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CONDITION SURVEY, HUNTER ARMY AIRFIELD  
SAVANNAH, GEORGIA

R. D. Jackson, et al

Army Engineer Waterways Experiment Station  
Vicksburg, Mississippi

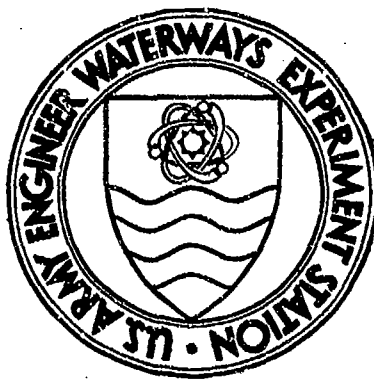
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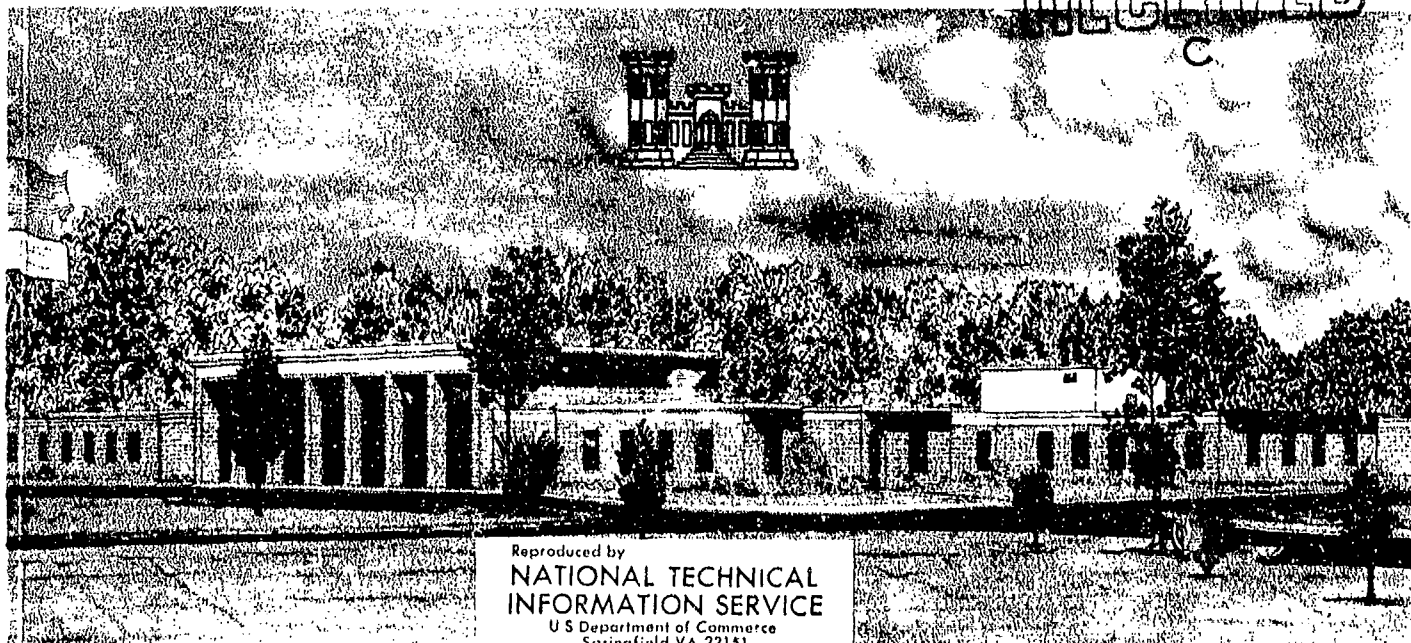
MISCELLANEOUS PAPER S-72-8

# CONDITION SURVEY, HUNTER ARMY AIRFIELD SAVANNAH, GEORGIA

by

R. D. Jackson, P. J. Vedros

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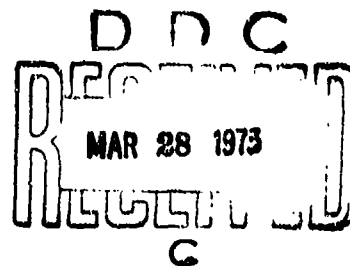


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## Foreword

Authority for performance of condition surveys at selected airfields is contained in Long Range Program, O&M, A FY 1971, Project Q6-1: "Engineering Criteria for Design and Construction - WES," dated May 1970.

The facilities at Hunter Army Airfield were inspected in March 1971 by Messrs. R. D. Jackson and S. J. Alford of the Flexible Pavement Branch, U. S. Army Engineer Waterways Experiment Station (WES). This report was prepared by Messrs. Jackson and P. J. Vedros under the general supervision of Messrs. J. P. Sale, R. G. Ahlvin, and R. L. Hutchinson of the Soils Division, WES.

COL Ernest D. Peixotto, CE, was Director of the WES during the conduct of the study and preparation of the report. Mr. F. R. Brown was Technical Director.

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Conversion Factors, British to Metric Units of Measurements

British units of measurement used in this report can be converted to metric units as follows:

<u>Multiply</u>	<u>By</u>	<u>To Obtain</u>
inches	2.54	centimeters
feet	0.3048	meters
square inches	6.4516	square centimeters
pounds	0.45359237	kilograms
pounds per square inch	0.6894757	newtons per square centimeter

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CONDITION SURVEY, HUNTER ARMY AIRFIELD  
SAVANNAH, GEORGIA

Purpose

1. The purpose of this report is to present the results of an inspection performed at Hunter Army Airfield (HAAF) in March 1971. The inspection was limited to visual observations, and no tests were conducted on any of the pavement facilities. A layout of the airfield is shown in plate 1.

Pertinent Background Data

General description of airfield

2. HAAF, formerly Hunter Air Force Base, is located in the southwest corner of Savannah, Georgia.

3. The airfield is located physiographically in the Sea Island section of the Coastal Plain province in an area of gently rolling topography. The soil in the area is generally a poorly graded sand, with scattered deposits of fine sand, silt, and lean clay. However, at lower depths occasional pockets of fat clays are found.

4. In March 1971, the airfield facilities consisted of an east-west runway 11,375 ft\* long and 200 ft wide, connecting taxiways, parking aprons, two warm-up aprons, former alert aprons and taxiway, and a compass swing base (see plate 1). The taxiways and aprons are of various lengths

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\* A table of factors for converting British units of measurements to metric units is presented on page vii.

and widths. Huey-type helicopters were utilizing the large parking apron and the former alert facilities for parking.

Previous reports

5. Previous reports covering the airfield facilities are listed below and pertinent data were extracted from them for use in this condition survey report.

a. Condition survey reports:

(1) U. S. Army Engineer Waterways Experiment Station, CE, "Condition Survey, Hunter Army Airfield, Savannah, Georgia," Miscellaneous Paper S-69-37, August 1969, Vicksburg, Miss.

(2) U. S. Army Rigid Pavement Laboratory, Ohio River Division, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Savannah, Georgia," July 1959, Mariemont, Ohio.

(3) U. S. Army Ohio River Division Laboratories, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Georgia," October 1956, Mariemont, Ohio.

(4) U. S. Army Ohio River Division Laboratories, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Savannah, Georgia," July 1953, Mariemont, Ohio.

(5) U. S. Army Ohio River Division Laboratories, CE, "Report of Rigid Pavement Condition Survey, Hunter Air Force Base, Savannah, Georgia," April 1951, Mariemont, Ohio.

b. Evaluation reports:

(1) U. S. Army Engineer Waterways Experiment Station, CE, "Airfield Pavement Evaluation, Hunter Air Force Base, Savannah, Georgia," Miscellaneous Paper No. 4-379, February 1960, Vicksburg, Miss.

(2) U. S. Army Engineer Waterways Experiment Station, CE, "Airfield Pavement Evaluation, Hunter Air Force Base, Savannah, Georgia," April 1958, Vicksburg, Miss.

(3) U. S. Army, Savannah District, CE, "Airfield Evaluation, Final Report," April 1945, Savannah, Georgia.

## History of Airfield Pavements

### Construction history

6. The majority of pavement facilities were constructed during the period of years 1941-1959. A summary of the construction history (from the evaluation report, reference 1, paragraph 5b) is shown in table 1. The pavements constructed after 1955 were designed to support a landing gear load of 100,000 lb carried on dual wheels spaced 37.5 in. c-c, each wheel having a tire contact area of 267 sq in. Typical sections of the primary runway and taxiways are shown in plates 2 and 3, respectively. Pavement thickness and other details for all pavement features are shown in the summary of physical property data shown in table 2.

### Traffic history

7. During 1967, HAAF was converted from an Air Force to an Army installation. Prior to the change, the pavements were utilized by heavy bomber and cargo-type aircraft. The Army is using the facilities for rotary-wing aircraft used for pilot training. Considerable traffic is recorded for Huey-type helicopters; however, these aircraft have little adverse effect on the pavements, which were designed for heavy loads. The runway and taxiway pavements are generally used for small Army aircraft; however, occasional use is made of the system by transient Air Force heavy-type aircraft.

## Condition of Pavement Surfaces

8. A visual inspection of the pavements in March 1971 indicated the airfield pavement to be generally in good condition.

### Flexible pavement surfaces

9. The surface of the east-west runway was excellent (photographs 1 and 2). The surfaces of the taxiways, aprons, and holding areas

at taxiways 1 and 5 presented a good appearance. The areas of the taxiways and aprons that were treated with a bituminous coating material in 1967 and 1968 (condition survey report, reference 1, paragraph 5a) showed numerous open cracks and scaling of the coating in some areas (photographs 3-6). A white deposit observed along the cracks in the holding pad at taxiway 5 is shown in photograph 7. The deposit was a fine-grained powder that apparently came from the limerock base course. Asphalt shoulder pavements of the former alert facilities are being utilized by Cobra-type helicopters as parking areas. Considerable deterioration of the pavement has occurred because of fuel spillage (photographs 8 and 9). The shoulder pavements of taxiway 6 had many cracks with vegetation growing in the cracks.

#### Rigid pavement facilities

10. The rigid pavement facilities condition survey (table 3) revealed that the majority of these pavements were in excellent condition. The pavement of the east apron and the small slabs of the holding pads (former alert parking aprons) were rated good. The defects in the former alert facilities areas probably were developed while they were being utilized by heavy Air Force aircraft. It seems unlikely that helicopters could have caused the defects. The slabs in apron B were in fair condition. Typical defects in this area are shown in photographs 10-14. Photograph 13 shows fuel stains at a Huey parking spot on the west apron.

### Airfield Maintenance

#### Recent maintenance

11. During 1970 the asphalt portion of the east-west runway was heater planed to remove the oxidized pavement surface. It was then rolled and a slurry seal was applied.

#### Planned maintenance

12. Maintenance scheduled for rigid pavement facilities during 1971 included replacing shattered slabs, cleaning and repairing cracks in broken slabs, and applying joint sealer to a large portion of the east apron and apron B. After such maintenance, these areas should be in good condition. Maintenance planned for the flexible pavements during 1971 was slurry-sealing of taxiways 1-3 and the holding area adjacent to taxiway 1. Also, the shoulder pavements of taxiway 6 were to be removed and replaced with hot-mix asphaltic concrete. An overlay of the flexible pavement of the overrun area at both ends of the east-west runway is planned for FY 1972.

#### Evaluation

13. The evaluation of the load-carrying capacity of the HAAF pavements was based on criteria contained in TM 5-827-2 and TM 5-827-3, "Flexible Airfield Pavement Evaluation," and "Rigid Airfield Pavement Evaluation," respectively, and on the strength values assigned for the 1960 evaluation. Evaluations are shown in table 4 for four life categories of airfield pavements and various types of landing gear wheel assemblies. An aircraft identification index is presented in table 5, which lists the various types of aircraft according to landing gear configurations.

Table 1  
Construction History

Facility	Length ft	Width ft	Pavement		Construction	
			Thickness in.	Type	Period	Agency
E-W runway						
Sta 0+00-105+00	10,500	200	4	AC	1951-1952	CE
Sta 95+00-105+00 (strengthened)	1,000	200	2	AC	1955-1956	CE
Sta 105+00-113+75	875	200	15	PCC	1955-1956	CE
Sta 0+00-3+00	300	200	19-22	PCC	1957	IE
Sta 3+00-105+00 (strengthened)	10,200	200	1	AC	1959	IE
Alert aprons and twy			20	PCC	1959	CE
Taxiway 6	1,300+	75	18	PCC	1957	CE
Taxiway 5						
Original	5,400+	100	4	AC	1951-1952	CE
Sta 62+50-83+00 (strengthened)	2,050	80	1-1/2	AC	1959	IE
Taxiway 1	1,670+	75	4	AC	1951-1952	CE
Taxiway 4	670+	75	4	AC	1951-1952	CE
Taxiway 3						
Southwest end	630+	75	4	AC	1951-1952	CE
Northeast end	2,200+	150	6	PCC	1941	CE
Strengthened	2,200+	150	4	AC	1952-1953	CE
Taxiway 2						
Southeast end	970+	75	4	AC	1951-1952	CE
Northwest end	900+	150	6	PCC	1941	CE
Strengthened	900+	150	4	AC	1952-1953	CE
E-W taxiway						
Original	5,300	150	6	PCC	1941	CE
Strengthened	5,300	150	4	AC	1952-1953	CE
Hangar aprons			13	PCC	1953-1954	CE
Compass swing base			15	PCC	1953-1954	CE
West apron			15	PCC	1953-1954	CE
Apron B			6	PCC	1942	CE
East apron						
Original			6	PCC	1942	CE
Strengthened			11	PCC	1955-1956	CE
North apron			15	PCC	1955-1956	CE
South apron			15	PCC	1953-1954	CE
Apron A			15	PCC	1953	CE

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S. J. ...

FACILITY			OVERLAY PAVEMENT			PAVEMENT			BASE			SUBGRADE		GENERAL CONDITION OF AREA OR CONSIDERED
FACILITY NUMBER AND IDENTIFICATION	LENGTH FT	WIDTH FT	THICK IN.	DESCRIPTION	FLEX. STR PSI	THICK IN.	DESCRIPTION	FLEX. STR PSI	THICK IN.	CLASSIFICATION	CBR OR K	CLASSIFICATION	CBR OR K	
E-W runway Sta 5+00 to 3+00	300	20	4	Asphaltic concrete	75	4	Portland-cement concrete	75	4	Limerock base Subbase	80	Sand	300	
Sta 3+00 to 11+00	900	200	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Sta 11+00 to 9+00	800	200	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Sta 9+00 to 10+00	1000	200	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Sta 10+00 to 11+75	875	200	4	Asphaltic concrete	73	4	Portland-cement concrete	73	4	Limerock base Subbase	80	Sand	300	
Taxiway 6	1300±	75	4	Asphaltic concrete	735	4	Portland-cement concrete	735	4	Limerock base Subbase	80	Sand	300	
Taxiway 1	1670±	75	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Taxiway 4	670±	75	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Taxiway 5 Sta 62+50 to 83+00	2050	80	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Original	5400±	120	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Taxiway 2 Southeast end	970±	75	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
N Ribvert end	900±	15	6	Asphaltic concrete	650	6	Portland-cement concrete	650	6	Limerock base Subbase	80	Sand	350	
Taxiway 3 Southwest end	630±	75	4	Asphaltic concrete		4	Asphaltic concrete		4	Limerock base Subbase	80	Sand	25	
Northwest end	2200±	150	6	Asphaltic concrete	650	6	Portland-cement concrete	650	6	Limerock base Subbase	80	Sand	350	
E-W taxiway	430±	15	6	Asphaltic concrete	730	6	Portland-cement concrete	730	6	Limerock base Subbase	80	Sand	350	
Runway aprons			13	Portland-cement concrete	800	13	Portland-cement concrete	800	13	Limerock base Subbase	80	Sand	300	
Alert aprons and taxiway			20	Portland-cement concrete	730	20	Portland-cement concrete	730	20	Limerock base Subbase	80	Sand	225	
North apron			15	Portland-cement concrete	730	15	Portland-cement concrete	730	15	Limerock base Subbase	80	Sand	300	
South apron			15	Portland-cement concrete	730	15	Portland-cement concrete	730	15	Limerock base Subbase	80	Sand	300	

(Continued) (1 of 2 sheets)

WES FORM 1000 11171-2  
MAY 1958

FACILITY			OVERLAY PAVEMENT				PAVEMENT				BASE		SUBGRADE		GENERAL CONDITION OF AREA CONSIDERED
FACILITY NUMBER AND IDENTIFICATION	LENGTH FT	WIDTH FT	THICK. IN	DESCRIPTION	FLEX. STR PSI	THICK. IN	DESCRIPTION	FLEX. STR PSI	THICK. IN	CLASSIFICATION	CBR OR K	CLASSIFICATION	CBR OR K		
West apron						14	Portland-cement concrete	730				Sand	300		
Compass-lying base						15	Portland-cement concrete	730				Sand	340		
East apron	1400	Var	11	Portland-cement concrete	730	6	Portland-cement concrete	650				Sand	450		
Apron area Apron extensions						7	Asphaltic concrete		6	8	Waterbound macadam Subbase	Sand	25		
Portion of old NE-SE runway	2200	150	4	Asphaltic concrete		6	Asphaltic concrete	650				Sand	24		
Portion of old NE-SW runway	700	150	4	Asphaltic concrete		6	Portland-cement concrete	650				Sand	350		
Apron A						15	Portland-cement concrete	730				Sand	340		
Apron E						5	Portland-cement concrete	730				Sand	340		

(2 of 2 sheets)

RES FORM 1000 2-11-57  
MAY 1958







Table 4  
SUMMARY OF PAVEMENT EVALUATION

FACILITY				OVERLAY PAVEMENT			PAVEMENT			BASE			SUBGRADE		LOAD CAPACITY		
IDENTIFICATION	LENGTH FT	WIDTH FT	TEST PIT NO.	THICK. IN.	DESCRIPTION	FLEX. STR. PSI	THICK. IN.	DESCRIPTION	FLEX. STR. PSI	THICK. IN.	DESCRIPTION	CBR %	CLASSIFICATION	CBR %	CATEGORY OF PAVEMENT LIFE AND OPERATIONAL USE	SINGLE 100 PSI TIRE PRESSURE	SINGLE 100 LB IN. CONTACT AREA
E-W runway Sta 0+00 to 3+00							22	Portland-cement concrete	750				Sand	300	Emergency Minimum Full Capacity	155,000+	65,000+
A			Selected figures for evaluation													155,000+	65,000+
Sta 3+00 to 11+00				1	Asphaltic concrete		4	Asphaltic concrete		8 8	Limerock Subbase	80 40	Sand	25	Emergency Minimum Full Capacity	155,000+	65,000+
Q			Selected figures for evaluation													155,000+	65,000+
Sta 11+00 to 95+00				1	Asphaltic concrete		4	Asphaltic concrete		6 8	Limerock Subbase	80 40	Sand	25	Emergency Minimum Full Capacity	155,000+	65,000+
K			Selected figures for evaluation													155,000+	65,000+
Sta 95+00 to 105+00				3	Asphaltic concrete		4	Asphaltic concrete		8 8	Limerock Subbase	80 40	Sand	25	Emergency Minimum Full Capacity	155,000+	65,000+
J			Selected figures for evaluation													155,000+	65,000+
Sta 105+00 to 113+75							15	Portland-cement concrete	730				Sand	300	Emergency Minimum Full Capacity	155,000+	65,000+
E			Selected figures for evaluation													155,000+	65,000+
Taxiway 6							18	Portland-cement concrete	735				Sand	300	Emergency Minimum Full Capacity	155,000+	65,000+
D			Selected figures for evaluation													155,000+	65,000+
Taxiway 1 and 5 Original							4	Asphaltic concrete		8 8	Limerock Subbase	80 40	Sand	25	Emergency Minimum Full Capacity	155,000+	65,000+
I			Selected figures for evaluation													155,000+	65,000+
Sta 62+50 to 83+00				1-1/2	Asphaltic concrete		4	Asphaltic concrete		8 8	Limerock Subbase	80 40	Sand	25	Emergency Minimum Full Capacity	155,000+	65,000+
P			Selected figures for evaluation													155,000+	65,000+
Taxiway 4							4	Asphaltic concrete		8 8	Limerock Subbase	80 40	Sand	25	Emergency Minimum Full Capacity	155,000+	65,000+
I			Selected figures for evaluation													155,000+	65,000+
Taxiway 3 Original							4	Asphaltic concrete		8 8	Limerock Subbase	80 40	Sand	25			
I			Selected figures for evaluation														
NE end				7	Asphaltic concrete		6	Portland-cement concrete	650				Sand	350	Emergency Minimum Full Capacity	155,000+	65,000+
L			Selected figures for evaluation													155,000+	65,000+
Taxiway 2 Original							4	Asphaltic concrete		8 8	Limerock Subbase	80 40	Sand	25			
I			Selected figures for evaluation														
SE end				7	Asphaltic concrete		6	Portland-cement concrete	650				Sand	350	Emergency Minimum Full Capacity	155,000+	65,000+
			Selected figures for evaluation													155,000+	65,000+
SE Taxiway				7	Asphaltic concrete		6	Portland-cement concrete	650				Sand	350			
			Selected figures for evaluation														
Runway 2							15	Portland-cement concrete	730				Sand	300	Emergency Minimum Full Capacity	155,000+	65,000+
			Selected figures for evaluation													155,000+	65,000+
Runway 1							13	Portland-cement concrete	730				Sand	300	Emergency Minimum Full Capacity	155,000+	65,000+
			Selected figures for evaluation													155,000+	65,000+

Table 1  
SUMMARY OF PAVEMENT EVALUATION

SUBGRADE		CATEGORY OF PAVEMENT LIFE AND OPERATIONAL USE	LOAD CARRYING CAPACITY IN LB OF GROSS PLANE LOAD FOR INDICATED LANDING GEAR TYPES AND CONFIGURATIONS AND LIFE CATEGORIES									TRAFFIC AREA	
CLASSIFICATION	CBN I		TRICYCLE ARRANGEMENT						BICYCLE				
			SINGLE 120 PSI TIME PRESSURE	SINGLE 100 SQ IN CONTACT AREA	SINGLE 121 SQ IN CONTACT AREA	1225 E-C 220 SQ IN CONTACT AREA EACH TIME	SINGLE TANDEM 80 SPACING 400 SQ IN CONTACT AREA	1225 E-C 227 SQ IN CONTACT AREA EACH TIME	1225 E-C 220 SQ IN CONTACT AREA EACH TIME	1225 E-C 227 SQ IN CONTACT AREA EACH TIME	C-48 GEAR CONFIGURATION		TWIN TWIN SPEC 121 241 SQ IN CONTACT AREA EACH TIME
and	300	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	A
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	B
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	185,000	200,000+	250,000+	230,000+	380,000	770,000+	390,000	
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	580,000	
		Full Capacity	155,000+	65,000+	95,000+	200,000	200,000+	250,000	230,000+	380,000+	770,000+	440,000	
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
and	300	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	A
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	500,000	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	295,000+	230,000+	380,000+	770,000+	430,000	
and	300	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	A
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	560,000	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	310,000	230,000+	380,000+	770,000+	470,000	
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	336,000+	230,000+	380,000+	770,000+	600,000+	A
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	320,000	230,000+	380,000+	770,000+	460,000	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000	210,000	230,000+	280,000	770,000+	370,000	
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	A
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	530,000	
		Full Capacity	155,000+	65,000+	95,000+	200,000	200,000+	270,000	230,000+	360,000	770,000+	400,000	
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	155,000	200,000+	220,000	230,000+	380,000+	770,000+	500,000	
and	25											C	
		Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+		600,000+
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+		530,000
and	300	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	A
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	180,000	230,000+	380,000+	750,000	275,000	
		Full Capacity	115,000	65,000+	95,000+	175,000+	200,000+	145,000	230,000+	290,000	600,000	245,000	
and	25											C	
		Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+		600,000+
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+		600,000+
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	410,000	
		Full Capacity	115,000	65,000+	95,000+	175,000	200,000+	230,000	230,000+	380,000+	770,000+	370,000	
and	25											A	
		Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+		600,000+
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+		600,000+
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	500,000	
		Full Capacity	155,000+	65,000+	95,000+	220,000	200,000+	320,000	230,000+	380,000+	770,000+	500,000	
and	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	575,000	
		Full Capacity	135,000	65,000+	95,000+	200,000	200,000+	260,000	230,000+	380,000+	770,000+	415,000	



Table 4 (Concluded)

FACILITY				OVERLAY PAVEMENT			PAVEMENT			BASE		SUBGRADE		LOAD CARRYING				
IDENTIFICATION	LENGTH FT.	WIDTH FT.	TEST PIT NO.	THICK. IN.	DESCRIPTION	FLEX STR. PSI.	THICK. IN.	DESCRIPTION	FLEX STR. PSI.	THICK. IN.	DESCRIPTION	COR. L.	CLASSIFICATION	COR. L.	CATEGORY OF PAVEMENT LIFE AND OPERATIONAL USE	LOAD CARRYING		
																SINGLE 100 PSI TIRE PRESSURE	SINGLE 100 LB IN. CONTACT AREA	1 FT. COUR.
East Apron H	Selected figures for evaluation			11	Portland- cement concrete	730	6	Portland- cement concrete c = 0.75	650				Sand	350	Emergency Minimum Full Capacity	155,000+ 155,000+ 155,000+ 150,000	65,000+ 65,000+ 65,000+ 65,000+	95 95 95 95
Taxiway 7 and 8 B	Selected figures for evaluation						20	Portland- cement concrete	800				Sand	225	Emergency Minimum Full Capacity	155,000+ 155,000+ 155,000+ 155,000+	65,000+ 65,000+ 65,000+ 65,000+	95 95 95 95
Apron Area N	Selected figures for evaluation						4	Asphaltic concrete		6 8	Waterbound macadam Subbase	80 40	Sand	25	Emergency Minimum Full Capacity	155,000+ 155,000+ 155,000+ 155,000+	65,000+ 65,000+ 65,000+ 65,000+	95 95 95 95
Apron Area N	Selected figures for evaluation			4	Asphaltic concrete		6	Asphaltic concrete					Sand	25	Emergency Minimum Full Capacity	155,000+ 155,000+ 155,000+ 120,000	65,000+ 65,000+ 65,000+ 65,000+	95 95 95 85
Apron Area O	Selected figures for evaluation			4	Asphaltic concrete		6	PT 10.7 Portland- cement concrete	650				Sand	350	Emergency Minimum Full Capacity	155,000+ 155,000+ 145,000 90,000	65,000+ 65,000+ 65,000+ 65,000+	95 95 95 95
Apron B S	Selected figures for evaluation						6	Portland- cement concrete	650				Sand	350	Emergency Minimum Full Capacity	150,000 110,000 85,000 55,000	65,000+ 65,000+ 60,000 40,000	95 85 65 45

Table 1 (Continued)

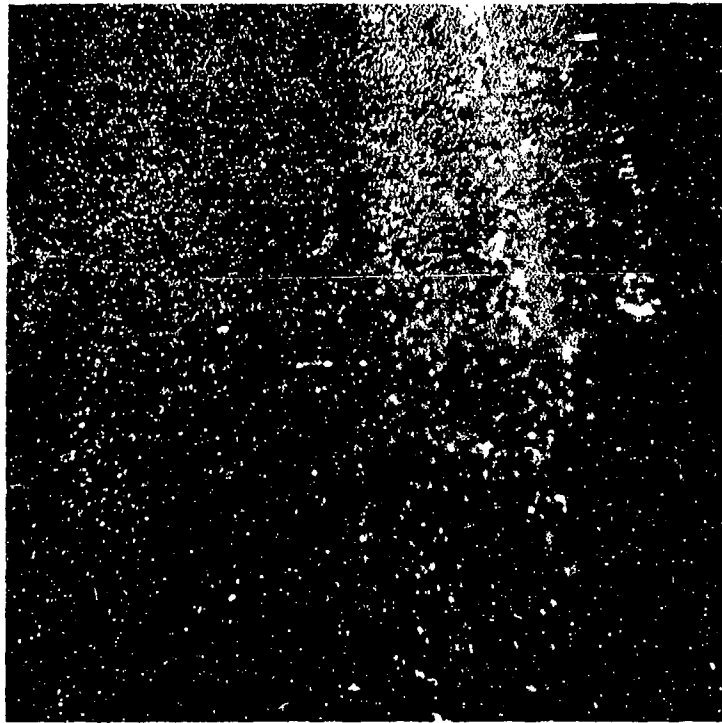
SUBGRADE		LOAD CARRYING CAPACITY IN LB OF GROSS PLANE LOAD FOR INDICATED LANDING GEAR TYPES AND CONFIGURATIONS AND LIFE CATEGORIES											TRAFFIC AREA
CLASSIFICATION	CBR	CATEGORY OF PAVEMENT LIFE AND OPERATIONAL USE	TRICYCLE ARRANGEMENT										
			SINGLE 10x15 TIRE PRESSURE	SINGLE 10x15 IN CONTACT AREA	SINGLE 24x30 IN CONTACT AREA	TWIN C C 24x30 IN CONTACT AREA EACH TIRE	SINGLE TANDEM 24x30 IN CONTACT AREA	TWIN C C 24x30 IN CONTACT AREA EACH TIRE	TWIN C C 24x30 IN CONTACT AREA EACH TIRE	TWIN TANDEM 24x30 IN CONTACT AREA EACH TIRE	C 50 GEAR CONFIGURATION	TWIN TANDEM 24x30 IN CONTACT AREA EACH TIRE	
Sand	350	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
Sand	225	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	B
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
Sand	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
Sand	25	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
Sand	350	Emergency	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	C
		Minimum	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
		Full Capacity	155,000+	65,000+	95,000+	220,000+	200,000+	330,000+	230,000+	380,000+	770,000+	600,000+	
Sand	350	Emergency	150,000	65,000+	95,000+	220,000	200,000+	200,000	230,000+	380,000+	770,000+	350,000	C
		Minimum	110,000	65,000+	85,000+	160,000	200,000+	150,000	230,000+	320,000	770,000+	270,000	
		Full Capacity	85,000	60,000	65,000	125,000	185,000	115,000	210,000	260,000	730,000	340,000	
Sand	350	Emergency	150,000	60,000	85,000+	220,000	200,000+	200,000	230,000+	380,000+	770,000+	350,000	C
		Minimum	110,000	60,000	85,000+	160,000	200,000+	150,000	230,000+	320,000	770,000+	270,000	
		Full Capacity	85,000	40,000	45,000+	85,000	135,000	80,000	155,000	190,000	525,000	340,000	

Note: A plus sign denotes allowable gross loading greater than the maximum gross weight of any existing aircraft having indicated gear configuration. (a) denotes allowable gross loading is less than the minimum gross weight of any existing aircraft having indicated gear configuration.

Table 5

Aircraft Identification Index  
 (For Gear Configurations Shown in Columns 1-10, Table 4)

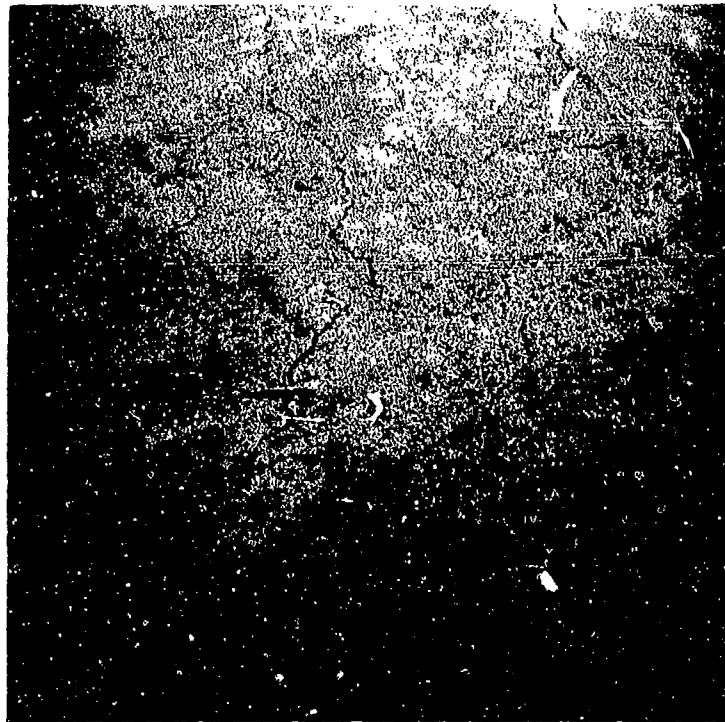
1	2	3	4	5	6	7	8	9	10
B-26-B	B-45-C	F-111	C-119	C-130		C-124	C-133	C-5A	B-52
B-45-A	F-84-F		C-54-G		B-50		C-135		B-52-A
B-57-B	F-84-G		C-118		KC-97		KC-135		
B-66-C	F-86-D		C-131		C-74		C-141		
C-45-F	F-86-F				C-121		KC-137		
C-45-G	F-86-H								
C-46-F	F-89 Series								
C-82	F-100-A								
C-123-B	F-101-A								
F-86-A	F-102								
F-86-E	C-47								
F-94-B	B-57								



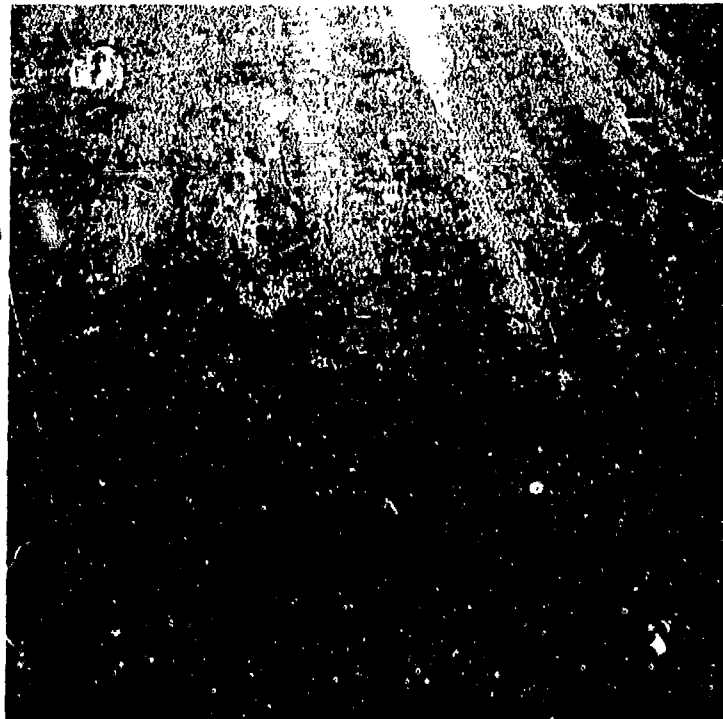
Photograph 1. Surface typical of asphalt portion  
of east-west runway



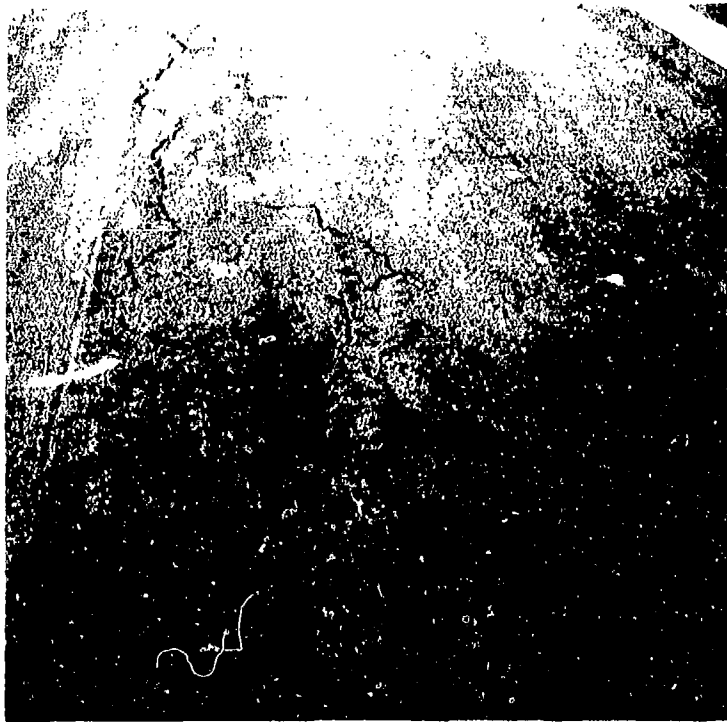
Photograph 2. Closeup view of slurry seal on  
east-west runway



Photograph 3. Open cracks near east end of the asphalt apron



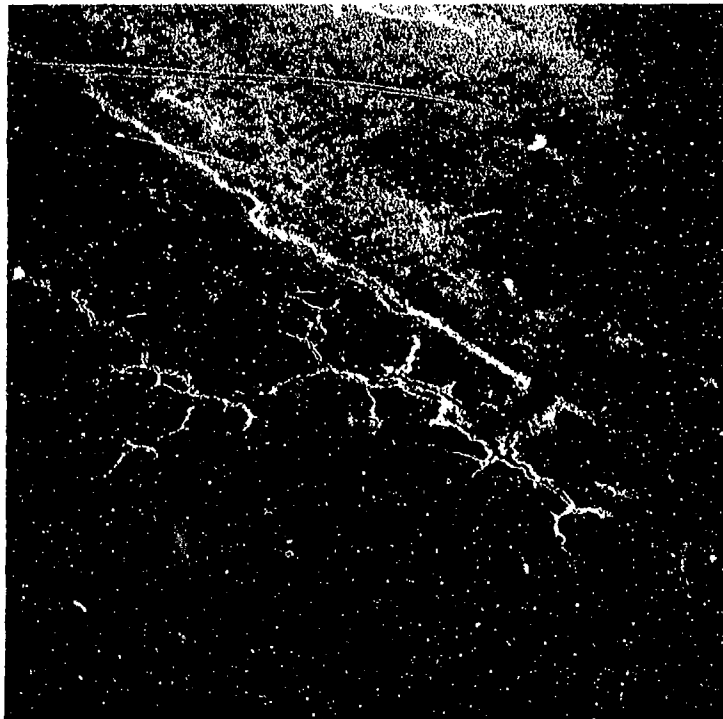
Photograph 4. Longitudinal cracks in taxiway 4



Photograph 5. Random cracking, taxiway 5



Photograph 6. Scaling of bituminous seal, east apron area



Photograph 7. White powdery substance along  
cracks in pavement of holding area at taxiway 5



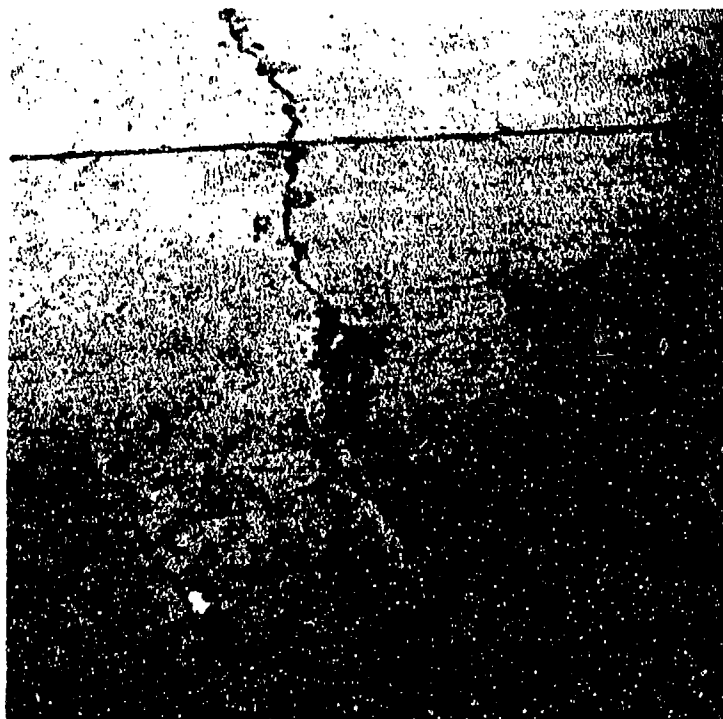
Photograph 8. Effects of fuel spillage on shoulder pavements of former alert area



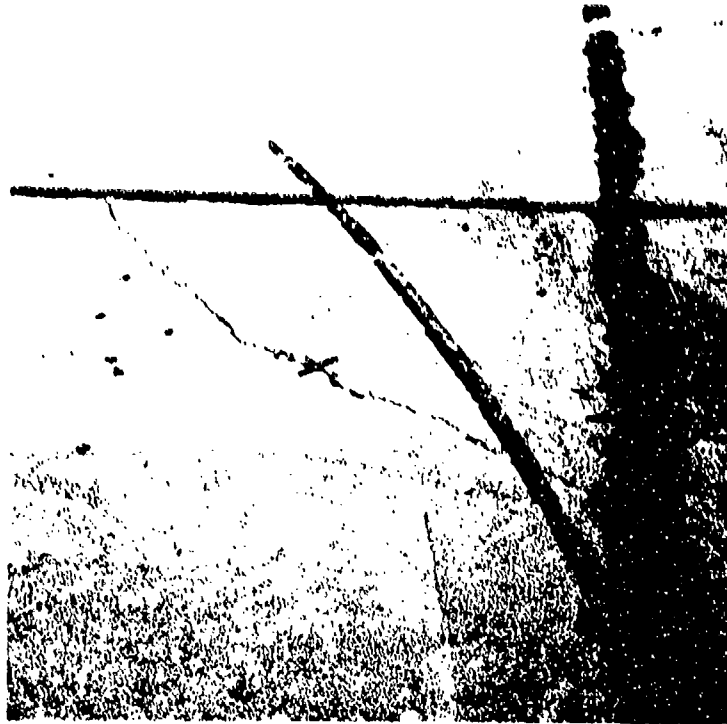
Photograph 9. Ruts in fuel spillage area on shoulder pavements of former alert area



Photograph 10. Shattered slab in apron B



Photograph 11. Spall along transverse crack,  
apron B



Photograph 12. Corner break of slab, apron B



Photograph 13. Typical popout in rigid pavement,  
apron B



Photograph 14. Spalling along longitudinal joint, apron B



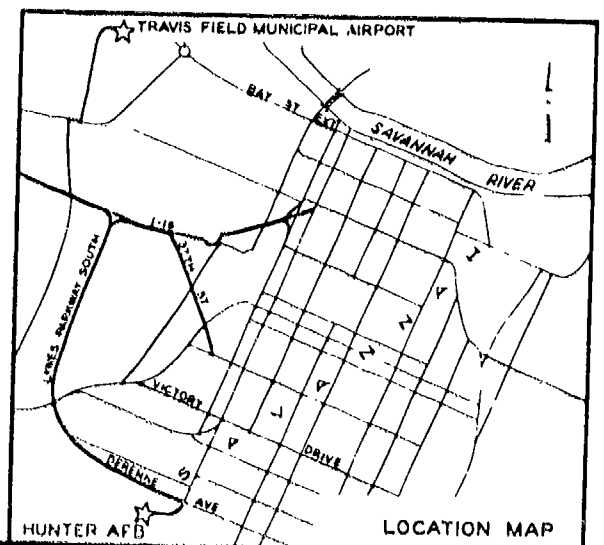
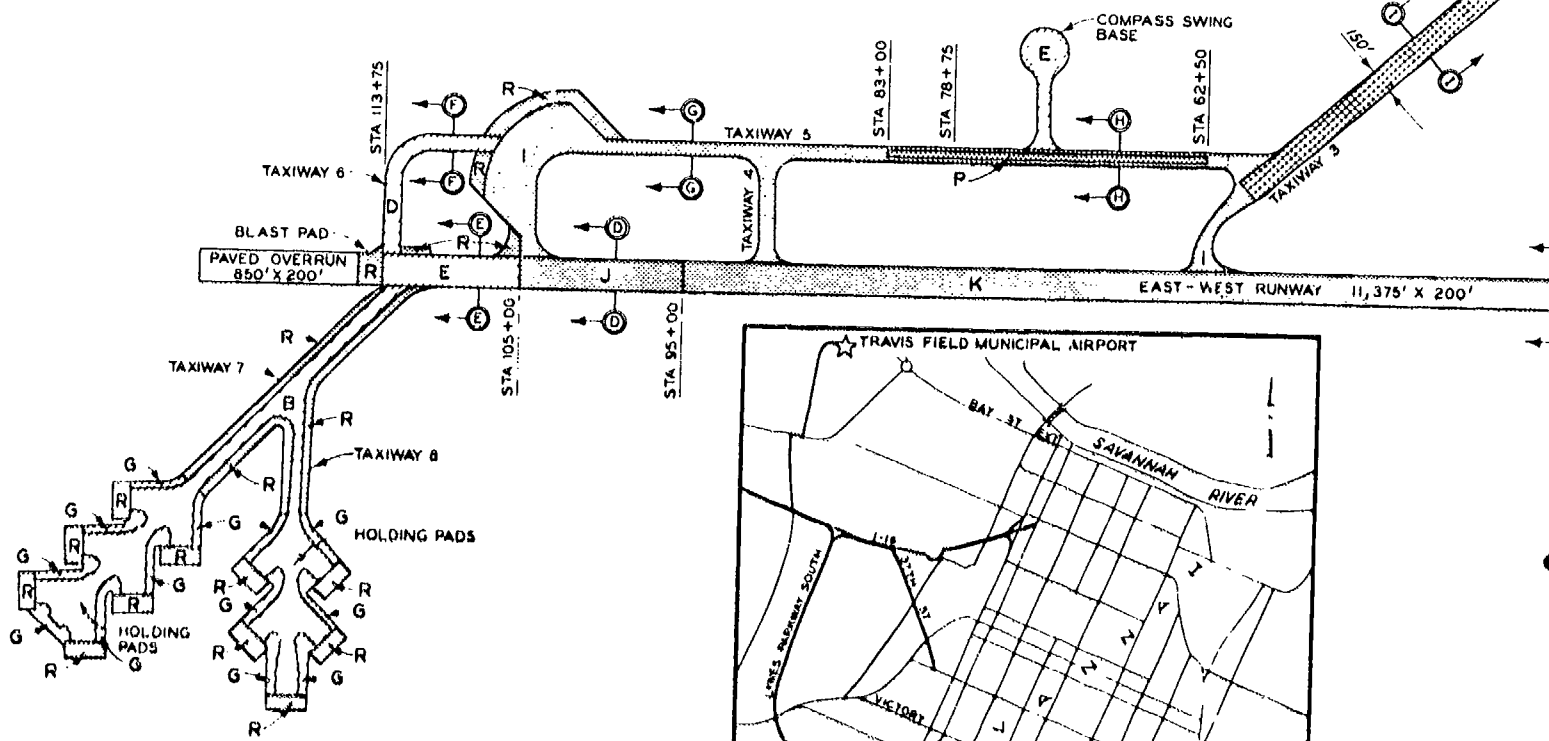
Photograph 15. Fuel stains at Huey parking spot, west apron



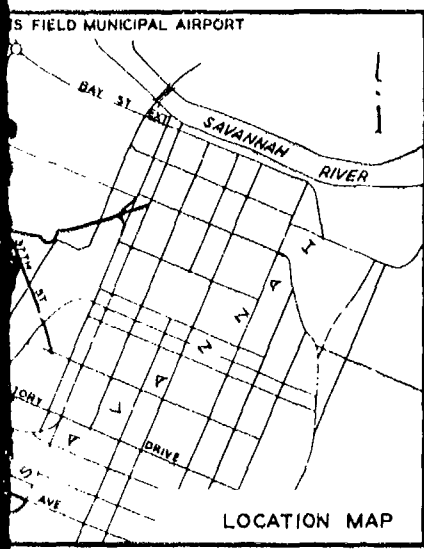
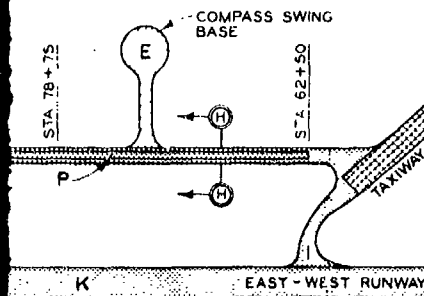
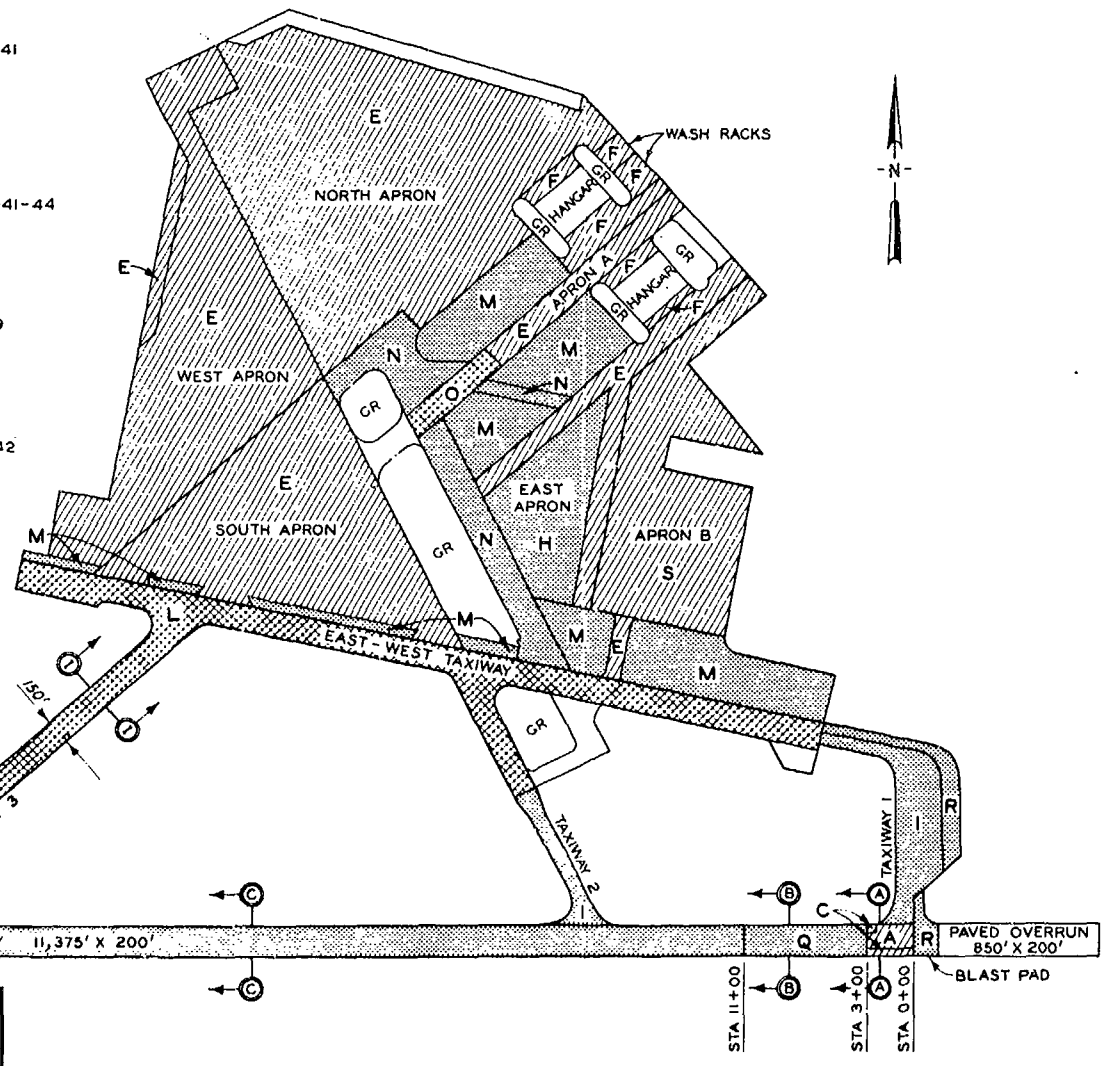
A		22-IN. PORTLAND-CEMENT CONCRETE	1957
B		20-IN. PORTLAND-CEMENT CONCRETE	1959
C		19-IN. PORTLAND-CEMENT CONCRETE	1957
D		18-IN. PORTLAND-CEMENT CONCRETE	1957
E		15-IN. PORTLAND-CEMENT CONCRETE	1953-58
F		13-IN. PORTLAND-CEMENT CONCRETE	1953-56
G		6-IN. PORTLAND-CEMENT CONCRETE	1959
H		11-IN. PORTLAND-CEMENT CONCRETE	1955-56
		8-6-6-8-IN. PORTLAND-CEMENT CONCRETE	1942
I		4-IN. ASPHALTIC CONCRETE	} 1951-52
		8-IN. LIMEROCK BASE	
		8-IN. SUBBASE	
J		1-IN. (MIN) ASPHALTIC CONCRETE OVERLAY	1959
		2-IN. ASPHALTIC CONCRETE OVERLAY	1955-56
		4-IN. ASPHALTIC CONCRETE	} 1951-52
		8-IN. LIMEROCK BASE	
		8-IN. SUBBASE	
K		1-IN. (MIN) ASPHALTIC CONCRETE OVERLAY	1959
		4-IN. ASPHALTIC CONCRETE	} 1951-52
		C-IN. LIMEROCK BASE	
		8-IN. SUBBASE	

**LEGEND**

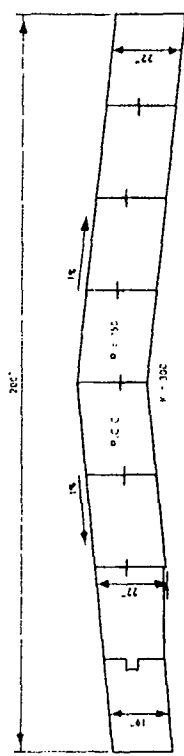
L		4-IN. ASPHALTIC CONCRETE	} 1952-53
		3-IN. BLACK BASE	
		8-6-6-8-IN. PORTLAND-CEMENT CONCRETE	
M		4-IN. ASPHALTIC CONCRETE	} 1952-53
		6-IN. WATERBOUND MACADAM BASE	
		8-IN. SUBBASE	
N		4-IN. ASPHALTIC CONCRETE OVERLAY	1941-44
		6-IN. ASPHALTIC CONCRETE	
O		4-IN. ASPHALTIC CONCRETE OVERLAY	} 1941-44
		8-6-6-8-IN. PORTLAND-CEMENT CONCRETE	
		8-IN. SUBBASE	
P		1½-IN. ASPHALTIC CONCRETE OVERLAY	1959
		4-IN. ASPHALTIC CONCRETE	} 1951-52
		8-IN. LIMEROCK BASE	
		8-IN. SUBBASE	
Q		1-IN. (MIN) ASPHALTIC CONCRETE OVERLAY	1959
		4-IN. ASPHALTIC CONCRETE	} 1951-52
		8-IN. LIMEROCK BASE	
		8-IN. SUBBASE	
R		BLAST AND SHOULDER PAVEMENT (BITUMINOUS)	
S		6-6-6-8-IN. PORTLAND-CEMENT CONCRETE	1942
GR		GRASS PLOTS	



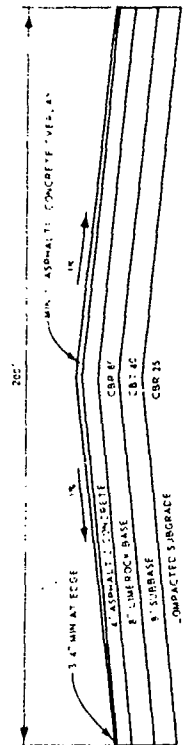
- 4-IN. ASPHALTIC CONCRETE } 1952-53
- 4-IN. BLACK BASE } 1952-53
- 6-6-8-IN. PORTLAND-CEMENT CONCRETE 1941
- 4-IN. ASPHALTIC CONCRETE } 1952-53
- 4-IN. WATERBOUND MACADAM BASE } 1952-53
- 4-IN. SUBBASE } 1952-53
- 4-IN. ASPHALTIC CONCRETE OVERLAY 1941-44
- 4-IN. ASPHALTIC CONCRETE } 1952-53
- 4-IN. ASPHALTIC CONCRETE OVERLAY 1941-44
- 6-6-8-IN. PORTLAND-CEMENT CONCRETE 1941-44
- 4-IN. ASPHALTIC CONCRETE OVERLAY 1959
- 4-IN. ASPHALTIC CONCRETE } 1951-52
- 4-IN. LIMEROCK BASE } 1951-52
- 4-IN. SUBBASE } 1951-52
- 4-IN. (MIN) ASPHALTIC CONCRETE OVERLAY 1959
- 4-IN. ASPHALTIC CONCRETE } 1951-52
- 4-IN. LIMEROCK BASE } 1951-52
- 4-IN. SUBBASE } 1951-52
- LAST AND SHOULDER PAVEMENT (BITUMINOUS)
- 6-6-8-IN. PORTLAND-CEMENT CONCRETE 1942
- GRASS PLOTS



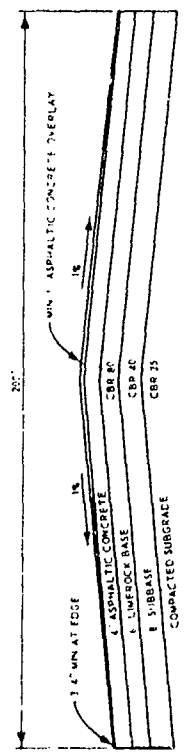
HUNTER AAF  
SAVANNAH, GEORGIA  
**PAVEMENT PLAN**  
SCALE IN FEET  
500 0 500 1000



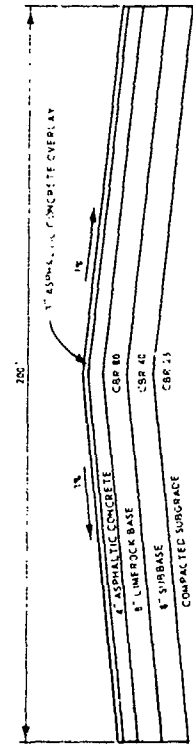
SECTION B-B  
STA 3+00 TO 11+00



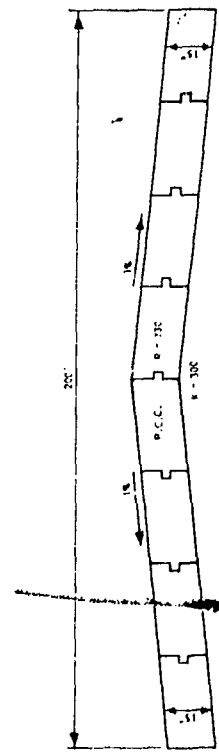
SECTION C-C  
STA 11+00 TO 95+00



SECTION D-D  
STA 95+00 TO 105+00

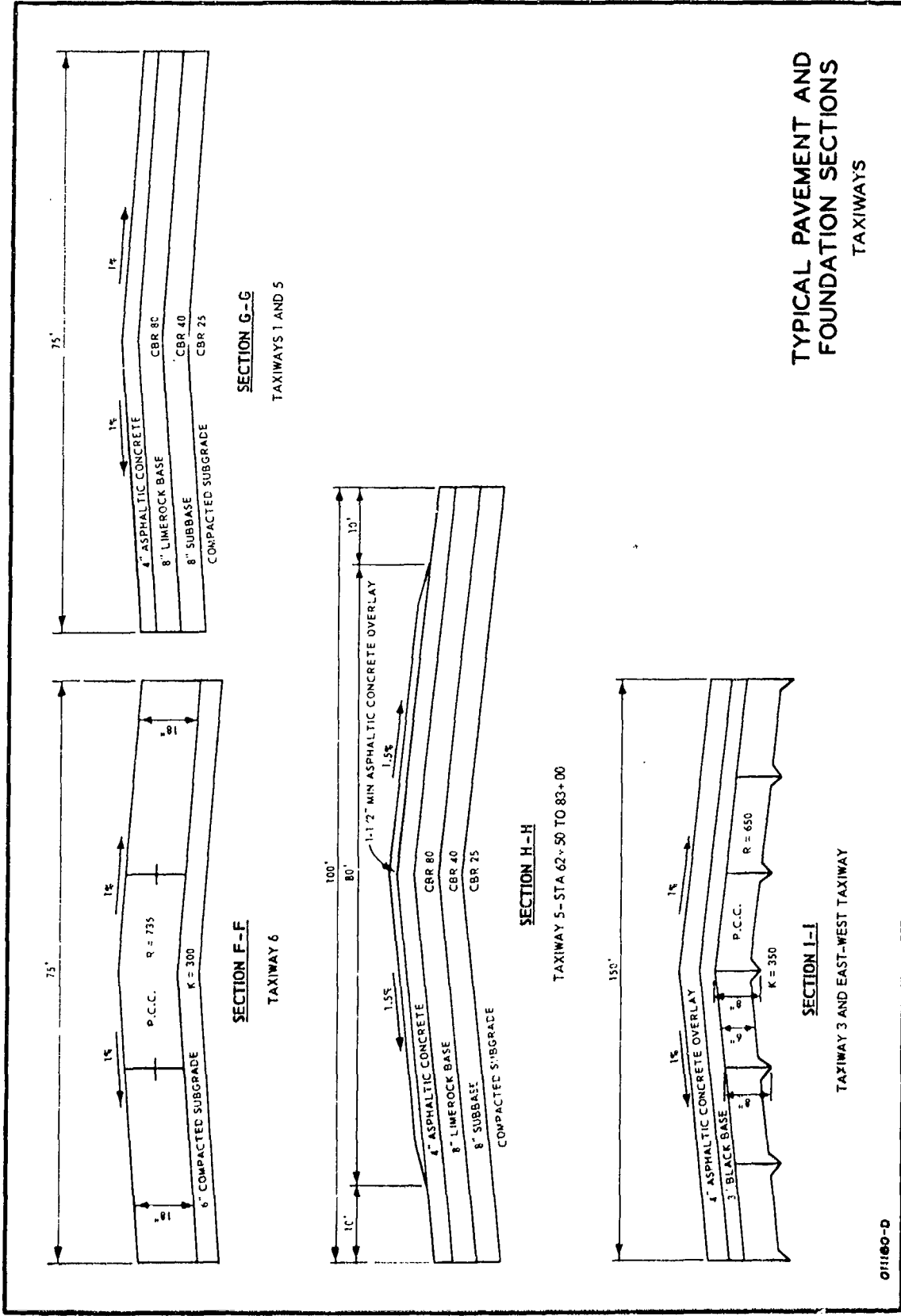


SECTION E-E  
STA 105+00 TO 113+75



TYPICAL PAVEMENT AND  
FOUNDATION SECTIONS  
EAST-WEST RUNWAY

01160-C



TYPICAL PAVEMENT AND FOUNDATION SECTIONS  
TAXIWAYS