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RESEARCH ON STRUCTURAL DYNAMIC TESTING
BY IMPEDANCE METHODS. VOLUME III.
FREE-BODY RESPONSE.

Alex Berman, et al

Kaman Aerospace Corporation

Prepared for:

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USAAMRDL TECHNICAL REPORT 72-63C
RESEARCH ON STRUCTURAL DYNAMIC
TESTING BY IMPEDANCE METHODS
VOLUME III
FREE-BODY RESPONSE

By

William G. Flannelly
Alex Berman
Nicholas Giansante

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EUSTIS DIRECTORATE
U. S. ARMY AIR MOBILITY RESEARCH AND DEVELOPMENT LABORATORY
FORT EUSTIS, VIRGINIA

CONTRACT DAAJ02-70-C-0012
KAMAN AEROSPACE CORPORATION
BLOOMFIELD, CONNECTICUT

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EUSTIS DIRECTORATE
FORT EUSTIS, VIRGINIA 23604

This program was conducted under Contract DAAJ02-70-C-0012 with Kaman Aerospace Corporation.

This report contains the theoretical derivation and the presentation of a methodology for system identification of structures. Computer experiments were run to verify this methodology.

The report has been reviewed by this Directorate and is considered to be technically sound. It is published for the exchange of information and the stimulation of future research.

This program was conducted under the technical management of Mr. Arthur J. Gustafson, Technology Applications Division.

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13. ABSTRACT A method is presented for determining free-body dynamic responses from data taken on a constrained structure. The method requires the matrix of complex amplitudes of the deflection at N points on the structure and the sinusoidal forces of excitation at frequency ω applied at M points on the structure. The forces of constraint are considered to be "applied" forces. There is no need for the force points to coincide with or be equal to the response points. The displacements can also, with no change in the analysis, represent displacements, linear or angular, in two or three directions at one geometrical point. Further, the method is not restricted to displacement response, and the same formulation is applicable to either velocity or acceleration response. A digital computer program was prepared for the IBM Model 360/40 computer using FORTRAN IV language to test the practicality of the subject theory. Computer experiments were conducted to test the sensitivity of the theory to measurement errors in the simulated test data and to numerically test the theory. The method was proven to be a practical means of predicting the free-body dynamic responses from simulated experimental data derived from a constrained structure. The theoretical development was shown to be numerically sound and insensitive to measurement errors.			

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RESEARCH ON STRUCTURAL DYNAMIC
TESTING BY IMPEDANCE METHODS

Volume III
Free-Body Response

Final Report

Kaman Report R-1001-3

By

Alex Berman
Nicholas Giansante
William G. Flannelly

Prepared by

Kaman Aerospace Corporation
Bloomfield, Connecticut

for

EUSTIS DIRECTORATE
U. S. ARMY AIR MOBILITY RESEARCH AND DEVELOPMENT LABORATORY
FORT EUSTIS, VIRGINIA

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FOREWORD

The work presented in this report was performed by Kaman Aerospace Corporation under Contract DAAJ02-70-C-0012 (Task 1F162204AA4301) for the Eustis Directorate, U. S. Army Air Mobility Research and Development Laboratory, Fort Eustis, Virginia. The program was implemented under the technical direction of Mr. Joseph H. McGarvey of the Reliability and Maintainability Division* and Mr. Arthur J. Gustafson of the Structures Division.** The report is presented in four volumes, each describing a separate phase of the basic theory of structural dynamic testing using impedance techniques.

Volume I presents the results of an analytical and numerical investigation of the practicality of system identification using fewer measurement points than there are degrees of freedom. The parameters in Lagrange's equations of motion, mass, stiffness, and damping for a mathematical model having fewer degrees of freedom than the linear elastic structure it represents may be determined directly from measured mobility data. Volume II describes the method of system identification wherein the necessary impedance data are experimentally determined by applying a force excitation at a single point on the structure. Volume III presents a method of determining the free-body dynamic responses from data obtained on a constrained structure. Volume IV describes a method of obtaining the equations for the combination of measured mobility matrices of a helicopter and its subsystems. The response of the combination of a helicopter and its subsystems is determined from data based on the experimental results of the main system and subsystems separately.

*Division name changed to Military Operations Technology Division.

**Division name changed to Technology Applications Division.

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LIST OF SYMBOLS

- $\{\bar{F}_j\}$ Vector of complex amplitude of applied force
[F] Matrix of applied force vectors
M Number of points at which force excitation is applied
N Number of points at which response measurements are made
 $\{\bar{y}_j\}$ Vector of complex amplitudes of deflection
[y] Matrix of complex amplitudes of deflection vectors
[Y] Complex displacement mobility matrix relating forces and responses (N×M)
 ω Frequency of applied sinusoidal forces

BRACKETS

- [] Matrix
↑↓ Diagonal matrix
{ } Column or row vector

SUPERSCRIPTS

- 1 Inverse

INTRODUCTION

The dynamic testing of helicopters, both full scale and model, is required to anticipate the response of the vehicle to the vibratory load spectrum to which it is subjected in flight. These responses were computed from an analytical model which is partly developed from, and verified by, data obtained from testing, such as resonant frequencies, resonant responses, and damping.

The uncertainties in the experimental determination of mode shapes and resonant frequencies for unconstrained structures relate to the inherent difficulties involved in supporting the structure so as to simulate free-body conditions. During vibration tests the model or full-scale vehicle must be supported in a manner to prevent interference with the response of the structure. In order to simulate the free-body boundary conditions of a helicopter in flight, the usual procedure has been to support the structure on a system which is relatively soft to negate the effect of rigid-body modes on the elastic modes of the helicopter. A commonly used technique consists of supporting the fuselage on cables and bungee attached to the hub. As helicopters become larger and heavier, it will be necessary to construct massive structures capable of supporting the total weight of the vehicle. This situation, which is expensive and requires substantial design effort, will continue to deteriorate. In addition, there is some uncertainty in the effects of the suspension dynamics and nonlinearities on the helicopter response.

A practical method which eliminates the need for soft suspensions in order to simulate in-flight boundary conditions will be of great help in the dynamic testing of helicopters. Such a method would yield better correlation between experimental and flight results and significantly reduce the cost of this testing.

The method described in this report has several desirable features. It uses the measured forces of constraint to convert the measured constrained helicopter responses to free-body responses. The structure being tested is considered to be supported on real supports, but the characteristics of the supports themselves are not required since only their measured reactions are used. The procedure uses only data which is actually measured, and no quantitative assumptions are employed.

THEORY

For sinusoidally varying forces at a frequency ω , applied at M points on a structure, a vector $\{\bar{f}_j\}$ can be defined which represents the complex amplitude of applied force at each of the points. Similarly, $\{\bar{y}_j\}$ is a vector representing the complex amplitudes of the deflection at each of N points resulting from the force vector $\{\bar{f}_j\}$. There is no necessity for the force excitation points to coincide with the points at which the deflection response is measured. The complex displacement mobility matrix, [Y], of order NxM represents the relationship between the applied forces and responses. The vectors $\{\bar{f}_j\}$, $\{\bar{y}_j\}$ and the matrix [Y] and the relationship among them are written

$$\{\bar{y}_j\} = \begin{Bmatrix} Y_1 \\ Y_2 \\ \vdots \\ Y_N \end{Bmatrix} \quad \{\bar{f}_j\} = \begin{Bmatrix} f_1 \\ f_2 \\ \vdots \\ f_M \end{Bmatrix} \quad [Y] = \begin{bmatrix} \frac{\partial y_1}{\partial f_1} & \frac{\partial y_1}{\partial f_2} & \dots & \frac{\partial y_1}{\partial f_M} \\ \frac{\partial y_2}{\partial f_1} & & & \\ \vdots & & & \\ \frac{\partial y_N}{\partial f_1} & \dots & \dots & \frac{\partial y_N}{\partial f_M} \end{bmatrix}$$

and

$$\{\bar{y}_j\} = [Y]\{\bar{f}_j\} \quad (1)$$

The previous expressions are not restricted to displacement response; exactly the same relationships apply for velocity and acceleration response. The displacements can also, with no modification in the analysis, represent displacements or rotations in two or three directions at one geometrical point by allotting one element in each vector for each generalized displacement. Similar considerations apply to the forces or moments. There is no necessity for the matrix, [Y], to be square; it will contain one row for each measured displacement, one column for each point at which an excitation is applied.

Consider a matrix, [F], containing several applied vectors and a matrix, [y], containing the corresponding deflections, as follows:

$$[F] = [\bar{f}_1 \mid \bar{f}_2 \mid \dots] \quad [y] = [\bar{y}_1 \mid \bar{y}_2 \mid \dots]$$

and then

$$[y] = [Y][F] \quad (2)$$

If [F] is a nonsingular matrix, then the desired result, the response of the points of interest to single forces, may be written

$$[Y] = [y][F]^{-1} \quad (3)$$

In Equation (3), the matrices [y] and [F] consist of measured data. When the externally applied loads only are included in [F], then [Y] is the mobility of the structure as tested on the actual supports. If [F] includes any of the forces of constraint, then [Y] is the mobility of the structure with those constraints removed. If all the forces of constraint are included in [F], then [Y] is the mobility of the unconstrained system or free body.

As indicated previously, [F] must be nonsingular and thus have an inverse. If there are M forces to be considered, including the forces at the constraints, then M sets of vectors of applied force, $\{\bar{F}_j\}$, must be applied and all of these vectors must be linearly independent. The independence criterion may be achieved by applying an external force at each constraint.

FORCES AT CONSTRAINTS

If an exciting force is applied at the j-th constraint, the force vector will be of the form

$$\{\bar{F}_j\} = \begin{pmatrix} f_{r_1} \\ f_{r_2} \\ \vdots \\ f_{r_j} + f \end{pmatrix} \quad (4)$$

where the f_{r_j} are the forces at the constraints due to the applied force f.

For an optimally designed support system, the maximum number of constraint forces is six and the [F] matrix relating the support forces will be of the following form:

$$[F_C] = \begin{bmatrix} f_{r_1} + f_1 & f_{r_1} & \dots & f_{r_1} \\ f_{r_2} & f_{r_2} + f_2 & & \vdots \\ \vdots & \vdots & & \vdots \\ f_{r_6} & f_{r_6} & & f_{r_6} + f_6 \end{bmatrix} \quad (5)$$

where the elements in each column represent the actual measurements taken for the force applied at each of the constraints. Because the loads are independent, the preceding matrix will always be nonsingular.

In conjunction with the force measurements, the displacements (or accelerations) are recorded at the points of interest on the structure and one column of $[y]$ is formed for each column of $[F_C]$.

Then, as in Equation (3),

$$[Y_{FB}] = [y][F_C]^{-1} \quad (6)$$

where $[Y_{FB}]$ represents the deflection of each point of interest on the structure due to each of the applied loads at the supports, and is the free-body mobility matrix. The $[F_C]$ matrix was obtained at one particular frequency, and the same procedure must be implemented over the frequency bandwidth of interest.

Normally it is required to determine the response of the helicopter due to force excitation at points other than the supports. Therefore, the structure must also be forced at these points and the forces at the constraints must be measured. If $[F_{CA}]$ is a matrix representing the forces of constraint for each point of excitation and $[F_A]$ is a matrix representing the applied loads, then the $[F]$ matrix becomes

$$[F] = \begin{bmatrix} [F_A] & \vdots & 0 \\ [F_{CA}] & \vdots & [F_C] \end{bmatrix} \quad (7)$$

However, if unit forces are applied, the matrix $[F_A]$ becomes the unit matrix and Equation (7) becomes

$$[F] = \begin{bmatrix} [I] & 0 \\ [F_{CA}] & [F_C] \end{bmatrix} \quad (8)$$

The inverse of this matrix is readily obtained and is given by

$$[F]^{-1} = \begin{bmatrix} [I] & 0 \\ [F_C]^{-1} & [F_{CA}] \end{bmatrix} \quad (9)$$

or by simply inverting the $[F]$ matrix in its entirety by numerical means.

ERROR ANALYSIS

The technique of structural dynamics testing utilizing impedance methods, to be of any practical engineering significance, must be functional with a reasonable degree of experimental error. Measurements of the complex amplitudes of displacement required to implement the method will be subject to experimental errors of various types. In general, all errors can be classified as either random or bias.

In the present analysis, the simulated test data were polluted with measurement error, and computer experiments were conducted to test the sensitivity of the method to error. The 20-degree-of-freedom "beam-type" representation of a helicopter and the 18-degree-of-freedom, three-dimensional mathematical model were analyzed incorporating measurement error. The simulated test data in the form of acceleration mobility and reaction force measurements were both polluted with a 5 percent bias error and a +5 percent random error on amplitude and a +1 degree error on phase angle. The random amplitude phase angle errors were assumed to be uniformly distributed. The resulting distribution of random error between the selected limits is conservative compared to limits applied to a normal distribution.

APPLICATION OF THE THEORY

The method has several attributes which make it especially suitable for practical application, including the use of only measured data, the lack of quantitative assumptions regarding the mass or stiffness distributions or the assumption of infinitely rigid supports. There are, however, as in all procedures, certain considerations involved in planning an efficient and accurate application of the method.

At each frequency it is necessary to conduct one test for each constraint. It is possible to constrain all rigid-body motions with six constraining forces. There is no necessity for such complete constraint, however. During the design of a test, test configurations should be considered which allow freedom of motion in the horizontal plane and around the vertical axis. Such an arrangement for a typical helicopter might consist of three supports at the base, each resisting only vertical motion and mounted on a device which allows unconstrained, but limited, motion in the horizontal plane. For sinusoidal force excitation at the rotor hub, it is necessary to measure the complex amplitude of the response of the points of interest on the structure as well as the forces induced at the constraint points. It is also necessary to apply vertical excitation at each support and measure each of the vertical forces of constraint. In addition, any other shaker position or orientation could be used while the three vertical forces are measured.

Theoretically, the stiffness characteristics of the supports are immaterial. However, these characteristics do affect the magnitudes of the forces and responses which will be measured. The performance of transducers such as accelerometers or load cells is dependent on the magnitude and frequency of the quantity being measured. Thus, for the most reliable results, the supports should be designed and the transducers selected for optimum operation.

A computer simulation of alternative test configurations can be an invaluable aid in the preliminary design stage of an actual test program. Prior to implementation of a test plan, a computer simulation applied to this method using an intuitive analytical model of the structure can be used to determine the sensitivity to error and the expected accuracy of the results of the various arrangements considered. It should include realistic experimental errors, approximate constraint characteristics and a frequency spectrum bounding the range of interest.

COMPUTER SIMULATION RESULTS

A computer simulation study was conducted to determine the practicality of the theory described in the subject report. The digital computer program listing in FORTRAN IV language and a description of the program input cards are presented in the appendix. Two basic configurations were analyzed, a 20-degree-of-freedom "beam-type" representation of a helicopter and an 18-degree-of-freedom, three-dimensional structure. Table I presents a description of the "beam-type" model including mass and stiffness distributions. The discrete masses, coordinate locations, member internal spring rates and ground springs are given in Table II for the 18-degree-of-freedom analytical model. For the models investigated, the simulated test data, acceleration mobility amplitude and reaction forces, were separately polluted with measurement errors of 5 percent bias error, +5 percent random error and +1 degree error on phase angle. Tables III and IV present a compilation of a portion of the computer experiments performed in implementing the theory presented in this report.

The effect of measurement error on the free-body real and imaginary acceleration frequency response for the 20-degree-of-freedom "beam-type" representation of a helicopter is shown in Figures 1 and 2, respectively. In each instance the exact curve represents the result of the analysis utilizing simulated experimental data with zero error. Figures 3 and 4 present similar data for the 18-degree-of-freedom, three-dimensional mathematical model. As evidenced by the aforementioned figures, the theoretical development presented herein is essentially insensitive to the degree of error incorporated in the analysis.

Figures 5 and 6 portray the results of implementation of the method using the measured forces of constraint to yield the free-body response of the 20-degree-of-freedom "beam-type" structure. Two simulated constrained modes are shown on the figures with the specimen supported on real supports of 10,000 lb/in. and 50,000 lb/in. spring rates, respectively. The free-body response of the structure achieved by deleting the effects of supports for both constrained boundary conditions is also presented. It is important to note that regardless of support system, application of the method yields identical unconstrained or free-body boundary conditions for the structure. Many computer experiments were conducted, including the effect of measurement error, to obtain the data presented in Figures 5 and 6. Comparison of the simulated "exact" free response and the response obtained incorporating realistic measurement error indicates the results

TABLE I. 20-DEGREE-OF-FREEDOM MODEL, "BEAM TYPE"

Sta No.	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
Sta (In.)	0	60	120	180	240	300	360	420	480	540	600	660	720	780	840	900	960	1020	1080	1140
Mass (Lb-Sec ² /In.)	1.05	3.67	2.18	2.18	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08	2.08
EI (Lb-In. ² x 10 ¹⁰)	.35	1.95	4.37	5.80	4.425	3.07	2.05	2.05	2.05	2.05	2.05	2.05	2.05	2.05	2.05	2.05	2.05	2.05	2.05	2.05
Springs to Ground (Lb/In.)		10,000		50,000																

TABLE II. THREE-DIMENSIONAL 18-DEGREE-OF-FREEDOM MODEL

Mass No.	Coordinate Numbers			Coordinate Locations*			Member Rate x 10 ⁻³			Internal Spring			Ground Springs Configuration		
	X	Y	Z	X	Y	Z	1	2	3	4	5	6	Coordinate	H-1	H-2
1	2	1	2	3	0	0	-50	-	200	100	100	100	4	1000	2000
2	2	4	5	6	100	-50	0	200	-	100	20	100	5	5000	2000
3	6	7	8	9	100	-50	-100	100	100	-	100	150	6	10,000	50,000
4	10	10	11	12	100	120	-120	100	20	100	-	50	13	1000	5000
5	3	13	14	15	100	50	0	100	100	150	50	-	15	10,000	20,000
6	1	16	17	18	300	20	-50	-	50	200	500	50	18	500	2000

* Note Conventional Right-Hand Coordinate System

TABLE III. 20-DEGREE-OF-FREEDOM "BEAM TYPE" CONFIGURATION

Case No.	Rand %	Bias Phase Deg	Seed	Rand %	Error - Force	Force	Phase	Seed	Spring	Constraint*	Applied Force**
No.	%	Deg	Seed	%	%	Deg	Deg	Seed	Lb/In.	Lb/In.	At Sta 1,6,10, and 18
1	0	0	0	0	0	0	0	0	10,000	10,000	10 100 10 100
2	5	1	4316	5	5	1	1	-	10,000	10,000	10 100 10 100
3	5	1	529591	5	5	1	1	-	10,000	10,000	10 100 10 100
4	5	1	75123	5	5	1	1	-	50,000	50,000	10 100 10 100
5	5	1	514272	5	5	1	1	-	50,000	50,000	10 100 10 100
6	5	1	7580101	5	5	1	1	-	50,000	50,000	10 100 10 100
7	5	1	124	5	5	1	1	-	50,000	10,000	10 100 10 100
8	5	1	16	5	5	1	1	-	100,000	10,000	10 100 10 100
9	5	1	893	5	5	1	1	-	10,000	10,000	10 100 10 100
10	5	1	11171	5	5	1	1	-	10,000	10,000	10 100 10 100
11	5	1	12	5	5	1	1	-	10,000	10,000	10 100 10 100
12	0	0	0	0	0	0	0	-	50,000	50,000	10 100 10 100

* Constraints located at Sta 6 and 18

** Applied forces at Sta 1, 6, 10 and 18

TABLE IV. THREE-DIMENSIONAL CONFIGURATION

Error on Force and Mobility							
Case No.	Results	Rand %	Bias %	Phase Deg	Seed	Constraints* Applied Forces**	
13	Conf H-1	0	0	0	-	1000 5000 10,000 1000 10,000	500
14	Conf H-1	0	0	0	-	1000 5000 10,000 1000 10,000	500
15	Conf H-1	5	5	1	243012	1000 5000 10,000 1000 10,000	50
16	Conf H-1	5	5	1	4911	1000 5000 10,000 1000 10,000	500
17	Conf H-2	0	0	0	-	2000 2000 50,000 5000 20,000	2000
18	Conf H-2	0	0	0	-	2000 2000 50,000 5000 20,000	2000
19	Conf H-2	5	5	1	821	2000 2000 50,000 5000 20,000	2000
20	Conf H-2	5	5	1	5632	2000 2000 50,000 5000 20,000	2000

* Constraint attachment numbers 4, 5, 6, 13, 15 and 18

**Applied forces at above stations 10, 10, 100, 10, 100 and 10 (pounds)

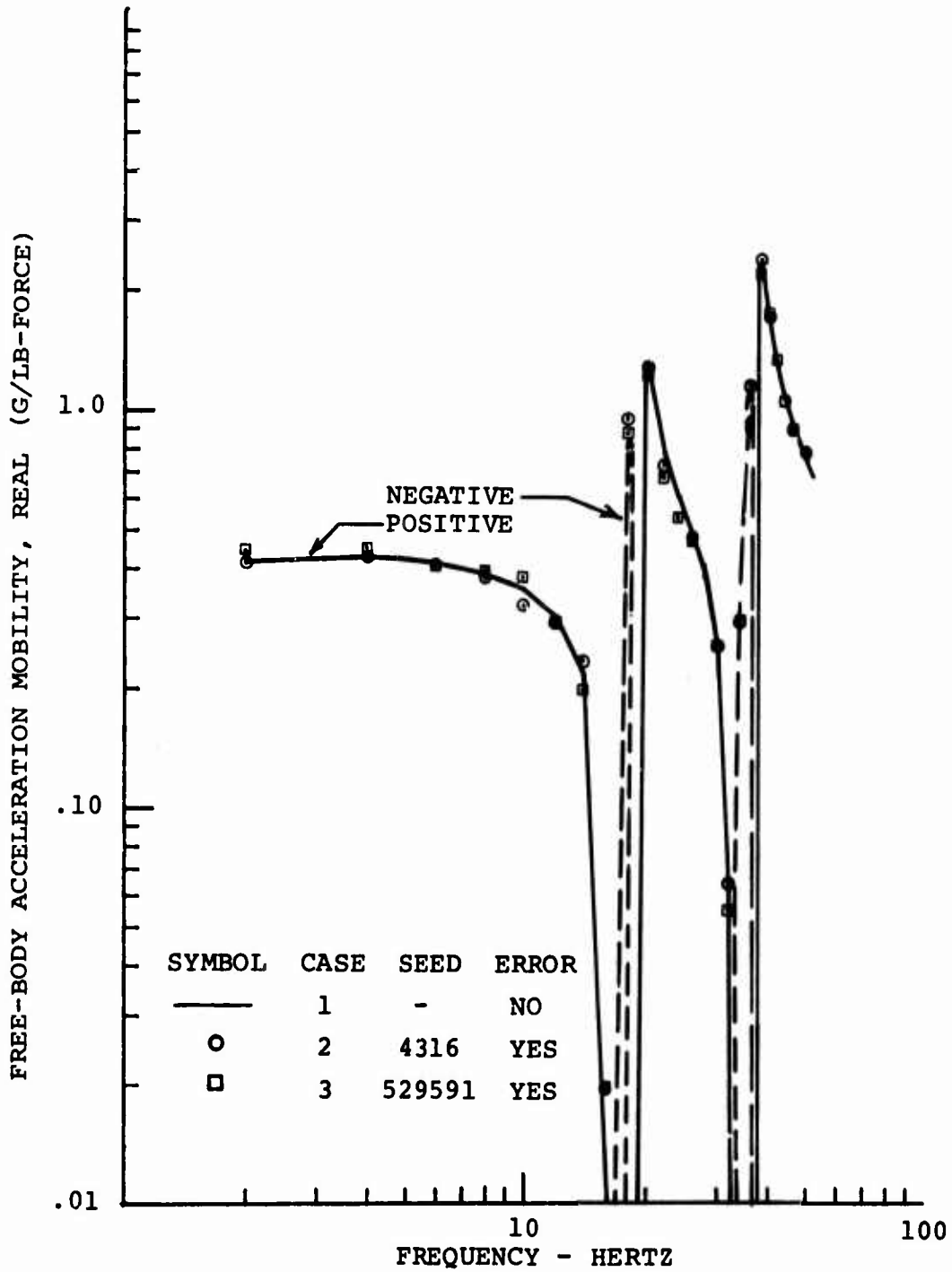


Figure 1. The Effect of Error on "Beam-Type" Model; Driving Point Real Acceleration Response.

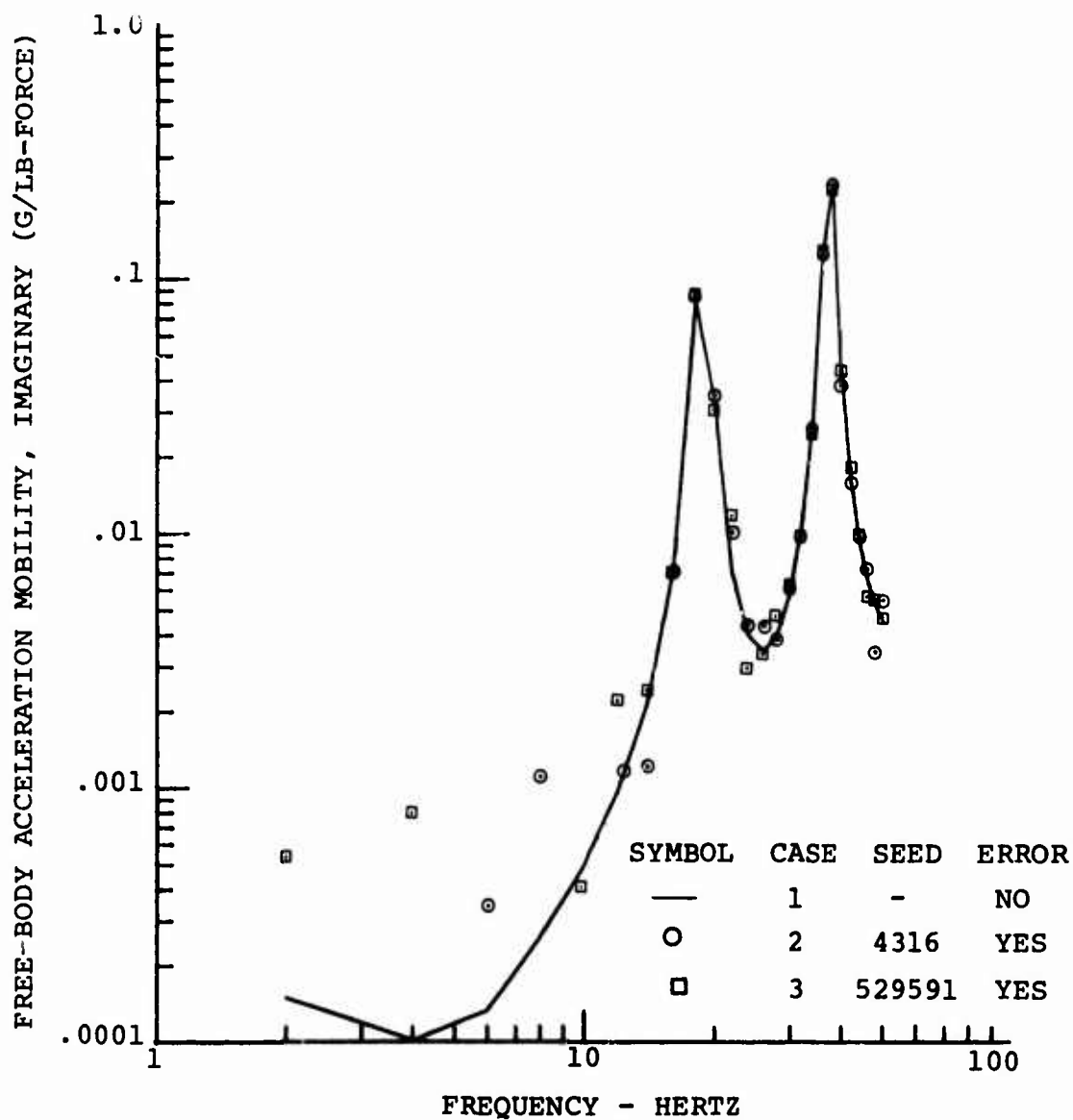


Figure 2. The Effect of Error on "Beam-Type" Model; Driving Point Imaginary Acceleration Response.

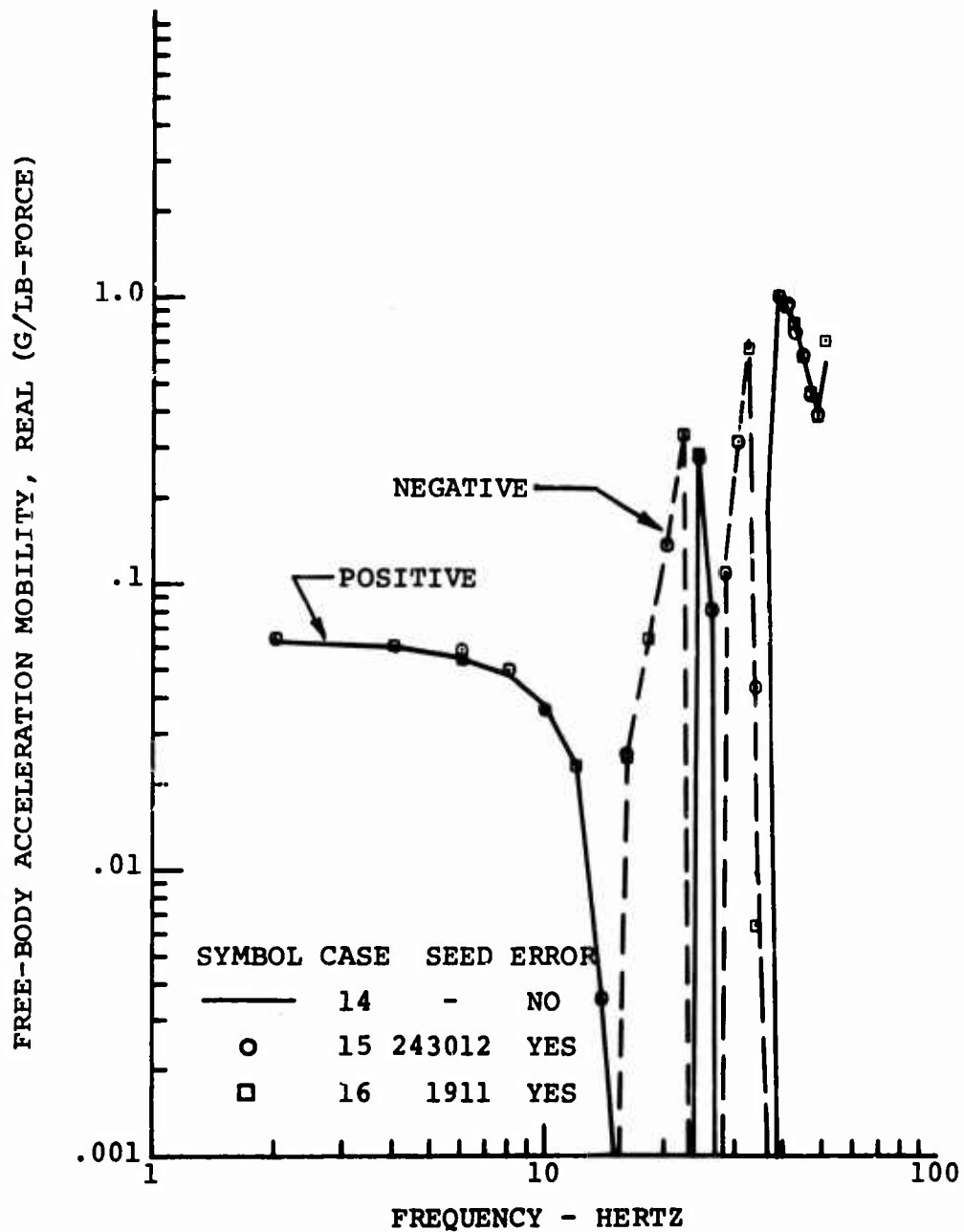


Figure 3. The Effect of Error on Three-Dimensional Model; Driving Point Real Acceleration Response.

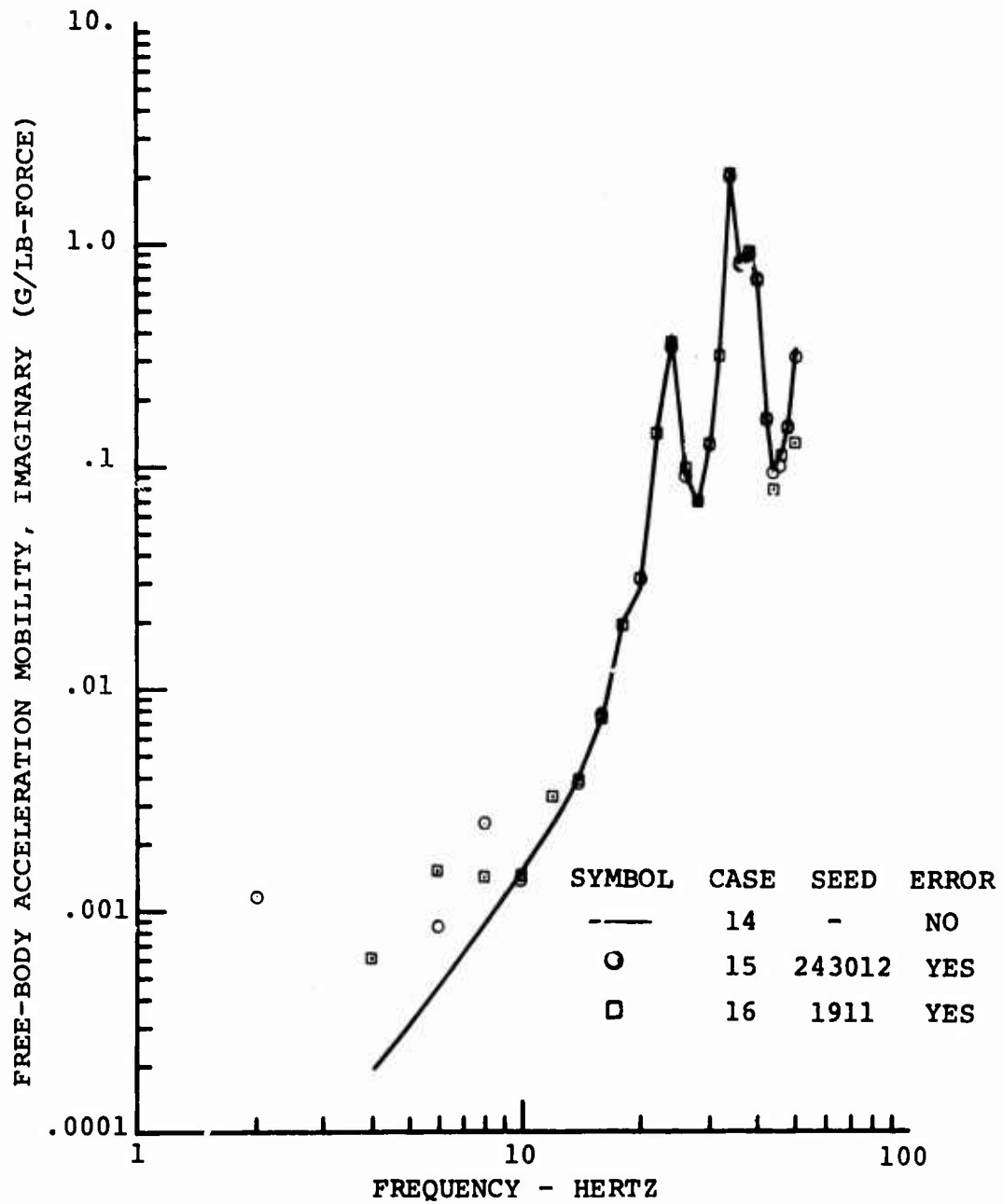


Figure 4. The Effect of Error on Three-Dimensional Model; Driving Point Imaginary Acceleration Response.

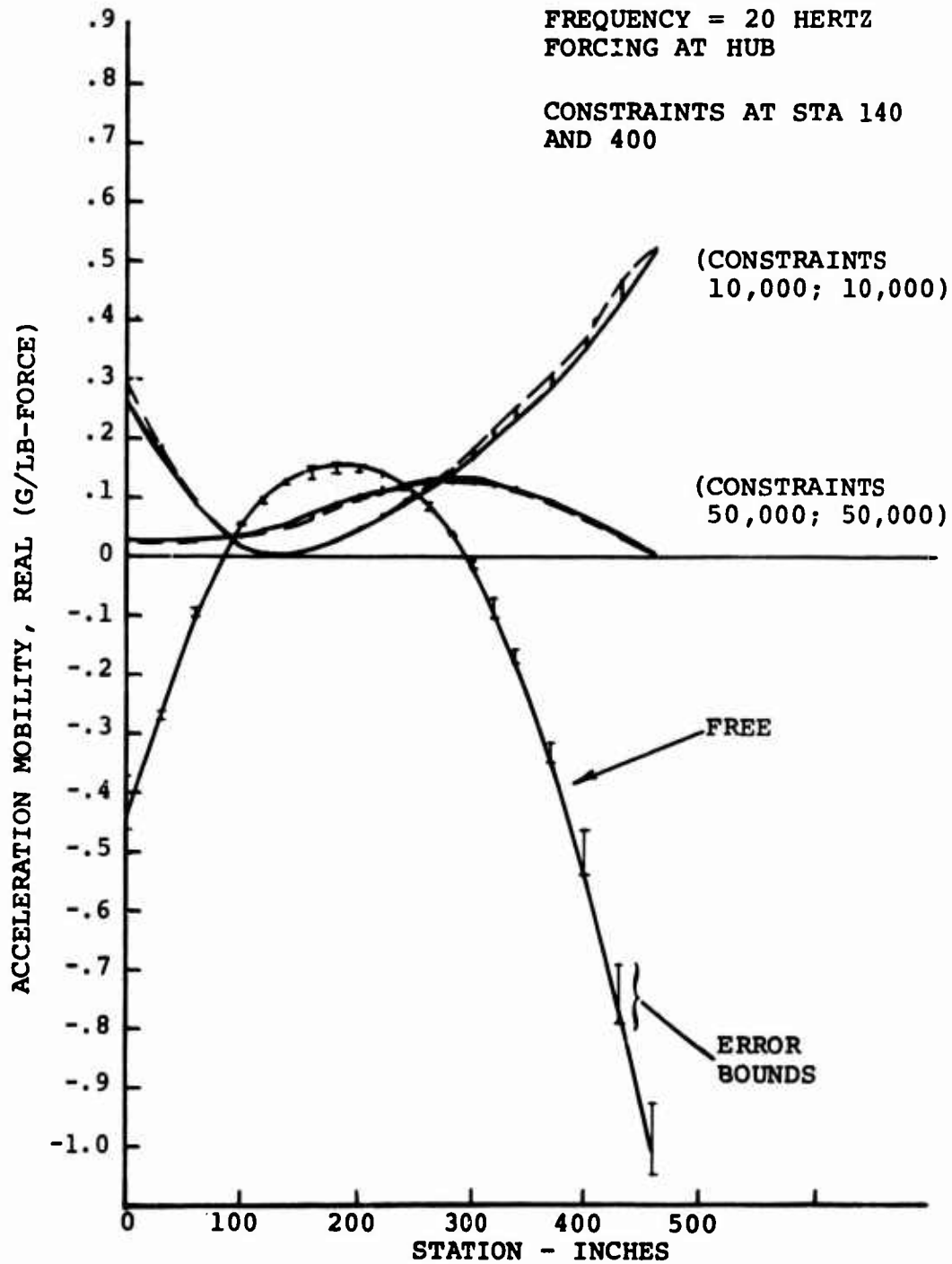


Figure 5. "Beam-Type" Model; Free-Body Real Acceleration Response.

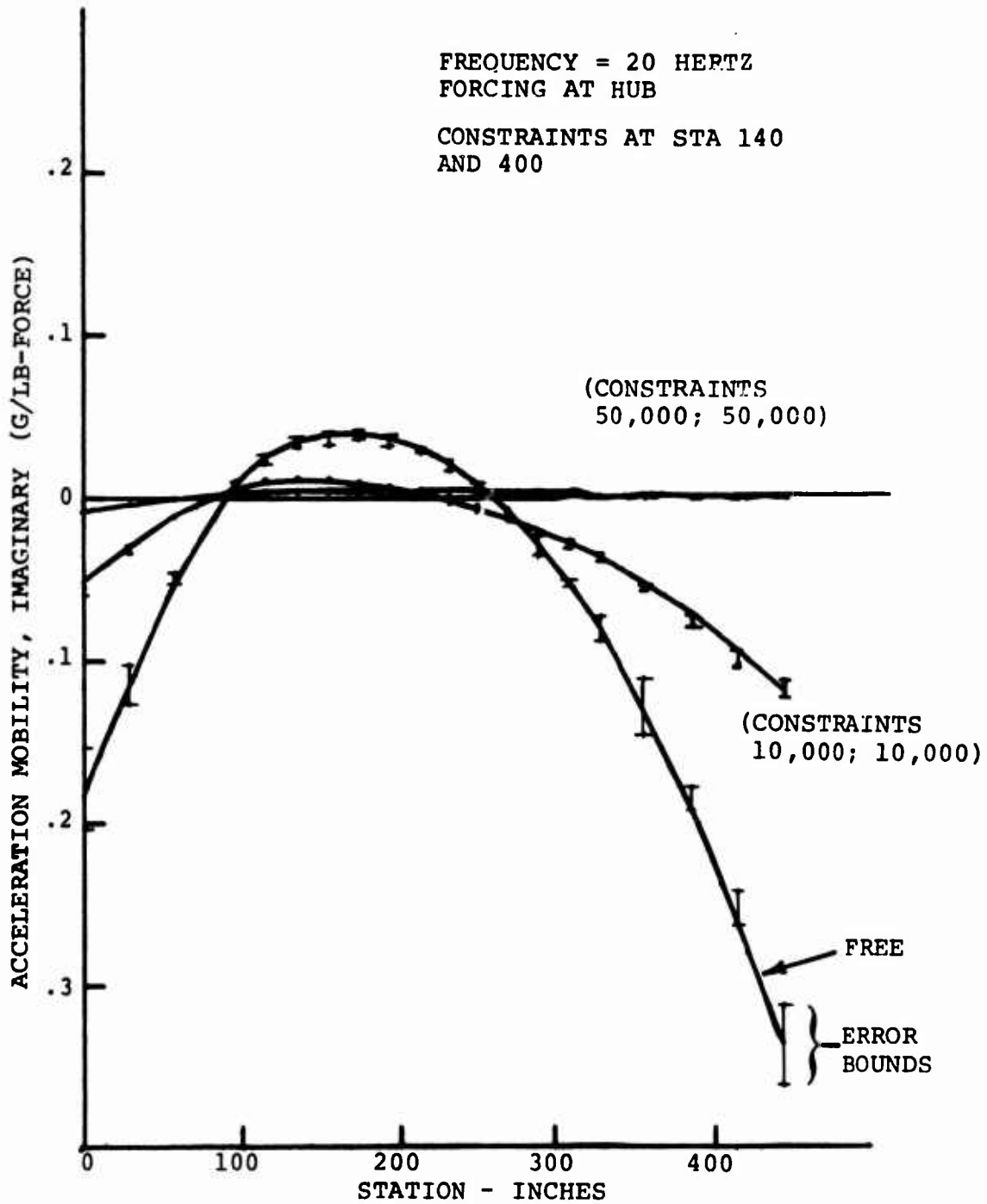


Figure 6. "Beam-Type" Model; Free-Body Imaginary Acceleration Response.

are effectively invariant with error and the method can be applied, with confidence, to yield the free-body characteristics of a structure.

The results of the investigation to determine the influence of measurement error on the methodology are shown in Figures 7, 8, 9, and 10. Figure 7 is a histogram obtained by considering the real acceleration mobility of a specific element on the constrained 20-degree-of-freedom "beam-type" model over the frequency range of interest. The percentage error was based on a comparison of the "exact" or zero error simulation and the conditions wherein measurement error was applied to acceleration mobility data and reaction force data. The solid line represents the theoretical cumulative frequency polygon and yields the probability of obtaining an error deviation below a specified value. Figure 8 shows similar data for the imaginary component of constrained acceleration mobility. Corresponding data for the free-body condition is given in Figures 9 and 10. Examining Figures 7, 8, 9, and 10 reveals an increase in error bandwidth accompanying removal of the system constraints. This is a consequence of polluting the reaction forces with measurement forces and the subsequent interaction with the acceleration mobility measurements also subjected to error. However, the errors are not prohibitively large and are within an acceptable range.

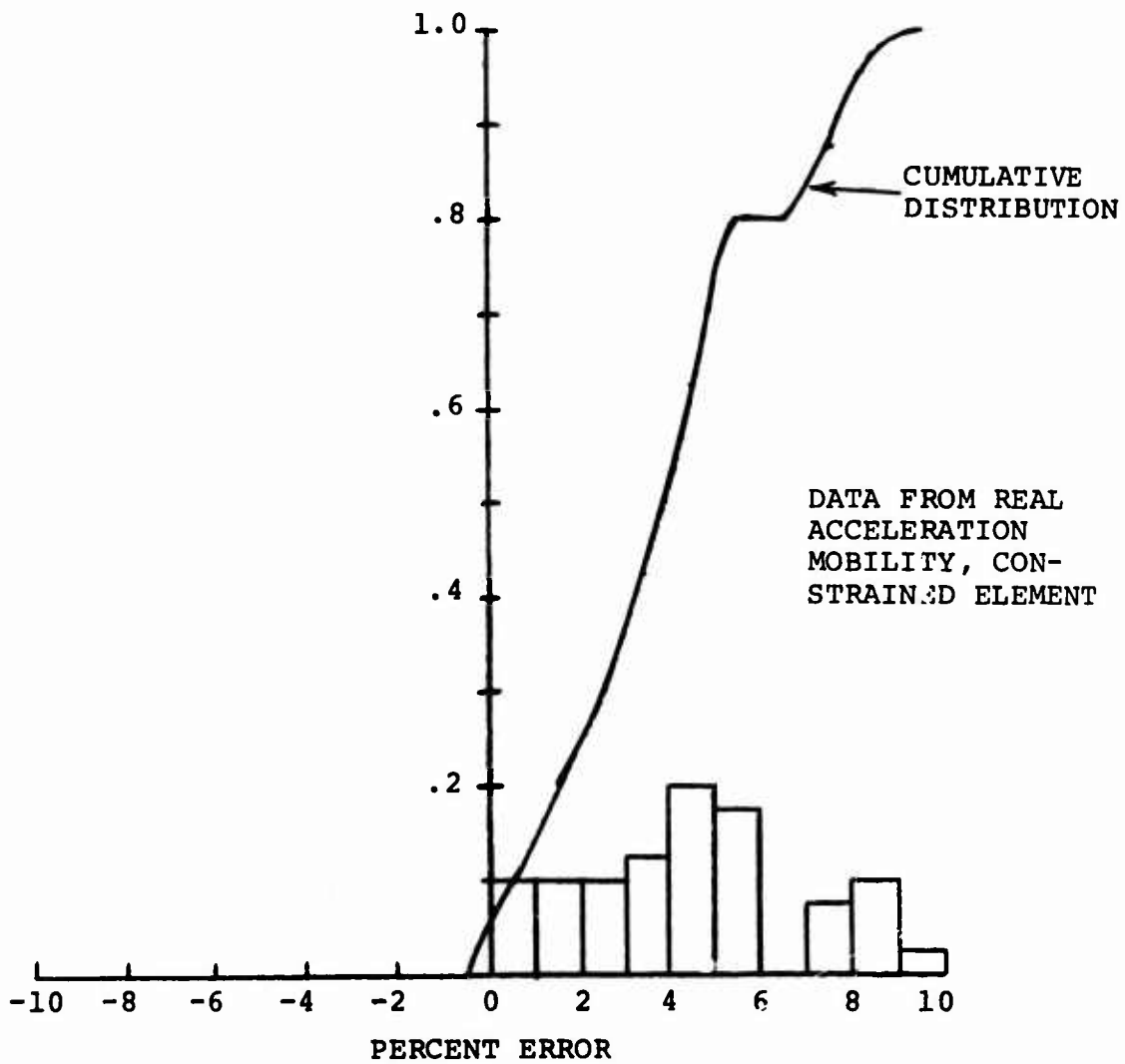


Figure 7. Error Distribution for Constrained Element Using Real Acceleration Mobility.

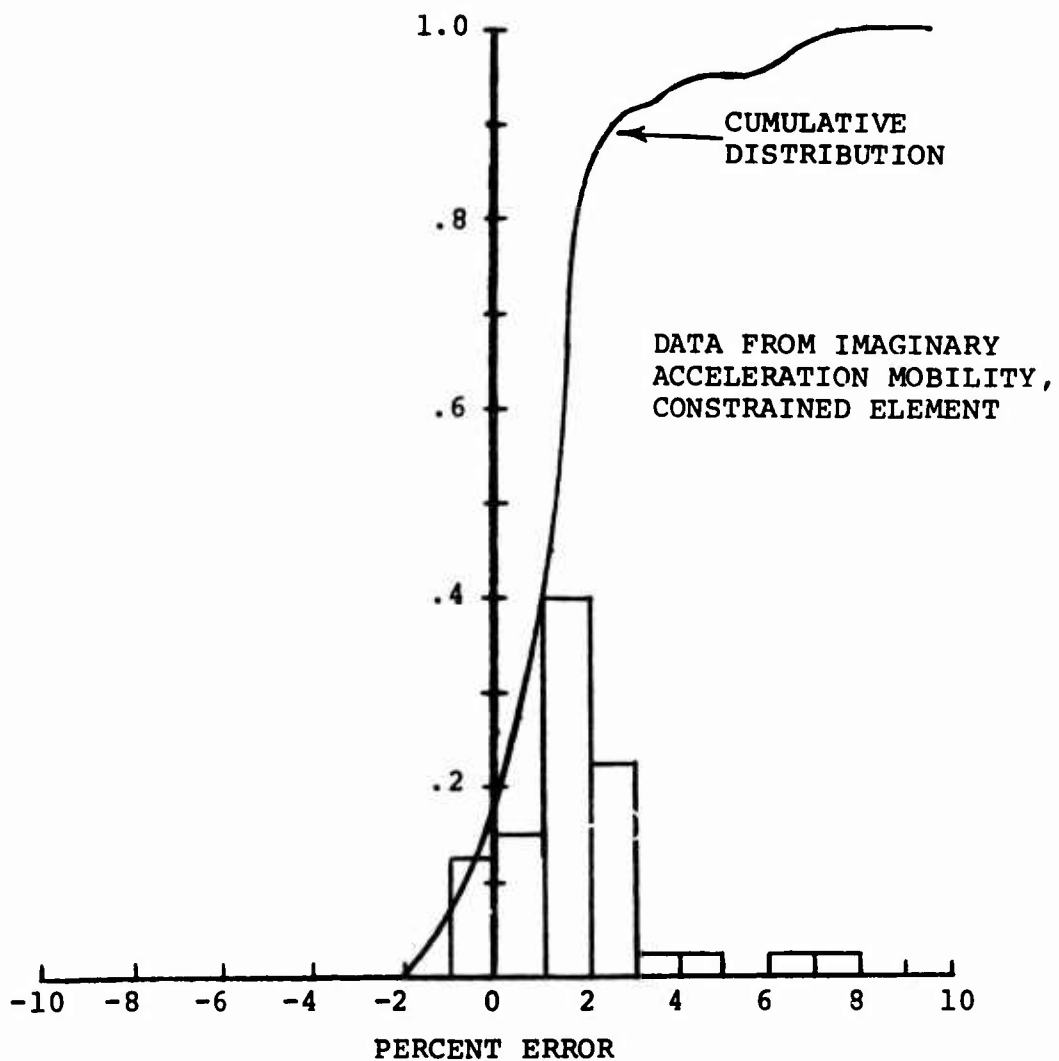


Figure 8. Error Distribution for Constrained Element Using Imaginary Acceleration Mobility.

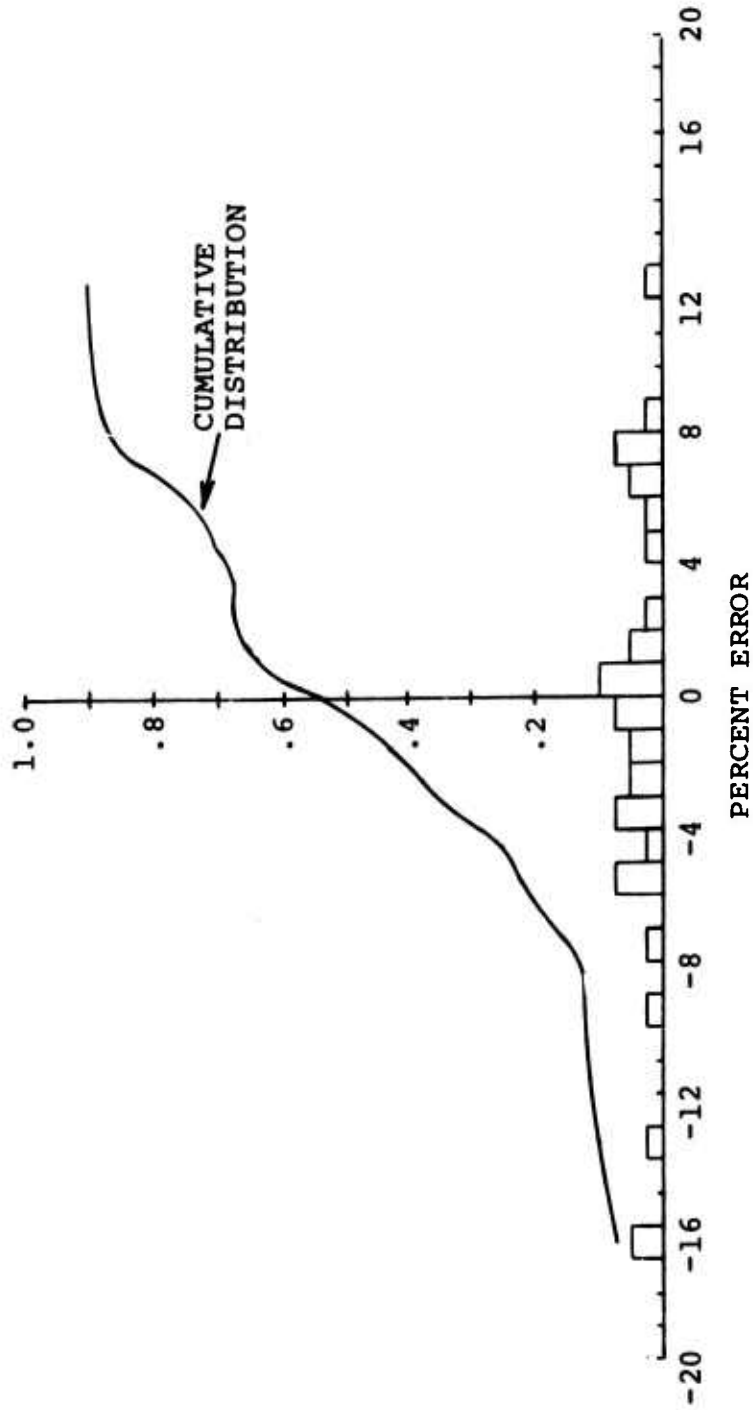


Figure 9. Error Distribution for "Beam-Type" Model Free Element Using Real Acceleration Mobility Data.

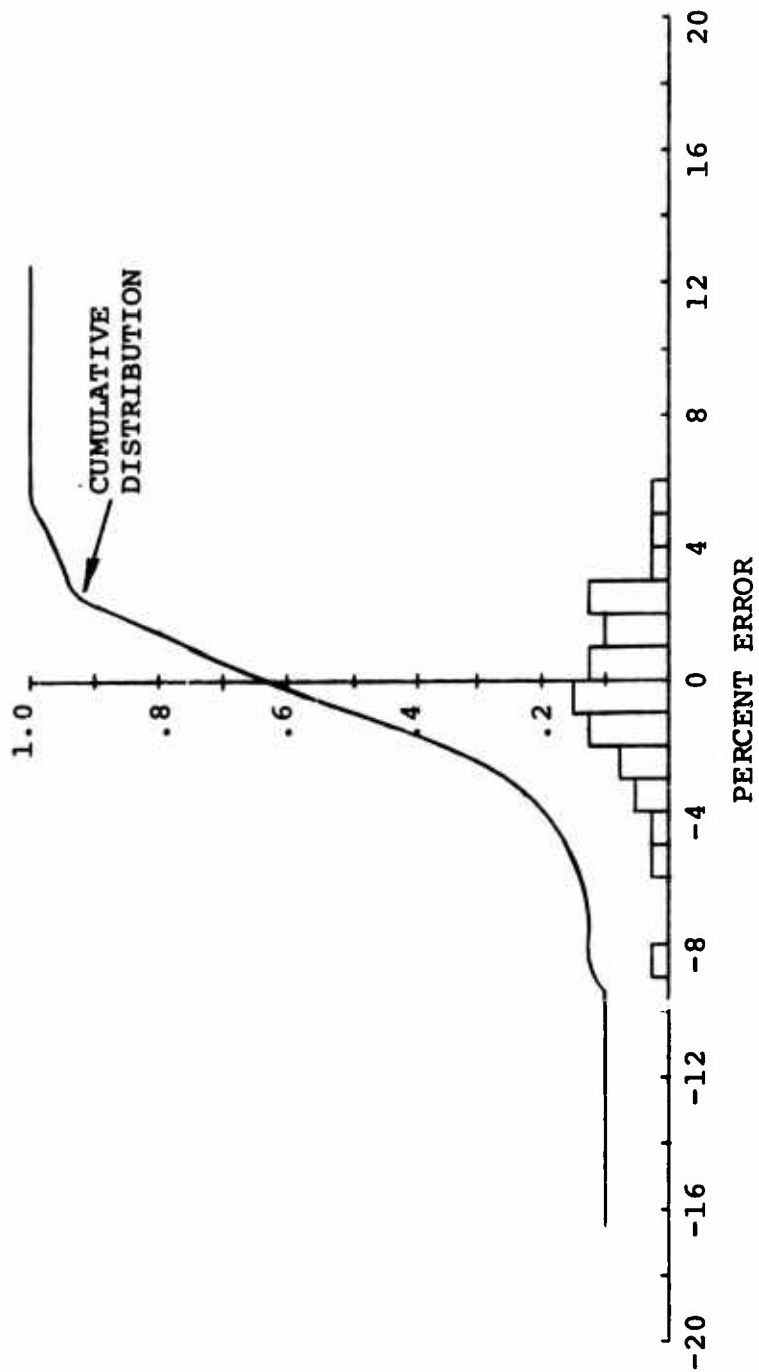


Figure 10. Error Distribution for "Beam-Type" Model Free Element Using Imaginary Acceleration Mobility Data.

CONCLUSIONS

1. The free-body dynamic response of a vehicle can be determined from experimental data taken on a constrained structure.
2. The methodology is insensitive to measurement error using simulated test data subjected to errors that are within the state of the measurement art.
3. The method can eliminate the need for soft suspension systems currently used in helicopter test techniques.
4. By eliminating the effect of the uncertain characteristics of the supporting structure, the accuracy and validity of all dynamic test results will be improved.
5. The method is inherently flexible and can be implemented using displacement, velocity or acceleration data, either linear or angular, in two or three directions at one geometrical point.
6. The theory was found to be experimentally practical and numerically sound.

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APPENDIX
COMPUTER PROGRAM

A digital computer program was generated to implement the theory presented in this report. The program was written for the IBM 360/40 operating system using FORTRAN IV language. A flow chart delineating the program logic is shown in Figure 11. A description of the input cards and a program source listing are included in this appendix.

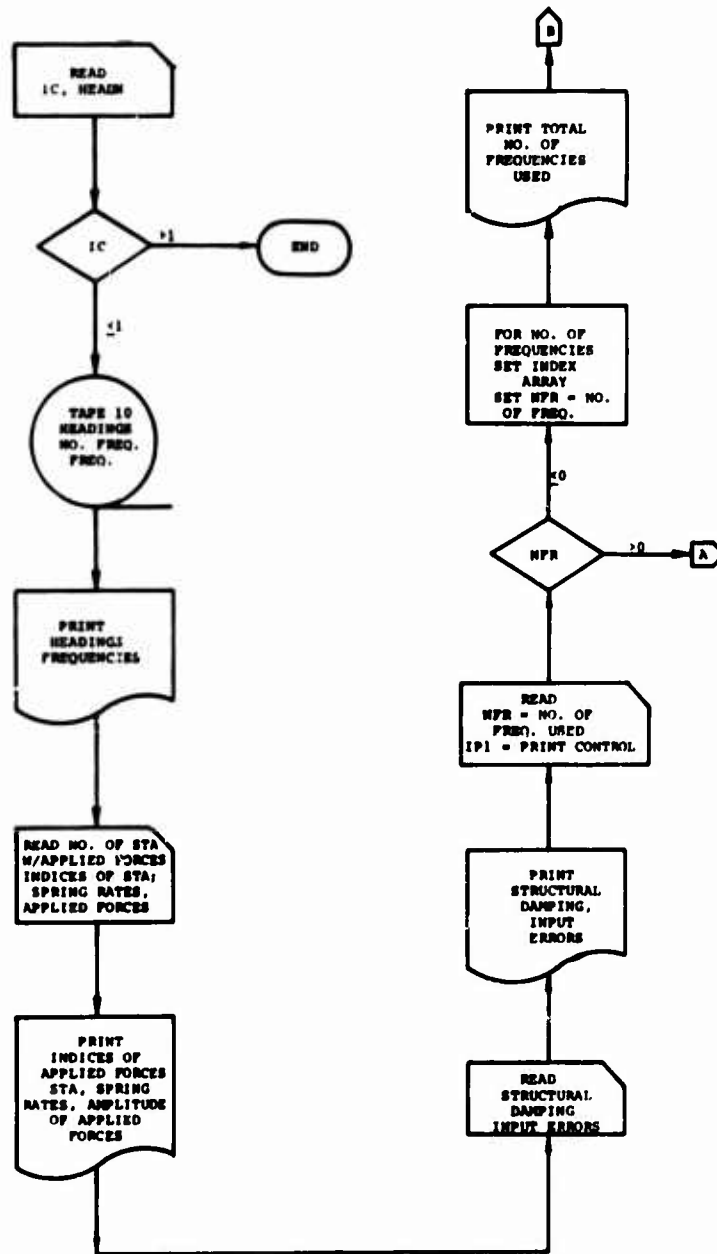


Figure 11. Computer Program Flow Chart.

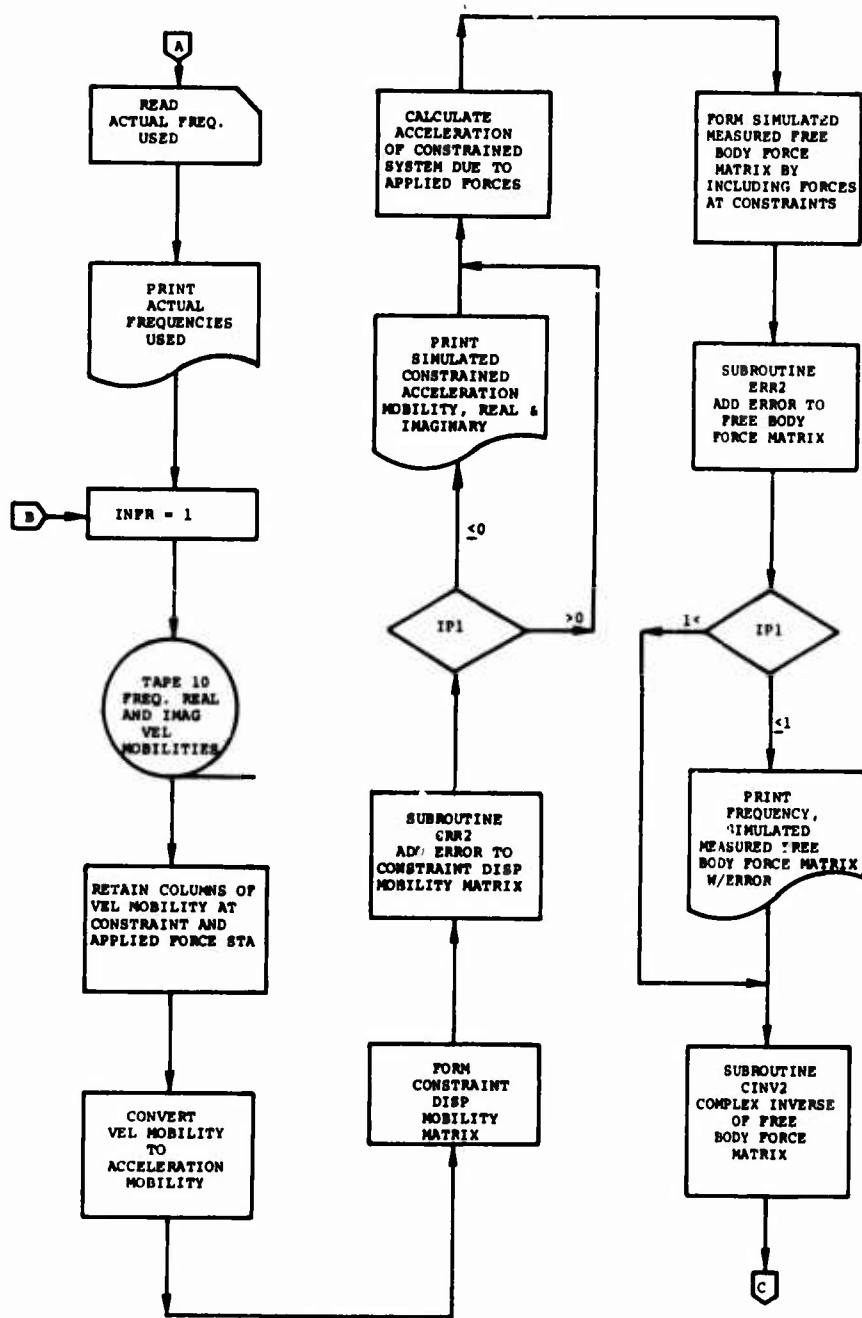


Figure 11 - Continued.

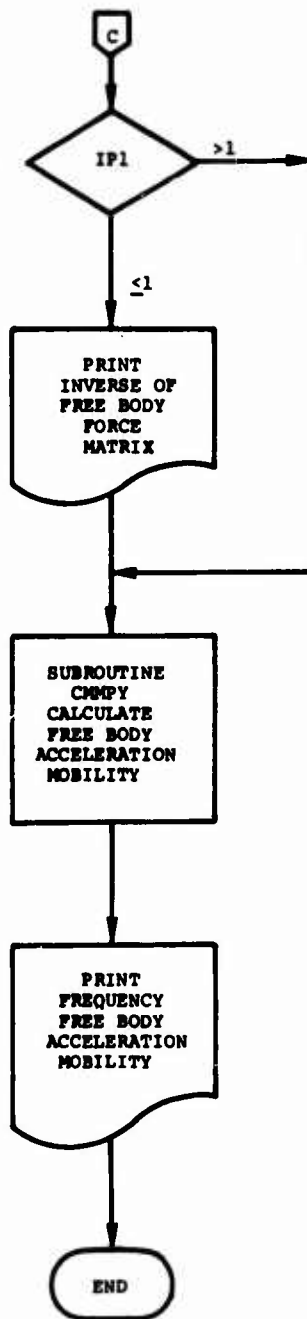


Figure 11 - Concluded.

DESCRIPTION OF INPUT CARDS

Note: All integer variables must be right justified with no decimal point.

Tape, Card Reader and Printer Assignments

- 1 Card Reader
- 3 Printer
- 10 Tape assignment. Contains mobility data, with zero error, for specified frequencies.

All input data must be in the following units:

Mass - lb-sec²/in.

Stiffness - lb/in.

Frequencies - Hz

PROGRAM FRESIM
FREE-BODY TEST METHOD SIMULATION

Card 1	Columns	1	IC	Program Control
		2-80	HEADN	Heading
Card 2		1-5	NS	No. of stations with applied force.
		5-10	INDS	The indices of the stations at which the forces are applied, including all constraint stations (FORMAT I5, 7I10, Maximum of 8).
Card 3		1-80	SPR	Spring rates at the above stations, if at constraints, otherwise zero (FORMAT 8F10.0, Maximum of 8).
Card 4		1-80	FAMP	Amplitude of applied forces at the above stations (FORMAT 8F10.0, Maximum of 8).
Card 5		1-10	G	Structural damping coefficient
		11-20	PCT	Random error applied to mobility amplitude, uniform between - and + PCT* element amplitude.
		21-30	PCTB	Bias error applied to mobility amplitude PCTB* element amplitude.
		31-40	PHE	Random error in degrees applied to mobility phase angle. Uniform between - PHE and + PHE.
		41-50	FPCT	Random error applied to force amplitude, uniform between - and + FPCT* Force amplitude.
		51-60	FPCTB	Bias error applied to force amplitude FPCTB* Force amplitude.
		61-70	FPHE	Random error in degrees applied to force phase angle. Uniform between - PHE and + PHE.
		71-80	IZ	Random number seed.

Card 6	Column	1-5	NFR	Number of frequencies to be used from Tape 10 (<u><</u> 100; if 0 use all frequencies on Tape 10).
		6-10	IPl	= 0 Print constrained displacement mobility; force matrix; free displacement mobility. = 1 Print force matrix; free displacement mobility. = 2 Print free displacement mobility.
Card 7		1-80	INDX	Indices of frequencies to be used from Tape 10. Indices must be in ascending order. Five columns per value, 16 values per card (FORMAT 16I5).

C	FREE BODY TEST METHOD SIMULATION	FRE	1
C		FRE	2
C	USES MOBILITY DATA FROM 'XACT'	FRE	3
C	SIMULATES SHAKING AT EACH SUPPORT TO OBTAIN FREE BODY	FRE	4
C		FRE	5
C		FRE	6
C		FRE	7
	INTEGER HEADN(20),HEAD(20),HT(7),INDS(8),INDX(100)	FRE	8
	REAL HZ(100),YR(20,21),Y:(20,21),SPK(8),FAMP(8),YDR(20,8),	FRE	9
	A YDI(20,8),CYDR(8,9),CYDI(8,9),FINVR(8,9),FINVI(8,9),	FRE	10
	B FYR(20,8),FYI(20,8)	FRE	11
C	INPUT HEADING, XACT TAPE HEADING	FRE	12
	100 READ (1,110) IC,HEADN	FRE	13
	110 FORMAT (11,A3,19A4)	FRE	14
	IF (IC.NE.0) REWIND 10	FRE	15
	IF (IC.GT.1) CALL EXIT	FRE	16
	READ (10) HT,HEAD,NF,ND,(HZ(I),I=1,NF)	FRE	17
	WRITE (3,120) HEADN,HT,HEAD,ND,(HZ(I),I=1,NF)	FRE	18
	120 FORMAT ('1'/T10,17(' FREE ')/T25,A3,19A4//T10,'TAPE HEADING' /	FRE	19
	A T25,7A4/T25,A3,19A4/T25,I2,' JEWELS OF FREEDOM'/T25,'FREQUENCIES	FRE	20
	B(HZ) ON TAPE'//T10,10F10.2))	FRE	21
C	INPUT SUPPORT SPRINGS AND APPLIED FORCES	FRE	22
	READ (1,130) NS,INDS,SPR,FAMP	FRE	23
	130 FORMAT (2I5,7I10/(8F10.0))	FRE	24
	WRITE (3,140) (INDS(I),I=1,NS)	FRE	25
	WRITE (3,150) (SPR(I),I=1,NS)	FRE	26
	WRITE (3,160) (FAMP(I),I=1,NS)	FRE	27
	140 FORMAT (/T10,'CONSTRAINTS AND FORCES'//T10,'STATION ',5X,8I10)	FRE	28
	150 FORMAT (T10,'SPRING',10X,8F10.0)	FRE	29
	160 FORMAT (T10,'APPLIED FORCE ',8F10.0)	FRE	30
C	INPUT ERRORS, FREQUENCIES	FRE	31
	READ (1,170) G,PCT,PCTB,PHE,FPLT,FPLTB,FPHE,IZ	FRE	32
	170 FORMAT (7F10.0,I10)	FRE	33
	WRITE (3,180) G	FRE	34
	180 FORMAT (/T10,'STRUCTURAL DAMPING',F6.4)	FRE	35
	IX=IZ*2+1	FRE	36
	WRITE (3,190) PCT,PCTB,PHE,IZ,FPLT,FPLTB,FPHE	FRE	37
	190 FORMAT (/T10,'MAX RAND ERROR =',F6.3,' BIAS ERROR =',F6.3,' OF E	FRE	38
	ALEMENTS, MAX RAND PHASE ERROR =',F5.2,' DEG. SEED =',I10/	FRE	39
	B T10,'FORCE ERRORS',4X,F6.3,15X,F6.3,37X,F5.2/)	FRE	40
	READ (1,200) NFR,IPI	FRE	41
	200 FORMAT (16I5)	FRE	42
	IF (NFR.GT.0) GO TO 230	FRE	43
	DO 210 I=1,NF	1FRE	44
	210 INDX(I)=I	1FRE	45
	NFR=NFR-I	FRE	46
	WRITE (3,220) NF	FRE	47
	220 FORMAT (/T10,'ALL ',I3,' FREQUENCIES USED')	FRE	48
	GO TO 250	FRE	49
	230 READ (1,200) (INDX(I),I=1,NFR)	FRE	50
	WRITE (3,240) (HZ(INDX(I)),I=1,NFR)	FRE	51
	240 FORMAT (/T10,'FREQUENCIES USED'//T10,10F12.4))	FRE	52
	250 INFR=I	FRE	53
C	START MAIN LOOP	FRE	54
	DO 410 L=1,NF	1FRE	55

	READ (10) FREQ, ((YR(I,J),YI(I,J),I=1,ND),J=1,ND)	1FRE	56
	IF(L.NE.INDX(INFR)) GO TO 410	1FRE	57
C	ELIMINATE COLUMNS AND CONVERT TO ACCL MOB	1FRE	58
	OMR=HZ(LI)*6.2832	1FRE	59
	OMRS=OMR*OMR	1FRE	60
	DO 260 I=1,ND	2FRE	61
	DO 260 J=1,NS	3FRE	62
	YDR(I,J)=-YI(I,INDS(J))*OMR	3FRE	63
260	YDI(I,J)=YR(I,INDS(J))*OMR	3FRE	64
C	FORM CONSTRAINT DISP MOB MATRIX AND ADD ERROR	1FRE	65
	DO 270 I=1,NS	2FRE	66
	DO 270 J=1,NS	3FRE	67
	CYDR(I,J)=-YDR(INDS(I),J)/OMRS	3FRE	68
270	CYDI(I,J)=-YDI(INDS(I),J)/OMRS	3FRE	69
	CALL ERR2 (YDR,YDI,PCT,PCTB,PHE,ND,NS,IX,20)	1FRE	70
	IF (IPL.GT.0) GO TO 300	1FRE	71
	WRITE (3,280) HZ(L)	1FRE	72
280	FORMAT ('1',T20,'SIMULATED MEASURED CONSTRAINED ACCELERATION MOBIL	1FRE	73
	ITY, REAL PART F = ',F10.2,' HZ'/)	1FRE	74
	CALL MOUT3 (YDR,ND,NS,20)	1FRE	75
	WRITE (3,290)	1FRE	76
290	FORMAT (//T50,'IMAGINARY PART'/)	1FRE	77
	CALL MOUT3 (YDI,ND,NS,20)	1FRE	78
C	ACCELERATIONS DUE TO FORCES AND FORCES AT CONSTRAINTS	1FRE	79
C	WITH ERRORS ON TOTAL FORCES	1FRE	80
300	DO 320 J=1,NS	2FRE	81
	F=FAMP(J)	2FRE	82
	DO 310 I=1,ND	3FRE	83
	YDR(I,J)=YDR(I,J)*F	3FRE	84
310	YDI(I,J)=YDI(I,J)*F	3FRE	85
	DO 320 I=1,NS	3FRE	86
	YYYY=(-CYDR(I,J)+G*CYDI(I,J))*F*SPK(I)	3FRE	87
	YYYYI=(-CYDI(I,J)-G*CYDR(I,J))*F*SPK(I)	3FRE	88
	CYDR(I,J)=YYYY	3FRE	89
320	CYDI(I,J)=YYYYI	3FRE	90
	DO 330 I=1,NS	2FRE	91
330	CYDR(I,I)=CYDR(I,I)+FAMP(I)	2FRE	92
	CALL ERR2 (CYDR,CYDI,FPCT,FPCTB,FPHE,NS,NS,IX,8)	1FRE	93
	IF (IPL.GT.1) GO TO 360	1FRE	94
	WRITE (3,340) HZ(L)	1FRE	95
340	FORMAT ('1',T20,'SIMULATED MEASURED FREE BODY FORCE MATRIX, REAL P	1FRE	96
	AART F = ',F10.2,' HZ'/)	1FRE	97
	CALL MOUT3 (CYDR,NS,NS,8)	1FRE	98
	WRITE (3,350)	1FRE	99
350	FORMAT (//T50,'IMAGINARY PART'/)	1FRE	100
	CALL MOUT3 (CYDI,NS,NS,8)	1FRE	101
C	INVERT FORCE MATRIX DIM 8 X 9	1FRE	102
360	CALL CINV2 (CYDR,CYDI,NS,FINVR,FINVI,8)	1FRE	103
	IF (IPL.GT.1) GO TO 380	1FRE	104
	WRITE (3,370)	1FRE	105
370	FORMAT (//T40,'INVERSE OF FORCE MATRIX, REAL PART'/)	1FRE	106
	CALL MOUT3 (FINVR,NS,NS,8)	1FRE	107
	WRITE (3,350)	1FRE	108
	CALL MOUT3 (FINVI,NS,NS,8)	1FRE	109
C	FREE BODY MOBILITY FY = YD * FINV	1FRE	110

380 CALL CMMPY (YDR,YDI,FINVR,FINVI,ND,NS,NS,FYR,FYI,20,8,20)	1FRE 111
WRITE (3,390) HZ(L),(INDS(I),I=1,NS)	1FRE 112
390 FORMAT ('1',T20,'FREE BODY ACC M ₀₀ MATRIX, REAL PART, F =',F10.2	1FRE 113
A , ' HZ'/T5,10I12)	1FRE 114
CALL MOUT3 (FYR,ND,NS,20)	1FRE 115
WRITE (3,290)	1FRE 116
CALL MOUT3 (FYI,ND,NS,20)	1FRE 117
400 INFR=INFR+1	1FRE 118
IF (INFR.GT.NFR) GO TO 100	1FRE 119
410 CONTINUE	1FRE 120
REWIND 12	FRE 121
GO TO 100	FRE 122
END	FRE 123

	SUBROUTINE CMMPY (A,B,C,D,N1,N2,N3,E,F,NRA,NRC,NRE)	CMY	1
C		CMY	2
C	COMPLEX MATRIX MULT OBJECT TIME DIMENSIONS	CMY	3
C	$E + I*F = (A + I*B)*(C + I*D) \quad I = \text{SQRT}(-1)$	CMY	4
C	NRA IS NO OF ROWS IN DIMENSION OF A,B	CMY	5
C	NRC IS NO OF ROWS IN DIMENSION OF C,D	CMY	6
C	NRE IS NO OF ROWS IN DIMENSION OF E,F	CMY	7
C	A,B ARE N1 X N2 C,D ARE N2 X N3 E,F ARE N1 X N3	CMY	8
C		CMY	9
	REAL A(NRA,1),B(NRA,1),C(NRC,1),D(NRC,1),E(NRE,1),F(NRE,1),	CMY	10
	A G(20,20)	CMY	11
	CALL MPPY2 (A,C,N1,N2,N3,E,NRA,NRC,NRE)	CMY	12
	CALL MPPY2 (B,D,N1,N2,N3,G,NRA,NRC,20)	CMY	13
	DO 100 I=1,N1	1CMY	14
	DO 100 J=1,N3	2CMY	15
100	E(I,J)=E(I,J)-G(I,J)	2CMY	16
	CALL MPPY2 (A,D,N1,N2,N3,F,NRA,NRC,NRE)	CMY	17
	CALL MPPY2 (B,C,N1,N2,N3,G,NRA,NRC,20)	CMY	18
	DO 110 I=1,N1	1CMY	19
	DO 110 J=1,N3	2CMY	20
110	F(I,J)=F(I,J)+G(I,J)	2CMY	21
	RETURN	CMY	22
	END	CMY	23

C	SUBROUTINE CINV2 (A,B,N,C,D,NR)	CIN	1
C	COMPLEX INVERSE C + I*D = INV OF A + I*B	CIN	2
C	OBJECT TIME DIMENSIONS NR IS NO OF ROWS IN DIM. OF A,B,C,D	CIN	3
C	THEY MUST BE DIMENSIONED WITH EXTRA COLUMN IN CALLING PROG.	CIN	4
C		CIN	5
C	A IS ASSUMED NON-SINGULAR	CIN	6
C	USES INVRS2, MPPY2	CIN	7
C		CIN	8
		CIN	9
	REAL A(NR,1),B(NR,1),C(NR,1),DIM(N,1),E(20,21):	CIN	10
	CALL INVRS2 (C,N,A,NR)	CIN	11
	CALL MPPY2 (C,B,N,N,N,E,NR,NR,20)	CIN	12
	CALL MPPY2 (B,E,N,N,N,D,NR,20,NR)	CIN	13
	DO 100 I=1,N	1CIN	14
	DO 100 J=1,N	2CIN	15
	E(I,J)=-E(I,J)	2CIN	16
100	D(I,J)=D(I,J)+A(I,J)	2CIN	17
	CALL INVRS2 (C,N,D,NR)	CIN	18
	CALL MPPY2 (E,C,N,N,N,D,20,NR,NR)	CIN	19
	RETURN	CIN	20
	END	CIN	21

C	SUBROUTINE MMPY2 (A,B,N1,N2,N3,C,NRA,NRB,NRC)	MY2	1
C	OBJECT TIME DIMENSIONS NRA IS NU OF ROWS IN A IN DIMENSION	MY2	2
C	STATEMENT IN MAIN PROG. ETC FOR NRB, NRC.	MY2	3
C	C = A * B	MY2	4
C	A (N1 X N2) B (N2 X N3) C (N1 X N3)	MY2	5
		MY2	6
	REAL A(NRA,1),B(NRB,1),C(NRC,1)	MY2	7
	DO 100 I=1,N1	1MY2	8
	DO 100 J=1,N3	2MY2	9
	C(I,J)=0.	2MY2	10
	DO 100 K=1,N2	3MY2	11
100	C(I,J)=C(I,J)+A(I,K)*B(K,J)	3MY2	12
	RETURN	MY2	13
	END	MY2	14

```
C SUBROUTINE RANDU (IX,IY,YFL)
      THIS SUBROUTINE IS FROM SSP VERS. II
      IY=IX*65539
      IF(IY)100,110,110
100 IY=IY+2147483647+1
110 YFL=IY
      YFL=YFL*.4656613E-9
      RETURN
      END
```

```
RAN 1
RAN 2
RAN 3
RAN 4
RAN 5
RAN 6
RAN 7
RAN 8
RAN 9
```

C	SUBROUTINE ERR2 (A,B,PCT,PCTB,PHE,N1,N2,IX,NR)	ERR	1
C	OBJECT TIME DIMENSIONS	ERR	2
C	EACH ELEMENT OF A COMPLEX MATRIX, $A + I \cdot B$, IS MODIFIED TO	ERR	3
C	INCLUDE A SMALL PHASE ERROR, PNE (DEG), A BIAS ERROR,	ERR	4
C	PCTB (RATIO) ON AMPLITUDE, AND A UNIFORM RANDOM ERROR	ERR	5
C	HAVING A +/- MAXIMUM OF PCT (RATIO) ON AMPLITUDE.	ERR	6
C	THE PHASE ERROR IS ALSO RANDOMLY DISTRIBUTED	ERR	7
C	TE NO SYMMETRIZATION IS PERFORMED	ERR	8
C		ERR	9
C	USES RANDU	ERR	10
	DIMENSION A(NR,1),B(NR,1)	ERR	11
	IF(PCT) 120,100,120	ERR	12
100	IF(PCTB) 120,110,120	ERR	13
110	IF(PHE) 120,140,120	ERR	14
120	P=PHE/57.296	ERR	15
	DO 130 I=1,N1	1ERR	16
	DO 130 J=1,N2	2ERR	17
	CALL RANDU (IX,IY,YFL)	2ERR	18
	IX=IY	2ERR	19
	E=2.0*P*(YFL-0.5)	2ERR	20
	A1=A(I,J)-E*B(I,J)	2ERR	21
	B(I,J)=B(I,J)+E*A(I,J)	2ERR	22
	A(I,J)=A1	2ERR	23
	CALL RANDU (IX,IY,YFL)	2ERR	24
	IX=IY	2ERR	25
	E=1.0+2.0*PCT*(YFL-0.5)+PCTB	2ERR	26
	A(I,J)=A(I,J)*E	2ERR	27
130	B(I,J)=B(I,J)*E	2ERR	28
140	RETURN	ERR	29
	END	ERR	30

	SUBROUTINE INVRS2 (A,N,B,NR)	INV	1
C	OBJECT TIME DIMENSIONS. ALWAYS SHOULD BE DIMENSIONED	INV	2
C	WITH ONE MORE COLUMN THAN ROW. NR SHOULD BE .LE. 20	INV	3
C	A = INVERSE OF B B UNDISTURBED	INV	4
C		INV	5
	DIMENSION A(NR,1),D(20,21),IROW(21),ICOL(21),B(NR,1)	INV	6
	DO 100 I=1,N	1INV	7
	DO 100 J=1,N	2INV	8
100	A(I,J)=B(I,J)	2INV	9
	M=N+1	INV	10
	DO 110 I=1,N	1INV	11
	IROW(I)=I	1INV	12
110	ICOL(I)=I	1INV	13
	DO 260 K=1,N	1INV	14
	AMAX= A(K,K)	1INV	15
	DO 130 I=K,N	2INV	16
	DO 130 J=K,N	3INV	17
	IF(ABS(A(I,J))-ABS(AMAX))130,120,120	3INV	18
120	AMAX= A(I,J)	3INV	19
	IC=I	3INV	20
	JC=J	3INV	21
130	CONTINUE	3INV	22
	KI=ICOL(K)	1INV	23
	ICOL(K)=ICOL(KI)	1INV	24
	ICOL(KI)=KI	1INV	25
	KI=IROW(K)	1INV	26
	IROW(K)=IROW(KI)	1INV	27
	IROW(KI)=K	1INV	28
	IF(AMAX) 160,140,160	1INV	29
140	WRITE (3,150)	1INV	30
150	FORMAT(' SOLUTION OF EXISTING MATRIX NOT POSSIBLE')	1INV	31
	GO TO 330	1INV	32
160	DO 170 J=1,N	2INV	33
	E=A(K,J)	2INV	34
	A(K,J)=A(KI,J)	2INV	35
170	A(KI,J)=E	2INV	36
	DO 180 I=1,N	2INV	37
	E=A(I,K)	2INV	38
	A(I,K)=A(I,KI)	2INV	39
180	A(I,KI)=E	2INV	40
	DO 210 I=1,N	2INV	41
	IF(I-K) 200,190,200	2INV	42
190	A(I,M)=1.	2INV	43
	GO TO 210	2INV	44
200	A(I,M)=0.	2INV	45
210	CONTINUE	2INV	46
	PVT=A(K,K)	1INV	47
	DO 220 J=1,M	2INV	48
220	A(K,J)=A(K,J)/PVT	2INV	49
	DO 250 I=1,N	2INV	50
	IF(I-K) 230,250,230	2INV	51
230	AMULT=A(I,K)	2INV	52
	DO 240 J=1,M	3INV	53
240	A(I,J)=A(I,J)-AMULT*A(K,J)	3INV	54
250	CONTINUE	2INV	55

```

      DO 260 I=1,N
260  A(I,K)=A(I,M)
      DO 250 I=1,N
      DO 270 L=1,N
      IF(IROW(I)-L)270,280,270
270  CONTINUE
280  DO 290 J=1,N
290  D(L,J)=A(I,J)
      DO 320 J=1,N
      DO 300 L=1,N
      IF(ICOL(J)-L) 300,310,300
300  CONTINUE
310  DO 320 I=1,N
320  A(I,L)=D(I,J)
330  RETURN
      END

```

```

2INV 56
2INV 57
1INV 58
2INV 59
2INV 60
2INV 61
2INV 62
2INV 63
1INV 64
2INV 65
2INV 66
2INV 67
2INV 68
2INV 69
1INV 70
1INV 71

```