=1,MXADJP),I=1,NUMNPB)
  IC=IC+1
  IF(IC.GT.NUMCLS) GO TO 105
  IF(NPOUT(IC).NE.NUMMCP(IC-1)) GO TO 103
  NLOW=1
  NHGH=NUMMCP(IC)-NPOUT(IC)
  GO TO 104
103 NPR=NUMMCP(IC-1)-NPOUT(IC)
  DO 102 I=1,NPR
  L=NPOUT(IC)-NPOUT(IC-1)+I
  NADJNP(I)=NADJNP(L)
  ITYPE(I)=ITYPE(L)
  THETA(I)=THETA(L)
  XMASS(I)=XMASS(L)
  SNPUU(I)=SNPUJ(L)
  SNPUW(I)=SNPUJW(L)
  SNPWW(I)=SNPWW(L)
  FAU(I)=FAU(L)
  FAW(I)=FAW(L)
  DO 102 J=1,MXADJP
  NPADJ(I,J)=NPADJ(L,J)
  SADUU(I,J)=SADUJ(L,J)
  SADUW(I,J)=SADUW(L,J)
102 SADWW(I,J)=SADWW(L,J)
  NLOW=NUMMCP(IC-1)-NPOUT(IC)+1
  NHGH=NUMMCP(IC)-NPOUT(IC)
  GO TO 104
105 REWIND 10
  REWIND 14
  DC 105 NC=1,NUMCLS
  READ(14) N1,N2,N3,N4,N5,N6,N7,(NADJNP(I),ITYPE(I),THETA(I),
  1XMASS(I),SNPUJ(I),SNPUJW(I),SNPWW(I),FAU(I),FAW(I),(NPADJ(I,J),
  2SADUU(I,J),SADUJ(I,J),SADUW(I,J),J=1,MXADJP),I=1,N7)
  WRITE(10)N1,N2,N3,N4,N5,N6,N7,(NADJNP(I),ITYPE(I),THETA(I),
  1XMASS(I),SNPUJ(I),SNPUJW(I),SNPWW(I),FAU(I),FAW(I),(NPADJ(I,J),
  2SADUU(I,J),SADUJ(I,J),SADUW(I,J),J=1,MXADJP),I=1,N7)
106 CONTINUE
  READ(9) (NPTN(I),NC,I=1,NUMNP)
  WRITE(10) (NPTN(I),I=1,NUMNP)
  REWIND 8
  REWIND 3
  IC=1
  NLOW=1
  NHGH=NUMMCP(IC)
204 DC 201 L=NLOW,NHGH
201 READ(3) N,NADJNP(L),NADJEL(L),(NPADJ(L,J),J=1,MXADJP),
  1(STNPU(K,L),STNPW(K,L),K=1,4),(STADU(K,L,J),STADW(K,L,J),
  2K=1,4),J=1,MXADJP)
C
  NUMNPB=NUMMCP(IC)-NPOUT(IC)
  WRITE(10) NLOW(IC),NPHIGH(IC),NPOUT(IC),NUMMCP(IC),NECLS(IC),
  1NPMCLS(IC),NUMNPB,(NADJNP(I),NADJEL(I),(NPADJ(I,J),J=1,MXADJP),
  2(STNPU(K,I),STNPW(K,I),K=1,4),(STADU(K,I,J),STADW(K,I,J),
  3K=1,4),J=1,MXADJP),I=1,NUMNPB)
C
  IC=IC+1
  IF(IC.GT.NUMCLS) GO TO 205
  IF(NPOUT(IC).NE.NUMMCP(IC-1)) GO TO 203
  NLOW=1
  NHGH=NUMMCP(IC)-NPOUT(IC)

```

GO TO 204

C
203 NPR=NUMCP(IC-1)-NPOUT(IC)
DO 202 I=1,NPR
L=NPOUT(IC)-NPOJT(IC-1)+1
NADJNP(I)=NADJNP(L)
NADJEL(I)=NADJEL(L)
DO 207 J=1,MXADJP
207 NPADJ(I,J)=NPADJ(L,J)
DO 202 K=1,4
STNPU(K,I)=STNPJ(K,L)
STNPW(K,I)=STNPW(K,L)
DO 202 J=1,MXADJP
STADU(K,I,J)=STADU(K,L,J)
202 STADW(K,I,J)=STADW(K,L,J)

C
NLOW=NUMCP(IC-1)-NPOUT(IC)+1
NHGH=NUMCP(IC)-NPOJT(IC)
GO TO 204

C
205 REWIND 10
REWIND 3
RETURN
END

C
C
C
OVERLAY(MOHAN,12,0)
PROGRAM LNK1J
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPELB,NUMNP,
1 NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2 KTAPE,KRJN,IPRINT,NUMST,MXSTRT,IELAST(20),IPLAST(20),
3 WGT(20),NSTART(79),EI(5,20),IPELTP,INT,NPRCDS,IMPBX
DIMENSION NLOW(80),NPHIGH(80),NPOUT(80),NUMCP(80),
1 NELCLS(80),NMPCLS(80)

C
DIMENSION JPLAST(20),CC(4,4)
DIMENSION NOOFEL(24),NP(24,4),ITYPE(24,4),THETA(24,4),
1 IC(24,4,4),B(24,4,8),P(24,8,4),EPSTI1(24,4),EPSPI1(24,4),
2 SIGI1(24,4),DJM(24,29),IDUM(24,29)
EQUIVALENCE (DJM,IDUM)
DIMENSION ALPHA(20),CAPPA(20),CUSTH(20),NOYILD(20),
1 SSTAR(20,10),HSTAR(20,10)
2 ,COHESN(20),FRCTN1(20),FRCTN2(20),SNSWCH(20),
3 CRESID(20),FRESID(20),JTENSN(20),
4 MYIELD(20),TRESID(20)
MOHAN=5HMOHAN
IF(NUMPEL.EQ.0) RETURN
REWIND 12
REWIND 8
DO 1 I=1,NUMNP
1 READ(8) T,NPLOW(I),NPLOW(1),NPLOW(J),J=1,MXADJP
READ(8) NUMCLS,(NPLOW(I),NPHIGH(I),NPOUT(I),NUMCP(I),
1 NELCLS(I),NMPCLS(I),I=1,NUMCLS)
REWIND 8
REWIND 4
DO 2 I=1,NUMNP
2 READ(4) N,R,D,TT,TH
READ(4) NZCNFS
DO 3 I=1,NZCNFS

110

```

READ(4) IZ, IELAST(IZ), JPLAST(IZ), WGT(IZ), (EI(J,IZ), J=1,5)
IF(JPLAST(IZ).EQ.0) GO TO 3
IF(JPLAST(IZ).GT.1) GO TO 4
READ(4) K, (SSTAR(IZ,J), J=1,K), (HSTAR(IZ,J), J=1,K)
NOYILD(IZ)=K
GO TO 3
4 IF(JPLAST(IZ).GT.2) GO TO 10
READ(4) ALPHA(IZ), CAPPA(IZ), COSTH(IZ)
GO TO 3
10 IF(JPLAST(IZ).GT.3) GO TO 5
READ(4) COHESN(IZ), FRCTN1(IZ), FRCTN2(IZ),
1     SNSWCH(IZ), CRESID(IZ), FRESID(IZ),
2     MYIELD(IZ), IRESID(IZ), JTENS(IZ)
GO TO 3
5 WRITE(6,6) IZ, JPLAST(IZ)
6 FORMAT(1H1,22HERROR IN L1+ ZONE DATA//1CX,7HZONE =,I5/
110X,7HJPLAST=,I5)
CALL EXIT
3 CONTINUE

C
JCLUS=1
NUMTEL=0
8 IF(NMPCLS(JCLUS).NE.0) GO TO 7
JCLUS=JCLUS+1
9 IF(JCLUS.LE.NJMCLS) GO TO 8
REWIND 12
REWIND 4
RETURN

C
7 NUMCEL=0
100 IF((NELCLS(JCLUS)-NUMCEL).LT.MXPELB) NUMELB=NELCLS(JCLUS)-NUMCEL
IF((NELCLS(JCLUS)-NUMCEL).GE.MXPELB) NUMELB=MXPELB
DO 200 KK=1, NUMELB
READ(4) JJ, NOOFEL(KK), IZONE, IPLAST(KK), NP(KK,1), NP(KK,2), NP(KK,3),
1 NP(KK,4), NCRACK, ITYPE(KK,1), ITYPE(KK,2), ITYPE(KK,3), ITYPE(KK,4),
2     THETA(KK,1), THETA(KK,2), THETA(KK,3), THETA(KK,4),
3     RI, RJ, RK, RL, ZI, ZJ, ZK, ZL
IF(JJ.EQ.JCLUS) GO TO 239
WRITE(6,240) JJ, JCLUS
240 FORMAT(1H1,7HJJ =, I5/7H JCLUS=, I5)
CALL EXIT
239 CONTINUE
202 IE=IELAST(IZONE)
A1=EI(1,IZONE)
A2=EI(2,IZONE)
A3=EI(3,IZONE)
A4=EI(4,IZONE)
A5=EI(5,IZONE)
NUME=NOOFEL(KK)
CALL ELCST(IE, ISTRES, A1, A2, A3, A4, A5, CC, NUME)
DO 203 I=1,4
DO 203 J=1,4
203 C(KK,I,J)=CC(I,J)

C
DO 204 I=1,4
DO 204 J=1,8
B(KK,I,J)=0.0
204 P(KK,J,I)=0.0

C
IF(NP(KK,4).NE.0) GO TO 208

```

C

AJ=RJ-RI
 AK=RK-RI
 BJ=ZJ-ZI
 BK=ZK-ZI
 HH=AJ*BK-AK*BJ
 AA=AJ-AK
 BB=BJ-BK

C

B(KK,1,1)=BB/HH
 B(KK,1,3)=BK/HH
 B(KK,1,5)=-BJ/HH
 IF(ISTRES.NE.0) GO TO 205
 RO=(AJ+AK)/3.
 ZO=(BJ+BK)/3.
 CAPRO=RI+RO
 B(KK,2,1)=(HH+BB*RO-AA*ZO)/(HH*CAPRO)
 B(KK,2,3)=(BK*RO-AK*ZO)/(HH*CAPRO)
 B(KK,2,5)=(-BJ*RO+AJ*ZO)/(HH*CAPRO)
 205 B(KK,3,2)=-AA/HH
 B(KK,3,4)=-AK/HH
 B(KK,3,6)=AJ/HH
 B(KK,4,1)=B(KK,3,2)
 B(KK,4,2)=B(KK,1,1)
 B(KK,4,3)=B(KK,3,4)
 B(KK,4,4)=B(KK,1,3)
 B(KK,4,5)=B(KK,3,6)
 B(KK,4,6)=B(KK,1,5)

C

IF(ISTRES.NE.0) CONST=HH/2.
 IF(ISTRES.EQ.0) CONST=HH*CAPRO/2.
 212 DO 206 I=1,8
 DO 206 J=1,4
 DO 207 N=1,4
 207 P(KK,I,J)=P(KK,I,J)+CONST*B(KK,N,I)*CC(N,J)
 206 CONTINUE
 GO TO 231

C

208 AJ=RJ-RI
 BJ=ZJ-ZI
 AA=SQRT(AJ*AJ+BJ*BJ)
 AL=RL-RI
 BL=ZL-ZI
 BB=SQRT(AL*AL+BL*BL)
 HH=AA*BB
 S1=-BJ/AA
 C1=AJ/AA
 IF(INCRACK.EQ.0) GO TO 213
 HH=AA
 BB=0.0
 213 CONTINUE
 IF(ISTRES.EQ.0) GO TO 209
 RC=AA/2.
 ZO=BB/2.
 GO TO 210
 209 AINT1=HH
 AINT2=HH*BB/2.
 AINT3=HH*AA/2.
 AINT4=HH*(RI+(AA*C1+BB*S1)/2.)
 AINT13=AA*AINT4/2.+HH*AA**2*C1/12.

```

    AINT14=BB* AINT4/2.+HH*BB**2*S1/12.
    AINT16=(HH/2.)*2
    RC=AINT13/AINT4
    ZC=AINT14/AINT4
210 DUMMY=RC*S1+ZC*C1
    B(KK,1,1)=(-AA*S1-BB*C1 +DUMMY)/HH
    B(KK,1,3)=(BB*C1-DJMMY)/HH
    B(KK,1,5)=DJMMY/HH
    B(KK,1,7)=(AA*S1-DJMMY)/HH
    IF(ISTRÉS.NE.0) GO TO 211
    IF(NCRACK.EQ.1) GO TO 211
    B(KK,2,1)=(HH* AINT1-BB* AINT3+ AINT16-AA* AINT2)/(HH* AINT4)
    B(KK,2,3)=(BB* AINT3- AINT16)/(HH* AINT4)
    B(KK,2,5)= AINT16/(HH* AINT4)
    B(KK,2,7)=(AA* AINT2- AINT16)/(HH* AINT4)
211 DUMMY=RC*C1-ZC*S1
    B(KK,3,2)=((BB*S1-AA*C1)+DUMMY)/HH
    B(KK,3,4)=(-BB*S1-DUMMY)/HH
    B(KK,3,6)=DJMMY /HH
    B(KK,3,6)=DJMMY /HH
    B(KK,3,8)=(AA*C1-DJMMY)/HH
    B(KK,4,1)=B(KK,3,2)
    B(KK,4,2)=B(KK,1,1)
    B(KK,4,3)=B(KK,3,4)
    B(KK,4,4)=B(KK,1,3)
    B(KK,4,5)=B(KK,3,6)
    B(KK,4,6)=B(KK,1,5)
    B(KK,4,7)=B(KK,3,8)
    B(KK,4,8)=B(KK,1,7)
    IF(ISTRÉS.NE.0) CONST=H
    IF(ISTRÉS.EQ.0) CONST= AINT4
    GO TO 212

```

C

```

231 DO 232 I=1,4
    EPST11(KK,I)=0.0
    EPSP11(KK,I)=0.0
232 SIG11(KK,I)=0.0
    IF(IPLAST(KK).NE.1) GO TO 237
    IDUM(KK,21)=NOYILD( IZONE)
    KYILD=NCYILD( IZONE)
    DO 233 I=1,KYILD
    DUM(KK,I)=SSTAR( IZONE,I)
233 DUM(KK,I+10)=HSTAR( IZONE,I)
    DO 234 I=1,8
234 DUM(KK,I+21)=0.0
    DUM(KK,27)=SSTAR( IZONE,1)
    GO TO 200
237 IF(IPLAST(KK).NE.2) GO TO 241

```

C

```

    DUM(KK,1)=ALPHA( IZONE)
    DUM(KK,2)=CAPPA( IZONE)
    DUM(KK,3)=COSTH( IZONE)
    IDUM(KK,4)=0
    IF(CAPPA( IZONE).EQ.0.0) IDUM(KK,4)=1
    DUM(KK,5)=0.0
    DO 238 I=1,4
238 DUM(KK,I+5)=0.0
    GO TO 200
241 IF(IPLAST(KK).NE.3) GO TO 235
    DUM(KK,1) = COHESN( IZONE)

```

```

DUM(KK,2) = FRCTN1( IZONE)
DUM(KK,3) = FRCTN2( IZONE)
DUM(KK,4) = SNSWCH( IZONE)
DUM(KK,5) = CRESID( IZONE)
DUM(KK,6) = FRESID( IZONE)
IDUM(KK,7) = MYIELD( IZONE)
IDUM(KK,8) = IRESID( IZONE)
IDUM(KK,9) = JTFNSN( IZONE)
AJ=RJ-RI
BJ=ZI-ZJ
AL=SQRT(AJ*AJ+BJ*BJ)
CO=AJ/AL
SI=BJ/AL
DUM(KK,10) = CO
DUM(KK,11) = SI
GO TO 200

C
235 WRITE(6,236) NOOFEL(K), IZONE, IPLAST(KK)
236 FORMAT(1H1,28HERROR IN ELEMENT DATA, 11H //
110X,12HELEMENT NO.=,15/10X,12HZONE NO. =,15/
210X,12HIPLAST =,15)
CALL EXIT

C
200 CONTINUE

C
NUMCEL=NUMCEL+NUMELB
NUMTEL=NUMTEL+NUMELB
EFFECT=0.0
WRITE(12) NUMTEL, NUMELB, (NOOFEL(K), IPLAST(K), (NP(K,J), I TYPE(K,J),
1THETA(K,J), J=1,4), ((C(K,J,I), I=1,4), (B(K,J,I), I=1,8), J=1,4),
2((P(K,J,I), J=1,8), IPST11(K,I), EPSPI1(K,I), SIGI1(K,I), I=1,4),
3(DUM(K,I), I=1,29), EFFECT, K=1, NUMELB)
IF(NUMCEL.LT.NELCLS(JCLUS)) GO TO 100
JCLUS=JCLUS+1
GO TO 9
END

C
C
C
SUBROUTINE ELOST( IELAST, ISTRES, E1, E2, E3, E4, E5, C, NUME)
DIMENSION C(4,4)
C**** FORM STRESS-STRAIN MATRIX
DO 1 I=1,4
DO 1 J=1,4
1 C(I,J)=0.0
IF(IELAST.NE.1) GO TO 20
C**** ISOTROPIC ELASTIC MATERIAL
IF(ISTRES.EQ.2) GO TO 4
C
AXISYMMETRIC OR PLANE STRAIN PROBLEM
EBAR=E1/((1.+E2)*(1.-2.*E2))
C(1,1)=EBAR*(1.-E2)
C(1,2)=EBAR*E2
C(1,3)=C(1,2)
C(2,1)=C(1,2)
C(2,2)=C(1,1)
C(2,3)=C(1,2)
C(3,1)=C(1,2)
C(3,2)=C(1,2)
C(3,3)=C(1,1)
C(4,4)=EBAR*(1.-2.*E2)/2.

```

```

      RETURN
      PLANE STRESS PROBLEM
C
      4 EBAR=E1/(1.-E2*E2)
        C(1,1)=EBAR
        C(3,1)=EBAR*E2
        C(1,3)=C(3,1)
        C(3,3)=C(1,1)
        C(4,4)=EBAR*(1.-E2)/2.
      RETURN
C**** ANISOTROPIC ELASTIC MATERIAL
C
      20 IF(IELAST.NE.2) GO TO 30
        IF(ISTRES.EQ.2) GO TO 2
        C(1,1)=E1
        C(1,2)=E1-2.*E5
        C(1,3)=E3
        C(2,1)=C(1,2)
        C(2,2)=C(1,1)
        C(2,3)=C(1,3)
        C(3,1)=C(1,3)
        C(3,2)=C(2,3)
        C(3,3)=E2
        C(4,4)=E4
      RETURN
C
      2 C(1,1)=2.*E5*(E1-2.*E5)/E1
        C(1,3)=2.*E3*E5/E1
        C(3,1)=C(1,3)
        C(3,3)=E2-E3**2/E1
        C(4,4)=E4
      RETURN
      21 WRITE(6,3) IELAST,NUME,ISTRES
      3  FORMAT(1H1/31H ERROR IN ELASTIC CONSTANT DATA/
113H IELAST      =,15/13H ELEMENT NO.=,15/
213H ISTRES      =,15)
      CALL EXIT
C
      30 IF(IELAST.NE.3) GO TO 21
C**** COMPRESSIBLE FLUID
      IF(ISTRES.EQ.2) GO TO 21
      DO 31 I=1,3
      DO 31 J=1,3
      31 C(I,J)=E1
      RETURN
C
      END
C
C
C
      OVERLAY(MOHAN,13,0 )
      PROGRAM LNK2
      COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPCL3,NUMNP,
1          NUMEL,ISTRES,NUMPCL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2          KTAPE,KRUN,IPRINT,NUMST,MXSTRT,FUZZ(239),
3          IPELTP,INT,NP3CDS,IMPBX
      COMMON/A/ U(1600),W(1600),NP3OUT(80),MPCLS(80),FNU(350),
1          FNW(350)
      COMMON/B/ NADJNP(400),ITYPE(400),THETA(400),XMASS(400),
1          SNPUU(400),SNPUW(400),SNPWW(400),FAU(400),FAW(400),

```



```

2  NPADJ(400,8),SADUU(400,8),SADUW(400,8),SADWW(400,8)
  DIMENSION ANAME(18),COM(1)
C
  EQUIVALENCE (MAXNP,COM(1))
  MCHAN=5HMOHAN
  IPELTP=12
  INT=8
  NPRCDS=0
  REWIND INT
  REWIND IPELTP
C
  READ(5,1) ITMAX,ERRMAX,NFAC,KTAPE,ICONTU,OVERLX
1  FORMAT(15,E10.0,3I5,E10.0)
C  ITMAX = MAX. NO. OF ITERATIONS PER SOLUTION
C  ERRMAX=MAX. ALLOWABLE ERROR (LBS)
C  NFAC  = INCREMENTS FOR NONLINEAR SOLUTION
C  KTAPE =0 USE TWO K TAPES
C          =1 USE ONE K TAPE
C  ICONTJ=0 CONTINUE SOLUTION FOR NONCONVERGENCE
C  OVERLX=OVER-RELAXATION FACTOR
  FACTOR=FLOAT(NFAC)
  WRITE(6,2) ITMAX,ERRMAX,FACTOR,KTAPE,ICONTU,OVERLX
2  FORMAT(1H1,7HITMAX =,15/8H ERRMAX=,1PE15.5/8H FACTOR=,1PE15.5/
18H KTAPE =,15/8H ICONTJ=,15/8H OVERLX=,1PE15.5//)
  IF(OVERLX.LE.0.0) CALL EXIT
  REWIND 8
  DO 3 I=1,NUMNP
3  READ(8) N,NPOUT(1),NPOJT(1),(NPOUT(J),J=1,MXADJP)
  READ(8) NMKCLS,(NPLOW,NPHIGH,NPOUT(1),NUMCP,NELCLS,NMPCLS(1),
1I=1,NMKCLS)
  READ(8) (NPLOW,NPLOW,I=1,NUMNP)
  READ(8) (J(I),N(I),I=1,NUMNP)
  REWIND 8
16 WRITE(6,17)
17 FORMAT(1H1,16HELASTIC SOLUTION//4X,6HKERROR,4X,6HNERROR,14X,
16HERRCNT,14X,6HERRMAX,4X,6HICOUNT,5X,5HITMAX,7X,3HINT,4X,
26HIPELTP//)
  ICOUNT=1
18 CALL ERROR(KERROR,ERRMAX,1,NERROR,ERRCNT,OVERLX)
  WRITE(6,19) KERROR,NERROR,ERRCNT,ERRMAX,ICOUNT,ITMAX,INT,IPELTP
19 FORMAT(2I10,1P2E20.5,4I10)
  ALAMB=0.0
  IF(KERROR.EQ.0) GO TO 22
  IF(ICOUNT.EQ.ITMAX) GO TO 20
  IF(ICOUNT.EQ.ITMAX) GO TO 20
  ICOUNT=ICOUNT+1
  GO TO 18
20 WRITE(6,21)
21 FORMAT(//39H HAVE NOT CONVERGED TO ELASTIC SOLUTION//)
22 IF(NUMPEL.EQ.0) GO TO 27
  DO 23 ICLJS=1,NMKCLS
  IF(NMPCLS(ICLJS).EQ.0) GO TO 23
  NUM=NMPCLS(ICLJS)
  DO 24 I=1,NUM
24 CALL PLASTF(0,0,ICLJS,0,1)
23 CONTINUE
  WRITE(6,25) ALAMB
25 FORMAT(//21H ELASTIC LOAD FACTOR=,1PE20.5//)
  IF(ALAMB.LE.1.0) GO TO 27
  IF(ALAMB.LT.1.0E+38) GO TO 54

```

```

AA=0.0
WRITE(INT) (COM(1), I=1, 16)
DO 55 I=1, NJMNP
55 WRITE(INT) AA, AA
ALAMB=2.0
GO TO 56
54 CONTINUE
DO 25 I=1, NJMNP
U(I)=J(I)/ALAMB
26 W(I)=W(I)/ALAMB
C
27 WRITE(INT) (COM(1), I=1, 16)
DO 38 I=1, NJMNP
38 WRITE(INT) U(I), W(I)
53 IF(NUMPCL.EQ.0) RETURN
DO 42 ICLJS=1, NMKCLS
IF(NMPCLS(ICLJS).EQ.0) GO TO 42
NUM=NMPCLS(ICLJS)
DO 43 I=1, NUM
43 CALL PLASTF(1, 1, ICLUS, 0, 1)
42 CONTINUE
IF(ALAMB.LE.1.0) RETURN
56 IF(KERRCR.EQ.0) GO TO 52
IF(ICONTJ.NE.0) RETURN
52 CONTINUE
FAC=(ALAMB-1.0)/FACTOR
DO 28 I=1, NJMNP
U(I)=U(I)*FAC
28 W(I)=W(I)*FAC
C
DO 35 IFAC=1, NFAC
ICOUNT=1
32 CALL ERROR(KERROR, ERRMAX, C, NERROR, ERRCNT, OVERLX)
WRITE(6, 19) KERROR, NERROR, ERRCNT, ERRMAX, ICOUNT, ITMAX, INT, IPELTP
C
IF(KERRCR.NE.0) GO TO 31
35 DO 41 I=1, NJMNP
41 WRITE(INT) U(I), W(I)
DO 29 ICLJS=1, NMKCLS
IF(NMPCLS(ICLJS).EQ.0) GO TO 29
NUM=NMPCLS(ICLJS)
DO 30 I=1, NUM
30 CALL PLASTF(1, 1, ICLUS, IFAC, 1)
29 CONTINUE
WRITE(6, 37) IFAC
37 FORMAT(/ / 32H HAVE FINISHED PLASTIC INCREMENT, I5 /)
NPRCDS=NPRCDS+1
IF(KERRCR.EQ.0) GO TO 36
IF(ICONTJ.NE.0) RETURN
GO TO 36
31 IF(ICOUNT.EQ.ITMAX) GO TO 33
ICOUNT=ICOUNT+1
GO TO 32
33 WRITE(6, 34) IFAC
34 FORMAT(/ / 57H HAVE NOT CONVERGED TO PLASTIC SOLUTION FOR INCREMENT
INC., I5 /)
GO TO 35
36 CONTINUE
C
REWIND IPELTP

```

REWIND INT
RETURN
END

117

C
C
C

```

SUBROUTINE FRROR(KERROR,ERRMAX,KSWTCH,NERROR,ERRCNT,OVERLX)
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPCLB,NUMNP,
1  NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2  KTAPE,KRJN,IPRINT,NUMLT,MXSTRT,FUZ(239),
3  IPELTP,INT,NPRCDS,IMPBX
COMMON/A/ U(1600),W(1600),NPJUT(80),NMPCLS(80),FNU(350),
1  FNW(350)
COMMON/B/ NADJNP(400), ITYPE(400), THETA(400), XMASS(400),
1  SNPUU(400),SNPUW(400),SNPWW(400),FAU(400),FAW(400),
2  NPADJ(400,8),SADUU(400,8),SADUW(400,8),SADWW(400,8)
DATA IRT/1/
C  KSWTCH=0 DC NONLINEAR PART OF ANALYSIS
C  =1 DC ELASTIC ANALYSIS ONLY
IF(IRT.EQ.0) GO TO 100
IRT=0
ISWTCH=0
JSWTCH=0
REWIND 10
REWIND 1
DO 5 I=1,MXNPB
FNU(I)=0.0
5 FNW(I)=0.0
IF((NUMNP.LE.MXNPB).AND.(NUMPEL.EQ.0)) ISWTCH=1
IF(KTAPE.NE.0) JSWTCH=1
IC=0
1 IC=IC+1
READ(10) NPLW,NPHIGH,NPJUT(IC),NUMCP,NELCLS,NMPCLS(IC),
1 NUMNPB,(NADJNP(I),ITYPE(I),THETA(I),XMASS(I),SNPUU(I),SNPUW(I),
2 SNPWW(I),FAJ(I),FAW(I),(NPADJ(I,J),SADUU(I,J),SADUW(I,J),
3 SADWW(I,J),J=1,MXADJP),I=1,NUMNPB)
IF(ISWTCH.EQ.1) GO TO 4
IF(JSWTCH.EQ.1) GO TO 2
WRITE(1) NPLW,NPHIGH,NPJUT(IC),NUMCP,NELCLS,NMPCLS(IC),
1 NUMNPB,(NADJNP(I),ITYPE(I),THETA(I),XMASS(I),SNPUU(I),SNPUW(I),
2 SNPWW(I),FAJ(I),FAW(I),(NPADJ(I,J),SADUU(I,J),SADUW(I,J),
3 SADWW(I,J),J=1,MXADJP),I=1,NUMNPB)
2 IF(NUMCP.LT.NUMNP) GO TO 1
REWIND 10
REWIND 1
IF(JSWTCH.EQ.1) GO TO 3
IO=1
GO TO 100
3 IO=10
4 REWIND 10
IC=10
100 IF((ISWTCH.EQ.1).OR.(JSWTCH.EQ.1)) GO TO 101
IF(IO.EQ.10) GO TO 102
IO=10
GO TO 103
102 IO=1
103 CONTINUE
101 KERROR=0
NERROR=0
ERRCNT=0.0

```

```

DC 104 ICLUS=1,NMKCLS
IF(KSWTCH.EQ.1) GO TO 113
IF(NUMPEL.EQ.0) GO TO 113
IF(NMPCLS(ICLUS).EQ.0) GO TO 113
DO 114 I=1,MXNPB
FNU(I)=0.0
114 FNU(I)=0.0
NUM=NMPCLS(ICLUS)
DO 115 I=1,NUM
115 CALL PLASTF(0,1,ICLUS,0,I)
113 IF(ISWTCH.EQ.1) GO TO 105
READ(IO) NPLGW,NPHGH,NPJ ,NUMCP,NELCLS,NMP
1 NUMNPB,(NADJNP(I),ITYPE(I),THETA(I),XMASS(I),SNPUU(I),SNPUW(I),
2 SNPWW(I),FAJ(I),FAW(I),NPADJ(I,J),SADU(I,J),SADUW(I,J),
3 SADWW(I,J),J=1,MXADJP),I=1,NUMNPB)
105 NLOW=NPLGW-NPOUT(ICLUS)
NHGH=NPHGH-NPOUT(ICLUS)
IF(KSWTCH.EQ.0) FAC=(1.0-1.0/ALAMB)/FACTOR
IF(KSWTCH.EQ.1) FAC=1.0
DO 106 I=NLOW,NHGH
ERRU=0.0
ERRW=0.0
L=I+NPOUT(ICLUS)
IF(ITYPE(I).EQ.2) GO TO 106
IF(ITYPE(I).EQ.3) GO TO 108
ERRU=FNU(I)+FAU(I)*FAC
ERRU=ERRU-SNPJU(I)*U(L)-SNPUW(I)*W(L)
NUM=NADJNP(I)
DO 107 J=1,NJM
NP=NPADJ(I,J)
107 ERRU=ERRU-SADUJ(I,J)*U(NP)-SADUW(I,J)*W(NP)
IF(ABS(ERRU).GT.ERRCNT) ERRCNT=ABS(ERRU)
IF(ABS(ERRU).LE.ERRMAX) GO TO 116
IF(ABS(ERRU).LE.ERRMAX) GO TO 116
KERROR=1
NERROR=NERROR+1
116 U(L)=J(L) + OVERLX*ERRU/SNPUU(I)
IF(ITYPE(I).EQ.1) GO TO 106
108 ERRW=FNU(I)+FAW(I)*FAC
ERRW=ERRW-SNPJW(I)*U(L)-SNPWW(I)*W(L)
NUM=NADJNP(I)
DO 109 J=1,NJM
NP=NPADJ(I,J)
NPR=NP-NPOUT(ICLUS)
DC 110 K=1,MXADJP
KK=K
IF(NPADJ(NPR,K).EQ.L) GO TO 112
110 CONTINUE
WRITE(6,111) ICLUS,NLOW,NHGH,NPOUT(ICLUS),I,L,NP,NPR
111 FORMAT(1H1,24HERROR IN COMPUTING SADUW//B15)
CALL EXIT
112 SADWU=SADUW(NPR,KK)
109 ERRW=ERRW-SADWU*U(NP)-SADWW(I,J)*W(NP)
IF(ABS(ERRW).GT.ERRCNT) ERRCNT=ABS(ERRW)
IF(ABS(ERRW).LE.ERRMAX) GO TO 117
KERROR=1
NERROR=NERROR+1
117 W(L) = W(L) + OVERLX*ERRW/SNPWW(I)
106 CONTINUE
104 CONTINUE

```

```

REWIND IO
RETURN
END
SUBROUTINE PLASTF(LSWTCH,MSWTCH,ICLUS,IFAC,INUM)
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPEL3,NUMNP,
1     NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2     KTAPF,KRJN,IPRINT,NUMST,MXSTRT,FUZ(239),
3     IPELTP,INT,NPRCDS,IMPBX
COMMON/A/ U(1600),W(1600),NPOUT(80),VMPCLS(80),FNU(350),
1     FNU(350)
COMMON/B/ NADJNP(400),JTYPE(400),SHETA(400),XMASS(400),
1     SNPUU(400),SNPUW(400),SNPWW(400),FAU(400),FAW(400),
2     NPADJ(400,8),SADUJ(400,8),SADUW(400,8),SADW(400,8)

```

C

```

DIMENSION BUFF(3280)
EQUIVALENCE (BUFF,NADJNP)
DIMENSION NOOFEL(24),IPLAST(24),NP(24,4),ITYPE(24,4),THETA(24,4),
1C(24,4,4),B(24,4,8),P(24,8,4),EPSTII(24,4),EPSPII(24,4),
2SIGII(24,4),DJM(24,29),IDUM(24,29)
EQUIVALENCE (BUFF(1),NOOFEL),(BUFF(25),IPLAST),(BUFF(49),NP),
1(BUFF(145),ITYPE),(BUFF(241),THETA),(BUFF(337),C),(BUFF(721),B),
2(BUFF(1489),P),(BUFF(2257),EPSTII),(BUFF(2353),EPSPII),
3(BUFF(2449),SIGII),(BUFF(2545),DJM),(BUFF(2545),IDUM)
DIMENSION EPSTII(4),EPSPII(4),SIGII(4),EPSDI(4),X(8),FPLAST(8),
1EFFECT(24)
DIMENSION STRESS(4),STRAIN(4),PSTRAN(4),CMAT(4,4),FMAT(4,4),
1GMAT(4,4),SIGNII(4),SIGNBI(4)
C LSWTCH=0 DO NOT JDATE ELEMENT TAPE
C =1 JDATE ELEMENT TAPE
C MSWTCH=0 FIND ALAMB VALUE ONLY
C =1 DO ALL NONLINEAR PART

```

```

IC=IPELTP
IF(IO.EQ.12) JO=3
IF(IO.EQ.3) JO=12
L=I+NPOUT(ICLUS)
READ(IO) NJMCEL,NELBUF,(NOOFEL(K),IPLAST(K),(NP(K,J),
1ITYPE(K,J),THETA(K,J),J=1,4),((C(K,J,I),I=1,4),(C(K,J,I),I=1,8),
2J=1,4),((P(K,J,I),J=1,8),EPSTII(K,I),EPSPII(K,I),SIGII(K,I),
3I=1,4),(DJM(K,I),I=1,29),EFFECT(K),K=1,NELBUF)
DO 3 I=1,NELBUF
DO 5 J=1,4
N=NP(I,J)
K=2*J-1
IF(J.EQ.4) GO TO 6
8 IF(ITYPE(I,J).EQ.1) GO TO 7
X(K)=U(N)
X(K+1)=W(N)
GO TO 5
7 X(K)=U(N)*COS(THETA(I,J))-W(N)*SIN(THETA(I,J))
X(K+1)=U(N)*SIN(THETA(I,J))+W(N)*COS(THETA(I,J))
GO TO 5
6 IF(N.NE.0) GO TO 8
X(7)=0.0
X(8)=0.0
5 CONTINUE
DO 19 J=1,8
IF(X(J).EQ.0.0) GO TO 19
GO TO 18
19 CONTINUE
GO TO 3

```

120

```

18 DO 9 J=1,4
   EPSTI(J)=EPSTI1(I,J)
   DO 9 K=1,8
9   EPSTI(J)=EPSTI(J)+B(I,J,K)*X(K)
   IF(IPLAST(I).NE.1) GO TO 200
   L=I
   CALL MISES(L, EPSTI, EPSPI, SIGI, EPSDI, EFFFI, SYI, SMAXI, SI, MSWCH,
1BLAMB)
   IF(MSWCH.EQ.0) GO TO 400
   IF(LSWCH.EQ.0) GO TO 100
   DO 101 J=1,4
101 DUM(I,J+21)=EPSDI(J)
   DUM(I,26)=EFFFI
   DUM(I,27)=SYI
   DUM(I,28)=SMAXI
   DUM(I,29)=SI
100 CONTINUE
   GO TO 300
200 IF(IPLAST(I).NE.2) GO TO 500
   L=I
   CALL COULMR(L, EPSTI, EPSPI, SIGI, KORNER, FYLDI, EPSDI, MSWCH, BLAMB)
   IF(MSWCH.EQ.0) GO TO 400
   IF(LSWCH.EQ.0) GO TO 201
   IDUM(I,4)=KORNER
   DUM(I,5)=FYLDI
   DO 202 J=1,4
202 DUM(I,J+5)=EPSDI(J)
201 CONTINUE
   GO TO 300
500 IF(IPLAST(I).NE.3) GO TO 900
   L=I
   CCHESN=DUM(I,1)
   FRCTN1=DUM(I,2)
   FRCTN2=DUM(I,3)
   SNSWCH=DUM(I,4)
   CRESID=DUM(I,5)
   FRESID=DUM(I,6)
   MYIELD=IDUM(I,7)
   IRESID=IDUM(I,8)
   JTENSN=IDUM(I,9)
   COSTH=DUM(I,10)
   SINTH=DUM(I,11)
   DO 501 J=1,4
   STRESS(J)=SIGI1(I,J)
   STRAIN(J)=EPSTI1(I,J)
   PSTRAIN(J)=EPSPI1(I,J)
   DO 501 K=1,4
501 CMAT(J,K)=C(I,J,K)
   CALL NCQJL(L, EPSTI, STRAIN, EPSPI, PSTRAIN, SIGI, STRESS,
1CCHESN, FRCTN1, SNSWCH, FRCTN2, CRESID,
2FRESID, MYIELD, IRESID, JTENSN, CMAT, ISTRES,
3MSWCH, SBAR, BLAMB, COSTH, SINTH, FMAT, GMAT,
4SIGNI1,0,SIGNB1)
   IF(MSWCH.EQ.0) GO TO 400
   IF(LSWCH.EQ.0) GO TO 300
   DUM(I,1)=CCHESN
   DUM(I,2)=FRCTN1
   DUM(I,3)=FRCTN2
   DUM(I,4)=SNSWCH
   DUM(I,5)=CRESID

```

```

DUM(I,6)=FRES ID
IDUM(I,7)=MY IELD
IDUM(I,8)=IRES ID
IDUM(I,9)=JT ENSN
DUM(I,10)=COSTH
DUM(I,11)=SINTH
GO TO 300

```

C

```

900 WRITE(6,901) NDOFEL(I), IPLAST(I)
901 FORMAT(1H1,15HERROR IN PLASTF//215)
CALL EXIT
300 IF(LSWTCH.EQ.1) GO TO 301
DO 12 J=1,8
FPLAST(J)=0.0
DO 12 K=1,4
12 FPLAST(J)=FPLAST(J)+PT(I,J,K)*(EPSP I(K)-EPSP I1(I,K))
301 CONTINUE
IF(LSWTCH.EQ.0) GO TO 10
DEPS=0.0
DO 20 J=1,4
DUMMY=EPSP I(J)-EPSP I1(I,J)
IF(J.EQ.4) GO TO 21
DEPS=DEPS+DUMMY*DUMMY
GO TO 20
21 DEPS=DEPS+DUMMY*DUMMY/2.0
20 CONTINUE
EFFECT(I)=EFFECT(I)+SQRT(2.*DEPS/3.0)
DO 11 J=1,4
EPST I1(I,J)=EPST I(J)
EPSP I1(I,J)=EPSP I(J)
11 SIG I1(I,J)=SIG I(J)
10 CONTINUE

```

C

```

IF(LSWTCH.EQ.1) GO TO 3
DO 13 J=1,4
NCDE=NP(I,J)
K=2*J-1
N=NODE-NPOUT(ICLUS)
IF(J.EQ.4) GO TO 14
16 IF(ITYPE(I,J).EQ.1) GO TO 15
FNU(N)=FNU(N)+FPLAST(K)
FNW(N)=FNW(N)+FPLAST(K+1)
GO TO 13
15 DUMJ= FPLAST(K)*COS(THETA(I,J))+FPLAST(K+1)*SIN(THETA(I,J))
DUMW=-FPLAST(K)*SIN(THETA(I,J))+FPLAST(K+1)*COS(THETA(I,J))
FNU(N)=FNU(N)+DUMJ
FNW(N)=FNW(N)+DUMW
GO TO 13
14 IF(NODE.NE.0) GO TO 16
13 CONTINUE
400 IF(MSWTCH.EQ.1) GO TO 3
IF(ABS(BLAMP).GT.ALAMB) ALAMB=ABS(BLAMP)
3 CONTINUE
IF(LSWTCH.EQ.0) GO TO 17

```

C

```

WRITE(JO) NJMCEL, NELBUF, (NDOFEL(K), IPLAST(K), (NP(K,J),
1 ITYPE(K,J), THETA(K,J), J=1,4), ((C(K,J,I), I=1,4), (B(K,J,I), I=1,8),
2 J=1,4), (PT(K,J,I), J=1,8), EPST I1(K,I), EPSP I1(K,I), SIG I1(K,I),
3 I=1,4), (DUM(K,I), I=1,29), EFFECT(K), K=1, NELBUF)
WRITE(INT) NELBUF, (NDOFEL(I), EFFECT(I), (NP(I,J), EPST I1(I,J),

```

```

1 EPSP11(I,J),(C(I,J,K),K=1,4),J=1,4),I=1,NELBUF)
  IF((IPRINT.NE.7).AND.(IPRINT.NE.99)) GO TO 17
  IF(ICLUS.NE.1) GO TO 30
  IF(INJM.NE.1) GO TO 60
  WRITE (6,30)
30  FORMAT(1H1,10X,28PSTRESSES IN PLASTIC ELEMENTS//)
  IF(IFAC.NE.0) GO TO 40
  WRITE(6,35)
35  FORMAT(5X,16HELASTIC SOLUTION//)
  GO TO 50
40  WRITE(6,45)IFAC
45  FORMAT(23H PLASTIC INCREMENT NO.=,I5//)
50  WRITE (6,55)
55  FORMAT(12H EL. NJMBER ,8X,12HSIGMAZ (PSI),8X,12HSIGMAT (PSI),8X,12
1  HSIGMAZ (PSI),8X,12H TAU (PSI)//)
60  WRITE(6,65)(NOOFEL(K),(SIGI1(K,I),I=1,4),K=1,NELBUF)
65  FORMAT(17,9X,1P4E20.5)
17 IF(NUMCEL.LT.NJMELP) RETURN
  RFWIND IO
  RFWIND JO
  IF(LSWTCH.EQ.1) IPELTP=JO
  RETURN
  END

```

C
C
C

```

SUBROUTINE MISES(I,EPSTI,EPSP I,SIGI,EPSDI,EEFFI,SYI,SMAXI,SI,
1MSWTCH,BLAMB)

```

C

```

COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPELB,NLMNP,
1  NJMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2  KTAPE,KRUN,IPRINT,NUMST,MXSTRT,FUZ(239),
3  IPELTP,INT,NPRCDS,IMPBX
COMMON/A/ U(1600),W(1600),NPJUT(80),NMPCLS(80),FNU(350),
1  FNW(350)
COMMON/B/ NADJNP(400),JTYPE(400),SHETA(400),XMASS(400),
1  SNPUJ(400),SNPUW(400),SNPWW(400),F4U(400),FAW(400),
2  NPADJ(400,8),SADUU(400,8),SADUW(400,8),SADWW(400,8)

```

C

```

DIMENSION BJFF(3280)
EQUIVALENCE (BJFF,NADJNP)
DIMENSION NOOFEL(24),IPLAST(24),NP(24,4),ITYPE(24,4),THETA(24,4),
1C(24,4,4),B(24,4,8),P(24,8,4),EPSTI1(24,4),EPSP11(24,4),
2SIGI1(24,4),DJM(24,29),IDUM(24,29)
EQUIVALENCE (BJFF(1),NOOFEL),(BUFF(25),IPLAST),(BUFF(49),NP),
1(BUFF(145),ITYPE),(BJFF(241),THETA),(BUFF(337),C),(BUFF(721),B),
2(BUFF(1489),P),(BUFF(2257),EPSTI1),(BUFF(2353),EPSP11),
3(BUFF(2449),SIGI1),(BUFF(2545),DUM),(BUFF(2545),IDUM)
C  DUM(M,1)=SSTAR(M,1)
C  DUM(M,11)=PSTAR(M,1)
C  DUM(M,21)=NOYILD(M)
C  DUM(M,22)=EPSDI1(M,1)
C  DUM(M,26)=EEFFI1(M)
C  DUM(M,27)=SYI1(M)
C  DUM(M,28)=SMAXI1(M)
C  DUM(M,29)=SI1(M)
DIMENSION EPSTI(4),EPSP I(4),SIGI(4),EPSDI(4)
DIMENSION SIGBAR(4),SIGMA(4),EPST(4),EP SD(4),DEPSD(4),SIGMAT(4),
1ESTAR(10),SSTAR(10),HSTAR(10),EPSDI1(4),DEPS(4)
NOYILD=IDUM(1,21)

```

C
C
C
C
C
C
C
C
C


```

      DC 400 J=1,NOYILD
      SSTAR(J)=DUM(I,J)
400 HSTAR(J)=DUM(I,J+10)
      DC 401 J=1,4
401 EPSDII(J)=DJM(I,J+21)
      EEFFI=DUM(I,26)
      SYI=DUM(I,27)
      SMAXI=DJM(I,28)
      SI=DUM(I,29)

C
      ESTAR(1)=0.0
      IF(NOYILD.EQ.1) GO TO 8
      DC 7 J=2,NOYILD
      ESTAR(J)=ESTAR(J-1)+(SSTAR(J)-SSTAR(J-1))/HSTAR(J-1)
      DC 1 J=1,4
      SIGBAR(J)=0.0
      DC 1 K=1,4
      1 SIGBAR(J)=SIGBAR(J)+C(I,J,K)*(EPSTI(K)-EPSPII(I,K))
      SBAR=(ABS(SIGBAR(1)-SIGBAR(2))**2+(ABS(SIGBAR(1)-SIGBAR(3))**2
      1 +(ABS(SIGBAR(2)-SIGBAR(3))**2+6.*(ABS(SIGBAR(4))**2
      SBAR=SQRT(SBAR/2.)
      IF(MSWTCH.EQ.1) GO TO 3

C
      BLAMB=SBAR/SYI
      RETURN
      3 IF(SBAR.GE.SYI ) GO TO 10
C
      CAS = 1 OR 3, NOW ELASTIC
      IF(SMAXI .GE.SYI ) GO TO 20
C
      CASE = 1 ALWAYS WAS ELASTIC AND STILL IS ELASTIC
      ICASE=1
      DC 4 J=1,4
      EPSPII(J)=0.0
      SIGI(J)=SIGBAR(J)
      4 EPSDII(J)=0.0
      EEFFI=0.0
      SYI=SYI
      SI=SBAR
      IF(SBAR.GT.SMAXI ) GO TO 5
      SMAXI=SBAR
      GO TO 6
      5 SMAXI=SBAR
      6 RETURN
C
      CASE = 3, WAS PREVIOUSLY PLASTIC,NOW ELASTIC
      20 ICASE=3
      DO 21 J=1,4
      EPSPII(J)=EPSPII(I,J)
      SIGI(J)=SIGBAR(J)
      21 EPSDII(J)=0.0
      EEFFI=EEFFI
      SYI=SYI
      SMAXI=SYI
      SI=SBAR
      RETURN

C
      10 IF(SBAR.GT.SYI ) GO TO 30
C
      CASE = 2 OR 4, JUST AT YIELD STRESS
      IF(SMAXI .GE.SYI ) GO TO 23
C
      CASE = 2, WAS PREVIOUSLY ELASTIC, NOW ON VERGE OF FLOW
      ICASE=2
      GO TO 24

```

```

C     CASE = 4, WAS PREVIOUSLY PLASTIC, NOW ON VERGE OF FLOW
23  ICASE=4
C
24  DO 22 J=1,4
    EPSPI(J)=EPSPI1(I,J)
22  SIGI(J)=SIGBAR(J)
    EPSDI(1)=(2.*SIGBAR(1)- SIGBAR(2)- SIGBAR(3))/(2.*SBAR)
    EPSDI(2)=( -SIGBAR(1)+2.*SIGBAR(2)- SIGBAR(3))/(2.*SBAR)
    EPSDI(3)=( -SIGBAR(1)- SIGBAR(2)+2.*SIGBAR(3))/(2.*SBAR)
    EPSDI(4)=3.*SIGBAR(4)/SBAR
    EEFI=EEFFI1
    SYI=SYI1
    SMXI=SYI
    SI=SBAR
    RETURN
C     CASES 5,6 OR 7, PLASTIC FLOW
30  IF(SMAXI1 .LT.SYI1 ) GO TO 100
C     CASES 5 OR 6
    IF(SI1 .GE.SYI1 ) GO TO 200
C     CASE = 6, WAS PLASTIC, UNLOADED, ELASTIC AT PREVIOUS TIME STEP
    ICASE=6
32  DUMMY=(SYI1 -SI1 )/(SBAR-SI1 )
    DO 31 J=1,4
    SIGMA(J)=SIGI1(I,J)+DUMMY*(SIGBAR(J)-SIGI1(I,J))
31  EPST(J)=EPSTI1(I,J)+DUMMY*(EPSTI(J)-EPSTI1(I,J))
    SIGB=(ABS(SIGMA(1)-SIGMA(2))**2+(ABS(SIGMA(1)-SIGMA(3))**2
1    +(ABS(SIGMA(2)-SIGMA(3))**2+6.*(ABS(SIGMA(4))**2
    SIGB=SQRT(SIGB/2.)
    EPSD(1)=(2.*SIGMA(1)- SIGMA(2)- SIGMA(3))/(2.*SIGB)
    EPSD(2)=( -SIGMA(1)+2.*SIGMA(2)- SIGMA(3))/(2.*SIGB)
    EPSD(3)=( -SIGMA(1)- SIGMA(2)+2.*SIGMA(3))/(2.*SIGB)
    EPSD(4)=3.*SIGMA(4)/SIGB
    GO TO 201
C     CASE = 7, WAS PREVIOUSLY PLASTIC, NOW PLASTIC
100 ICASE=7
    GO TO 32
C     CASE = 5, WAS PREVIOUSLY PLASTIC, NOW FURTHER FLOW
200 ICASE=5
    DO 33 J=1,4
    SIGMA(J)=SIGI1(I,J)
    EPST(J)=EPSTI1(I,J)
33  EPSD(J)=EPSDI1(I,J)
    SIGB=SYI1
C     COMPUTE NEW PLASTIC STRAINS
201 IF(ISTRES.NE.2) GO TO 202
C     PLANE STRESS
    XNU=C(I,1,3)/C(I,1,1)
    E=C(I,1,1)*(1.-XNU*XNU)
    A=(7.-13.*XNJ+7.*XNU*XNU)/4.+0.75*(2.-5.*XNU+2.*XNU*XNU)*
1    ((SIGMA(4)/SIGB)*(SIGMA(4)/SIGB)-(SIGMA(1)/SIGB)*(SIGMA(3)/
2    SIGB))
    A=A/(1.-XNU*XNU)**2
    B=((5.-4.*XNJ)/2.)*((SIGBAR(1)/SIGB)*(SIGMA(1)/SIGB)+
1    (SIGBAR(3)/SIGB)*(SIGMA(3)/SIGB))-((4.-5.*XNU)/2.)*
2    ((SIGBAR(1)/SIGB)*(SIGMA(3)/SIGB)+(SIGBAR(3)/SIGB)*
3    (SIGMA(1)/SIGB))+9.*(1.-XNU)*(SIGBAR(4)/SIGB)*(SIGMA(4)/SIGB)
    B=B/(1.-XNU*XNU)
    GO TO 203
C     PLANE STRAIN OR AXISYMMETRIC
202 XNU=(C(I,1,2)/C(I,1,1))/(1.+C(I,1,2)/C(I,1,1))

```

```

E=C(1,1,1)*(1.+XNU)*(1.-2.*XNU)/(1.-XNU)
A=9./(4.*(1.+XNJ)**2)
B=(3./(1.+XNJ))*(SIGBAR(1)/SIGB)*EPSD(1)+(SIGBAR(2)/SIGB)*EPSD(2)
1  +(SIGBAR(3)/SIGB)*EPSD(3)+(SIGBAR(4)/SIGB)*EPSD(4)
C FIND LOCAL VALJE OF H
203 IF(NOIILD.GT.1) GO TO 204
J=1
205 H=HSTAR( J)
GC TO 207
204 DO 206 K=1,NOIILD
J=K
IF(K.EQ.NOIILD) GO TO 205
IF(SYI1 .LT.SSTAR( K+1)) GO TO 205
206 CONTINUE
207 ALPHA=A-(H/E)**2
BETA=2.*H/E+B
GAMMA=(SBAR/SIGB)**2-1.0
IF(ALPHA.NE.0.0) GO TO 208
IF(BETA.GT.0.0) GO TO 209
IERROR=1
212 WRITE(6,210) T,NOUFEL(1),1
210 FORMAT(1H1,50HERROR ENCOUNTERED IN PLASTIC FORCES, MISES ROUTINE//
110H TIME =,1PE15.5/10H ELEMENT =,15/10H NUMBER =,15)
WRITE(6,500) (EPSTI1(1,J),J=1,4),(EPSP11(1,J),J=1,4),
1(EPSD11( J),J=1,4),EEFF11 (EPSTI(J),J=1,4),
2(EPST(J),J=1,4) , (EPSD(J),J=1,4)
500 FORMAT(10H EPSTI1 =,1P4E15.5/10H EPSP11 =,1P4E15.5/
110H EPSD11 =,1P4E15.5/10H EEFF11 =,1PE15.5/10H EPSTI =,1P4E15
2.5/10H EPST =,1P4E15.5 /10H EPSD =,
31P4E15.5)
WRITE(6,501)(SIGI1(1,J),J=1,4), SYI1 ,SMAXI1 ,SI1 ,
1(SIGBAR(J),J=1,4),SHAR,(SIGMA(J),J=1,4),SIGB
501 FORMAT(10H SIGI1 =,1P4E15.5/10H SYI1 =,1PE15.5/
110H SMAXI1 =,1PE15.5/10H SI1 =,1PE15.5/
210H SIGBAR =,1P4E15.5/10H SBAR =,1PE15.5/
310H SIGMA =,1P4E15.5/10H SIGB =,1PE15.5)
WRITE(6,502) E,XNU,A,B,H,ALPHA,BETA,GAMMA,IERROR
502 FORMAT(10H E =,1PE15.5/10H XNU =,1PE15.5/
110H A =,1PE15.5/10H B =,1PE15.5/10H H =,1PE15.5/
210H ALPHA =,1PE15.5/10H BETA =,1PE15.5/10H GAMMA =,1PE15.5/
310H IERROR =,15)
CALL EXIT
209 DELTA=GAMMA/BETA
GC TO 211
208 DUMMY=BETA*BETA-4.*ALPHA*GAMMA
503 IF(DUMMY.GT.0.0) GO TO 213
DELTA=BETA/(2.*ALPHA)
IF(DELTA.GT.0.0) GO TO 211
IERROR=3
GC TO 212
213 DUMMY=SQRT(DUMMY)
DELTA1=(BETA+DUMMY)/(2.*ALPHA)
DELTA2=(BETA-DUMMY)/(2.*ALPHA)
IF((DELTA1.GT.0.0).OR.(DELTA2.GT.0.0)) GO TO 600
IERROR=4
GC TO 212
600 IF((DELTA1.GT.0.0).AND.(DELTA2.GT.0.0)) GO TO 601
IF(DELTA1.GT.0.0) DELTA=DELTA1
IF(DELTA2.GT.0.0) DELTA=DELTA2
GO TO 211

```

```

601 IF (DELTA1.GE.DELTA2) DELTA=DELTA2
    IF (DELTA1.LT.DELTA2) DELTA=DELTA1
211 DFEFF=SIGB*DELTA/E

```

C

```

    IC=1
    DE=0.0
    S=SY11
    DEEFFL=0.0
    DEEFFH=0.0
    SIGIBM=SBAR-SY11
    DEEFFM=0.0
    ISW=1
215 DO 215 J=1,4
    SIGI(J)=0.0
    DO 216 K=1,4
216 SIGI(J)=SIGI(J)+C(I,J,K)*(EPSTI(K)-E2SP11(I,K)-DEEFF*FPSD (K))
    SIGIB=(ABS(SIGI(1)-SIGI(2))**2 +(ABS(SIGI(2)-SIGI(3))**2
1      +(ABS(SIGI(3)-SIGI(1))**2+6.*(ABS(SIGI(4))**2
    SIGIB=SQRT(SIGIB/2.)
    SCI=S+H*(DEEFF-DE)
    IF (ISW .EQ. 2) GO TO 217
    IF (SIGIB-SCI .GT. SIGIBM) GO TO 227
    SIGIBM=SIGIB-SOI
    DEEFFM=DEEFF
227 CONTINUE
226 IF (ABS(SOI-SIGIB).LE.0.01*SOI) GO TO 217
    IF (DEEFFH.EQ.0.0) GO TO 218
    IF (SOI.GT.SIGIB) GO TO 219
    DEEFFL=DEEFF
220 DEEFF=(DEEFFL+DEEFFH)/2.
    GO TO 215
219 DEEFFH=DEEFF
    GO TO 220
218 IF (SOI.GT.SIGIB) GO TO 219
    DEEFFL=DEEFF
    DEEFF=2.*DEEFF
    IC=IC+1
    IF (IC.LE.20) GO TO 215
    DEEFF=DEEFFM
    ISW=2
    GO TO 215
217 DO 221 J=1,4
221 SIGI(J)=SIGI(J)*SOI/SIGIB
    EEFFI=EEFFI1+DEEFF
    IF (NOYILD.EQ.1) GO TO 222
    IF (EEFFI1.GE.ESTAR(NOYILD)) GO TO 222
    J=1
224 IF (EEFFI1.LT.ESTAR(J)) GO TO 223
    J=J+1
    GO TO 224
223 IF (EEFFI1.LE.ESTAR(J)) GO TO 222
    S=SSTAR(J)
    H=HSTAR(J)
    DE=ESTAR(J)-EEFFI1
    DEEFFL=DEEFF
    DEEFF=2.*DEEFF
    IC=1
    DEEFFH=0.0
    ISW=1
    GO TO 215

```

```

222 DO 225 J=1,4
225 EPSPI(J)=EPSPI(1,J)+DEEFF*EPSD(J)
   EPSD(1)=(2.*SIGI(1)-SIGI(2)-SIGI(3))/(2.*SOI)
   EPSD(2)=( -SIGI(1)+2.*SIGI(2)-SIGI(3))/(2.*SOI)
   EPSD(3)=( -SIGI(1)-SIGI(2)+2.*SIGI(3))/(2.*SOI)
   EPSD(4)=3.*SIGI(4)/SOI
   SYI=SOI
   SMAXI=SGI
   SI=SOI
   RETURN
END

```

C
C
C

```

SUBROUTINE COJLMR(I,EPSTI,EPSPI,SIGI,KORNER,FYLDI,EPSDI,MSWCH,
1BLAMB)
COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPCL3,NUMNP,
1 NJMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,CLAMB,
2 KTAPE,KRJN,IPRINT,NUMST,MXSTRT,FUZ(239),
3 IPELTP,INT,NPRCDS,IMPBX
COMMON/A/ U(1600),W(1600),NPJUT(80),VMPCLS(80),FNU(350),
1 FNW(350)
COMMON/B/NADJNP(400),JTYPE(400),SHETA(400),XMASS(400),
1 SNPUU(400),SNPUW(400),SVPWW(400),FAU(400),FAW(400),
2 NPADJ(400,8),SADUJ(400,8),SADUW(400,8),SADWW(400,8)
DIMENSION BJFF(3280)
EQUIVALENCE (BJFF,NADJNP)
DIMENSION NOOFEL(24),IPLAST(24),NP(24,4),ITYPE(24,4),THETA(24,4),
1C(24,4,4),B(24,4,8),P(24,8,4),EPSTI1(24,4),EPSPI1(24,4),
2SIGI1(24,4),DJM(24,29),IDUM(24,29)
EQUIVALENCE (BJFF(1),NOOFEL),(BJFF(25),IPLAST),(BJFF(49),NP),
1(BJFF(145),ITYPE),(BJFF(241),THETA),(BJFF(337),C),(BJFF(721),B),
2(BJFF(1489),P),(BJFF(2257),EPSTI1),(BJFF(2353),EPSPI1),
3(BJFF(2449),SIGI1),(BJFF(2545),DUM),(BJFF(2545),IDUM)
DUM(M,3)=COSTH
DUM(M,4)=KORNER
DUM(M,5)=FYLDI1
DUM(M,6)=FPSDI1
DIMENSION EPSTI(4),EPSPI(4),SIGI(4),EPSDI(4)
DIMENSION EPSDI1(4),SIGBAR(4),SIGMA(4),EPST(4),EPSD(4),ED(4),SX(4)

SIMPLE COULOMB MOHR YIELD CONDITION
ALPHA=DUM(I,1)
CAPPA=DUM(I,2)
COSTH=DUM(I,3)
KORNER=IDUM(I,4)
FYLDI1=DUM(I,5)
DO 1 J=1,4
1 EPSDI1(J)=DJM(I,J+5)

DO 2 J=1,4
SIGBAR(J)=0.0
DO 2 K=1,4
2 SIGBAR(J)=SIGBAR(J)+C(I,J,K)*(EPSTI(K)-EPSPI1(I,K))
AI1BAR=SIGBAR(1)+SIGBAR(2)+SIGBAR(3)
AI2BAR=((SIGBAR(1)-SIGBAR(2))*(SIGBAR(1)-SIGBAR(2))
1 +(SIGBAR(2)-SIGBAR(3))*(SIGBAR(2)-SIGBAR(3))
2 +(SIGBAR(3)-SIGBAR(1))*(SIGBAR(3)-SIGBAR(1)))/6.0
3 +SIGBAR(4)*SIGBAR(4)
IF(AI2BAR.LE.0.0) GO TO 3

```

```

FYLDBR=ALPHA*AI1BAR+SQRT(AI2BAR)
GO TO 4
3 FYLDBR=ALPHA*AI1BAR
4 IF (MSWICH.EQ.1) GO TO 33
  IF (CAPPA.EQ.0.0) GO TO 65
  BLAMB=FYLDBR /CAPPA
  RETURN
65 BLAMB=1.E+38
  RETURN
33 IF (FYLDBR.GE.CAPPA) GO TO 6
  KORNER=0
  DO 5 J=1,4
    SIGI(J)=SIGBAR(J)
    EPSDI(J)=0.0
  5 EPSPI(J)=EPSPI1(I,J)
    FYLDI=FYLDBR
    RETURN
  6 IF (FYLDBR.GT.CAPPA) GO TO 10
    DO 7 J=1,4
      SIGI(J)=SIGBAR(J)
    7 EPSPI(J)=EPSPI1(I,J)
      FYLDI=FYLDBR
      IF (AI2BAR.LT.0.001) GO TO 8
      KORNER=0
      DUM1=SQRT(AI2BAR)
    64 EPSDI(1)=ALPHA+(2.*SIGI(1)- SIGI(2)- SIGI(3))/(6.*DUM1)
      EPSDI(2)=ALPHA+( -SIGI(1)+2.0*SIGI(2)- SIGI(3))/(6.*DUM1)
      EPSDI(3)=ALPHA+( -SIGI(1)- SIGI(2)+2.0*SIGI(3))/(6.*DUM1)
      EPSDI(4)=SIGI(4)/DUM1
      CALL LGTH(EPSDI)
      RETURN
    8 KORNER=1
    DO 9 J=1,4
      EPSDI(J)=0.0
    9 RETURN
  10 IF (FYLDI.LT.CAPPA) GO TO 12
    DO 11 J=1,4
      SIGMA(J)=SIGI1(I,J)
      EPST(J)=EPSTI1(I,J)
    11 EPSD(J)=EPSDI1(J)
    GO TO 16
  12 DUMMY=(CAPPA-FYLDI)/(FYLCBR-FYLDI)
    DO 13 J=1,4
      SIGMA(J)=SIGI1(I,J)+DUMMY*(SIGBAR(J)-SIGI1(I,J))
    13 EPST(J)=EPSTI1(I,J)+DUMMY*(EPSTI(J)-EPSTI1(I,J))
      AI2=((SIGMA(1)-SIGMA(2))*(SIGMA(1)-SIGMA(2))
        1 +(SIGMA(2)-SIGMA(3))*(SIGMA(2)-SIGMA(3))
        2 +(SIGMA(3)-SIGMA(1))*(SIGMA(3)-SIGMA(1)))/6.0
        3 +SIGMA(4)*SIGMA(4)
      IF (AI2.LE.1.0E-5) GO TO 14
      DUMMY=SQRT(AI2)
      KORNER=0
      EPSD(1)=ALPHA+(2.*SIGMA(1)- SIGMA(2)- SIGMA(3))/(6.*DUMMY)
      EPSD(2)=ALPHA+( -SIGMA(1)+2.*SIGMA(2)- SIGMA(3))/(6.*DUMMY)
      EPSD(3)=ALPHA+( -SIGMA(1)- SIGMA(2)+2.*SIGMA(3))/(6.*DUMMY)
      EPSD(4)=SIGMA(4)/DUMMY
      CALL LGTH(EPSD)
      GO TO 16
  14 KORNER=1
    DO 15 J=1,4

```

129

```

15 EPSD(J)=0.0
16 IF(KORNER.EQ.1) GO TO 28
17 AX=C(1,1,1)*EPSD(1)+C(1,1,2)*EPSD(2)+C(1,1,3)*EPSD(3)
  AY=C(1,2,1)*EPSD(1)+C(1,2,2)*EPSD(2)+C(1,2,3)*EPSD(3)
  AZ=C(1,3,1)*EPSD(1)+C(1,3,2)*EPSD(2)+C(1,3,3)*EPSD(3)
  AW=C(1,4,4)*EPSD(4)
  B1=AX+AY+AZ
  B2=((SIGBAR(1)-SIGBAR(2))*(AX-AY)+(SIGBAR(2)-SIGBAR(3))*(AY-AZ)
1    +(SIGBAR(3)-SIGBAR(1))*(AZ-AX))/6.0+(SIGBAR(4)*AW)
  B3=(ABS(AX-AY)**2+(ABS(AY-AZ)**2+(ABS(AZ-AX)**2
  B3=B3/6.0+AW*AW
  D1=B3-(ALPHA*B1)*(ALPHA*B1)
  D2=2.*(ALPHA*ALPHA*AI1BAR*B1-ALPHA*CAPPA*P1-B2)
  D3=AI2BAR-CAPPA*CAPPA+2.*ALPHA*CAPPA*AI1BAR-ALPHA*ALPHA
1    *AI1BAR*AI1BAR
  IF(D1.NE.0.0) GO TO 18
  ALAMB=-D3/D2
  IF(ALAMB.GT.0.0) GO TO 24
  IERROR=1
  ALAMB1=0.0
  ALAMB2=0.0
  DUMMY=0.0
  GO TO 23
18 DUMMY=D2*D2-4.*D1*D3
  IF(DUMMY.GE.0.0) GO TO 19
  IF(ABS(DUMMY/(D2*D2)).LT.0.015) GO TO 19
  IERROR=2
  ALAMB=0.0
  ALAMB1=0.0
  ALAMB2=0.0
  GO TO 23
19 IF(DUMMY.GT.0.0) GO TO 20
  ALAMB=-D2/(2.*D1)
  IF(ALAMB.GT.0.0) GO TO 24
  IERROR=3
  ALAMB1=0.0
  ALAMB2=0.0
  GO TO 23
20 ALAMB1=(-D2+SQR(DUMMY))/(2.*D1)
  ALAMB2=(-D2-SQR(DUMMY))/(2.*D1)
  IF((ALAMB1.GT.0.0).OR.(ALAMB2.GT.0.0)) GO TO 21
  IERROR=4
  GO TO 23
21 IF((ALAMB1.GT.0.0).AND.(ALAMB2.GT.0.0)) GO TO 22
  IF(ALAMB1.GT.0.0) ALAMB=ALAMB1
  IF(ALAMB2.GT.0.0) ALAMB=ALAMB2
  GO TO 24
22 IF(ALAMB1.GE.ALAMB2) ALAMB=ALAMB2
  IF(ALAMB1.LT.ALAMB2) ALAMB=ALAMB1
  GO TO 24
23 WRITE(6,100) T,NOFFL(1),1
100 FORMAT(1H1,45HERROR IN PLASTIC FORCES, COULOMB-MOHR ROUTINE//
110H TIME      =,1P4E15.5/10H ELEMENT =,15/10H NUMBER =,15)
  WRITE(6,101) (EPST11(I,J),J=1,4),(EPSP11(I,J),J=1,4),
1    1(EPSD11(J),J=1,4),(EPST1(J),J=1,4),(EPST(J),J=1,4),
2    2(EPSD(J),J=1,4)
101 FORMAT(10H EPST11 =,1P4E15.5/10H EPSP11 =,1P4E15.5/
110H EPSD11 =,1P4E15.5/10H EPST1 =,1P4E15.5/
210H EPST =,1P4E15.5/10H EPSD =,1P4E15.5)
  WRITE(6,102) (SIG11(I,J),J=1,4),(SIGBAR(J),J=1,4),(SIGMA(J),J=1,4)

```

```

102 FORMAT(10H SIGI1   =,1P4E15.5/10H SIGBAR   =,1P4E15.5/
110H SIGMA   =,1P4E15.5)
WRITE(6,103) ALPHA,CAPPA,COSTH,KORNER,FYLDI1,FYLD8R,A11BAR,
1A12BAR
103 FORMAT(10H ALPHA   =,1PE15.5/10H KAPPA   =,1PE15.5/10H COS(TH) =,
11PE15.5/10H KORNER   =,15/10H FYLDI1   =,1PE15.5/10H FYLD8R   =,
21PE15.5/10H I1BAR   =,1PE15.5/10H I2BAR   =,1PE15.5)
WRITE(6,104) C(1,1,1),C(1,1,2),C(1,1,3),C(1,4,4)
104 FORMAT(10H C(1,1)  =,1PE15.5/10H C(1,2)  =,1PE15.5/10H C(1,3)  =,
11PE15.5/10H C(4,4)  =,1PE15.5)
WRITE(6,105) AX,AY,AZ,AW,B1,B2,B3,D1,D2,D3
105 FORMAT(10H AX      =,1PE15.5/10H AY      =,1PE15.5/10H AZ      =,
11PE15.5/10H AW      =,1PE15.5/10H B1      =,1PE15.5/10H B2      =,
21PE15.5/10H B3      =,1PE15.5/10H D1      =,1PE15.5/10H D2      =,
31PE15.5/10H D3      =,1PE15.5)
WRITE(6,106) ALAMB,ALAMB1,ALAMB2,DUMMY,IERROR
106 FORMAT(10H ALAMB   =,1PE15.5/10H ALAMB1   =,1PE15.5/10H ALAMB2   =,
11PE15.5/10H DUMMY    =,1PE15.5/10H IERROR    =,15)
CALL EXIT
24 ALAMB1=0.0
ALAMBH=0.0
IC=1
25 DO 26 J=1,4
SIGI(J)=0.0
DO 25 K=1,4
26 SIGI(J)=SIGI(J)+C(1,J,K)*(EPSTI(K)-EPSPI1(I,K)-ALAMB*FPSD(K))
A11=SIGI(1)+SIGI(2)+SIGI(3)
A12=((ABS(SIGI(1)-SIGI(2)))**2+(ABS(SIGI(2)-SIGI(3)))**2
1 + (ABS(SIGI(3)-SIGI(1)))**2)/6.0+SIGI(4)*SIGI(4)
FSTAR=ALPHA*A11+SQRT(A12)-CAPPA
IF(FSTAR.NE.0.0) GO TO 53
IF(A12.GT.0.001) GO TO 51
27 KORNER=1
DO 50 J=1,4
50 EPSPI(J)=EPSPI1(I,J)+ALAMB*EPSD(J)
GO TO 36
51 KORNER=0
FYLDI=CAPPA
FPSDI(1)=ALPHA+(2.*SIGI(1)-SIGI(2)-SIGI(3))/(6.*SQRT(A12))
EPSDI(2)=ALPHA+(-SIGI(1)+2.*SIGI(2)-SIGI(3))/(6.*SQRT(A12))
EPSDI(3)=ALPHA+(-SIGI(1)-SIGI(2)+2.*SIGI(3))/(6.*SQRT(A12))
EPSDI(4)=SIGI(4)/SQRT(A12)
CALL LGTH(EPSDI)
DO 52 J=1,4
52 EPSPI(J)=EPSPI1(I,J)+ALAMB*EPSD(J)
RETURN
53 ABAR2=(ABS(CAPPA/ALPHA-A11))**2/3.
BBAR2=2.*A12
RBAR2=ABAR2+BBAR2
IF(RBAR2.LE.0.001) GO TO 27
IF((CAPPA/ALPHA-A11).LE.0.0) GO TO 27
A12S=(CAPPA-ALPHA*A11)**2
BBARST=SQRT(2.*A12S)
IF(BBAR2.GT.0.0) GO TO 54
ALAMBH=ALAMB
GO TO 57
54 BBAR=SQRT(BBAR2)
IF(ABS(BBARST-BBAR).LE.0.01*BBARST) GO TO 61
C=====
PRINT 999,I,NCUFFL(I),IC,ALAMB,BBAR,BBARST

```


131

999 FORMAT(3I10,3E15.5)

C=====*****

```

      IC=IC+1
      IF(IC.LE.20) GO TO 55
      IERROR=8
      ALAMB1=BBARST
      ALAMB2=BBAR
      DUMMY=AI2
      GO TO 23
55  IF(ALAMBH.GT.0.0) GO TO 58
      IF(BBAR.LT.BBARST) GO TO 56
      ALAMBL=ALAMB
      ALAMB=2.*ALAMB
      GO TO 25
56  ALAMBH=ALAMB
57  ICOUNT=I
      GO TO 60
58  IF(BBAR.GT.BBARST) GO TO 59
      ALAMBH=ALAMB
      GO TO 60
59  ALAMBL=ALAMB
60  ALAMB=(ALAMBH+ALAMBL)/2.
      GO TO 25
61  S=AI1/3.
      DO 62 J=1,3
62  SX(J)=SIGI(J)-S
      SX(4)=SIGI(4)
      DUMMY=BBARST/BBAR
      DO 63 J=1,3
      SX(J)=DUMMY*SX(J)
63  SIGI(J)=S+SX(J)
      SIGI(4)=DUMMY*SX(4)
      GO TO 51
28  XNU=C(I,1,2)/(C(I,1,1)+C(I,1,2))
      EBAR=C(I,1,1)/(1.-XNU)
      EMOD=EBAR*(1.+XNU)*(1.-2.*XNU)
      DO 29 J=1,3
29  EPSD(J)=EPSTI(J)-(CAPPA*(1.-2.*XNU)/(3.*ALPHA*EMOD))-EPSPI1(I,J)
      EPSD(4)=EPSTI(4)-EPSPI1(I,4)
      CALL LGTH(EPSD)
      ALGTH=(ABS(EPSD(1))**2+(ABS(EPSD(2))**2+(ABS(EPSD(3))**2
      ALGTH=SQRT(ALGTH)
      DELTAD=EPSD(1)+EPSD(2)+EPSD(3)
      COSTHB=DELTAD/(SQRT(3.)*ALGTH)
      IF(ALGTH.EQ.0.0) COSTHB=1.0
      IF(COSTHB.GE.COSTH) GO TO 42
C
      ABAR2=(ABS(CAPPA/ALPHA-AI1BAR))**2/3.
      BBAR2=2.*AI2BAR
      RBAR2=ABAR2+BBAR2
      IF(RBAR2.LT.1.0E-5) GO TO 42
      IF((CAPPA/ALPHA-AI1BAR).LE.0.0) GO TO 42
      S=AI1BAR/3.
      AI2S=(CAPPA-ALPHA*AI1BAR)**2
      HBARST=SQRT(2.*AI2S)
      BBAR=SQRT(BBAR2)
      DO 30 J=1,3
30  SX(J)=(BBARST/BBAR)*(SIGBAR(J)-S)
      SX(4)=(BBARST/BBAR)*SIGBAR(4)
      DO 31 J=1,3

```

```

31 SIGI(J)=SX(J)+S
   SIGI(4)=SX(4)
   ED(1)=(SIGI(1)-XNU*(SIGI(2)+SIGI(3)))/EMOD
   ED(2)=(SIGI(2)-XNU*(SIGI(1)+SIGI(3)))/EMOD
   ED(3)=(SIGI(3)-XNU*(SIGI(1)+SIGI(2)))/EMOD
   ED(4)=2.*(1.+XNJ)*SIGI(4)/EMOD
   DO 32 J=1,4
32 EPSPI(J)=EPSTI(J)-ED(J)
   KCRNER=0
   FYLDI=CAPPA
   DUM1=SQRT(A12S)
   GO TO 64
42 DC 43 J=1,3
43 EPSPI(J)=EPSTI(J)-(CAPPA*(1.-2.*XNU)/(3.*ALPHA*EMOD))
   EPSPI(4)=EPSTI(4)
36 S=CAPPA/(3.*ALPHA)
   DC 37 J=1,3
37 SIGI(J)=S
   SIGI(4)=0.0
   KORNER=1
   FYLDI=CAPPA
   DO 38 J=1,4
38 EPSDI(J)=0.0
   RETURN
   END

```

C
C
C

```

SUBROUTINE LGTH(E)
  DIMENSION E(4),A(4)
  DO 1 I=1,4
1  A(I)=ABS(E(I))
  B=AMAX1(A(1),A(2),A(3),A(4))
  DO 2 I=1,4
2  E(I)=E(I)/B
  RETURN
  END

```

C
C
C

```

SUBROUTINE NCOJL(NJME,EPSTI,EPSTII,EPSP I,EPSP II,SIGI,SIGII,
1COHESN,FRCTN1,SNWCH,FRCTN2,CRESID,
2FRESID,MYIELD,IRESID,JTENS,CMAT,ISTRES,
3MSWCH,SBAR,BLAMB,COSTH,SINTH,F,G,
4SIGNI,NSWCH,SIGNBI)
  DIMENSION CMAT(4,4),EPSTI(4),EPSTII(4),EPSP I(4),EPSP II(4),
1  SIGI(4),SIGII(4)
  DIMENSION F(4,4),G(4,4),SIGNI(4),SIGNII(4),SIGNBI(4),EPTNI(4),
1  EPTNII(4)
C  MSWCH=0 COMPUTE BLAMB ONLY
C  NSWCH=1 COMPUTE F,G, AND SIGNII ONLY
  DO 1 I=1,4
  DO 1 J=1,4
  F(I,J)=0.0
1  G(I,J)=0.0
  F(1,1)=COSTH*COSTH
  F(3,1)=SINTH*SINTH
  F(4,1)=SINTH*COSTH
  F(2,2)=1.0
  F(1,3)=F(3,1)

```

```

F(3,3)=F(1,1)
F(4,3)=-F(4,1)
F(1,4)=-2.*F(4,1)
F(3,4)=-F(1,4)
F(4,4)=F(1,1)-F(3,1)
DO 2 I=1,4
DO 2 J=1,4
2 G(I,J)=F(I,J)
G(4,1)=-G(4,1)
G(4,3)=-G(4,3)
G(1,4)=-G(1,4)
G(3,4)=-G(3,4)

```

C

```

DO 3 I=1,4
SIGNI1(I)=0.0
DO 3 J=1,4
3 SIGNI1(I)=SIGNI1(I)+F(I,J)*SIGI1(J)
IF(NSWCH.EQ.1) RETURN
DO 4 I=1,4
EPTNI1(I)=0.0
EPTNI(I)=0.0
DO 4 J=1,4
EPTNI1(I)=EPTNI1(I)+G(J,I)*EPSTI1(J)
4 EPTNI(I)=EPTNI(I)+G(J,I)*EPSTI(J)
DO 5 I=1,4
SIGNBI(I)=SIGNI1(I)
DO 5 J=1,4
5 SIGNBI(I)=SIGNBI(I)+CMA*(I,J)*(EPTNI(J)-EPTNI1(J))
SBAR=SIGNBI(4)

```

C

```

TNPHI1=TAN(FRCTN1)
TNPHI2=TAN(FRCTN2)
TAUNT=ABS(SIGNBI(4))
SNI=SIGNBI(3)
IF(MSWCH.EQ.1) GO TO 10
IF(SNI.GT.0.0.AND.JTENS.NEQ.0) GO TO 6
AUMER=TAUNT+SNI*TNPHI1
DENOM=COHESN
IF(DENOM.GT.0.0) GO TO 7
IF(AUMER.GT.0.0) GO TO 6
IF(FRCTN1.GT.FRCTN2) GO TO 50
BLAMB=0.0
GO TO 9
6 BLAMB=1.0E+38
GO TO 9
7 IF(AUMER.GT.0.0) GO TO 8
AUMER=TAUNT+SNI*TNPHI2
IF(AUMER.GT.0.0) GO TO 8
BLAMB=0.0
GO TO 9
8 BLAMB=AUMER/DENOM
IF(FRCTN1.EQ.FRCTN2) RETURN
SIGFF=SNI/BLAMB
IF(SIGFF.GE.SNSWCH) RETURN
50 AUMER=TAUNT+SNI*TNPHI2
DENOM=COHESN-SNSWCH*(TNPHI1-TNPHI2)
BLAMB=AUMER/DENOM
IF(AUMER.LE.0.0) BLAMB=0.0
9 RETURN

```

C

134

```

10 IF(JTENS.NE.1) GO TO 100
   IF(SNI.GE.0.0) GO TO 150
   GO TO 105
100 IF(TNPHI1.LE.0.0) GO TO 105
   IF(SNI.LT.COHE.N/TNPHI1) GO TO 105
   IF(IRESID.EQ.0) GO TO 160
   COHE.N=CRESID
   FRCTN1=FRESID
   FRCTN2=FRESID
   GO TO 160
105 IF(MYIELD.EQ.1.AND.IRESID.EQ.1) GO TO 110
   IF(SNI.LT.SNSWCH) GO TO 120
110 TAUNT.B=COHE.N-SNI*TNPHI1
   GO TO 130
120 TAUNT.B=COHE.N-SNSWCH*TNPHI1-(SNI-SNSWCH)*TNPHI2
130 IF(TAUNT.LT.TAUNT.B) GO TO 170
   IF(MYIELD.EQ.1) GO TO 140
   IF(IRESID.EQ.0) GO TO 140
   COHE.N=CRESID
   FRCTN1=FRESID
   FRCTN2=FRESID
   IF(FRESID.GT.0.0) GO TO 132
   IF(SNI.GE.0.0) GO TO 150
   GO TO 135
132 IF(SNI.LT.COHE.N/TAN(FRESID)) GO TO 135
   GO TO 160
135 TAUNT.B=COHE.N - SNI*TAN(FRESID)
140 SIGNI(4)=SIGNBI(4)*TAUNT.B/TAUNT
   SIGNI(3)=SIGNBI(3)
   SIGNI(2)=SIGNBI(2)
   SIGNI(1)=SIGNBI(1)
   MYIELD=1
   GO TO 15
150 SIGNI(4)=0.0
   SIGNI(3)=0.0
   SIGNI(2)=SIGNBI(2)
   SIGNI(1)=SIGNBI(1)
   MYIELD=1
   GO TO 15
160 SIGNI(4)=0.0
   SIGNI(3)=COHE.N/TAN(FRCTN1)
   SIGNI(2)=SIGNBI(2)
   SIGNI(1)=SIGNBI(1)
   MYIELD=1
   GO TO 15
170 DO 180 I=1,4
180 SIGNI(I)=SIGNBI(I)
C
15 DO 16 I=1,4
   SIGI(I)=0.0
   DO 15 J=1,4
16 SIGI(I)=SIGI(I)+G(I,J)*SIGNI(J)
C
   IF(ISTRES.EQ.2) GO TO 17
   XNU=CMAT(1,3)/(CMAT(1,1)+CMAT(1,3))
   EBAR=CMAT(1,1)/(1.-XNU)
   EMOD=EBAR*(1.+XNU)*(1.-2.*XNU)
   GO TO 18
17 XNU=CMAT(1,3)/CMAT(1,1)
   EMOD=CMAT(1,1)*(1.-XNU*XNU)

```

```

18 EPSPI(1)=(SIGI(1)-XNU*(SIGI(2)+SIGI(3)))/EMOD
   EPSPI(2)=(SIGI(2)-XNU*(SIGI(3)+SIGI(1)))/EMOD
   EPSPI(3)=(SIGI(3)-XNU*(SIGI(1)+SIGI(2)))/EMOD
   EPSPI(4)=SIGI(4)/CMAT(4,4)
   DO 19 I=1,4
19 EPSPI(I)=EPSTI(I)-EPSPI(I)

C
   RETURN
   END

C
C
C
   OVERLAY(MOHAN,14,0 )
   PROGRAM LNK3

C
   COMMON MAXNP,MXCLS,MXADJP,MXZONE,MXNPB,NZONES,MXPCLB,NUMNP,
1        NUMEL,ISTRES,NUMPEL,NUMELP,PERIOD,NMKCLS,FACTOR,ALAMB,
2        KTAPE,KRUN,IPRINT,NUMST,MXSTRT,FUZ(239),
3        IPELTP,INT,NPRCDS,IMPRX

C
   COMMON/A/ U(1600),W(1600),VPCUT(80),VMPCLS(80),FNU(350),
1        FNU(350)

C
   DIMENSION STNPJ(4,350),STNPW(4,350),STADU(4,350,8),
1  STADW(4,350,8),NADJNP(350),NADJEL(350),NPADJ(350,8)

C
   DIMENSION ITYPE(350),SHETA(350),XMASS(350),SNPLU(350),SNPUW(350),
1  SNPWW(350),FAU(350),FAW(350),SADUU(350,8),SADUW(350,8),
2  SADWW(350,8)

C
   DIMENSION NPTN(1600),COM(16),SIG(350,4),EPST(24,4),EFFECT(24),
1  SIGPL(350,4),SIGMX(350),SIGMN(350),THETA(350),NOOFEL(24),
2  NP(24,4),EPS(24,4),C(24,4,4),SIGMAP(4),EPSE(4),SIGMA(4)

C
   EQUIVALENCE (SIGMX,FNU),(SIGMN,FNU),(MAXNP,COM(1))

C
   EQUIVALENCE (STNPU(1),ITYPE),(STNPU(351),SHETA),(STNPU(701),XMASS),
1  (STNPU(1021),SNPUJ),(STNPW(1),SNPUW),(STNPW(351),SNPWW),
2  (STNPW(701),FAU),(STNPW(1051),FAW),(STADU(1),SADUU),
3  (STADU(2801),SADUW),(STADU(5601),SADWW)
   EQUIVALENCE(FUZ(1),NOOFEL),(FUZ(25),NP),(FUZ(121),EPS)

C
   MOHAN=5HMOHAN
   PI=3.1415927
   ISWCH=0
   IF((NUMNP.LE.MXNPB).AND.(NUMPEL.EQ.0)) ISWCH=1
   REWIND 10
   REWIND INT
   REWIND 1

C*****
   IF(IMPBX.NE.1) GO TO 1
   REWIND 15
   READ(15)DUMMY,DUMMY,(DUMMY1,DUMMY2,I=1,NUMNP),(DUMMY1,DUMMY2,
1  DUMMY3,DUMMY4,DUMMY5,I=1,NUMEL)
C*****
1  READ(10) N1,N2,N3,N4,N5,N6,N7,(NADJNP(I),ITYPE(I),SHETA(I),
1  XMASS(I),SNPJ(I),SNPUW(I),SNPWW(I),FAU(I),FAW(I),
2  (NPADJ(I,J),SADUJ(I,J),SADUW(I,J),SADWW(I,J),J=1,MXADJP),I=1,N7)
   IF(N4.LT.NUMNP) GO TO 1
   READ(10) (NPTN(I),I=1,NUMNP)

```

136

```

C      IC=1
2      READ(10) NPLOW,NPHIGH,NPOUT(IC),NUMCP,N5,NMPCLS(IC),N7,(NADJNP(I),
1      NADJEL(I),(NPADJ(I,J),J=1,MXADJP),(STNPU(K,I),STNPW(K,I),
2      K=1,4),((STADJ(K,I,J),STADW(K,I,J),K=1,4),J=1,MXADJP),I=1,N7)
      IF(IISWTCHEQ.1) GO TO 30
      WRITE(1) NPLOW,NPHIGH,NPOUT(IC),NUMCP,N5,NMPCLS(IC),N7,(NADJNP(I),
1      NADJEL(I),(NPADJ(I,J),J=1,MXADJP),(STNPU(K,I),STNPW(K,I),
2      K=1,4),((STADJ(K,I,J),STADW(K,I,J),K=1,4),J=1,MXADJP),I=1,N7)
      IC=IC+1
      IF(NUMCP.LT.NJMNP) GO TO 2
30     REWIND 10
      REWIND 1
      READ(INT)(COM(I),I=1,16)
C
      IF(IPELTP.EQ.12) IOUTAP=3
      IF(IPELTP.EQ. 3) IOUTAP=12
      WRITE(6,3) IOUTAP
3     FORMAT(1H1,26HOUTPUT HISTORY TAPE IS NO.,I5//)
C
      REWIND IOUTAP
      WRITE(IOUTAP)(COM(I),I=1,16),NPRCDS,(NPV(I),I=1,NUMNP)
C
      NPRCDS=NPRCDS+1
      DO 100 IPRCDS=1,NPRCDS
C
      DO 200 I=1,NUMNP
      READ(INT) UBAR,WBAR
      IF(IPRCDS.GT.1) GO TO 201
      U(I)=UBAR
      W(I)=WBAR
      GO TO 200
201    U(I)=U(I)+UBAR
      W(I)=W(I)+WBAR
200    CONTINUE
C
      DO 101 ICLUS=1,NMKCLS
C
      DO 4 I=1,MXNPB
      DO 4 J=1,4
4      SIGPL(I,J)=0.0
C
      IF(IPRCDS.GT.1) GO TO 104
      WRITE(6,25)
25     FORMAT(1H1,16HELASTIC SOLUTION//)
      IF(NUMPEL.EQ.0) GO TO 102
      GO TO 106
C
104    NUM=IPRCDS-1
      WRITE(6,5) NJM
5      FORMAT(1H1,22HPLASTIC INCREMENT NO. =,I5//)
C
106    CONTINUE
      NUM=NMPCLS(ICLUS)
      IF(NUM.EQ.0) GO TO 102
      DO 6 II=1,NUM
C
      READ(INT) NUMELB,(NOOFEL(I),EFFECT(I),(NP(I,J),EPST(I,J),EPSP(I,J),
1      IC(I,J,K),K=1,4),J=1,4),I=1,NUMELB)
C

```

```

      DO 7 I=1,NUMELB
      DC 32 J=1,4
32  EPSE(J)=EPST(I,J)-EPSP(I,J)
      DO33 J=1,4
      SIGMA(J)=0.0
      DO33 K=1,4
33  SIGMA(J)=SIGMA(J)+C(I,J,K)*EPSE(K)
C
      DC 8 J=1,4
      SIGMAP(JT)=0.0
      DC 8 K=1,4
      8  SIGMAP(J)=SIGMAP(J)+C(I,J,K)*EPSP(I,K)
      DC 9 J=1,4
      NODE=NP(I,J)
      IF(NODE.EQ.0) GO TO 9
      NPR=NODE-NPOJT(TCLUS)
      DC 10 K=1,4
10  SIGPL(NPR,K)=SIGPL(NPR,K)+SIGMAP(K)
      9  CONTINUE
      7  CONTINUE
      6  CONTINUE
      WRITE(6,36)
36  FORMAT(1H0,15HNODE POINT DATA//)
102 IF(IISWCH.EQ.1) GO TO 103
      READ(1) NPLW,NPHGH,N3,NUMCP,N5,N6,NUMNPB,(NADJNP(I),
      1NADJEL(I),(NPADJ(I,J),J=1,MXADJP),(STNPU(K,I),STNPW(K,I),
      2K=1,4),(STADU(K,I,J),STADW(K,I,J),K=1,4),J=1,MXADJP),I=1,NUMNPB)
C
      NLOW=NPLW-NPOUT(ICLUS)
      NHGH=NPHGH-NPOJT(ICLUS)
      DO 11 I=NLOW,NHGH
      DUM=NADJEL(I)
      DO 11 J=1,4
11  SIGPL(I,J)=SIGPL(I,J)/DUM
C
103 CONTINUE
      NLOW=NPLW-NPOJT(ICLUS)
      NHGH=NPHGH-NPOJT(ICLUS)
      DO 12 I=NLOW,NHGH
      NO=I+NPOJT(TCLUS)
      NUM=NADJNP(I)
      DO 13 J=1,4
      SIG(I,J)=STNPJ(J,I)*U(NO)+STNPW(J,I)*W(NO)
      DC 14 K=1,NUM
      NODE=NPADJ(I,K)
14  SIG(I,J)=SIG(I,J)+STADU(J,I,K)*U(NODE)+STADW(J,I,K)*W(NODE)
      SIG(I,J)=SIG(I,J)-SIGPL(I,J)
13  CONTINUE
      DUM1=(SIG(I,1)-SIG(I,3))/2.
      DUM2=DUM1*DUM1+SIG(I,4)*SIG(I,4)
      IF(DUM2.GT.0.0) GO TO 15
      RADIUS=0.0
      GO TO 16
15  RADIUS=SQRT(DUM2)
16  DUM3=(SIG(I,1)+SIG(I,3))/2.
      SIGMX(I)=DUM3+RADIUS
      SIGMN(I)=DUM3-RADIUS
      IF(DUM1.GE.0.0) GO TO 17
      THE=ATAN(SIG(I,4)/(-DUM1))
      THE=(PI-THE)/2.

```

138

```

GC TO 20
17 IF(DUM1.GT.0.0) GO TO 19
   IF(SIG(1,4).EQ.0.0) GO TO 18
   THE=PI/4.
   GO TO 20
18 THE=0.0
   GC TO 20
19 THE=0.5*ATAN(SIG(1,4)/DUM1)
20 THETA(1)=THE*180./PI
12 CONTINUE
   WRITE(6,21)
21 FORMAT(10H NEW NODE,10X,6PU (IN),12X,12HSIGMAR (PSI),8X,
   112HSIGMAZ (PSI),8X,12HSIGMX (PSI),9X,11HTHETA (DEG)/
   210H OLD NCDE,10X,6PU (IN),12X,12HSIGMAT (PSI),8X,12HTAU (PSI),
   38X,12HSIGMN (PSI)//)
   DO 22 T=NLOW,NHGH
   NPNEW=1+NPCUT(1CLUS)
   NPOLD=NPTN(NPNEW)
C*****
   IF(IMPBX.EQ.1)WRITE(15)NPOLD,U(NPNEW),W(NPNEW)
C*****
22 WRITE(5,23) NPNEW,U(NPNEW),SIG(1,1),SIG(1,3),SIGMX(1),THETA(1),
1      NPOLD,W(NPNEW),SIG(1,2),SIG(1,4),SIGMN(1)
23 FORMAT(17,3X,1P5E20.5/17,3X,1P4E20.5//)
   NMRCDS=NHGH-NLOW+1
101 CONTINUE
   REWIND 1
100 CONTINUE
   REWIND IOJTAP
   REWIND INT
   REWIND 1
   RETURN
END

```


APPENDIX B - INTERPOLATION CODE FOR DETERMINING MPBX DISPLACEMENTS

B.1 - Code Description

This code determines the displacement of desired points along multiple position borehole extensometers (MPBX) by interpolating between node point displacements determined in the static SLAM finite element code. The code is written entirely in FORTRAN IV and consists of a main program, and five subroutines.

The code accepts either punched card input, which is read via tape 5, or input stored on magnetic tape, read as tape 1, output of the static SLAM code. Output is printed via tape 6. The code is presently operational on the CDC 6500, using the Purdue MACE operating system.

B.2 - Data Deck Setup

The following description of the data deck setup assumes that, in general, all numbers are right-oriented in their fields. Inclusion of the decimal point in floating point (real) numbers overrides the right-orientation requirement. Integer data are entered in 5-column fields while all floating point data are entered in 10-column fields.

<u>CARD</u>	<u>VARIABLE</u>	<u>FORMAT</u>
1.1	ANAME	(18A4)
	ANAME = Problem descriptor to be printed as output, up to 72 characters.	
2.1	ITAPE	(I5)
	ITAPE = Counter to indicate whether node point and element input is to be on punched cards or magnetic tape.	
	= 0, Node point and element input data are on punched cards.	
	= 1, Node point and element input data are on magnetic tape.	
3.1	NUMNP, NUMEL	(2I5)
	NUMNP = Number of node points (≤ 1600)	
	NUMEL = Number of elements	
	<u>Note:</u> Card 3.1 omitted if ITAPE = 1.	
4.1	NPNUM, R, Z, U, V	(I5,2F10.3, 2E10.4)
	NPNUM = Node point number	
	R = Radial (horizontal) coordinate (ft) of node point	
	Z = Vertical coordinate (ft) of node point	
	U = Horizontal displacement (inches) of node point (positive to right)	
	V = Vertical displacement (inches) of node point (positive down).	
	<u>Note:</u> Card 4.1 Repeated NUMNP times if ITAPE = 0; card is omitted if ITAPE = 1	
5.1	NUME, NPI, NPJ, NPK, NPL	(5I5)
	NUME = Element Number	
	NPI to NPL = Node numbers at vertices of rectangular element. NPI may be any node, but NPJ, NPK, NPL must be given in <u>clockwise</u> order around element starting from NPI. If NPI = 0, element is considered to be a triangle.	
	<u>Note:</u> Card 5.1 repeated NUMEL times if ITAPE = 0; card is omitted if ITAPE = 1.	

<u>CARD</u>	<u>VARIABLE</u>	<u>FORMAT</u>
6.1	NBX	(I5)
	NBX = Number of MPBX lines considered.	
6.2	NMPBX, PBX (1), PBX (2), PBX (3), PBX (4)	(I5, 4F10.2)
	NMPBX = MPBX identification number	
	PBX (1) = Radial (horizontal) coordinate (ft) of leftmost end of MPBX. If MPBX is vertical, this is the coordinate for the lower end.	
	PBX (2) = Vertical coordinate (ft) of leftmost end of MPBX.	
	PBX (3) = Radial (horizontal) coordinate (ft) of rightmost end of MPBX.	
	PBX (4) = Vertical coordinate (ft) of rightmost end of MPBX.	

Note: Card 6.2 repeated NBX times.

B.3 - Output

The output for each MPBX considered gives the MPBX identification number, coordinates of the end points, and displacements parallel to the MPBX line of points on the line. Displacements are determined for each point on the MPBX line where it intersects a line joining two adjacent node points.

Displacements along the MPBX line are defined to be positive if they occur in the direction from the leftmost end point toward the rightmost end point (irrespective of which end corresponds to the tunnel face). When the MPBX is vertical positive displacement corresponds to movement from the lower end point toward the higher end point.

```

PROGRAM WHP(INPUT,OUTPUT,TAPE5=INPUT,TAPE6=OUTPUT,TAPE1)
DIMENSION PBX(10,5),NP(1600,10),R(1600),Z(1600),
IU(1600),V(1600),NADNP(1600)
WRITE(6,5)
5  FORMAT(*1      DISPLACEMENTS ALONG MPBX LINES*//
1*NOTE--DISPLACEMENTS ARE MEASURED POSITIVE IF IN */
2*DIRECTION FROM LEFTMOST ENDPOINT TO RIGHTMOST ENDPOINT.*//
3*IF MPBX IS VERTICAL, POSITIVE DISPLACEMENT */
4*IS MEASURED IN DIRECTION FROM LOWEST TO HIGHEST POINT*)
500 READ(5,6) ANAME
6  FORMAT(1H4)
WRITE(6,6) ANAME
READ(5,7) ITAPE
IF (ITAPE.EQ.1) GO TO 1000
READ(5,7) NUMNP, NUMEL
7  FORMAT(3I5)
C READ IN NODE POINT DATA INCLUDING DISPLACEMENT
READ(5,8) (NPNUM, R(NPN), Z(NPN), U(NPN), V(NPN),
1APN=1, NUMNP)
GO TO 1010
1000 REWIND 1
READ(1) NUMNP, (R(1), Z(1), I=1, NUMNP), NUMEL
8  FORMAT(15,2F10.3,2E10.4)
C READ IN ELEMENT DATA
C READ IN SUPROUTINE
1010 CALL GETNP(NUMNP, NUMEL, NADNP, NP, ITAPE)
IF (ITAPE.EQ.0) GO TO 1020
READ (1) (I, U(I), V(I), J=1, NUMNP)
1020 DO 1015 I=1, NUMNP
Z(I)=-Z(I)
1015 V(I)=-V(I)
READ(5,7) NBX
DO 400 IBX=1, NBX
C MPBX INPUT-- LEFTMOST ENDPOINT READ FIRST
READ(5,9) NMPBX, (PBX(IBX,1), I=1, 4)
9  FORMAT(15,4F10.2)
WRITE(6,10) NMPBX, (PBX(IBX,1), I=1, 4)
10  FORMAT(// * MPBX NO. = *14*, END POINTS - LEFT = *
1F8.2*, ZLEFT = *F8.2/33X*RIGHT = *F8.2*, ZRIGHT = *F8.2/)
WRITE(6,12)
12  FORMAT(5X,*R-COORD.*5X,*Z-COORD.*5X,*DISPLACMT.(IN.)*//
PBX(IBX,2)=-PBX(IBX,2)
PBX(IBX,4)=-PBX(IBX,4)
C CHECK IF MPBX IS VERTICAL
IF (ABS(PBX(IBX,1)-PBX(IBX,3)).GT.0.01) GO TO 20
PBX(IBX,5)=9999.0
GO TO 25
C CALCULATE SLOPE OF MPBX
20  PBX(IBX,5)=(PBX(IBX,4)-PBX(IBX,2))/(PBX(IBX,3)-PBX(IBX
1,1))
IF (PBX(IBX,5).EQ.0.0) PBX(IBX,4)=PBX(IBX,4)+0.001
25  CALL MAX(PBX(IBX,2), PBX(IBX,1), PBX(IBX,4), PBX(IBX,3),
1PBXTOP, PBXBOT, RINTER, IXCORE)
DO 200 APN=1, NUMNP
IF (NADNP(APN).EQ.0) GO TO 200
NADJP=NADNP(APN)
DO 300 IAP=1, NADJP
C CHECK IF NODE LINE HAS BEEN USED
IF (NP(NPN, IAP)-NPN) 300, 300, 30
30  NPADJ=NP(NPN, IAP)

```

```

C CHECK IF NODE LINE IS VERTICAL
  IF (ABS(R(NPN)-R(NPADJ)).GT.0.01) GO TO 40
  SLOPNP=.99999.0
  GC TO 50
C CALCULATE SLOPE OF NODE POINT LINE
40 SLOPNP=(Z(NPADJ)-Z(NPN))/(R(NPADJ)-R(NPN))
C CHECK FOR MPBX PARALLEL TO NODE PT. LINE
50 IF (ABS(SLOPNP-PBX(IBX,5)).LT.0.01) GO TO 300
  IF (SLOPNP.NE.99999.0) GO TO 85
  RINTER=R(NPN)
  IF (RINTER.LT.PBX(IBX,1).OR.RINTER.GT.PBX(IBX,3))
    1 GC TO 300
    CALL MAX(7(NPN),R(NPN),7(NPADJ),R(NPADJ),ZTOP,ZBOT,RBC
    1T,NPCODE)
    ZINTER=PBX(IBX,2)+PPX(IBX,5)*(RINTER-PBX(IBX,1))
    IF (ZINTER.LT.ZBOT.OR.ZINTER.GT.ZTOP) GO TO 300
    GC TO (60,70),NPCODE
60 ATCP=NPN
  NBCT=NPADJ
  GC TO 80
70 NTCP=NPADJ
  NBCT=NPN
80 CALL INTERP(ZBOT,ZTOP,ZINTER,PBX(IBX,5),U(NBOT),V(NBOT
1),U(ATCP),V(NTOP),DISPL)
  GC TO 350
85 CALL MAX(R(NPN),7(NPN),R(NPADJ),7(NPADJ),RRIGHT,RLEFT,
1ZLEFT,NPCODE)
  IF (PBX(IBX,5).EQ.99999.0) GO TO 90
  RINTER=(PPX(IBX,2)-ZLEFT+SLOPNP*RLEFT-PBX(IBX,5)*PBX(I
1BX,1))/(SLOPNP-PBX(IBX,5))
  IF (RINTER.LT.PBX(IBX,1).OR.RINTER.GT.PBX(IBX,3)) GO TO 300
90 IF (RINTER.LT.RLEFT.OR.RINTER.GT.RRIGHT) GO TO 300
  ZINTER=ZLEFT+SLOPNP*(RINTER-RLEFT)
  IF (ZINTER.LT.PBXBOT.OR.ZINTER.GT.PBXTOP) GO TO 300
  GC TO (100,110),NPCODE
100 NLEFT=NPADJ
  NRIGHT=NPN
  GC TO 120
110 NLEFT=NPN
  NRIGHT=NPADJ
120 CALL INTERP(RLEFT,RRIGHT,RINTER,PBX(IBX,5),U(NLEFT),V(
1NLEFT),U(NRIGHT),V(NRIGHT),DISPL)
350 ZINTER=-ZINTER
  WRITE(6,13) RINTER,ZINTER,DISPL
13 FORMAT(3X,F8.2,5X,F8.2,5X,E12.4)
300 CONTINUE
200 CONTINUE
400 CONTINUE
  STOP
  END

C
C
C
C
SUBROUTINE GFTNP(NUMNP,NUMEL,NADJNP,NPADJ,ITAPE)
  DIMENSION NPADJ(1600,10),NADJNP(1600)
  MXADJP=8

C
  DO 5 I=1,NUMNP
    NADJNP(I)=0
  DO 5 J=1,MXADJP

```

```

5 NPADJ(1,J)=0
C
  DO 7 M=1,NUMEL
    IF (ITAPE.EQ.1) GO TO 6
    READ(5,4) NUMI,NPI,NPJ,NPK,NPL
    GO TO 7
6 READ(1) NUME,NPI,NPJ,NPK,NPL
7 CALL ADJNP(NJMNP,NPADJ,NUME,NPI,NPJ,NPK,NPL)
C
  CALL VADJNP(NADJNP,NUMNP,NPADJ)
4  FORMAT(5I5)
  RETURN
C
  END
C
C
C
  SUBROUTINE ADJNP(NUMNP,NPADJ,NUME,
1 NPI,NPJ,NPK,NPL)
  DIMENSION NPADJ(1600,10),NA(4)
C
C**** FORM TABLE OF ADJACENT NODAL POINTS
C  MXADJP=MAX. NO. OF ADJACENT NODAL POINTS ALLOWED
C  NJMNP =NO. OF NODE POINTS
C  NPADJ =ADJACENT NODE POINT NUMBER
C  NPI   =ELEMENT NODE POINT I
C  NPJ   =ELEMENT NODE POINT J
C  NPK   =ELEMENT NODE POINT K
C  NPL   =ELEMENT NODE POINT L, IF = 0 , TRIAN ELEMENT
C  NUME  =ELEMENT NUMBER BEING CONSIDERED
C  NOTE- TABLE ASSUMED TO BE ALREADY ZEROED OUT
C
  MXADJP=8
  NA(1)=NPI
  NA(2)=NPJ
  NA(3)=NPK
  NA(4)=NPL
  ICCUNT=1
9  NPNUM=NA(1)
  MX=NA(2)
  JCCUNT=1
5  DO 1 I=1,MXADJP
    J=I
    IF (NPADJ(NPNUM,I).EQ.MX) GO TO 2
    IF (NPADJ(NPNUM,I).EQ.0) GO TO 3
1  CONTINUE
    WRITE (6,101) NUME,NPNUM,MX,(NPADJ(NPNUM,I),I=1,MXADJP)
    CALL EXIT
C
3  NPADJ(NPNUM,J)=MX
2  JCCUNT=JCCUNT+1
  IF (JCCUNT.GT.3) GO TO 4
  IF (JCCUNT.GT.2) GO TO 102
  MX=NA(3)
  IF (NPL.EQ.0) GO TO 5
102 MX=NA(4)
  GO TO 5
C
4  GO TO (6,7,8,103),ICCUNT
6  ICCUNT=2

```

```

      NA(1)=NPJ
      NA(2)=NPK
      NA(3)=NPL
      NA(4)=NPI
      GC TO 9
C
      7 ICCUNT=3
      NA(2)=NPL
      NA(3)=NPI
      NA(1)=NPK
      NA(4)=NPJ
      GC TO 9
C
      8 ICCUNT=4
      NA(1)=NPL
      IF (NPL.EQ.0) GO TO 103
      NA(2)=NPI
      NA(3)=NPJ
      NA(4)=NPK
      GC TO 9
C
      101 FORMAT(43HERROR IN FORMING ADJACENT NODAL POINT ARRAY/
      121H ELEMENT NUMBER      =,15/19HNODE POINT NUMBER =,15/
      217H ADJACENT POINT=,15/15H NPADJ(NPNUM,1)/(21X,15))
C
      103 RETURN
C
      END
C
C
C
C
      SUBROUTINE VADJNP(NADJNP,NUMNP,NPADJ)
      DIMENSION NADJNP(1600),NPADJ(1600,10)
C
C**** FORM VECTOR INDICATING THE NUMBER OF ADJACENT NODE POINTS
C      AT EACH NODE POINT
C
C      MXADJP=MAX. NO. OF ADJACENT NODE POINTS ALLOWED
C      NADJNP=NO. OF ADJACENT NODE POINTS AT EACH NODE POINT
C      NUMNP =NO. OF NODE POINTS
C      NPADJ =ADJACENT NODE POINT NUMBER
C      MXADJP=8
      DO 12 M=1,NUMNP
      DO 10 I=1,MXADJP
      J=I
      IF (NPADJ(M,I).EQ.0) GO TO 11
10  CONTINUE
      NADJNP(M)=MXADJP
      GC TO 12
11  NADJNP(M)=J-1
12  CONTINUE
      RETURN
C
      END
C
C
C
C
      SUBROUTINE MAX(A1,B1,A2,B2,AMAX,AMIN,BMIN,N)
      IF (A1-A2)10,20,20
10  AMAX=A2

```

```

      AMIN=A1
      BMIN=B1
      N=2
      RETURN
20    AMAX=A1
      AMIN=A2
      BMIN=B2
      N=1
      RETURN
      END
C
C
C
      SUBROUTINE INTERP(AMIN,AMAX,AINTER,SLOPE,UMIN,VMIN,UMA
      IX,VMAX,CISPL)
      IF(ABS(AMIN-AMAX).GT.0.01) GO TO 20
      FACTOR=0.0
      GO TO 30
20    FACTOR=(AINTER-AMIN)/(AMAX-AMIN)
30    UINTER=UMIN+FACTOR*(UMAX-UMIN)
      VINTER=VMIN + FACTOR*(VMAX-VMIN)
      ANGLE=ATAN(SLOPE)
      DISPL=UINTER*COS(ANGLE)+VINTER*SIN(ANGLE)
      RETURN
      END

```