

AD-A044 595

DOUGLAS AIRCRAFT CO LONG BEACH CALIF
THE DETERMINATION OF DEFLECTION AND STRESS DISTRIBUTION FOR A T--ETC(U)
NOV 76 P H DENKE, J B HOFFMAN

F/G 1/3

F33615-75-C-3105

UNCLASSIFIED

MDC-J7173

AFFDL-TR-76-114

NL

1 OF 2
AD
A044595



AD A 044595

AFFDL-TR-76-114

12
B.S.

**THE DETERMINATION OF DEFLECTION AND STRESS
DISTRIBUTION FOR A TRANSPARENT LAMINATED BEAM**

Douglas Aircraft Company
McDonnell Douglas Corporation
3855 Lakewood Boulevard
Long Beach, California 90846

DDC
RECEIVED
SEP 19 1977
ALBUQUERQUE
CO

MARCH 1977

Final Report for Period January 1976-March 1977

Approved for public release; distribution unlimited

AD NO. _____
DDC FILE COPY

Air Force Flight Dynamics Laboratory
Air Force Wright Aeronautical Laboratories
Air Force Systems Command
Wright-Patterson Air Force, Ohio

NOTICE

When Government drawings, specifications, or other data are used for any purpose other than in connection with a definitely related Government procurement operation, the United States Government thereby incurs no responsibility nor any obligation whatsoever; and the fact that the government may have formulated, furnished, or in any way supplied the said drawings, specifications, or other data, is not to be regarded by implication or otherwise as in any manner licensing the holder or any other person or corporation, or conveying any rights or permission to manufacture, use, or sell any patented invention that may in any way be related thereto.

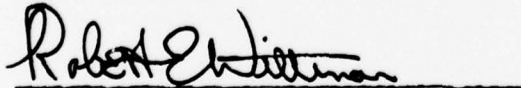
This report has been reviewed and cleared for open publication and/or public release by the appropriate Office of Information (OI) in accordance with AFR 190-170 and DODD 5230.9. There is no objection to unlimited distribution of this report to the public at large or by DDC to the National Technical Information Service (NTIS).

This technical report has been reviewed and is approved for publication.



DONALD C. CHAPIN, Project Manager
Improved Windshield Protection ADPO
Vehicle Equipment Division

FOR THE COMMANDER:



ROBERT E. WITTMAN, Program Manager
Improved Winshield Protection ADPO
Vehicle Equipment Division

Copies of this report should not be returned unless return is required by security considerations, contractual obligations, or notice on a specific document.

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER AFFDL-TR- 76-114	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle) THE DETERMINATION OF DEFLECTION AND STRESS DISTRIBUTION FOR A LAMINATED TRANSPARENT BEAM,	5. TYPE OF REPORT & PERIOD COVERED Final Report January 1976 - March 1977	
	6. PERFORMING ORG. REPORT NUMBER MDC-J7173	
7. AUTHOR(s) Paul H. Denke and Joseph B. Hoffman	8. CONTRACT OR GRANT NUMBER(s) F33615-75-C-3105	
9. PERFORMING ORGANIZATION NAME AND ADDRESS Douglas Aircraft Company McDonnell Douglas Corporation Long Beach, California 90846	10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS Project: 2202 Task: 02 Work Unit: 01	
11. CONTROLLING OFFICE NAME AND ADDRESS Air Force Flight Dynamics Laboratories (AFFDL/FEW) Air Force Wright Aeronautical Laboratories Air Force Systems Command Wright-Patterson Air Force Base, Ohio 45433	12. REPORT DATE November 1976	
	13. NUMBER OF PAGES 173	
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office) <i>(2) 173 p. 11 Nov 76</i>	15. SECURITY CLASS. (of this report) Unclassified	
15a. DECLASSIFICATION/DOWNGRADING SCHEDULE		
16. DISTRIBUTION STATEMENT (of this Report) Approved for public release, distribution unlimited		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from 16)		
18. SUPPLEMENTARY NOTES		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Computer Program Stress Distribution Elastic Deflection Structural Ply Interlayer Transparent Laminated Beam Windshield Static Loading		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This report documents the theory and procedures for calculating the elastic deflection and stress distribution of a flat, laminated beam with fixed ends or simply-supported ends comprised of transparency materials subjected to a normal point load at the midpoint of the beam. The development of a computer program using the basic theory is presented along with illustrative examples and a "user's manual." The program results have been favorably compared with actual test data and with a finite element method.		

116400

Dr. P. Poirier (ASD/ADDs) and Dr. P. J. Nikolai (AFFDL/FB), Wright-Patterson Air Force Base, Dayton, Ohio, provided the EISPACK sub-routines utilized in the Source Coding, Appendix B, in this report.

This report was submitted to the Air Force in November 1976 and covers work performed during the period of January 1976 and March 1977.

TABLE OF CONTENTS

SECTION		PAGE
I	INTRODUCTION	1
II	ANALYSIS	4
	DERIVATION OF EQUATIONS	4
	Assumptions	6
	Equilibrium, Structural Ply Element, i Odd	6
	Equilibrium, Interlayer Element, i Even	7
	Compatibility, Structural Plies, i Odd	9
	Compatibility, Interlayers, i Even	10
	Strains, Structural Plies, i Odd	10
	Strains, Interlayers, i Even	11
	Force-Deformation Relations	12
	Matric Formulation	12
	Matric Differential Equation	21
	Nature of the Roots of the Characteristic Equation	22
	Algebraic Modes	22
	Nature of the Algebraic Solutions	25
	Response	25
	Interlayer Shear Flows, Structural Ply Shears and Moments	29
	Boundary Conditions	30
	Effective Beam Stiffness	32
	EQUATION SUMMARY	33
	Input	34
	Equations	34
	Output (printed)	39
	ILLUSTRATIVE EXAMPLES	40
III	COMPUTER PROGRAM USER'S MANUAL	53
IV	CONCLUSIONS	63

TABLE OF CONTENTS (Continued)

SECTION		PAGE
V	RECOMMENDATIONS	65
APPENDIX A	PROPERTIES OF THE EIGENVALUES AND EIGENVECTORS . .	67
APPENDIX B	SOURCE CODING (MAIN PROGRAM AND SUB-ROUTINES) . .	69
APPENDIX C	INTERMEDIATE MATRICES (FOR CASE A)	117
APPENDIX D	OUTPUT FOR ILLUSTRATIVE CASE A, B, C, AND D AND FOR THE TEST BEAM	132
REFERENCES	157

LIST OF FIGURES

FIGURE	TITLE	PAGE
1	Fixed ended laminated beam	4
2	A typical transparency nine ply cross section	5
3	Half-beam	5
4	Structural ply equilibrium	7
5	Interlayer equilibrium	8
6	Compatibility	9
7	Interlayer shear strain	11
8	Boundary conditions	30
8a	Beam dimensions	33
9	Cross sectional properties for Case A	41
10	Cross sectional properties for Case B	42
11	Cross sectional properties for Case C	43
12	Cross sectional properties for Case D	44
13	Deflection versus beam length	48
14	Strain (in/in) versus thickness for Case A at center of beam	51
15	Finite element model of a fixed ended beam - Case A . .	52
16	Complete card deck	54
17	Card deck flow diagram	55
18	Sample coding sheet for control cards and separating cards	56
19	Sample input data coding sheet (Case A)	58
20	Sample data cards	59
21	Sample data cards	60

LIST OF TABLES

TABLE	TITLE	PAGE
1	SUMMARY OF INPUT DATA FOR CASE A	45
2	SUMMARY OF OUTPUT DATA FOR CASE A	46
3	DEFLECTION - (INCHES)	47
4	EFFECTIVE STIFFNESS - (POUNDS-INCHES SQUARE)	50

LIST OF ABBREVIATIONS AND SYMBOLS

(Used in Analysis, Section II)

A	A square coefficient matrix in the governing matrix differential equation
A_i	Cross-sectional area of i^{th} layer
$a_{1,2}$	k_A^{-1}
$a_{2,1}$	$\Delta_q k_\eta \Delta_q^T$
$a_{2,3}$	$-\Delta_q k_\eta \bar{t}$
$a_{5,4}$	$K_I^{-1} \bar{t}^T k_\eta \bar{t}$
$a_{5,2}$	$K_I^{-1} a_{2,3}^T a_{1,2}$
B	Coefficient matrix in the boundary condition equation
b	Beam width
C	A column matrix of arbitrary constants
\bar{C}	Column matrix of unit elements
C_{A_k}	Coefficient of k^{th} algebraic mode
C_{g_k}	$\bar{C}_{g_k} e^{\lambda g k \ell}$

C_k	k^{th} arbitrary constant
C_ℓ, C_g, C_A	Column matrices of the arbitrary constants $C_{\ell_k}, C_{g_k},$ and C_{A_k}
$C_{\ell_k}, \bar{C}_{g_k}$	Arbitrary constants in the expressions for $Y_{E\ell}$ and Y_{Eg}
e	Base of natural logarithms
E_i	Young's modulus of i^{th} layer
EI_{eff}	Effective beam bending stiffness
F	Load at beam centerline
$F_{g_k}(x)$	$e^{\lambda_{\ell k}(\ell-x)}$
$F_\ell(x), F_g(x)$	Column matrices of the $F_{\ell_k}(x)$'s and the $F_{g_k}(x)$'s
$F_{\ell_k}(x)$	$e^{\lambda_{\ell k}x}$
$F_{\ell D}(x), F_{gD}(x)$	$F_\ell(x), F_g(x)$ diagonalized
G	An eigenvector of A
G_{g_k}	k^{th} eigenvector of A corresponding to λ_{g_k}

G_i	Shear modulus of the i^{th} layer
G_k	k^{th} eigenvector of A
$G_{\ell k}$	k^{th} eigenvector of A corresponding to $\lambda_{\ell k}$
G_{u_k}, G_{H_k} G_{v_k}, G_{θ_k} G_{β_k}, G_{ϕ_k}	Partitions of G_k corresponding to the partitions $u, H, v, \theta, \beta,$ and ϕ of Y
H	Column matrix of structural ply axial loads
H_i	Axial load in i^{th} layer
H_{ℓ}, H_g	Rectangular matrices consisting of the modal columns $G_{\ell k}$ and $G_{g k}$
I	Unit (identity) matrix
I_i	Cross-sectional moment of inertia of i^{th} layer
k_A	Diagonal matrix of structural ply axial stiffnesses
K_I	Sum of cross-sectional moments of inertia of structural plies
k_I	Column matrix of structural ply bending stiffnesses
k_{ℓ}	Column matrix of interlayer shear stiffnesses
k_{η}	Diagonal matrix of interlayer shear stiffnesses

L	Length of beam
ℓ	L/2
M	Column matrix of structural ply bending moments
M_i	Bending moment in i^{th} layer
M_{xq}	A matrix of structural ply thicknesses
O	Null matrix
P	F/2
Q	A Boolean integrating matrix
q	Column matrix of interlayer shear flows
q_i	Shear flow on lower surface of i^{th} layer
S	$a_{2,1}$
$S(x)$	Total shear on the beam cross section
$S_{1,1}, S_{1,2}$	} Partitions of S
$S_{2,1}, S_{2,2}$	
T	(Superscript) Indicates transposed matrix
T	$k_A^u(0)$
\bar{t}	$t_\ell + \bar{t}_s$

t_i	Thickness of i^{th} layer
t_ℓ	Column matrix of interlayer thicknesses
t_s	Column matrix of structural ply thicknesses
\bar{t}_s	Column matrix of average structural ply thicknesses
T_1, T_2	Partitions of T
u	Column matrix of longitudinal displacements at layer centerlines
u_i	Longitudinal displacement at centerline of i^{th} layer
$u_{(0)}$	A column matrix of longitudinal displacements of structural ply centerlines in the rigid body translation parallel to x algebraic mode
$u_{(1)}$	A column matrix of longitudinal displacements of structural ply centerlines in the rigid body rotation about y algebraic mode
$u_{(2)}$	A column matrix of additional longitudinal displacements of structural ply centerlines in the shear parallel to z algebraic mode
$\bar{u}_{(2)}$	A partition of $u_{(2)}$
v	Column matrix of structural ply transverse shears
v_i	Shear in i^{th} layer

v_i, v	Vertical displacement at centerline of i^{th} layer
W	$k_A Q \bar{t}$
w_i	Normal load per unit length on lower surface of i^{th} layer
W_1, W_2	Partitions of W
x, y, z	Coordinates with origin at beam centerline
$Y_A(x)$	Rectangular matrix of the algebraic modes
$Y_{A_k}(x)$	k^{th} algebraic mode
$\left. \begin{array}{l} Y_{Au}(x), Y_{AH}(x) \\ Y_{Av}(x), Y_{A\theta}(x) \\ Y_{A\beta}(x), Y_{A\phi}(x) \end{array} \right\}$	Partitions of $Y_A(x)$ corresponding to the partitions $u, H, v, \theta, \beta,$ and ϕ of Y
Y_E	A column matrix representing the exponential beam response
Y_{E_ℓ}	A column matrix representing exponential beam responses corresponding to eigenvalues $\lambda_{\ell k}$
Y_{E_g}	A column matrix representing exponential beam responses corresponding to eigenvalues λ_{gk}
$Y(x)$	A column matrix of beam responses

α	A scalar coefficient of $u_{(0)}$ in the equation for $u_{(1)}$
β_i, β	Curvature of i^{th} layer = $d\theta_i/dx$
γ_i	Shear strain of i^{th} layer
Δ_n	A longitudinal displacement differencing matrix
Δq	A shear flow differencing matrix
Δ_{ts}	A Boolean coefficient matrix
$\bar{\Delta}_n$	Column matrix of longitudinal displacement differences at layer interfaces
ϵ_{x_i}	Axial strain of i^{th} layer
ζ_i	Vertical displacement of lower surface of i^{th} layer
η	Column matrix of longitudinal displacements at layer interfaces
η_i	Longitudinal displacement of lower surface of i^{th} layer
θ_i, θ	Slope of i^{th} layer = dv_i/dx
λ	An eigenvalue of A
λ_{g_k}	k^{th} eigenvalue of A greater than zero

λ_k	k^{th} eigenvalue of A
$\lambda_{\ell k}$	k^{th} eigenvalue of A less than zero
Σ	Summation sign
Σ_u	A Boolean coefficient matrix
ϕ	$d\beta/dx$
$\bar{\phi}_i$	Rotation of a vertical section of the i^{th} layer
ψ	Column matrix of constants in the boundary condition equation

SECTION I

INTRODUCTION

Bird impact hazards to high speed, low flying, aircraft have become one of the major flight safety problems of the jet age. The Douglas Aircraft Company's Windshield Technology Demonstrator Program is part of an effort by the Air Force Flight Dynamics Laboratory Improved Windshield Protection Program to develop technologies that will allow the design of aircraft transparent enclosures to increase protection against birdstrikes.

One of the basic objectives of the Windshield Technology Demonstrator Program is to develop windshield design technology for application to high performance military aircraft. Pursuant to this effort, a need arose for a simplified analytical method of rapidly assessing the structural effectiveness of candidate laminate configurations.

Typical laminated configurations consist of alternating structural plies and interlayers. Structural plies are composed of high modulus materials such as polycarbonate, glass or acrylic. Interlayers are composed of materials that may be less stiff by several orders of magnitude.

A realistic method of static analysis for a laminated beam carrying a concentrated load at the center, representing a bird impact, and having various types of end supports, would be a useful tool. Such a method should avoid the frequently made Bernoulli-Euler assumption that plane sections remain plain during deformation, because shear deformations in the soft interlayers are large and must be considered. The significance of these deformations is shown experimentally in Reference 1. Other rough assumptions that would unnecessarily diminish the effectiveness of the analysis should also be avoided. The method should accommodate laminates composed of nine layers or more, with different material properties for each layer.

Apparently no method meeting these requirements exists in the literature, although related work is described. Reference 2 contains an analysis of a three layered beam based on the assumption that the angle of rotation between a cross section of the center layer and its undeformed position is a constant factor times the slope along the length of the beam. This assumption is considered unnecessary, and its effects on the reliability of the results are hard to assess. The solution is based on the energy method, which introduces further approximations. Reference 3 presents methods of analysis of multiple layered beams based on the Bernoulli-Euler hypothesis; consequently, the approach is not applicable to beams with soft interlayers. Reference 4 offers an analysis of three layered beams involving simplifying assumptions which are also not considered acceptable for the present application.

Therefore, a new approach to the problem was developed, and is presented in subsequent sections of this report. The approach is based on the assumption that each structural ply can be treated as a beam to which the Bernoulli-Euler hypothesis is applicable, but no such assumption is applied to the cross section as a whole. Structural plies can bend and stretch, but shear deformations of these plies are considered negligible. Interlayers are assumed to carry shear, but not axial loads or bending moments. Deformations through the thickness of the beam are considered negligible, but stresses normal to the layers are assumed to exist. The equations of equilibrium and compatibility are written. The resulting set of differential equations is then expressed as a single matrix differential equation, which is solved exactly.

The analytical results have been translated into an efficient Fortran program. This program applies to nine layer laminates, which is an adequate number in most cases. The program can be easily extended to cover more layers, if necessary. Fewer layers can be accommodated by introducing negligible stiffness properties (E and G) for some of the layers. The method applies to fixed ended beams, but the program can be modified to be applicable to other boundary conditions. Extension of the method to other loading conditions is possible.

The most significant simplification involved in the analysis is the assumption that deformations through the thickness are negligible. This assumption greatly simplifies the analysis without slighting the primary feature of windshield laminate behavior, which is the relative freedom of structural plies to slide past each other because of the low stiffness of interlayer materials. The only negative effect of the assumption is that stresses normal to the layers are not correctly predicted. This is believed to be a localized effect confined to the center and ends of the beam where loads are applied. The elimination of this assumption is a possible subject for additional research. An analysis which accounts for transverse deformations might provide data that would be useful in defining adhesive strength needed to prevent delamination in regions of high transverse loads.

Results of the analysis have been correlated with finite element results and test data as described in a subsequent section. The comparisons are good and verify the validity of the basic assumptions.

Although the present method is intended to apply to fixed ended beams, it can also be applied to a beam with pinned ends by considering a fixed ended beam twice as long as the beam under consideration. The data output by the computer program between the quarter points of the fixed ended beam is applicable to the pin ended case.

SECTION II
ANALYSIS

DERIVATION OF EQUATIONS

This analysis applies to the fixed ended laminated beam of Figure 1. As the figure shows, L is the length of the beam, and F is the concentrated load acting at the center.

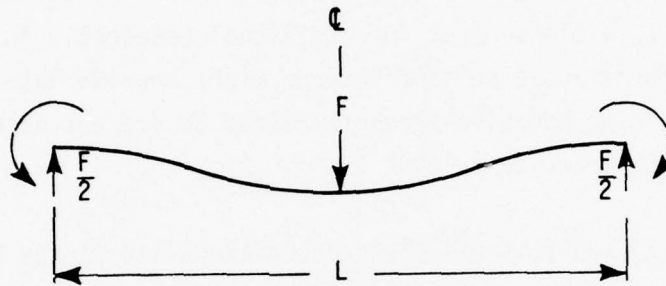


Figure 1. Fixed ended laminated beam.

The cross section is shown in Figure 2. The cross hatched layers are called "structural plies", while the other layers are "interlayers". The width of the beam is b , and the thickness of the i th layer is t_i . Different material properties can be assigned to each layer. The flexibilities of the interlayers are assumed to be large compared to the structural plies. The analysis can be applied to a beam having fewer than nine layers by assigning negligible values of Young's modulus, E , and the shear modulus, G , to some of the layers.

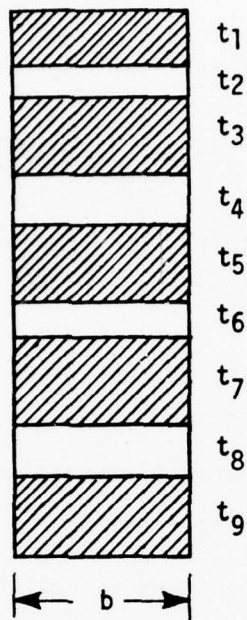


Figure 2. A typical transparency nine ply cross section.

Because of symmetry, consider one-half the beam as shown in Figure 3. In the figure $\ell = L/2$, and $P = F/2$. The figure also shows the xyz reference frame, and the definition of the vertical displacement v .

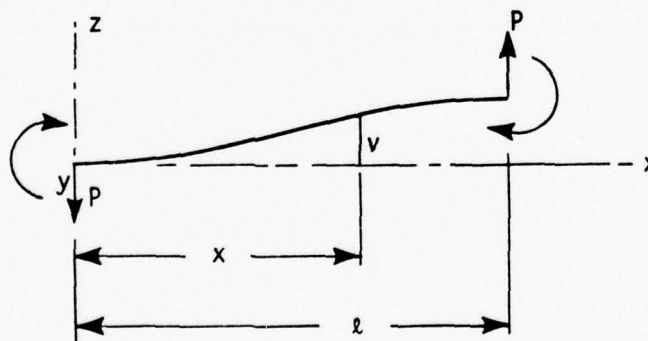


Figure 3. Half-beam.

Assumptions

1. Structural plies carry axial load, bending moment, shear, and normal load.
2. Interlayers carry shear and normal stresses only.
3. Plane sections of structural plies remain plane and normal to their elastic axes during deformation.
4. Normal strains through the thickness for structural plies and interlayers are negligible and can be considered equal to zero.
5. Shear strains for structural plies are negligible.
6. All stress-strain relations are linear.
7. Structural ply bending conforms to small displacement theory.

Equilibrium, Structural Ply Element, i Odd

Figure 4 shows the equilibrium of an element of the i th structural ply. H_i , V_i and M_i are the axial load, shear and bending moment acting on the element cross section. The transverse load per inch, w_i , and the shear flow, q_i , act upon the lower face. The corresponding forces on the upper face are w_{i-1} and q_{i-1} , since the $i-1$ st element is next above. Note that i is odd for structural plies.

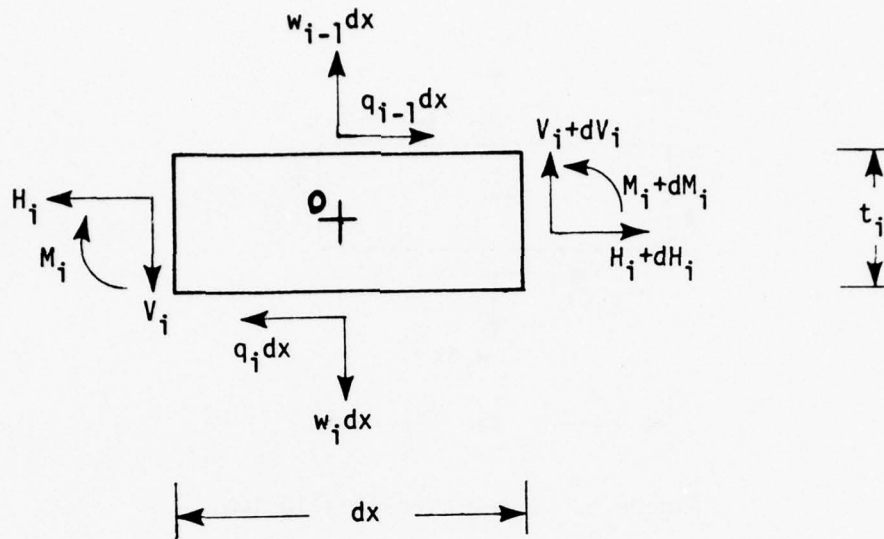


Figure 4. Structural ply equilibrium.

Summing forces in two directions and moments about 0 gives:

$$\left\{ \begin{array}{l} \frac{dH_i}{dx} + q_{i-1} - q_i = 0 \quad (i \text{ odd}) \quad (1) \\ \frac{dV_i}{dx} + w_{i-1} - w_i = 0 \quad (i \text{ odd}) \quad (2) \\ \frac{dM_i}{dx} + V_i - q_{i-1} \frac{t_i}{2} - q_i \frac{t_i}{2} = 0 \quad (i \text{ odd}) \quad (3) \end{array} \right.$$

where $1/2 dV_i$ has been deleted from Equation 3 to reduce complexity of problem. This is allowable due to insignificance of the term.

Equilibrium, Interlayer Element, i Even

Figure 5 shows the equilibrium of an element of the i th interlayer. The notation is consistent with the notation established for structural plies, although no axial loads or bending moments are acting, because of the assumption that these forces are negligible for interlayers. Note that i is even for these plies.

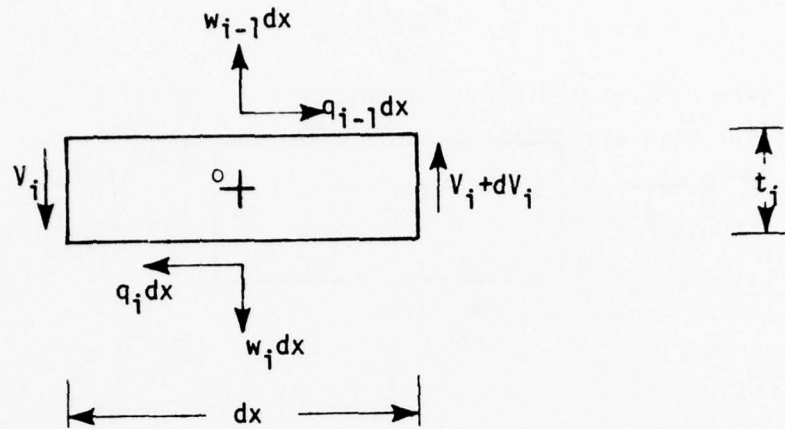


Figure 5. Interlayer equilibrium.

Summing forces in two directions and moments about 0 gives:

$$\left\{ \begin{array}{l} q_{i-1} - q_i = 0 \end{array} \right. \quad (i \text{ even}) \quad (4)$$

$$\left\{ \begin{array}{l} \frac{dV_i}{dx} + w_{i-1} - w_i = 0 \end{array} \right. \quad (i \text{ even}) \quad (5)$$

$$\left\{ \begin{array}{l} V_i - q_{i-1} \frac{t_i}{2} - q_i \frac{t_i}{2} = 0 \end{array} \right. \quad (i \text{ even}) \quad (6)$$

From Equation 4, $q_{i-1} = q_i$, and, from Equation 6, $V_i = q_i t_i$.

Therefore, from Equation 5,

$$t_i \frac{dq_i}{dx} + w_{i-1} - w_i = 0 \quad (i \text{ even}) . \quad (6a)$$

Also note that

$$q_0 = q_9 = 0 \quad (6b)$$

$$w_0 = w_9 = 0 \quad (6c)$$

since transverse forces and shear flows are assumed to be zero on the upper and lower surfaces of the beam.

Compatibility, Structural Plies, i Odd

Figure 6 shows the relationships between the displacements of the centerline of a structural ply and the displacements of the upper and lower surfaces of the same layer. In the figure, u_i , v_i and θ_i are the longitudinal and transverse displacements and the slope of the centerline of the i th ply. The longitudinal and transverse displacements of the lower surface of the i th ply are denoted η_i and ζ_i . Consequently the corresponding displacements of the upper surface are η_{i-1} and ζ_{i-1} , since the upper surface of the i th ply is the lower surface of the $(i-1)$ st ply.

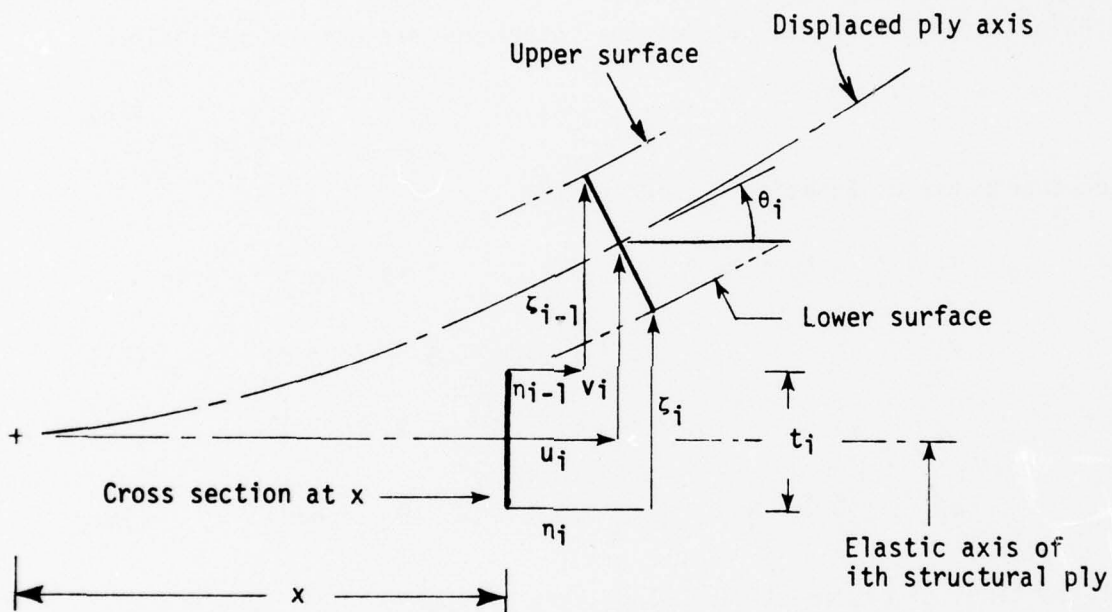


Figure 6. Compatibility.

Then from Figure 6,
$$\eta_{i-1} = u_i - \theta_i \frac{t_i}{2} \quad (7)$$

and
$$\eta_i = u_i + \theta_i \frac{t_i}{2} \text{ for small displacements.} \quad (8)$$

These equations are consistent with the assumption that cross sections of the structural ply remain normal to the elastic axis during deformation.

Also from Figure 6,

$$\zeta_{i-1} = v_i, \quad (9)$$

$$\zeta_i = v_i, \quad (10)$$

$$\text{and } \theta_i = \frac{dv_i}{dx}. \quad (11)$$

Equations 9 and 10 hold because strains through the thickness of structural plies are considered negligible. Equation 11 is consistent with the assumption that structural ply bending conforms to the theory of small displacements.

Compatibility, Interlayers, i Even.

Strains through the thickness of the interlayer are assumed negligible.

$$\therefore \zeta_{i-1} = \zeta_i. \quad (12)$$

In consequence of Equations 9, 10 and 12:

$$\zeta_0 = v_1 = \zeta_1 = \zeta_2 = v_3 = \zeta_3 = \zeta_4 = \dots = \zeta_7 = \zeta_8 = v_9 = \zeta_9,$$

$$\therefore v_i = v \quad i = 1, 3, \dots 9 \quad (i \text{ odd}) \quad (13)$$

$$\zeta_i = v \quad i = 0, 1, 2 \dots 9 \quad (\text{all } i) \quad (14)$$

$$\theta_i = \frac{dv}{dx} = \theta \quad i = 1, 3 \dots 9 \quad (i \text{ odd}) \quad (15)$$

All vertical displacements and slopes are the same.

Strains, Structural Plies, i Odd.

Longitudinal strain, ϵ , is equal to the derivative of longitudinal displacement:

$$\epsilon_{x_i} = \frac{du_i}{dx} \quad \text{axial strain.}$$

In small displacement theory, curvature β is equal to the rate of change of slope, therefore,

$$\beta_i = \frac{d\theta_i}{dx} = \frac{d\theta}{dx} = \beta . \quad (15a)$$

The notation, $\frac{d\beta}{dx} = \phi$, (15b)
to denote the rate of change of beam curvature, is useful subsequently.

Strains, Interlayers, i Even

Figure 7 shows the relationships between the shear strain of the i th interlayer, γ_i , the slope of the beam, θ , and the longitudinal displacement of the i th interlayer upper and lower surfaces. In the figure, $\bar{\phi}_i$ denotes the rotation of a cross section of the i th interlayer relative to its undisplaced position.

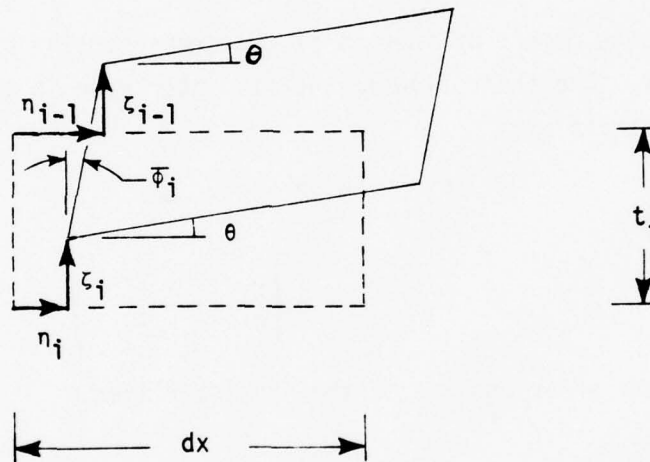


Figure 7. Interlayer shear strain.

From Figure 7,
$$\gamma_i = \theta + \bar{\phi}_i \quad (15c)$$

\therefore
$$\gamma_i = \frac{dv}{dx} + \frac{n_{i-1} - n_i}{t_i} , \quad (15d)$$

Force-Deformation Relations

The longitudinal force on the i th interlayer is given in terms of the longitudinal strain by

$$H_i = E_i A_i \epsilon_{x_i} .$$

$$\therefore H_i = E_i A_i \frac{du_i}{dx} \quad (i \text{ odd}) \quad (16)$$

where E_i and A_i are the values of Young's modulus and the cross-sectional area of the i th structural ply. The bending moment in the i th structural ply is given in terms of the curvature by

$$M_i = E_i I_i \beta_i .$$

$$\therefore M_i = E_i I_i \frac{d^2v}{dx^2} \quad (i \text{ odd}) \quad (17)$$

where I_i is the moment of inertia of the cross section of the i th structural ply. The shear flow in the i th interlayer is given in terms of the shear strain by

$$q_i = G_i b \gamma_i .$$

$$\therefore q_i = G_i b \left(\frac{dv}{dx} + \frac{\eta_i - 1 - \eta_i}{t_i} \right) \quad (i \text{ even}) \quad (18)$$

where G_i is the shear modulus of the i th interlayer.

Matric Formulation

Equation 1 can be written for each structural ply thus:

$$\frac{dH_1}{dx} + q_0 - q_1 = 0$$

$$\frac{dH_3}{dx} + q_2 - q_3 = 0$$

$$\frac{dH_5}{dx} + q_4 - q_5 = 0$$

$$\frac{dH_7}{dx} + q_6 - q_7 = 0$$

$$\frac{dH_9}{dx} + q_8 - q_9 = 0$$

But, from Equations 4 and 6b:

$$q_0=0 \quad q_1=q_2 \quad q_3=q_4 \quad q_5=q_6 \quad q_7=q_8 \quad q_9=0$$

$$\therefore \frac{dH_1}{dx} - q_2 = 0 ,$$

$$\frac{dH_3}{dx} + q_2 - q_4 = 0 ,$$

$$\frac{dH_5}{dx} + q_4 - q_6 = 0 ,$$

$$\frac{dH_7}{dx} + q_6 - q_8 = 0 ,$$

$$\frac{dH_9}{dx} + q_8 = 0 .$$

Writing these equations in matrix form gives:

$$\begin{pmatrix} \frac{dH_1}{dx} \\ \frac{dH_3}{dx} \\ \frac{dH_5}{dx} \\ \frac{dH_7}{dx} \\ \frac{dH_9}{dx} \end{pmatrix} + \begin{bmatrix} -1 & & & & \\ & 1 & & & \\ & & -1 & & \\ & & & 1 & & \\ & & & & -1 & \\ & & & & & 1 \end{bmatrix} \begin{pmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \end{pmatrix} = 0 \quad (18a)$$

$$\frac{dH}{dx} \quad \Delta q \quad q$$

Equation 18a has been derived in detail to show the method of converting the scalar differential equations into matrix form. Subsequent matrix equations are derived in a similar manner, although the details are omitted.

The matrices in Equation 18a are denoted according to the symbols written under the equation. Thus, H is a column matrix of structural ply longitudinal forces, dH/dx is its derivative, q is a column matrix of interlayer shear flows, and Δ_q is a Boolean differencing matrix.

Therefore,

$$\frac{dH}{dx} + \Delta_q q = 0. \quad (19)$$

From Equations 3, 4 and 6b,

$$\begin{pmatrix} \frac{dM_1}{dx} \\ \frac{dM_3}{dx} \\ \frac{dM_5}{dx} \\ \frac{dM_7}{dx} \\ \frac{dM_9}{dx} \\ \frac{dM}{dx} \end{pmatrix} + \begin{pmatrix} V_1 \\ V_3 \\ V_5 \\ V_7 \\ V_9 \\ V \end{pmatrix} - \begin{bmatrix} \frac{t_1}{2} & & & & & \\ & \frac{t_3}{2} & & & & \\ & & \frac{t_5}{2} & & & \\ & & & \frac{t_7}{2} & & \\ & & & & \frac{t_9}{2} & \\ & & & & & \end{bmatrix} \begin{pmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \\ q \end{pmatrix} = 0.$$

M_{xq}

The matrices are denoted according to the symbols written under the equation. The symbol M denotes a column matrix of structural ply bending moments, dM/dx is the derivative of M, V is a column matrix of structural ply shear forces, and M_{xq} is a rectangular matrix of structural ply thicknesses.

$$\therefore \frac{dM}{dx} + V - M_{xq} q = 0. \quad (22)$$

From Equations 2, 6a and 6c:

$$\left. \begin{aligned} \frac{dV_1}{dx} - w_1 &= 0 \\ t_2 \frac{dq_2}{dx} + w_1 - w_2 &= 0 \\ \frac{dV_3}{dx} + w_2 - w_3 &= 0 \\ t_4 \frac{dq_4}{dx} + w_3 - w_4 &= 0 \\ \frac{dV_5}{dx} + w_4 - w_5 &= 0 \\ t_6 \frac{dq_6}{dx} + w_5 - w_6 &= 0 \\ \frac{dV_7}{dx} + w_6 - w_7 &= 0 \\ t_8 \frac{dq_8}{dx} + w_7 - w_8 &= 0 \\ \frac{dV_9}{dx} + w_8 &= 0 \end{aligned} \right\}$$

Adding these equations gives

$$\frac{dV_1}{dx} + \frac{dV_3}{dx} + \frac{dV_5}{dx} + \frac{dV_7}{dx} + \frac{dV_9}{dx} + t_2 \frac{dq_2}{dx} + t_4 \frac{dq_4}{dx} + t_6 \frac{dq_6}{dx} + t_8 \frac{dq_8}{dx} = 0 \quad (23)$$

The matrix form of this equation is:

$$\left[\begin{array}{c|c|c|c|c} 1 & & & & \\ \hline & 1 & & & \\ \hline & & 1 & & \\ \hline & & & 1 & \\ \hline & & & & 1 \end{array} \right] \begin{Bmatrix} dV_1/dx \\ dV_3/dx \\ dV_5/dx \\ dV_7/dx \\ dV_9/dx \end{Bmatrix} + \left[\begin{array}{c|c|c|c} t_2 & & & \\ \hline & t_4 & & \\ \hline & & t_6 & \\ \hline & & & t_8 \end{array} \right] \begin{Bmatrix} dq_2/dx \\ dq_4/dx \\ dq_6/dx \\ dq_8/dx \end{Bmatrix} = 0$$

$$\bar{c}^T \quad \frac{dV}{dx} \quad t_\ell^T \quad \frac{dq}{dx}$$

The matrices are denoted according to the symbols written under the equation. The matrices dV/dx and dq/dx are the derivatives of the column matrices V and q previously defined, \bar{c} is a Boolean column matrix, and t_ℓ is a column matrix of interlayer thicknesses. The superscript T indicates a transposed matrix.

$$\therefore \quad \bar{c}^T \frac{dV}{dx} + t_\ell^T \frac{dq}{dx} = 0 \quad (23a)$$

From Equation 16:

$$\begin{Bmatrix} H_1 \\ H_3 \\ H_5 \\ H_7 \\ H_9 \end{Bmatrix} = \left[\begin{array}{c|c|c|c|c} E_1 A_1 & & & & \\ \hline & E_3 A_3 & & & \\ \hline & & E_5 A_5 & & \\ \hline & & & E_7 A_7 & \\ \hline & & & & E_9 A_9 \end{array} \right] \begin{Bmatrix} du_1/dx \\ du_3/dx \\ du_5/dx \\ du_7/dx \\ du_9/dx \end{Bmatrix}$$

$$H \quad \quad \quad k_A \quad \quad \quad \frac{du}{dx}$$

The matrices are denoted according to the symbols written under the equation. The symbol u denotes a column matrix of longitudinal displacements of structural ply centerlines, du/dx is the derivative of u , and k_A is a diagonal matrix of structural ply axial stiffnesses.

$$\therefore \quad H = k_A \frac{du}{dx} \quad (24)$$

From Equation 17:

$$\begin{Bmatrix} M_1 \\ M_3 \\ M_5 \\ M_7 \\ M_9 \end{Bmatrix} = \begin{Bmatrix} E_1 I_1 \\ E_3 I_3 \\ E_5 I_5 \\ E_7 I_7 \\ E_9 I_9 \end{Bmatrix} \frac{d^2 v}{dx^2}$$

M k_I

The matrices are denoted according to the symbols written under the equation. The symbol k_I denotes a column matrix of structural ply bending stiffnesses.

$$\therefore M = k_I \frac{d^2 v}{dx^2} \quad (25)$$

From Equation 18:

$$\begin{Bmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \end{Bmatrix} = \begin{Bmatrix} G_2 b \\ G_4 b \\ G_6 b \\ G_8 b \end{Bmatrix} \frac{dv}{dx} + \begin{bmatrix} \frac{G_2 b}{t_2} & & & \\ & \frac{G_4 b}{t_4} & & \\ & & \frac{G_6 b}{t_6} & \\ & & & \frac{G_8 b}{t_8} \end{bmatrix} \begin{Bmatrix} n_1 - n_2 \\ n_3 - n_4 \\ n_5 - n_6 \\ n_7 - n_8 \end{Bmatrix}$$

q k_ℓ k_n Δn̄

The matrices are denoted according to the symbols written under the equation. The symbol Δn̄ denotes a column matrix of longitudinal displacement differences at layer interfaces, k_ℓ is a column matrix of interlayer shear stiffnesses, and k_n is a diagonal matrix of interlayer shear stiffnesses divided by interlayer thicknesses.

$$\therefore q = k_\ell \frac{dv}{dx} + k_n \overline{\Delta n}$$

But

$$\overline{\Delta n} = \Delta_n n$$

where

$$\Delta_n = \begin{bmatrix} 1 & -1 & & & \\ & & 1 & -1 & \\ & & & & 1 & -1 \\ & & & & & & 1 & -1 \end{bmatrix}$$

and

$$n = \{ n_1 | n_2 | n_3 | n_4 | n_5 | n_6 | n_7 | n_8 \} \quad (\text{column}) .$$

Δn is a rectangular differencing matrix, and n is a column matrix of longitudinal displacements of layer interfaces.

$$\therefore q = k_l \frac{dv}{dx} + k_n \Delta_n n . \quad (26)$$

From Equations 7, 8 and 15,

$$\begin{pmatrix} n_1 \\ n_2 \\ n_3 \\ n_4 \\ n_5 \\ n_6 \\ n_7 \\ n_8 \end{pmatrix} = \begin{pmatrix} u_1 \\ u_3 \\ u_3 \\ u_5 \\ u_5 \\ u_7 \\ u_7 \\ u_9 \end{pmatrix} + \begin{pmatrix} t_1/2 \\ -t_3/2 \\ t_3/2 \\ -t_5/2 \\ t_5/2 \\ -t_7/2 \\ t_7/2 \\ -t_9/2 \end{pmatrix} \theta$$

$$\therefore \begin{pmatrix} n_1 \\ n_2 \\ \cdot \\ \cdot \\ \cdot \\ n_8 \end{pmatrix} = \begin{bmatrix} 1 & & & & & & & \\ & 1 & & & & & & \\ & & 1 & & & & & \\ & & & 1 & & & & \\ & & & & 1 & & & \\ & & & & & 1 & & \\ & & & & & & 1 & \\ & & & & & & & 1 \end{bmatrix} \begin{pmatrix} u_1 \\ u_3 \\ u_3 \\ u_5 \\ u_5 \\ u_7 \\ u_7 \\ u_9 \end{pmatrix} + \frac{1}{2} \begin{bmatrix} 1 & & & & & & & \\ & -1 & & & & & & \\ & & -1 & & & & & \\ & & & -1 & & & & \\ & & & & -1 & & & \\ & & & & & -1 & & \\ & & & & & & -1 & \\ & & & & & & & -1 \end{bmatrix} \begin{pmatrix} t_1 \\ t_3 \\ t_5 \\ t_7 \\ t_9 \end{pmatrix} \theta$$

$n \qquad \qquad \Sigma_u \qquad \qquad u \qquad \qquad \Delta_{ts} \qquad \qquad t_s$

The matrices are denoted according to the symbols written under the equation. The symbol n denotes a column matrix of longitudinal displacements, t_s is a column matrix of structural ply thicknesses, and Σ_u and Δ_{ts} are Boolean coefficient matrices.

$$\therefore n = \Sigma_u u + \frac{1}{2} \Delta_{ts} t_s \frac{dv}{dx} \quad (27)$$

From Equation 24,

$$\frac{du}{dx} - a_{1,2} H = 0 \quad (28)$$

where $a_{1,2} = k_A^{-1}$. (29)

Eliminate n from Equations 26 and 27:

$$q = (k_\ell + k_n \cdot \frac{1}{2} \Delta_n \Delta_{ts} t_s) \frac{dv}{dx} + k_n \Delta_n \Sigma_u u$$

But $k_\ell = k_n t_\ell$, $\frac{1}{2} \Delta_n \Delta_{ts} t_s = \bar{t}_s$, and $\Delta_n \Sigma_u = -\Delta_q^T$,

where
$$\bar{t}_s = \begin{pmatrix} \frac{1}{2}(t_1 + t_3) \\ \frac{1}{2}(t_3 + t_5) \\ \frac{1}{2}(t_5 + t_7) \\ \frac{1}{2}(t_7 + t_9) \end{pmatrix}$$

and \bar{t}_s is a column matrix of average structural ply thicknesses.

$$\therefore q = k_n \bar{t} \frac{dv}{dx} - k_n \Delta_q^T u \quad (30)$$

where $\bar{t} = t_\ell + \bar{t}_s = \begin{pmatrix} \frac{1}{2}t_1 + t_2 + \frac{1}{2}t_3 \\ \frac{1}{2}t_3 + t_4 + \frac{1}{2}t_5 \\ \frac{1}{2}t_5 + t_6 + \frac{1}{2}t_7 \\ \frac{1}{2}t_7 + t_8 + \frac{1}{2}t_9 \end{pmatrix}$.

Eliminate q and $\frac{dv}{dx}$ from Equations 15, 19 and 30

$$\therefore \frac{dH}{dx} - a_{2,1} u - a_{2,3} \theta = 0 \quad (31)$$

where $a_{2,1} = \Delta_q k_n \Delta_q^T$ (32)

and $a_{2,3} = -\Delta_q k_n \bar{t}$. (33)

Eliminate V from Equations 22 and 23a.

$$\therefore \bar{t}_s^T \frac{dq}{dx} - \bar{c}^T \frac{d^2M}{dx^2} + t_\ell^T \frac{dq}{dx} = 0, \quad (34)$$

since $\bar{c}^T M_{xq} = \bar{t}_s^T$. Eliminate M from Equations 25 and 34.

$$\text{Then, } \bar{t}^T \frac{dq}{dx} - K_I \frac{d^4v}{dx^4} = 0 \quad (35)$$

$$\text{where } K_I = (EI)_1 + (EI)_3 + (EI)_5 + (EI)_7 + (EI)_9. \quad (36)$$

Therefore, from Equations 15, 15a, 15b, and 35

$$\bar{t}^T \frac{dq}{dx} - K_I \frac{d\phi}{dx} = 0. \quad (37)$$

Eliminate q from Equations 30 and 37. Then,

$$\frac{d\phi}{dx} - K_I^{-1} a_{2,3}^T \frac{du}{dx} - a_{5,4} \frac{d^2v}{dx^2} = 0 \quad (38)$$

$$\text{where } a_{5,4} = K_I^{-1} \bar{t}^T k_n \bar{t}. \quad (38a)$$

Therefore, from Equations 15, 15a, 28 and 38,

$$\frac{d\phi}{dx} - a_{5,2}^H - a_{5,4} \beta = 0, \quad (39)$$

$$\text{where } a_{5,2} = K_I^{-1} a_{2,3}^T a_{1,2}. \quad (40)$$

Equations 15, 15a, 15b, 28, 31 and 39 are a set of 14 linear, ordinary, first order, homogeneous differential equations with constant coefficients, in the 14 dependent variables u_i , H_i , v , θ , β and ϕ . The number of variables is 14 since the matrices u and H each contain five variables. The theory of differential equations shows that, for a set of equations of this kind, n linearly independent solutions exist, where n is the number of equations, equal to 14 in this case. See Reference 5, article 12-8.

Matric Differential Equation

From Equations 15, 15a, 15b, 28, 31 and 39,

$$\left\{ \begin{array}{l} du/dx \\ dH/dx \\ dv/dx \\ d\theta/dx \\ d\beta/dx \\ d\phi/dx \end{array} \right\} - \left[\begin{array}{c|c|c|c|c} a_{1,2} & & & & \\ \hline a_{2,1} & & a_{2,3} & & \\ \hline & & 1 & & \\ \hline & & & 1 & \\ \hline & a_{5,2} & & a_{5,4} & \\ \hline \end{array} \right] \left\{ \begin{array}{l} u \\ H \\ v \\ \theta \\ \beta \\ \phi \end{array} \right\} = 0 \quad (41)$$

$\frac{dY}{dx} \qquad \qquad \qquad A \qquad \qquad \qquad Y$

The matrices are denoted according to the symbols written under the equation. The symbol Y denotes a column matrix of beam responses, dY/dx is its derivative with respect to x , and A is a square matrix of constant coefficients.

$$\therefore \frac{dY}{dx} - AY = 0 \quad (42)$$

This linear matric differential equation can be solved by taking

$$Y = Ge^{\lambda x} \quad (43)$$

where G is an unknown column matrix, λ is an unknown scalar and e is the base of natural logarithms. Eliminating Y from Equations 42 and 43 gives

$$(A - \lambda I)G = 0 \quad (44)$$

Evidently, λ and G are an eigenvalue and an eigenvector of the characteristic Equation 44. Therefore the solution of Equation 42 can be written

$$Y = \sum_{k=1}^{14} C_k G_k e^{\lambda_k x} \quad (45)$$

where G_k , and λ_k are the k th eigenvector and eigenvalue of Equation 44, and the C_k 's are arbitrary constants needed to satisfy the boundary conditions.

Nature of the Roots of the Characteristic Equation

The eigenvalues (roots) of Equation 44 are expected to be all real, from the nature of the physical problem. The roots can be easily shown to occur in pairs equal in magnitude and opposite in sign, such that if λ_α and λ_β are a pair of roots, then $\lambda_\beta + \lambda_\alpha = 0$. (See Appendix A.) It is shown subsequently that at least six repeated roots, equal to zero, exist. No more than six such roots are expected, because of the nature of the problem.

Algebraic Modes

The solutions of Equation 42 of the form given by Equation 43 are here called exponential solutions or exponential modes. The following paragraphs show that other solutions exist which are algebraic functions of x . Consequently, these solutions are called algebraic solutions or algebraic modes. Six linearly independent algebraic modes are shown to exist. These solutions are also linearly independent of the exponential solutions, since they are algebraic. Therefore, the characteristic equation (Equation 44) can yield at most only eight linearly independent exponential modes. Consequently, six of the roots of Equation 44 must be zero.

In view of the existence of the algebraic modes, the complete solution of Equation 42 can be written in the form

$$Y = \sum_{k=1}^8 C_k G_k e^{\lambda_k x} + \sum_{k=1}^6 C_{A_k} Y_{A_k} \quad (46)$$

where the C_{A_k} 's are arbitrary constants, and the Y_{A_k} 's (algebraic modes) are column matrices whose elements are algebraic functions of x . The only requirements for the Y_{A_k} 's is that they must satisfy Equation 42, and the G_k 's and Y_{A_k} 's must be linearly independent. The following Y_{A_k} 's satisfy these requirements:

Rigid Body Translation Parallel to x -

$$Y_{A_1} = \left\{ \begin{array}{c} u(0) \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{array} \right\} \text{ } \left. \vphantom{\begin{array}{c} u(0) \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{array}} \right\} \text{ 5 rows} \quad \text{where } u(0) = \left\{ \begin{array}{c} 1 \\ 1 \\ 1 \\ 1 \\ 1 \end{array} \right\} \quad (47)$$

Uniform Extension Parallel to x -

$$Y_{A_2} = \left\{ u(0)x \mid k_A u(0) \mid 0 \mid 0 \mid 0 \mid 0 \right\} \text{ (Column)} \quad (48)$$

Rigid Body Translation Parallel to z -

$$Y_{A_3} = \left\{ \underbrace{0 \mid 0}_{\substack{5 \\ \text{rows}}} \mid \underbrace{1 \mid 0 \mid 0 \mid 0}_{\substack{5 \\ \text{rows}}} \right\} \text{ (Column)} \quad (49)$$

Rigid Body Rotation about y -

$$Y_{A_4} = \left\{ u(1) \mid \underbrace{0 \mid x}_{\substack{5 \text{ rows}}} \mid 1 \mid 0 \mid 0 \right\} \text{ (Column)} \quad (50)$$

$$\text{where } u(1) = Q\bar{t} + \alpha u(0) \quad Q = \begin{bmatrix} -1 & -1 & -1 & -1 \\ -1 & -1 & -1 \\ -1 & -1 \\ -1 \\ 0 & 0 & 0 & 0 \end{bmatrix} \quad \alpha = \text{a scalar} \quad (51)$$

Bending about y -

$$Y_{A_5} = \left\{ u(1)x \mid k_A u(1) \mid x^2/2 \mid x \mid 1 \mid 0 \right\} \text{ (Column)} \quad (52)$$

Shear Parallel to z -

$$Y_{A_6} = \left\{ \frac{1}{2}u_{(1)}x^2 + u_{(2)} \mid k_A u_{(1)}x \mid \frac{x^3}{6} \mid \frac{x^2}{2} \mid x \mid 1 \right\} \text{ (Column)} \quad (53)$$

where $u_{(2)}$ = a column matrix with 5 rows.

α and $u_{(2)}$ must satisfy the equation

$$S u_{(2)} - T \alpha = W \quad (54)$$

$$\text{where } S = \Delta_q k_n \Delta_q^T, \quad T = k_A u_{(0)}, \quad W = k_A Q \bar{t}. \quad (55)$$

The matrix S is singular. Equation 54 is a matrix equation equivalent to five scalar simultaneous equations in the five unknown elements of $u_{(2)}$ and the unknown α . However one element of $u_{(2)}$ may be chosen arbitrarily since S has one degree of singularity. In this respect $u_{(2)}$ has the character of an eigenvector. Partition S , T , W and $u_{(2)}$:

$$S = \underbrace{\begin{bmatrix} S_{1,1} & S_{1,2} \\ S_{2,1} & S_{2,2} \end{bmatrix}}_{\substack{1 \text{ col.} & 4 \text{ col.}}} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad T = \left\{ \begin{array}{l} T_1 \\ T_2 \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad W = \left\{ \begin{array}{l} W_1 \\ W_2 \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad (56)$$

$$u_{(2)} = \left\{ \begin{array}{l} 0 \\ \bar{u}_{(2)} \end{array} \right\} \left\{ \begin{array}{l} 1 \text{ row} \\ 4 \text{ rows} \end{array} \right\} \quad (57)$$

where the element in the first row of $u_{(2)}$ has been chosen equal to zero.

From Equations 54, 56 and 57,

$$\begin{cases} S_{1,2} \bar{u}_{(2)} - T_1 \alpha = W_1 \\ S_{2,2} \bar{u}_{(2)} - T_2 \alpha = W_2 \end{cases} \quad (58)$$

$$\therefore \alpha = \frac{1}{T_1} [S_{1,2} \bar{u}(2) - W_1] \quad (59)$$

$$\text{and } \bar{u}(2) = (S_{2,2} - \frac{1}{T_1} T_2 S_{1,2})^{-1} (W_2 - \frac{1}{T_1} T_2 W_1) \quad (60)$$

Nature of the Algebraic Solutions

The first five algebraic solutions ($Y_{A_1}, Y_{A_2}, \dots, Y_{A_5}$) correspond to solutions provided by the engineering theory of tension and bending of beams. These solutions have been appropriately named to reflect this fact. Thus Y_{A_1} can be recognized as corresponding to a rigid body translation of the beam parallel to x , since the longitudinal motions of the centerlines of the structural plies are all equal to 1, and all other responses are zero. The mode Y_{A_4} can be recognized as a rigid body rotation about the centerline of the bottom structural ply, represented by $Q\bar{\epsilon}$, plus a rigid body translation parallel to x , represented by $\alpha u(0)$. The mode Y_{A_6} corresponds to the responses calculated by engineering theory when the beam is subjected to a constant shear load. However the responses given by elementary theory involve an approximation, whereas the responses given by Y_{A_6} exactly satisfy the differential equations.

Response

Appendix A shows that the eigenvalues λ_k occur in pairs, such that for a pair λ_α and λ_β ,

$$\lambda_\beta = -\lambda_\alpha \quad (61)$$

Appendix A also shows that the corresponding eigenvectors have the property that if

$$G_{\alpha} = \begin{pmatrix} G_{u_{\alpha}} \\ G_{H_{\alpha}} \\ G_{v_{\alpha}} \\ G_{\theta_{\alpha}} \\ G_{\beta_{\alpha}} \\ G_{\phi_{\alpha}} \end{pmatrix}, \text{ then } G_{\beta} = \begin{pmatrix} G_{u_{\alpha}} \\ -G_{H_{\alpha}} \\ -G_{v_{\alpha}} \\ G_{\theta_{\alpha}} \\ -G_{\beta_{\alpha}} \\ G_{\phi_{\alpha}} \end{pmatrix} \quad (62)$$

where $G_{u_{\alpha}}$, $G_{H_{\alpha}}$ etc. are partitions of G_{α} corresponding to the partitions u, H etc. of the matrix Y (Equation 41). Because half of the eigenvalues λ are positive, the corresponding functions $e^{\lambda x}$ can become very large as x increases. To avoid this difficulty, proceed as follows: See Equation 46. Let

$$Y_E = \sum_{k=1}^8 C_k G_k e^{\lambda_k x} \quad (\text{exponential response})$$

$$= Y_{El} + Y_{Eg} \quad (63)$$

where

$$Y_{El} = \sum_{k=1}^4 C_{l_k} G_{l_k} e^{\lambda_{l_k} x}, \quad Y_{Eg} = \sum_{k=1}^4 \bar{C}_{g_k} G_{g_k} e^{\lambda_{g_k} x}, \quad (64)$$

and where λ_{l_k} and λ_{g_k} are a pair of eigenvalues having the properties $\lambda_{l_k} = -\lambda_{g_k}$, $\lambda_{g_k} > 0$. G_{l_k} and G_{g_k} are corresponding eigenvectors. Now

$$Y_{Eg} = \sum_{k=1}^4 \bar{C}_{g_k} G_{g_k} e^{\lambda_{g_k}(x-l+l)} \quad (65)$$

$$= \sum_{k=1}^4 \bar{C}_{g_k} G_{g_k} e^{-\lambda_{g_k}(l-x)} e^{\lambda_{g_k} l} \quad (66)$$

$$= \sum_{k=1}^4 C_{g_k} G_{g_k} e^{\lambda_{l_k}(l-x)} \quad (67)$$

where $C_{g_k} = \bar{C}_{g_k} e^{\lambda_{g_k} l}$. (68)

From Equation 46:

$$Y = \sum_{k=1}^4 G_{\ell_k} e^{\lambda_{\ell_k} x} C_{\ell_k} + \sum_{k=1}^4 G_{g_k} e^{\lambda_{\ell_k}(\ell-x)} C_{g_k} + \sum_{k=1}^6 C_{A_k} Y_{A_k}. \quad (69)$$

Thus the troublesome eigenvalues λ_{g_k} , which are greater than zero, have been lumped into the arbitrary constants C_{g_k} , and thus eliminated. The functions $e^{\lambda_{\ell_k} x}$ and $e^{\lambda_{\ell_k}(\ell-x)}$ are less than or equal to 1 for $0 < x < \ell$, since $\lambda_{\ell_k} < 0$. Let

$$F_{\ell_k}(x) = e^{\lambda_{\ell_k} x} \quad \text{and} \quad F_{g_k}(x) = e^{\lambda_{\ell_k}(\ell-x)} \quad (70)$$

$$\therefore Y = \sum_{k=1}^4 G_{\ell_k} F_{\ell_k}(x) C_{\ell_k} + \sum_{k=1}^4 G_{g_k} F_{g_k}(x) C_{g_k} + \sum_{k=1}^6 C_{A_k} Y_{A_k} \quad (71)$$

$$\therefore Y(x) = H_{\ell} F_{\ell D}(x) C_{\ell} + H_g F_{g D}(x) C_g + Y_A(x) C_A \quad (72)$$

where the symbols in this equation are defined by Equations 73, 74, 75 and 7

$$H_{\ell} = \begin{bmatrix} G_{\ell_1} & G_{\ell_2} & G_{\ell_3} & G_{\ell_4} \end{bmatrix} \quad H_g = \begin{bmatrix} G_{g_1} & G_{g_2} & G_{g_3} & G_{g_4} \end{bmatrix} \quad (73)$$

$$F_{\ell}(x) = \begin{Bmatrix} F_{\ell_1}(x) \\ F_{\ell_2}(x) \\ F_{\ell_3}(x) \\ F_{\ell_4}(x) \end{Bmatrix} \quad F_g(x) = \begin{Bmatrix} F_{g_1}(x) \\ F_{g_2}(x) \\ F_{g_3}(x) \\ F_{g_4}(x) \end{Bmatrix} \quad (74)$$

$F_{\ell D}(x)$, $F_{g D}(x)$ = $F_{\ell}(x)$, $F_g(x)$ diagonalized

$$C_{\ell} = \begin{Bmatrix} C_{\ell_1} \\ C_{\ell_2} \\ C_{\ell_3} \\ C_{\ell_4} \end{Bmatrix} \quad C_g = \begin{Bmatrix} C_{g_1} \\ C_{g_2} \\ C_{g_3} \\ C_{g_4} \end{Bmatrix} \quad C_A = \begin{Bmatrix} C_{A_1} \\ C_{A_2} \\ \vdots \\ C_{A_6} \end{Bmatrix} \quad (75)$$

$$Y_A(x) = \left[Y_{A_1}(x) \mid Y_{A_2}(x) \mid \dots \mid Y_{A_6}(x) \right] \cdot \quad (76)$$

Partition $Y(x)$, H_ℓ , H_g , $Y_A(x)$:

$$\begin{pmatrix} u(x) \\ H(x) \\ v(x) \\ \theta(x) \\ \beta(x) \\ \phi(x) \end{pmatrix} = \begin{pmatrix} H_{u\ell} \\ H_{H\ell} \\ H_{v\ell} \\ H_{\theta\ell} \\ H_{\beta\ell} \\ H_{\phi\ell} \end{pmatrix} F_{\ell D}(x) C_\ell + \begin{pmatrix} H_u \\ -H_{H\ell} \\ -H_{v\ell} \\ H_{\theta\ell} \\ -H_{\beta\ell} \\ H_{\phi\ell} \end{pmatrix} F_{gD}(x) C_g + \begin{pmatrix} Y_{Au}(x) \\ Y_{AH}(x) \\ Y_{Av}(x) \\ Y_{A\theta}(x) \\ Y_{AB}(x) \\ Y_{A\phi}(x) \end{pmatrix} C_A \quad (77)$$

$Y(x) \qquad H_\ell \qquad H_g \qquad Y_A(x)$

The matrices are denoted according to the symbols written under the equation. The partitions of $Y_A(x)$ are defined, from Equation 76, by the following equations:

$$Y_{Au}(x) = \left[u(0) \mid u(0)x \mid 0 \mid u(1) \mid u(1)x \mid \frac{1}{2}u(1)x^2 + u(2) \right] \quad (78)$$

$$Y_{AH}(x) = \left[0 \mid k_A u(0) \mid 0 \mid 0 \mid k_A u(1) \mid k_A u(1)x \right] \quad (79)$$

$$Y_{Av}(x) = \left[0 \mid 0 \mid 1 \mid x \mid x^2/2 \mid x^3/6 \right] \quad (80)$$

$$Y_{A\theta}(x) = \left[0 \mid 0 \mid 0 \mid 1 \mid x \mid x^2/2 \right] \quad (81)$$

$$Y_{AB}(x) = \left[0 \mid 0 \mid 0 \mid 0 \mid 1 \mid x \right] \quad (82)$$

$$Y_{A\phi}(x) = \left[0 \mid 0 \mid 0 \mid 0 \mid 0 \mid 1 \right] \quad (83)$$

Interlayer Shear Flows, Structural Ply Shears and Moments

From Equations 15 and 30

$$q = k_n \bar{\epsilon} \theta - k_n \Delta_q^T u \quad (84)$$

From Equations 15b, 22 and 25,

$$V = V_q q - k_I \phi \quad (85)$$

where $V_q = M_{xq}$.

From Equations 15a and 25

$$M = k_I \beta \quad (86)$$

Let $S(x)$ = total shear on the cross section at station x .

$$\therefore S = \begin{bmatrix} 1 & | & 1 & | & 1 & | & 1 & | & 1 \end{bmatrix} \begin{Bmatrix} V_1 \\ V_3 \\ V_5 \\ V_7 \\ V_9 \end{Bmatrix} + \begin{bmatrix} t_2 & | & t_4 & | & t_6 & | & t_8 \end{bmatrix} \begin{Bmatrix} q_2 \\ q_4 \\ q_6 \\ q_8 \end{Bmatrix} \quad (87)$$

$u_{(0)}^T$ V t_{ℓ}^T q

The matrices are denoted according to the symbols written under the equation.

Therefore $S = u_{(0)}^T V + t_{\ell}^T q \quad (88)$

Eliminate V from Equations 85 and 88,

$$\therefore S = \bar{\epsilon}^T q - K_I \phi \quad (89)$$

Eliminate q from Equations 84 and 89,

$$\therefore S = a_{2,3}^T u + K_I a_{5,4} \theta - K_I \phi \quad (90)$$

Boundary Conditions

Figure 8 shows the boundary conditions for a fixed ended beam.

At $x = 0$ -

$$\begin{cases} u(0) = 0 & (5 \text{ conditions}) \\ v(0) = 0 & (1 \text{ condition}) \\ \theta(0) = 0 & (1 \text{ condition}) \end{cases}$$

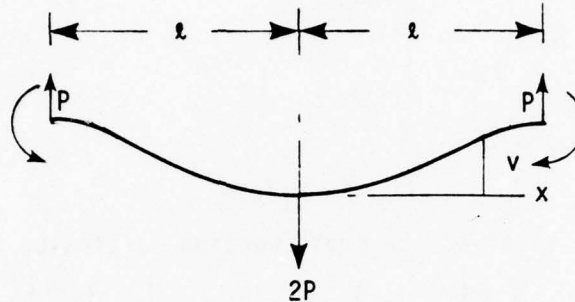


Figure 8. Boundary conditions.

For all x -

The total shear on the cross section is constant

$$\therefore S(x) = P.$$

$S(x)$ is most easily evaluated at $X = 0$, therefore take

$$S(0) = P \quad (1 \text{ condition}).$$

From Equation 90,

$$-K_I \phi(0) = P \quad \text{and} \quad \phi(0) = -K_I^{-1} P, \quad (92)$$

since $u(0) = \theta(0) = 0$.

Condition 1. Fixed Ends at $x = \pm l$. For $x = l$ -

$$\begin{cases} u(l) = 0 & (5 \text{ conditions}) \\ \theta(l) = 0 & (1 \text{ condition}) \end{cases} \quad (93)$$

From Equation 77

$$\left. \begin{aligned}
 H_{u\ell} F_{\ell D}(0) C_{\ell} + H_{u\ell} F_{gD}(0) C_g + Y_{Au}(0) C_A &= 0 \\
 H_{v\ell} F_{\ell D}(0) C_{\ell} - H_{v\ell} F_{gD}(0) C_g + Y_{Av}(0) C_A &= 0 \\
 H_{\theta\ell} F_{\ell D}(0) C_{\ell} + H_{\theta\ell} F_{gD}(0) C_g + Y_{A\theta}(0) C_A &= 0 \\
 H_{\phi\ell} F_{\ell D}(0) C_{\ell} + H_{\phi\ell} F_{gD}(0) C_g + Y_{A\phi}(0) C_A &= -K_I^{-1} P \\
 H_{u\ell} F_{\ell D}(\ell) C_{\ell} + H_{u\ell} F_{gD}(\ell) C_g + Y_{Au}(\ell) C_A &= 0 \\
 H_{\theta\ell} F_{\ell D}(\ell) C_{\ell} + H_{\theta\ell} F_{gD}(\ell) C_g + Y_{A\theta}(\ell) C_A &= 0
 \end{aligned} \right\} \quad (94)$$

$$\therefore \left[\begin{array}{ccc|c}
 H_{u\ell} F_{\ell D}(0) & H_{u\ell} F_{gD}(0) & Y_{Au}(0) & C_{\ell} \\
 H_{v\ell} F_{\ell D}(0) & -H_{v\ell} F_{gD}(0) & Y_{Av}(0) & C_g \\
 H_{\theta\ell} F_{\ell D}(0) & H_{\theta\ell} F_{gD}(0) & Y_{A\theta}(0) & C_A \\
 H_{\phi\ell} F_{\ell D}(0) & H_{\phi\ell} F_{gD}(0) & Y_{A\phi}(0) & \\
 H_{u\ell} F_{\ell D}(\ell) & H_{u\ell} F_{gD}(\ell) & Y_{Au}(\ell) & \\
 H_{\theta\ell} F_{\ell D}(\ell) & H_{\theta\ell} F_{gD}(\ell) & Y_{A\theta}(\ell) &
 \end{array} \right] \begin{pmatrix} C_{\ell} \\ C_g \\ C_A \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ -K_I^{-1} P \\ 0 \\ 0 \end{pmatrix} \quad (95)$$

B
C
 ψ

The matrices are denoted according to the symbols written under the equation. The symbol C denotes a column matrix of arbitrary constants, ψ is a column matrix of boundary conditions, and B is a coefficient matrix.

$$\therefore BC = \psi \quad (96)$$

From Equations 70 and 74

$$F_{\ell D}(0) = F_{gD}(\ell) = I \quad (97)$$

$$\text{and } F_{\ell D}(\ell) = F_{gD}(0) \quad (98)$$

where I is a unit (identity) matrix. Therefore; $B =$

$$\begin{bmatrix}
 H_{u\ell} & H_{u\ell}F_{\ell D}(\ell) & u(0) & 0 & 0 & u(1) & 0 & u(2) \\
 H_{v\ell} & -H_{v\ell}F_{\ell D}(\ell) & 0 & 0 & 1 & 0 & 0 & 0 \\
 H_{\theta\ell} & H_{\theta\ell}F_{\ell D}(\ell) & 0 & 0 & 0 & 1 & 0 & 0 \\
 H_{\phi\ell} & H_{\phi\ell}F_{\ell D}(\ell) & 0 & 0 & 0 & 0 & 0 & 1 \\
 H_{u\ell}F_{\ell D}(\ell) & H_{u\ell} & u(0) & u(0)\ell & 0 & u(1) & u(1)\ell & \frac{1}{2}u(1)\ell^2 + u(2) \\
 H_{\theta\ell}F_{\ell D}(\ell) & H_{\theta\ell} & 0 & 0 & 0 & 1 & \ell & \frac{\ell^2}{2}
 \end{bmatrix}$$

... (99)

Effective Beam Stiffness

For a fixed-ended monolithic beam carrying a concentrated load at the center,

$$v(\ell) = \frac{P\ell^3}{12EI} \quad (100)$$

Therefore define the effective stiffness of the laminated beam as

$$EI_{\text{eff}} = \frac{P\ell^3}{12v(\ell)} \quad (101)$$

EQUATION SUMMARY

This section summarizes the equations for the laminated beam. Figure 8a shows the basic beam dimensions.

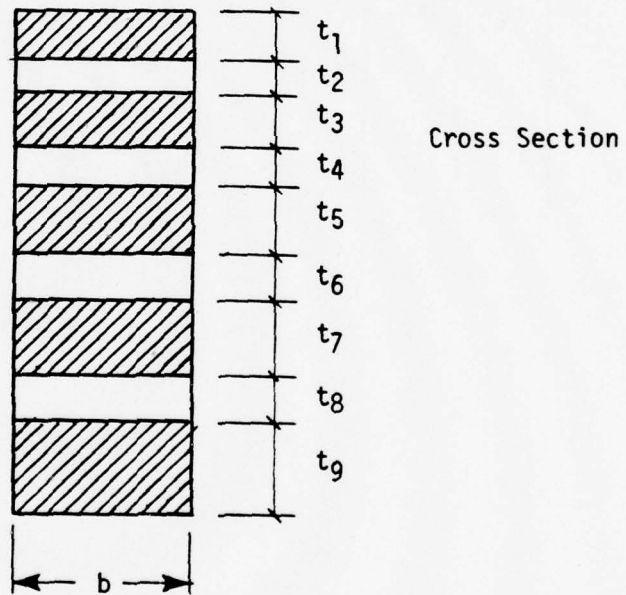
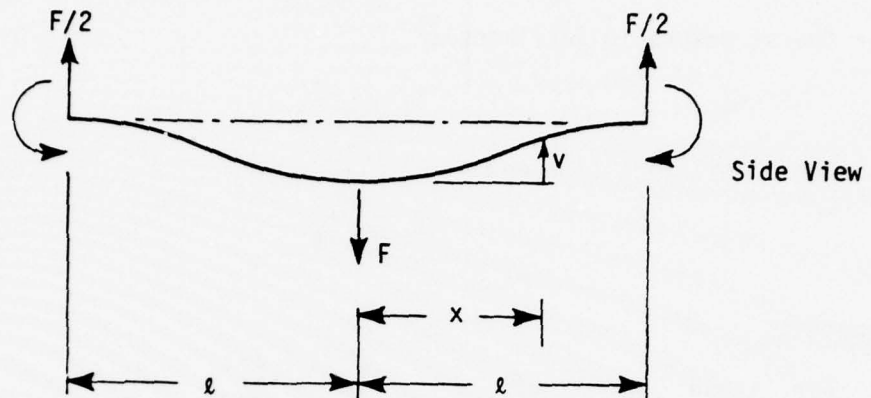


Figure 8a. Beam dimensions.

Odd numbered layers are structural plies.
Even numbered layers are interlayers.

Input

F, ℓ , b, $x = x_j$, $j = 0, 1, 2, \dots, n_x$ where $x_0 = 0$, $x_{n_x} = \ell$.

For the structural plies (i odd):

$$t_i, E_i$$

For the interlayers (i even):

$$t_i, G_i$$

Equations

For i odd: $A_i = bt_i$ $I_i = \frac{bt^3_i}{12}$ (102)

$$k_A = \begin{bmatrix} E_1 A_1 \\ E_3 A_3 \\ E_5 A_5 \\ E_7 A_7 \\ E_9 A_9 \end{bmatrix} \quad k_n = \begin{bmatrix} \frac{G_2 b}{t_2} \\ \frac{G_4 b}{t_4} \\ \frac{G_6 b}{t_6} \\ \frac{G_8 b}{t_8} \end{bmatrix} \quad \dots (103)$$

$$K_I = E_1 I_1 + E_3 I_3 + E_5 I_5 + E_7 I_7 + E_9 I_9 \quad (104)$$

$$\bar{t} = \begin{pmatrix} \frac{1}{2}t_1 + t_2 + \frac{1}{2}t_3 \\ \frac{1}{2}t_3 + t_4 + \frac{1}{2}t_5 \\ \frac{1}{2}t_5 + t_6 + \frac{1}{2}t_7 \\ \frac{1}{2}t_7 + t_8 + \frac{1}{2}t_9 \end{pmatrix} \quad \Delta_q = \begin{bmatrix} -1 & & & \\ 1 & -1 & & \\ & 1 & -1 & \\ & & 1 & -1 \\ & & & 1 \end{bmatrix} \quad (105)$$

$$\left. \begin{aligned}
 a_{1,2} &= k_A^{-1} \\
 a_{2,1} &= \Delta_q k_n \Delta_q^T \\
 a_{2,3} &= -\Delta_q k_n \bar{t} \\
 a_{5,2} &= K_I^{-1} a_{2,3}^T a_{1,2} \\
 a_{5,4} &= K_I^{-1} \bar{t}^T k_n \bar{t}
 \end{aligned} \right\} \quad (106)$$

$$A =_{14 \times 14} \begin{bmatrix}
 0 & a_{1,2} & 0 & 0 & 0 & 0 \\
 a_{2,1} & 0 & 0 & a_{2,3} & 0 & 0 \\
 0 & 0 & 0 & 1 & 0 & 0 \\
 0 & 0 & 0 & 0 & 1 & 0 \\
 0 & 0 & 0 & 0 & 0 & 1 \\
 0 & a_{5,2} & 0 & 0 & a_{5,4} & 0
 \end{bmatrix} \quad (107)$$

Find the eigenvalues and eigenvectors of

$$(A - \lambda I)G = 0 \quad (108)$$

Six of the eigenvalues should be zero. The remaining eight eigenvalues should be real and should occur in pairs such that for any pair, λ_ℓ and λ_g ,

$$\lambda_\ell < 0, \quad \lambda_g > 0 \quad \text{and} \quad \lambda_\ell + \lambda_g = 0 \quad (109)$$

If the eigenvalues do not occur in this manner, print an error message. Otherwise, discard the eigenvalues which are equal to or greater than zero, and the corresponding eigenvectors. Denote the remaining eigenvalues and eigenvectors λ_{ℓ_i} and G_{ℓ_i} , $i = 1, 2, 3, 4$.

Form

$$H_\ell = \left[G_{\ell 1} \mid G_{\ell 2} \mid G_{\ell 3} \mid G_{\ell 4} \right] \quad F_\ell(\ell) = \begin{pmatrix} \lambda_{\ell 1}^\ell \\ e \\ \lambda_{\ell 2}^\ell \\ e \\ \lambda_{\ell 3}^\ell \\ e \\ \lambda_{\ell 4}^\ell \\ e \end{pmatrix} \quad (110)$$

$F_{\ell D}(\ell) = F_\ell(\ell)$ diagonalized. Partition H_ℓ :

$$H_\ell = \begin{bmatrix} H_{u\ell} \\ H_{H\ell} \\ H_{V\ell} \\ H_{\theta\ell} \\ H_{\beta\ell} \\ H_{\phi\ell} \end{bmatrix} \quad \text{Form } u(0) = \begin{pmatrix} 1 \\ 1 \\ 1 \\ 1 \\ 1 \end{pmatrix} \quad Q = \begin{bmatrix} -1 & -1 & -1 & -1 \\ & -1 & -1 & -1 \\ & & -1 & -1 \\ & & & -1 \\ 0 & 0 & 0 & 0 \end{bmatrix} \quad (111)$$

$$S = \Delta_q k_n \Delta_q^T ; \quad T = k_A u(0) ; \quad W = k_A Q \bar{\epsilon} . \quad (112)$$

Partition S, T and W :

$$S = \underbrace{\begin{bmatrix} S_{1,1} & S_{1,2} \\ S_{2,1} & S_{2,2} \end{bmatrix}}_{\substack{1 \text{ col.} & 4 \text{ cols.}}} \left. \begin{array}{l} \left. \begin{array}{l} S_{1,1} \\ S_{1,2} \end{array} \right\} 1 \text{ row,} \\ \left. \begin{array}{l} S_{2,1} \\ S_{2,2} \end{array} \right\} 4 \text{ rows} \end{array} \right\} \quad T = \left. \begin{array}{l} \left. \begin{array}{l} T_1 \\ T_2 \end{array} \right\} 1 \text{ row,} \\ \left. \begin{array}{l} T_1 \\ T_2 \end{array} \right\} 4 \text{ rows} \end{array} \right\} \quad W = \left. \begin{array}{l} \left. \begin{array}{l} W_1 \\ W_2 \end{array} \right\} 1 \text{ row} \\ \left. \begin{array}{l} W_1 \\ W_2 \end{array} \right\} 4 \text{ rows} \end{array} \right\} \quad \dots(113)$$

Discard $S_{1,1}$ and $S_{2,1}$.

$$\bar{u}(2) = (S_{2,2} - \frac{1}{T_1} T_2 S_{1,2})^{-1} (W_2 - \frac{1}{T_1} T_2 W_1) \quad (114)$$

$$\alpha = \frac{1}{T_1} \left[S_{1,2} \bar{u}(2) - W_1 \right] \quad (115)$$

$$u(1) = Q\bar{t} + \alpha u(0) \quad (116)$$

$$u(2) = \begin{Bmatrix} 0 \\ \bar{u}(2) \end{Bmatrix} \quad (117)$$

Form

$$B = \begin{bmatrix} H_{u\ell} & H_{u\ell} F_{\ell D}(\ell) & u(0) & 0 & 0 & u(1) & 0 & u(2) \\ H_{v\ell} & -H_{v\ell} F_{\ell D}(\ell) & 0 & 0 & 1 & 0 & 0 & 0 \\ H_{\theta\ell} & H_{\theta\ell} F_{\ell D}(\ell) & 0 & 0 & 0 & 1 & 0 & 0 \\ H_{\phi\ell} & H_{\phi\ell} F_{\ell D}(\ell) & 0 & 0 & 0 & 0 & 0 & 1 \\ H_{u\ell} F_{\ell D}(\ell) & H_{u\ell} & u(0) & u(0)^\ell & 0 & u(1) & u(1)^\ell & \frac{1}{2} u(1)^{\ell^2+u(2)} \\ H_{\theta\ell} F_{\ell D}(\ell) & H_{\theta\ell} & 0 & 0 & 0 & 1 & \ell & \frac{\ell^2}{2} \end{bmatrix} \quad \dots(118)$$

$$P = F/2, \text{ and} \quad (119)$$

$$\psi = \left\{ 0^* \mid 0 \mid 0 \mid -K_I^{-1} P \mid 0^* \mid 0 \right\} \text{ (column)} \quad (120)$$

$$\text{Solve } BC = \psi \text{ for } C. \quad (121)$$

Form the following for $x = x_j$. The values of x_j are input.

$$\text{Then } F_{\ell_k}(x_j) = e^{\lambda_{\ell_k} x_j} \text{ and } F_{g_k}(x) = e^{\lambda_{\ell_k}(\ell - x_j)}. \quad (122)$$

* These null matrices each have five rows.

Then

$$F_{\ell}(x_j) = \begin{Bmatrix} F_{\ell_1}(x_j) \\ F_{\ell_2}(x_j) \\ F_{\ell_3}(x_j) \\ F_{\ell_4}(x_j) \end{Bmatrix} \quad F_g(x_j) = \begin{Bmatrix} F_{g_1}(x_j) \\ F_{g_2}(x_j) \\ F_{g_3}(x_j) \\ F_{g_4}(x_j) \end{Bmatrix} \quad (123)$$

$$F_{\ell D}(x_j) \text{ and } F_{gD}(x_j) = F_{\ell}(x_j) \text{ and } F_g(x_j) \text{ diagonalized.} \quad (124)$$

$$\left. \begin{aligned} Y_{Au}(x_j) &= \left[u(0) \mid u(0)x_j \mid 0 \mid u(1) \mid u(1)x_j \mid \frac{1}{2}u(1)x_j^2 + u(2) \right] \\ Y_{AH}(x_j) &= \left[0 \mid k_A^u(0) \mid 0 \mid 0 \mid k_A^u(1) \mid k_A^u(1)x_j \right] \\ Y_{Av}(x_j) &= \left[0 \mid 0 \mid 1 \mid x_j \mid x_j^2/2 \mid x_j^3/6 \right] \\ Y_{A\theta}(x_j) &= \left[0 \mid 0 \mid 0 \mid 1 \mid x_j \mid x_j^2/2 \right] \\ Y_{AB}(x_j) &= \left[0 \mid 0 \mid 0 \mid 0 \mid 1 \mid x_j \right] \\ Y_{A\phi}(x_j) &= \left[0 \mid 0 \mid 0 \mid 0 \mid 0 \mid 1 \right] \end{aligned} \right\} \quad (125)$$

$$\text{Partition } C : \quad C = \begin{Bmatrix} C_{\ell} \\ C_g \\ C_A \end{Bmatrix} \quad \left. \begin{array}{l} 4 \text{ rows} \\ 4 \text{ rows} \\ 6 \text{ rows} \end{array} \right\} \quad (126)$$

$$\begin{Bmatrix} u(x_j) \\ H(x_j) \\ v(x_j) \\ \theta(x_j) \\ \beta(x_j) \\ \phi(x_j) \end{Bmatrix} = \begin{Bmatrix} H_{u\ell} \\ H_{H\ell} \\ H_{v\ell} \\ H_{\theta\ell} \\ H_{\beta\ell} \\ H_{\phi\ell} \end{Bmatrix} F_{\ell D}(x_j) C_{\ell} + \begin{Bmatrix} H_{u\ell} \\ -H_{H\ell} \\ -H_{v\ell} \\ H_{\theta\ell} \\ -H_{\beta\ell} \\ H_{\phi\ell} \end{Bmatrix} F_{gD}(x_j) C_g + \begin{Bmatrix} Y_{Au}(x_j) \\ Y_{AH}(x_j) \\ Y_{Av}(x_j) \\ Y_{A\theta}(x_j) \\ Y_{AB}(x_j) \\ Y_{A\phi}(x_j) \end{Bmatrix} C_A \quad (127)$$

$$q(x_j) = k_n \bar{\epsilon} \theta(x_j) - k_n \Delta_q^T u(x_j) \quad (128)$$

$$V_q = \frac{1}{2} \begin{bmatrix} t_1 & & & & & & & & \\ & t_3 & & & & & & & \\ & & t_5 & & & & & & \\ & & & t_7 & & & & & \\ & & & & t_9 & & & & \end{bmatrix} \quad k_I = \begin{Bmatrix} E_1 I_1 \\ E_3 I_3 \\ E_5 I_5 \\ E_7 I_7 \\ E_9 I_9 \end{Bmatrix} \quad (129)$$

$$V(x_j) = V_q q(x_j) - k_I \phi(x_j) \quad (130)$$

$$M(x_j) = k_I \beta(x_j) \quad (131)$$

$$(EI)_{\text{eff.}} = \frac{P \ell^3}{12v(x_{n_x})} \quad (132)$$

Output (printed)

$u(x_j)$, $H(x_j)$, $v(x_j)$, $\theta(x_j)$, $\beta(x_j)$, $\phi(x_j)$
 $q(s_j)$, $V(x_j)$, $M(x_j)$ for $j = 0, 1, 2 \dots n_x$
 $(EI)_{\text{eff}}$

ILLUSTRATIVE EXAMPLES

Four sample cases have been selected to illustrate the usefulness of this program. Cross sections and material properties for the four cases are shown in Figures 9, 10, 11 and 12. The end conditions have been defined as fixed. The load is one pound and has been applied at the beam center line. The outer glass shield is fixed at the ends, although for a typical windshield the glass floats on an interlayer and is free-floating at the ends. The program is set up to handle a total of nine plies. For beams with less than nine plies, such as Cases B, C and D, small values were assigned to the thickness, modulus of elasticity, and shear modulus to negate the influence of those fictitious plies.

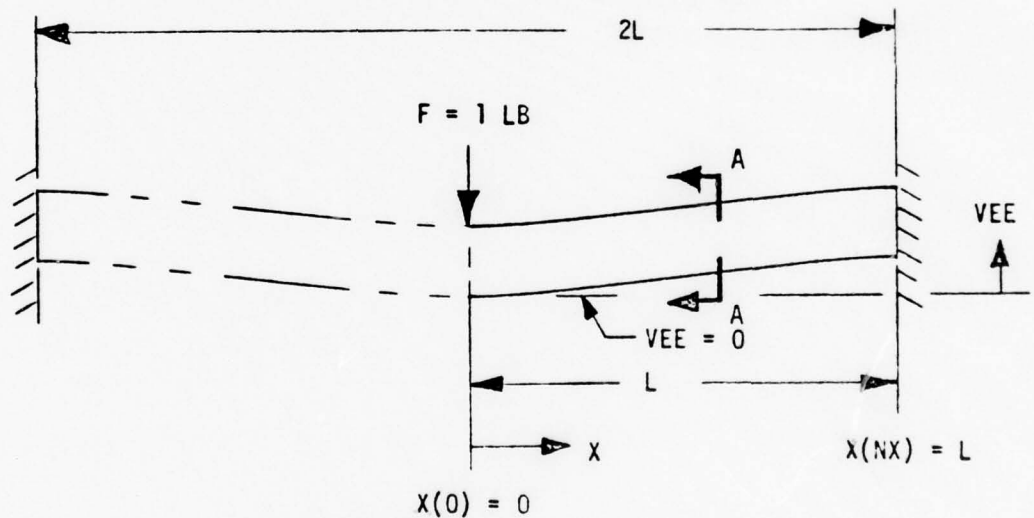
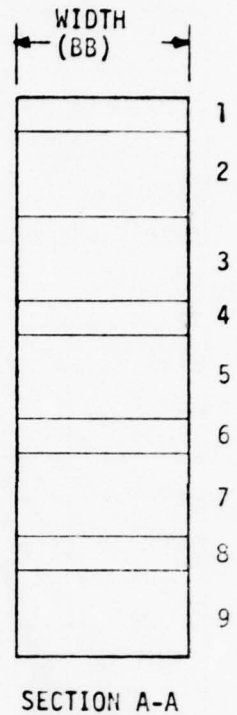
Input data and output data for five examples are presented in Appendix D. Case A was analyzed for a beam ten inches long and for a beam 30 inches long. Case D and Case C were analyzed for a beam ten inches long. Case B was analyzed for a beam 20 inches long. Table 1 is a summary and explanation of input data for Case A. Table 2 is a summary and explanation of output data for Case A. Table 3 lists a summary of deflections for the five examples as well as additional deflections for the four cases at various lengths measured at the beam center line. The deflections do not vary with the cube of the length and this deviation is shown in Figure 13, which compares a laminated beam to a multiply beam without interlayer for Cases A, B, C and D.

At the end of output data for each example in Appendix D, there is a printout for EI(EFF) which is defined as "effective stiffness". This value is calculated from the deflection at the beam center line using the formula for a beam with fixed ends and a center point load which is:

$$EI(EFF) = \frac{FL^3}{192V}$$

CASE A				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.100	10×10^6	4×10^6	GLASS
2	0.250	183	73	CIP SILICONE
3	0.250	340,000	109,000	POLYCARBONATE
4	0.100	183	73	CIP SILICONE
5	0.250	340,000	109,000	POLYCARBONATE
6	0.100	183	73	CIP SILICONE
7	0.250	340,000	109,000	POLYCARBONATE
8	0.100	183	73	CIP SILICONE
9	0.250	340,000	109,000	POLYCARBONATE



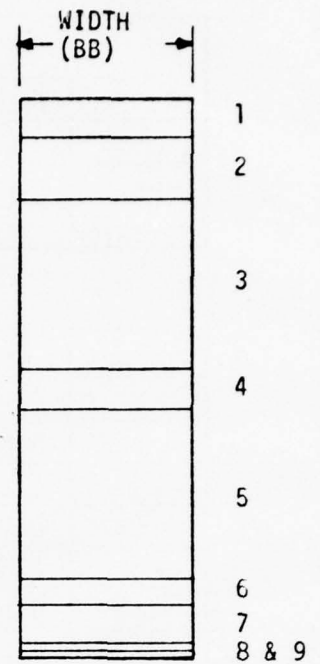
DEFLECTED SHAPE OF BEAM - NO SCALE

See Tables 1 and 2 for symbols

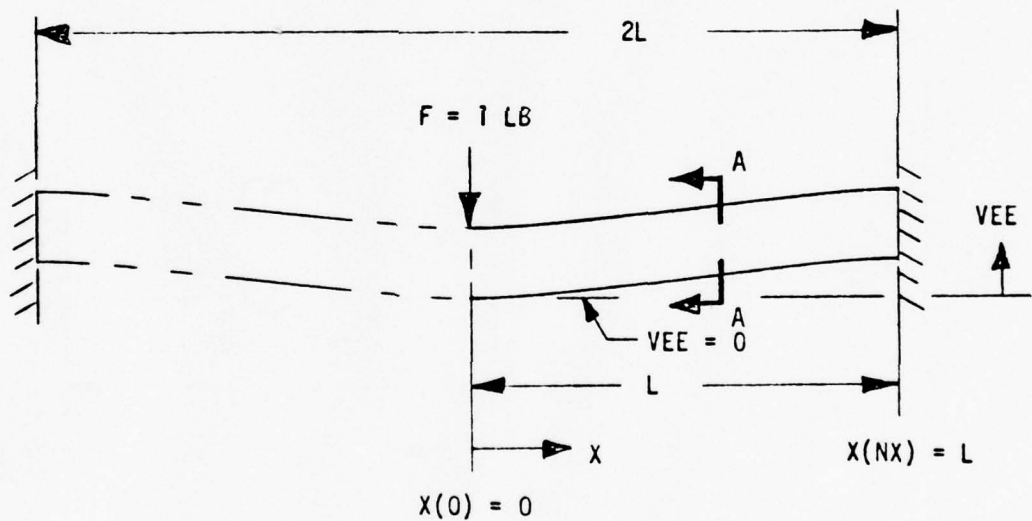
Figure 9. Cross sectional properties for Case A.

CASE B				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.110	10×10^6	4×10^6	GLASS
2	0.188	1,100	440	PPG 112
3	0.500	10×10^6	4×10^6	GLASS
4	0.120	1,100	440	PPG 112
5	0.500	10×10^6	4×10^6	GLASS
6	0.080	1,100	440	PPG 112
7	0.110	10×10^6	4×10^6	GLASS
8	0.021	1	1	FICTITIOUS
9	0.021	1	1	FICTITIOUS



SECTION A-A



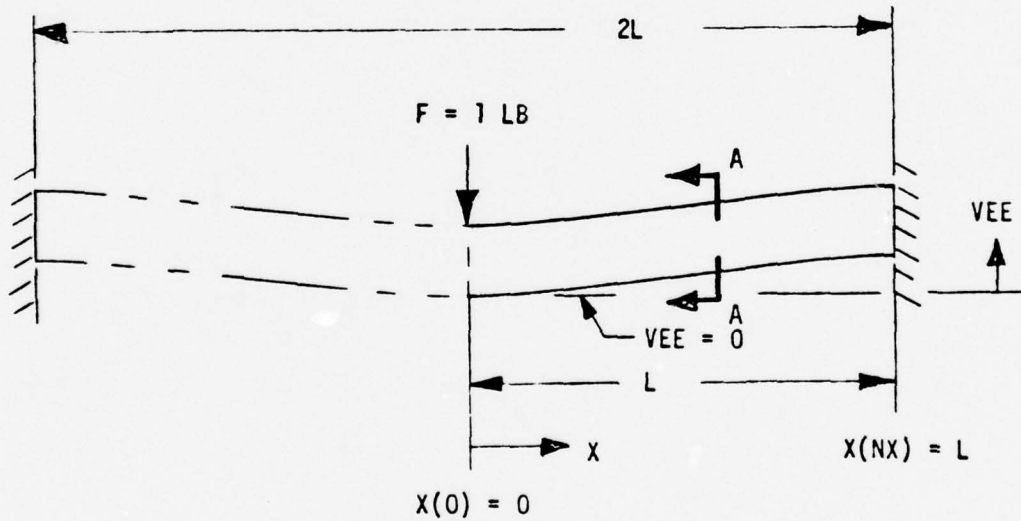
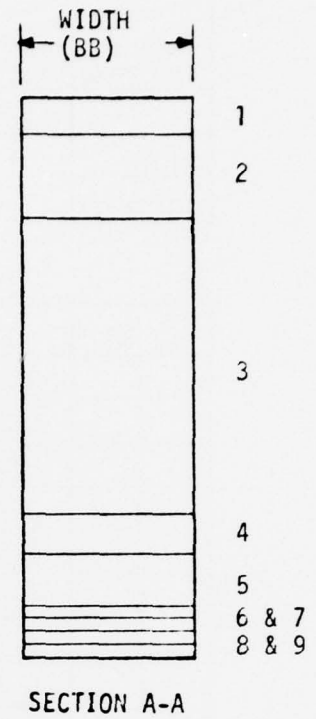
DEFLECTED SHAPE OF BEAM - NO SCALE

See Tables 1 and 2 for symbols

Figure 10. Cross sectional properties for Case B.

CASE C				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.105	10×10^6	4×10^6	GLASS
2	0.250	183	73	CIP SILICONE
3	0.870	340,000	109,000	POLYCARBONATE
4	0.120	183	73	CIP SILICONE
5	0.150	340,000	109,000	POLYCARBONATE
6	0.03875	1	1	FICTITIOUS
7	0.03875	1	1	FICTITIOUS
8	0.03875	1	1	FICTITIOUS
9	0.03875	1	1	FICTITIOUS

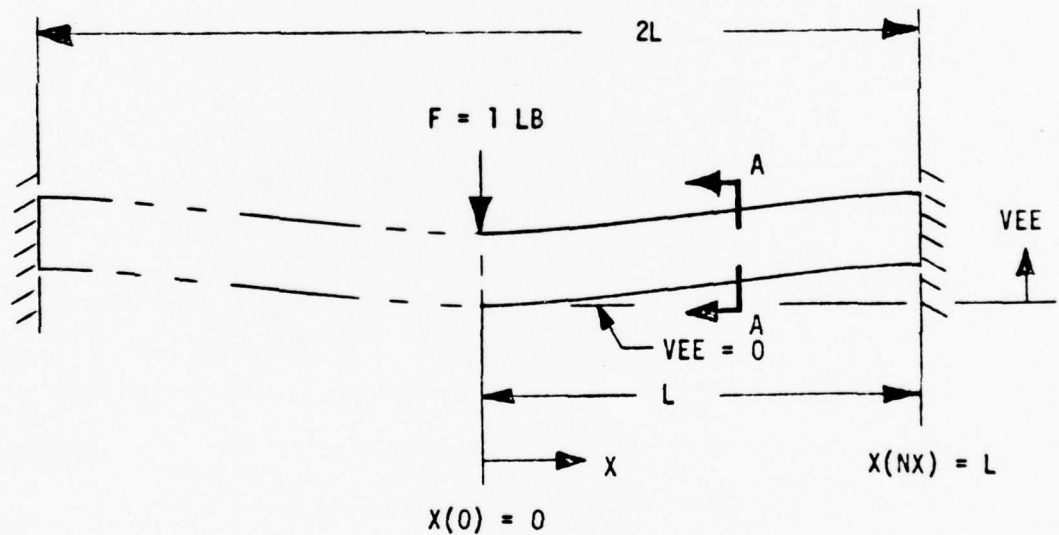
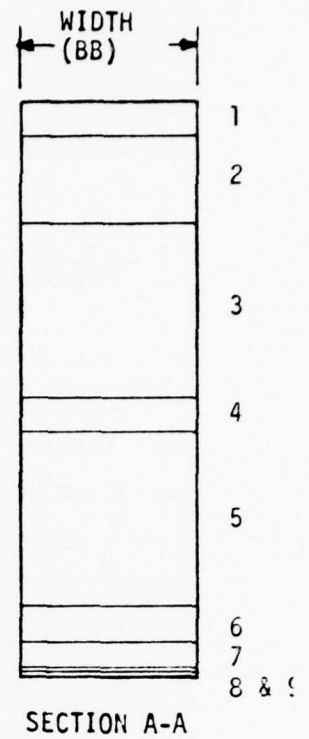


DEFLECTED SHAPE OF BEAM - NO SCALE
See Tables 1 and 2 for symbols

Figure 11. Cross sectional properties for Case C.

CASE D				
END COND	FIXED			
L	5	10	15	25
WIDTH	1			
TOT THICK	1.65			

PLY NO.	T	E	G	TYPE
1	0.100	10×10^6	4×10^6	GLASS
2	0.250	1,100	440	PPG 112
3	0.500	340,000	109,000	POLYCARBONATE
4	0.100	1,100	440	PPG 112
5	0.500	340,000	109,000	POLYCARBONATE
6	0.100	1,100	440	PPG 112
7	0.080	490,000	181,000	ACRYLIC
8	0.010	1	1	FICTITIOUS
9	0.010	1	1	FICTITIOUS



DEFLECTED SHAPE OF BEAM - NO SCALE

See Tables 1 and 2 for symbols

Figure 12. Cross sectional properties for Case D.

TABLE 1. SUMMARY OF INPUT DATA FOR CASE A

COMPUTER PROGRAM SYMBOLS	DESCRIPTION	INPUT DATA
NX	Number of Increments	10.0
F	Load (Pounds)	1.0
L	Half Length (Inches)	5.0
BB	Width (Inches)	1.0
T	Ply Thickness (Inches) (5 Structural Plies and 4 Interlayer Plies)	.10 .25, .25, .10, .25, .10, .25, .10, .25
E	Modulus of Elasticity (PSI)	10^7 , 183, 340,000, 183, 340,000, 183, 340,000, 183, 340,000
G	Shear Modulus (PSI)	4×10^6 , 73, 109,000, 73, 109,000, 73, 109,000, 73, 109,000
X	Distance from Center of Beam (Inches)	0, .50, 1.0, 1.5, 2.0, 2.5, 3.0, 3.5, 4.0, 4.5, 5.0

TABLE 2. SUMMARY OF OUTPUT DATA FOR CASE A

COMPUTER PROGRAM SYMBOLS	DESCRIPTION	OUTPUT DATA
X(0)	Increment Number	0
U	Horizontal Displacement of Center of Structural Plies (Inches)	0
H	Axial Load in Structural Plies (Pounds)	-.0959, -.0979, -.00192, -.00389, -.1918
VEE	Vertical Deflection (Inches) (Zero Deflection is Point of Maximum Deflection at Center)	0
QXJ	Shear Flow in Interlayer (Pounds Per Inch)	0
VXJ	Shear Force in Ply (Pounds)	.16, .085, .085, .085, .085
MXJ	Moment on Structural Ply (Inch-Pounds)	.322, .171, .171, .171, .171
THETA	Slope (Radians)	-5.551115E-17
BETA	Rate of Change of Slope	.000386
PHI	Rate of Change of BETA	-.000192

TABLE 3. DEFLECTION - (INCHES)

LENGTH OF BEAM - INCHES	OUTPUT FROM PROGRAM			
	10	20	30	50
<u>CASE A</u> 4 Ply Polycarbonate	.00153	.00733	.0152	.0339
<u>CASE B</u> 2-Ply Glass	.0000239	.000175	.000520	.00178
<u>CASE C</u> 1-Ply Polycarbonate	.000252	.00178	.00511	.0172
<u>CASE D</u> 2-Ply Polycarbonate	.000364	.00142	.00292	.00731

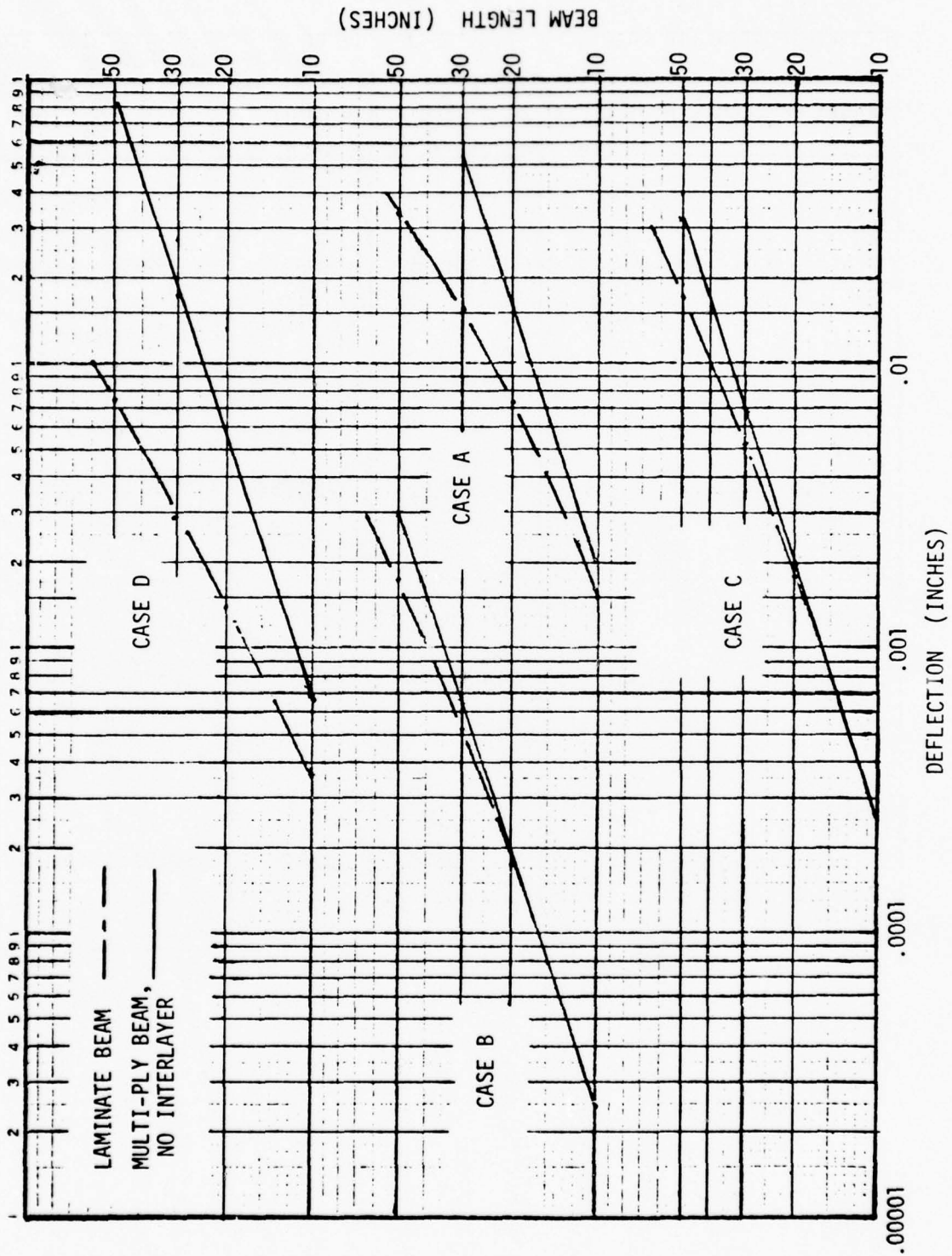


Figure 13. Deflection versus beam length.

where F is load, L is length and V is deflection. Effective stiffness varies with length. Table 4 is a comparison of effective stiffness, at various lengths for the four cases, to the stiffness if all the plies acted as individual beams. The reason why effective stiffness increases with length is that the interlayers are more effective in transferring shear between structural plies, as length increases. Thus, the behavior of the longer beams approaches monolithic.

Figure 14 is a plot of strain (inches per inch) versus thickness (inches) for Case A (4 ply polycarbonate) at the center of the beam with a one pound load applied at the center (beam length is ten inches). The values were calculated from the output data using the moment in the structural plies (MXJ) and the axial load in the structural plies (H).

Figure 15 shows a finite element model for a beam ten inches long with a cross section identical to Case A. The beam was analyzed using the computer aided Structural Design computer program* to provide a comparison. The deflections agreed within one percent. This comparison is presented as verification of the validity of the basic assumptions for the enclosed derivations.

An actual test (Reference 1) was conducted to determine the deflection of the midpoint of a beam with a cross section similar to Case C, Figure 11. The beam length was 34.7 inches. The face-ply (ply No. 1) was "free floating". The deflection of the test beam was .00882 inch. The data output for the program is given in Appendix D and shows a deflection of 0.00913 inch which agrees within 3.5 percent. This comparison is presented as additional verification of the validity of the basic assumptions for the enclosed derivations.

* Ref 9

TABLE 4. EFFECTIVE STIFFNESS (POUNDS-INCHES SQUARE)

LENGTH OF BEAM - (INCHES)	OUTPUT FROM PROGRAM				SINGLE ACTING BEAMS (NO INTERLAYER)
	10	20	30	50	
<u>CASE A</u>					
4-Ply Polycarbonate	3,397	5,683	9,238	19,212	2,600
<u>CASE B</u>					
2-Ply Glass	217,500	237,900	270,600	366,000	209,100
<u>CASE C</u>					
1-Ply Polycarbonate	20,700	23,450	27,500	37,900	19,720
<u>CASE D</u>					
2-Ply Polycarbonate	14,326	29,370	48,175	89,080	7,900

LENGTH = 10.0 INCH ———·
 LENGTH = 50.0 INCH - - - -·

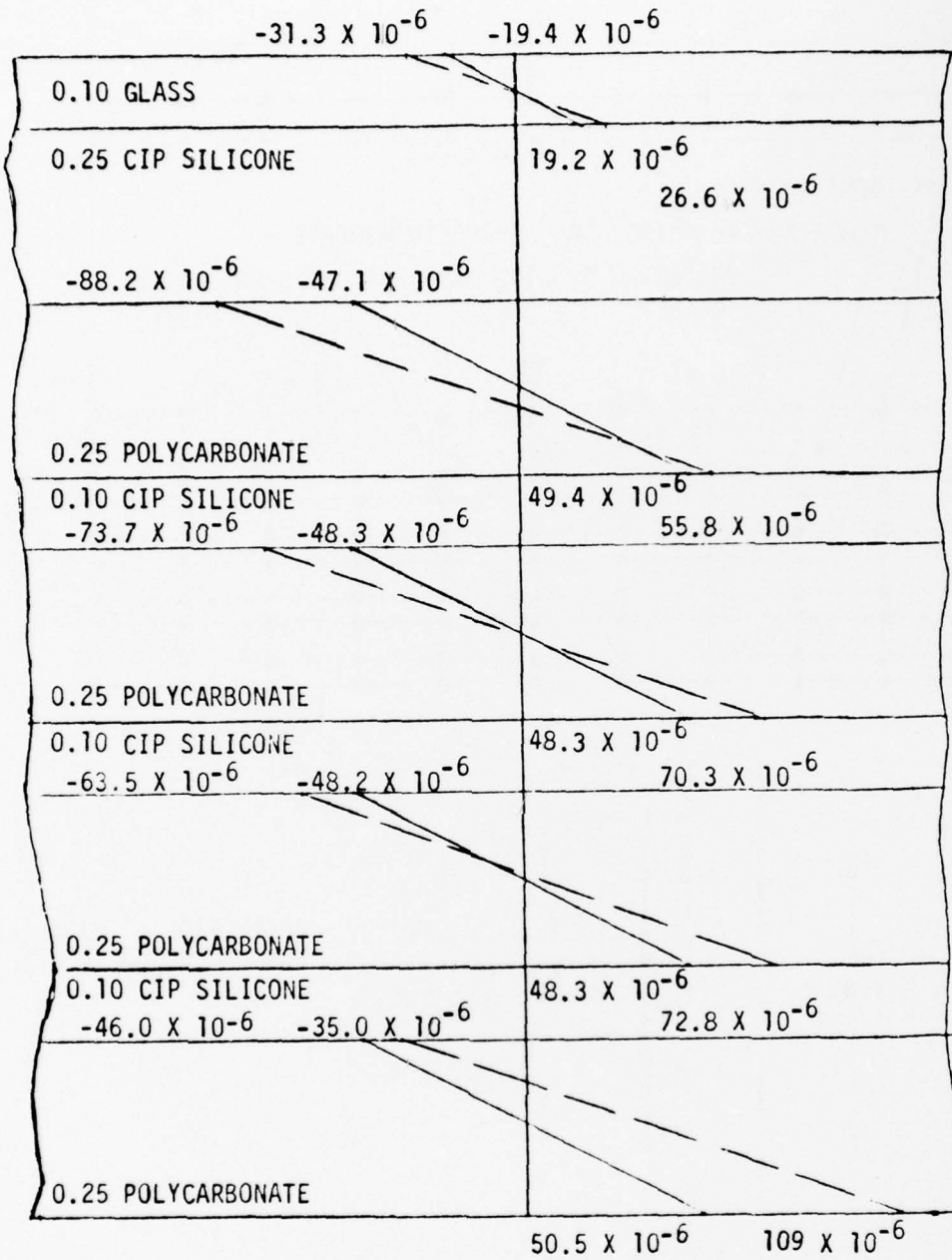


Figure 14. Strain (in/in) versus thickness for Case A at center of beam.

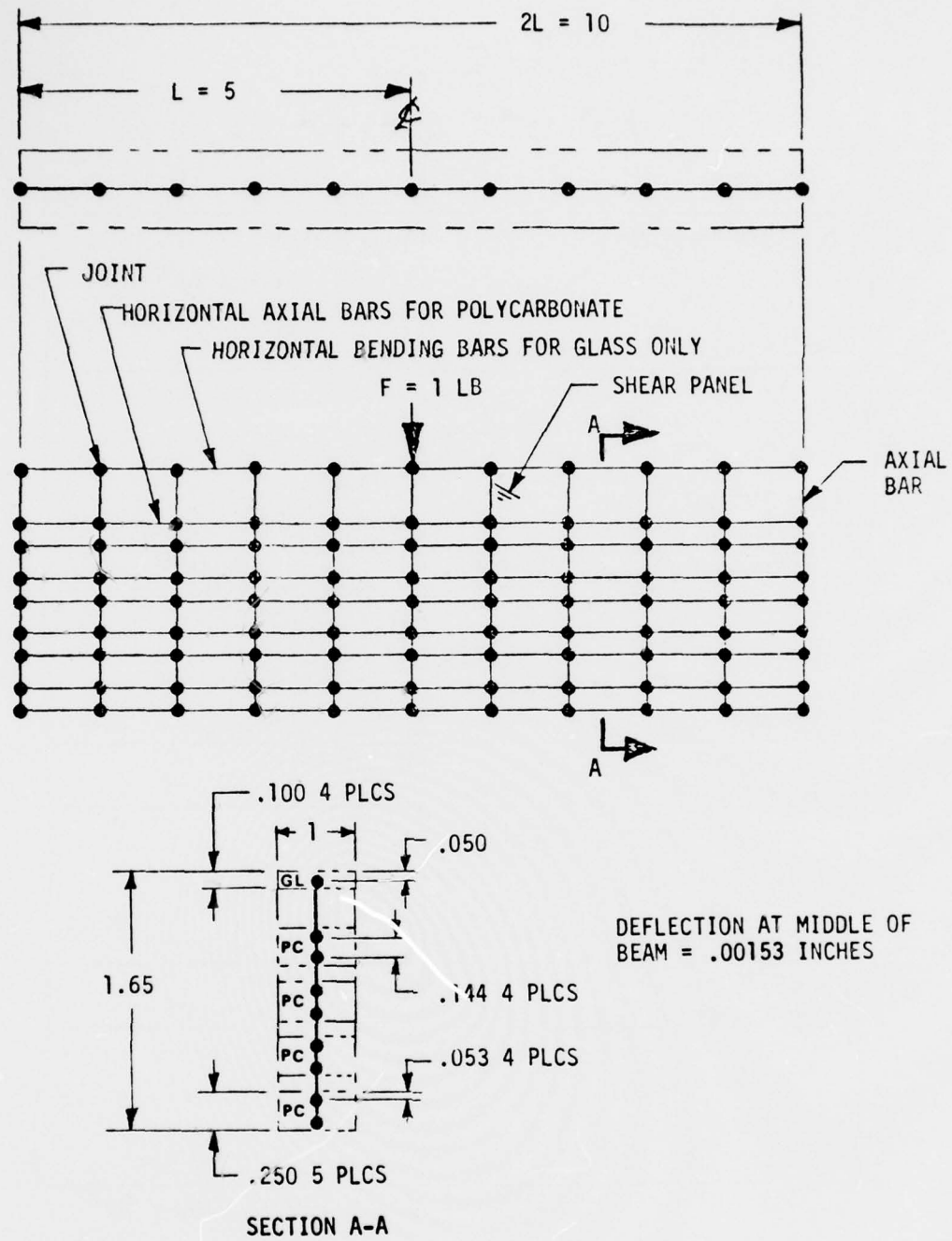


Figure 15. Finite element model of a fixed ended beam - Case A.

SECTION III

COMPUTER PROGRAM USER'S MANUAL

A FORTRAN computer program has been coded which determines the internal loads, deflections, and relative stiffness of a laminated, fixed ended beam with one concentrated load at the center. The program is based on the equations and procedures embodied in the preceding sections of this report. A copy of the source coding is contained in Appendix B. Appendix C lists the intermediate matrices which were run for Case A. The output for four illustrative problems and one test beam is contained in Appendix D. The program was written in CDC FORTRAN Extended Version 4-0 for utilization of a CDC6600 computer.

Figure 16 shows a complete card deck ready to load for a particular computer installation. Certain obvious changes to the information shown on the cards, such as time charge numbers, programmer's name, and program identification, would have to be made to suit the requirements of another installation. A review of the source coding shown in Appendix B may reveal other minor changes that might be needed at another installation.

The complete card deck consists of essentially five categories: a job card, two control cards, a source deck, input data cards, and separating cards, called 7, 8, 9 Cards and 6, 7, 8, 9 Cards. Figure 17 shows the relationship of these components to the complete deck.

The job card, control cards, and separating cards may be obtained by entering the appropriate data on the data coding sheet, Figure 18, and submitting the sheet to procure the key-punched cards. The line entries are described as follows:

Line 1 - This line describes the job card which identifies the user and lists other pertinent identifying data.

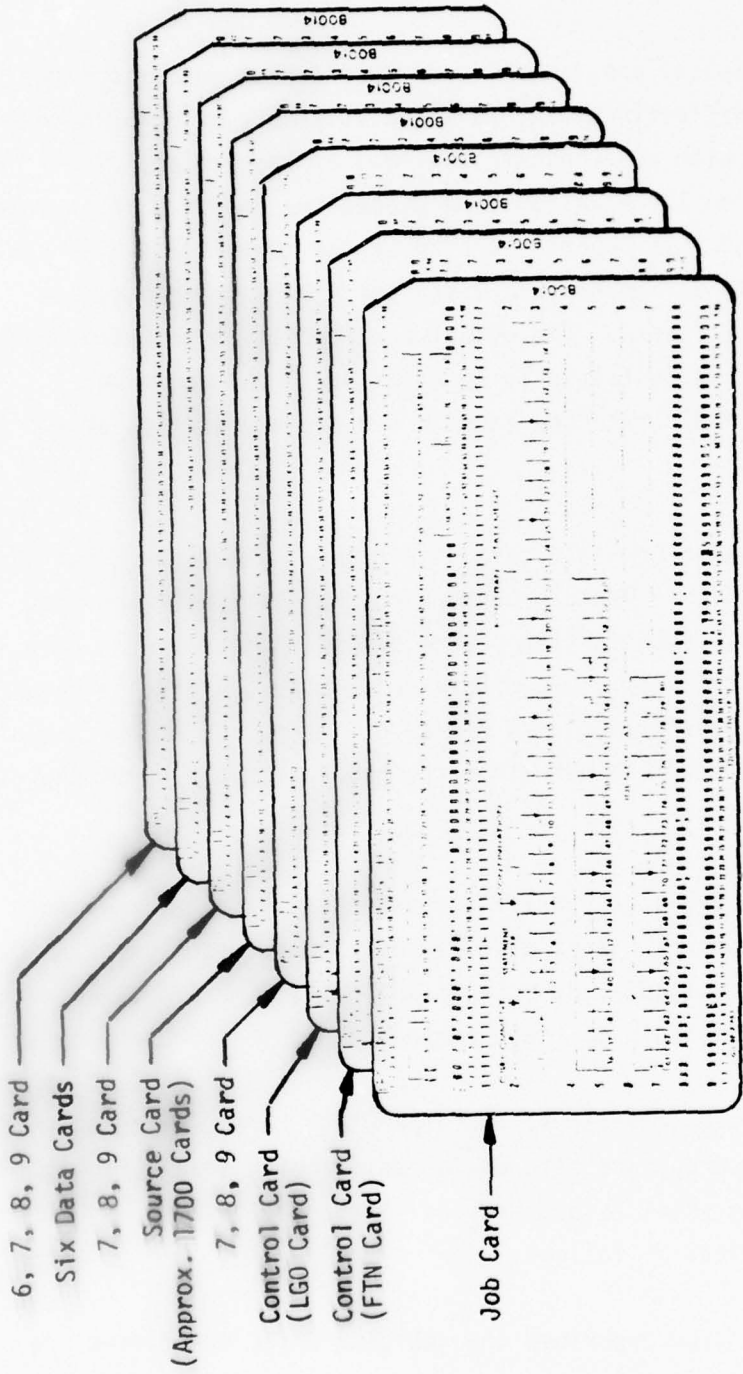


Figure 16. Complete card deck.

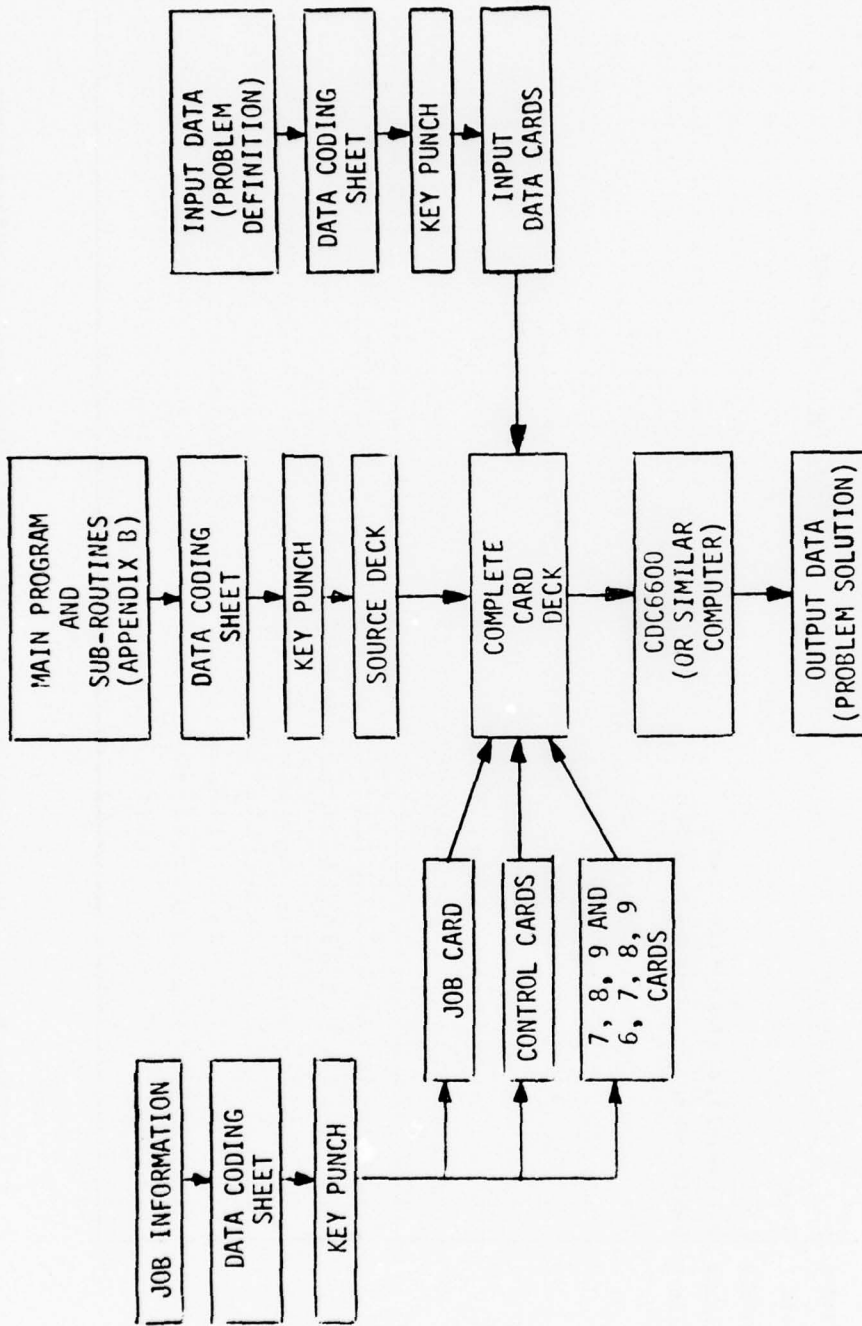


Figure 17. Card deck flow diagram.

Line 2 - This line refers to the FTN control card and provides access to library subroutines and compilation of the FORTRAN source deck.

Line 3 - This line refers to an LGO control card, which is a load and go card. One LGO card must be inserted for every set of data cards.

Line 4 - This line refers to a card that separates the source deck from the data cards and control cards. A minimum of two cards is required and an additional 7, 8, 9 Card must be added for each additional set of data cards.

Line 5 - This line refers to the last card in the deck, a 6, 7, 8, 9 Card.

The source deck is a set of approximately 1,700 cards and may be compiled from the main program and subroutines listed in Appendix B. This information may be entered on data coding sheets and submitted to key-punch to procure a source deck.

The input data card deck is a set of six cards and may be obtained by entering the appropriate information on the data coding sheet, Figure 19. The data sheet may be submitted to procure the key-punched cards shown in Figures 20 and 21 for illustrative Case A, Table 1. The program statements are shown for reference. A set of input data cards must be included for each problem to be solved. The line entries shown in Figure 19 for Case A are described as follows:

Line 1 - Columns 1 through 8 show the number of spanwise increments to the center line of the beam.

Line 1 - Columns 9 through 16 are the load in pounds.

Line 1 - Columns 17 through 24 are the length to the center line of the beam in inches.

Line 1 - Columns 25 through 32 are the width of the beam in inches.

Line 2 - Columns 1 through 8 are the thickness of the first ply in inches and the thickness of each of the remaining eight plies is shown in Columns 9 through 72.

Line 3 - Columns 1 through 8 are the modulus of elasticity (PSI) for Ply Number 1 and the modulus of elasticity for Ply Numbers 2 through 8 are shown in Columns 9 through 72.

Line 4 - Columns 1 through 8 are the shear modulus (PSI) for Ply Number 1 and the shear modulus for Ply Numbers 2 through 8 are shown in Columns 9 through 72.

Lines 5 and 6 - Columns 1 through 8 of Line 5 are the distance from the center of the beam to "X" increment in inches. The number of entries on Lines 5 and 6 correspond to the number of increments on Line 1, Columns 1 through 8. There are ten entries on Lines 5 and 6.

Lines 7 through 12 - These entries are the appropriate computer symbols corresponding to the entries on Lines 1 through 6 above and are for reference only.

Data cards must meet the following requirements:

- ° Each card field is eight (8) card columns wide.

- There cannot be any zeros or blank fields imbedded in the data (i.e., If a layer is to be dropped from the problem, its thickness and other parameters must be input to the problem as relatively small numbers to cause its effective elimination.).
- Numbers must be right justified in their fields.
- A trailing decimal point may be dropped.

SECTION IV CONCLUSIONS

The analysis and computer program presented in this report were developed to provide a tool for evaluating laminated combinations of transparent materials during initial design of windshields and windows. Historically, formulas and theories from engineering handbooks have been used for this purpose, but these methods are highly approximate.

The laminated beam analyzed in the present approach is considered to represent a strip cut from a typical aircraft windshield transparency. The primary characteristic of such a laminated beam is the relative ease of structural plies to slide past each other because of the softness of interlayer materials. Equilibrium and compatibility equations are written for the beam, based upon assumptions that preserve the important features of laminated beam behavior. The resulting set of differential equations is solved exactly.

The computer code presented can expedite the application of the theory to practical problems. Utilizing this program, the deflections for a series of typical laminated beams were examined, and were compared to the results of a proven finite element computer program. The two sets of computed values agreed within one percent. Data obtained from another beam, tested as noted in Reference 1, were compared to calculated results provided by the present method. The outer ply of the test beam was "free floating" at the ends, rather than fixed, as assumed in the analysis. To simulate this condition, a negligible value of Young's modulus was assigned to the outer ply. The calculated deflection was 3.5 percent greater than the measured value. This agreement is considered reasonably close, in view of the approximate method used to account for the "free floating" outer ply.

The applications of the computer program described in Section II under the heading "Illustrative Examples" were run on the CDC6600

computer, employing 21,000 decimal words of core. Each case required 0.879 seconds CPU time and 7.588 seconds I/O time. The I/O time includes compilation of the entire program.

SECTION V RECOMMENDATIONS

The new computer program is recommended as a tool for use in the early stages of aircraft windshield design as a means of screening laminate configurations. As an example, a strip of a certain width can be cut out of a transparency being studied. This strip can be flattened to form a laminated beam having pinned or fixed ends as appropriate. The width of the strip can be selected so that the resulting beam, supported only at the ends, roughly represents the transparency supported on all edges. A load can be applied at the center to represent a bird impact. The number of plies, structural ply and interlayer materials, and ply thicknesses can be varied, so that established stress and/or displacement constraints are met. The appropriate constraints are that ultimate stresses and maximum allowable displacements are not exceeded. Weights per square foot of transparency surface can then be calculated and compared. The laminates showing the most promise on this basis can be selected for further study.

Another use for the present code is to provide an effective bending stiffness for a laminate. This effective stiffness can be selected as the EI value of a monolithic beam necessary to give the same maximum deflection as a beam composed of the given laminate under the same load. The computer program outputs such a value. This value can then be employed as a means of developing a finite element model of the laminated transparency and supporting structure, in which the laminate is replaced by a single layer, as a means of reducing engineering, computing, and elapsed time to perform either a static or a dynamic analysis. Since effective stiffness depends upon beam length, the user should estimate the half length of the wave produced in the transparency when impact occurs. Such an estimate can be made on the basis of past experience, including previous studies employing the Bird Impact Math Model (Reference 6) or test results. This half wave length can then be taken as the length of the simply supported beam for which an effective EI

value can be calculated. An approach of this kind is probably most useful for calculating transparency displacements and loads on edge attachments and supporting structure.

The feasibility of extending the method to cover other loadings and boundary conditions should be studied. For example, an option applicable to an end condition in which the layers are prevented from sliding relative to each other, as by a bolt, although the end tangent can rotate, might be useful. A beam uniformly loaded over part of its length is a candidate case. The feasibility of applying the approach to dynamic problems also should be studied, and the possibility of eliminating the assumption that transverse strains are negligible should be considered. Eliminating this assumption would mean that the method would provide reasonably reliable estimates of transverse stresses, and perhaps a means of forecasting delamination. Any of these improvements would increase the realism of the analytical results. Consequently, the usefulness of the approach as a design tool would be enhanced.

If any of these improvements were accomplished, they could be verified by comparison with results obtained by finite element analysis.

APPENDIX A
PROPERTIES OF THE EIGENVALUES
AND EIGENVECTORS

The matrix differential equation (Equation 42) can be written in the following form by rearranging the rows of Y :

$$\begin{Bmatrix} dY_a/dx \\ dY_b/dx \end{Bmatrix} = \begin{bmatrix} 0 & a_b \\ b_a & 0 \end{bmatrix} \begin{Bmatrix} Y_a \\ Y_b \end{Bmatrix} \quad (\text{A.1})$$

where

$$b_a = \begin{bmatrix} a_{2,1} & a_{2,3} & 0 \\ 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix} \quad Y_a = \begin{Bmatrix} u \\ \theta \\ \phi \end{Bmatrix} \quad (\text{A.2})$$

$$a_b = \begin{bmatrix} a_{1,2} & 0 & 0 \\ 0 & 1 & 0 \\ a_{5,2} & a_{5,4} & 0 \end{bmatrix} \quad Y_b = \begin{Bmatrix} H \\ \beta \\ v \end{Bmatrix} \quad (\text{A.3})$$

The corresponding characteristic equation is

$$\begin{bmatrix} -\lambda I & a_b \\ b_a & -\lambda I \end{bmatrix} \begin{Bmatrix} G_a \\ G_b \end{Bmatrix} = 0 \quad (\text{A.4})$$

where

$$G_a = \begin{Bmatrix} G_u \\ G_\theta \\ G_\phi \end{Bmatrix} \quad G_b = \begin{Bmatrix} G_H \\ G_\beta \\ G_v \end{Bmatrix} \quad (\text{A.5})$$

$$\therefore \begin{cases} -\lambda G_a + a_b G_b = 0 \\ b_a G_a - \lambda G_b = 0 \end{cases} \quad (\text{A.6})$$

$$(\text{A.7})$$

Changing the signs of λ and G_b leaves Equations A.5 and A.6 unchanged.

Therefore if λ and $\begin{Bmatrix} G_a \\ G_b \end{Bmatrix}$ are an eigenvalue and an eigenvector of Equation A.4, then $-\lambda$ and $\begin{Bmatrix} G_a \\ -G_b \end{Bmatrix}$ are also an eigenvalue and an eigenvector.

APPENDIX B

SOURCE CODING (MAIN PROGRAM AND SUB-ROUTINES)

10
20
30
40
50
60
70
80
90
100
110
120
130
140
150
160
170
180
190
200
210
220
230
240
250
260
270
280
290
300
310
320
330
340
350
360

```
PROGRAM MAIN(INPUT=512,TAPE5=INPUT,OUTPUT=512,TAPE6=OUTPUT)
EQUIVALENCE (AL,LA),(TEMP(1,1),WORK(1000))
REAL L,KA,KETA,KI,LAMBL,MXJ
COMPLEX EIGVAL(14),EIGVEC(14,14)
DIMENSION WORK(3000),T(9),E(9),G(9),X(100),KA(5,5)
2. KETA(4,4),XKI(5),TBAR(4),DELTAQ(5,4),A12(5,5),A21(5,5)
3. A23(5),A52(5),A54(1),A(14,14),FLDL(4,4),Q(5,4),UI(5)
4. U2(5),B(14,14),HL(14,4),LAMBL(4),PSI(14),FLD(4,4)
5. FGD(4,4),ANS(14),QXJ(4) ,VQ(5,4),VXJ(5),MXJ(5)
6. TEMP(14,6)
7. S22(4,4),S12(4),XT(5),W(5)
C NX = NUMBER OF X'S.
C F = FORCE.
C L = LENGTH.
C BB = WIDTH.
WRITE(6,935)
READ(5,10) NX,F,L,BB
10 FORMAT(18,3F8.0)
C READ T,E,G.
READ(5,20) (T(J),J=1,9)
READ(5,20) (E(J),J=1,9)
READ(5,20) (G(J),J=1,9)
X(1) = 0.0
NX1 = NX+1
C READ X.
READ(5,20) (X(J),J=2,NX1)
20 FORMAT(9F8.0)
WRITE(6,1) NX,F,L,BB
WRITE(6,2) (T(J),J=1,9)
WRITE(6,3) (E(J),J=1,9)
WRITE(6,4) (G(J),J=1,9)
WRITE(6,5) (X(J),J=1,NX1)
1 FORMAT(11H NX,F,L,BB ,I5,1P3E16.6)
2 FORMAT(3H T ,1P5E16.6/3H ,1P4E16.6)
3 FORMAT(3H E ,1P5E16.6/3H ,1P4E16.6)
4 FORMAT(3H G ,1P5E16.6/3H ,1P4E16.6)
```

```

370
380
390
400
410
420
430
440
450
460
470
480
490
500
510
520
530
540
550
560
570
580
590
600
610
620
630
640
650
660
670
680
690
700
710
720

5 FORMAT(3H X ,1P5E16.6/3H ,1P6E16.6)
C
FORM VQ
DO 90 I=1,5
DO 90 J=1,4
90 VQ(I,J) = 0.0
DO 95 I=1,4
VQ(I,I) = 0.5*T(2*I-1)
95 VQ(I+1,I) = 0.5*T(2*I+1)
C
FORM KA
DO 100 I=1,5
DO 100 J=1,5
100 KA(I,J) = 0.0
DO 150 I=1,5
150 KA(I,I) = E(2*I-1)*BB*T(2*I-1)
C
FORM KETA
DO 160 I=1,4
DO 160 J=1,4
160 KETA(I,J) = 0.0
DO 170 I=1,4
170 KETA(I,I) = G(2*I)*BB/T(2*I)
C
FORM KI,XKI
KI = 0.0
DO 180 I=1,5
XKI(I) = E(2*I-1)*BB*T(2*I-1)**3/12.0
180 KI = KI+XKI(I)
C
FORM TBAR
DO 190 I=1,4
190 TBAR(I) = 0.5*T(2*I-1)+T(2*I)+0.5*T(2*I+1)
C
FORM DELTAQ
DO 200 I=1,5
DO 200 J=1,4
200 DELTAQ(I,J) = 0.0
DO 210 I=1,4
DELTAQ(I,I) = -1.0
210 DELTAQ(I+1,I) = 1.0
C
FORM A12

```

```

730 DO 220 I=1,5
740 DO 220 J=1,5
750 A12(I,J) = 0.0
760 220 IF(I .EQ. J) A12(I,J) = 1.0/KA(I,J)
770 FORM A21
780 CALL MULT(DELTAQ,5,4,KETA,4,4,WORK)
790 CALL TMULT(WORK,5,4,DELTAQ,5,4,A21)
800 FORM A23
810 DO 230 I=1,20
820 WORK(I) = -WORK(I)
830 CALL MULT(WORK,5,4,TBAR,4,1,A23)
840 FORM A52
850 CALL LTMULT(A23,5,1,A12,5,5,A52)
860 DO 240 I=1,5
870 240 A52(I) = A52(I)/KI
880 FORM A54
890 CALL LTMULT(TBAR,4,1,KETA,4,4,WORK)
900 CALL MULT(WORK,1,4,TBAR,4,1,A54)
910 A54(1) = A54(1)/KI
920 FORM A
930 DO 250 I=1,14
940 DO 250 J=1,14
950 250 A(I,J) = 0.0
960 DO 260 I=1,5
970 DO 260 J=1,5
980 A(I+5,J) = A21(I,J)
990 260 A(I,J+5) = A12(I,J)
1000 DO 270 J=1,5
1010 270 A(14,J+5) = A52(J)
1020 DO 280 I=1,5
1030 280 A(I+5,12) = A23(I)
1040 A(14,13) = A54(1)
1050 DO 290 I=11,13
1060 290 A(I,I+1) = 1.0
1070 EPSILN = 0.00001
1080 DO 299 J=1,14

```

```

1090 WRITE(6,298) (A(I,J),I=1,14)
1100 FORMAT(3H A ,1P7E16.6/1H ,1P7E16.6)
1110 298 CONTINUE
1120 IND = 1
1130 CALL EIGEN ROUTINE.
1140 CALL RGEIG(14,14,A,IND,EIGVAL,EIGVEC,WORK)
1150 DO 310 I=1,14
1160 310 WORK(I) = REAL(EIGVAL(I))
1170 WRITE(6,311) (WORK(I),I=1,14)
1180 311 FORMAT(13H EIGENVALUES ,1P7E16.6/1H ,1P7E16.6)
1190 C TEST EIGENVALUES.
1200 DO 320 I=1,14
1210 COM = AIMAG(EIGVAL(I))
1220 IF (ABS(COM) .LE. EPSILN) GO TO 320
1230 WRITE(6,315) COM,I
1240 315 FORMAT(19H COMPLEX EIGENVALUE,1PE13.6,I10)
1250 GO TO 9999
1260 320 CONTINUE
1270 340 KK = 1
1280 KOUNT = 0
1290 NNEG = 0
1300 NPOS = 0
1310 DO 371 I=1,14
1320 IF (WORK(I) .GT. EPSILN) NPOS=NPOS+1
1330 IF (WORK(I) .LT. -EPSILN) NNEG=NNEG+1
1340 IF (ABS(WORK(I)) .GT. EPSILN) GO TO 350
1350 KOUNT = KOUNT+1
1360 GO TO 371
1370 350 ITEST = 0
1380 DO 360 J=1,14
1390 IF (ABS(WORK(I)+WORK(J)) .GT. EPSILN) GO TO 360
1400 ITEST = 1
1410 GO TO 361
1420 360 CONTINUE
1430 361 IF (ITEST .EQ. 1) GO TO 363
1440 WRITE(6,362)

```

```

1450
1460
1470
1480
1490
1500
1510
1520
1530
1540
1550
1560
1570
1580
1590
1600
1610
1620
1630
1640
1650
1660
1670
1680
1690
1700
1710
1720
1730
1740
1750
1760
1770
1780
1790
1800

362 FORMAT(22H UNMATCHED EIGENVALUES)
GO TO 9999
363 IF(WORK(I) .GT. 0) GO TO 371
IF(KK .LE. 4) GO TO 365
WRITE(6,364)
364 FORMAT(9H KK GT 4)
GO TO 9999
STORE DESIRED EIGENVALUES.
365 LAMBL(KK) = WORK(I)
DO 369 K=1,14
COM = AIMAG(EIGVEC(K,I))
IF(COM .LE. EPSILN) GO TO 369
WRITE(6,368) COM,I,K
368 FORMAT(16H COMPLEX VECTOR ,1PE13.6,2I5)
GO TO 9999
369 CONTINUE
STORE DESIRED EIGENVECTORS,
DO 370 K=1,14
370 HL(K,KK) = REAL(EIGVEC(K,I))
KK = KK+1
371 CONTINUE
IF((KOUNT .EQ. 6) .AND. (NPOS .EQ. 4) .AND. (NNEG .EQ. 4))
160 TO 400
WRITE(6,375) KOUNT,NPOS,NNEG
375 FORMAT(17H KOUNT,NPOS,NNEG ,3I5)
GO TO 9999
400 CONTINUE
FORM FLDL
DO 408 I=1,4
DO 408 J=1,4
408 FLDL(I,J) = 0.0
DO 409 I=1,4
409 FLDL(I,I) = EXP(LAMBL(I)*L)
FORM Q
DO 410 I=1,5
DO 410 J=1,4

```

1810
1820
1830
1840
1850
1860
1870
1880
1890
1900
1910
1920
1930
1940
1950
1960
1970
1980
1990
2000
2010
2020
2030
2040
2050
2060
2070
2080
2090
2100
2110
2120
2130
2140
2150
2160

```
410 Q(I,J) = 0.0
DO 420 I=1,4
DO 420 J=1,4
420 Q(I,J) = -1.0
FORM U1 AND U2
DO 425 I = 1,5
U2(1) = 0.0
XT(1) = 0.0
DO 425 J = 1,5
425 XT(I) = XT(I) + KA(I,J)
CALL MULT ( KA,5,5,Q,5,4,WORK(1) )
CALL MULT ( WORK(1),5,4,TBAR,4,1,W )
DO 426 I = 1,4
426 S12(I) = A21(1,I+1)
CALL MULT ( XT(2),4,1,S12,1,4,S22 )
XT11 = 1.0 / XT(1)
DO 427 I = 1,4
U2(I+1) = W(I+1) - XT11 * XT(I+1) * W(1)
DO 427 J = 1,4
427 S22(I,J) = A21(I+1,J+1) - XT11 * S22(I,J)
CALL SLEQ ( S22,U2(2),4,IER )
CALL MULT ( S12,1,4,U2(2),4,1,ALPHA )
ALPHA = XT11 * ( ALPHA - W(1) )
CALL MULT ( Q,5,4,TBAR,4,1,U1 )
DO 430 I = 1,5
430 U1(I) = U1(I) + ALPHA
FORM B
DO 435 I=1,14
DO 435 J=1,14
435 B(I,J) = 0.0
DO 440 I=1,5
DO 440 J=1,4
B(I,J) = HL(I,J)
440 B(I+8,J+4) = HL(I,J)
DO 450 J=1,4
450 B(6,J) = HL(11,J)
```

```

2170      DO 460 J=1,4
2180      B(7,J) = HL(12,J)
2190      460 B(14,J+4) = HL(12,J)
2200      DO 470 J=1,4
2210      470 B(8,J) = HL(14,J)
2220      DO 480 I=1,5
2230      B(1,9) = 1.0
2240      B(1+8,9) = 1.0
2250      480 B(1+8,10) = L
2260      B(6,11) = 1.0
2270      B(7,12) = 1.0
2280      B(8,14) = 1.0
2290      B(14,12) = 1.0
2300      B(14,13) = L
2310      B(14,14) = L*L/2.0
2320      DO 490 I=1,5
2330      B(I,12) = U1(I)
2340      B(I+8,12) = U1(I)
2350      B(I+8,13) = U1(I)*L
2360      B(I,14) = U2(I)
2370      490 B(I+8,14) = 0.5*U1(I)*L*U2(I)
2380      DO 500 J=1,4
2390      DO 500 I=1,5
2400      500 WORK(I+5*(J-1)) = HL(I,J)
2410      CALL MULT(WORK,5,4,FLDL,4,4,WORK(21))
2420      DO 510 J=1,4
2430      DO 510 I=1,5
2440      IJ = I+20+5*(J-1)
2450      B(I+8,J) = WORK(IJ)
2460      510 B(I,J+4) = WORK(IJ)
2470      DO 520 J=1,4
2480      520 WORK(J) = HL(12,J)
2490      CALL MULT(WORK,1,4,FLDL,4,4,WORK(5))
2500      DO 530 J=1,4
2510      B(14,J) = WORK(J+4)
2520      530 B(7,J+4) = WORK(J+4)

```

```

2530 DO 540 J=1,4
2540 WORK(J) = -HL(11,J)
2550 CALL MULT(WORK,1,4,FLDL,4,4,WORK(5))
2560 DO 550 J=1,4
2570 B(6,J+4) = WORK(J+4)
2580 DO 560 J=1,4
2590 WORK(J) = HL(14,J)
2600 CALL MULT(WORK,1,4,FLDL,4,4,WORK(5))
2610 DO 570 J=1,4
2620 B(8,J+4) = WORK(J+4)
2630 P = F/2.0
2640 FORM PSI
2650 DO 580 J=1,14
2660 PSI(J) = 0.0
2670 PSI(8) = -P/KI
2680 CALL SOLUTION OF SIMULTANEOUS LINEAR EQUATIONS ROUTINE.
2690 CALL SLEQ(B,PSI,14,IER)
2700 LOOP OVER ALL X.
2710 DO 1000 JELT=1,NXI
2720 FORM FLD,FGD
2730 DO 700 I=1,4
2740 DO 700 J=1,4
2750 FLD(I,J) = 0.0
2760 FGD(I,J) = 0.0
2770 IF(I .NE. J) GO TO 700
2780 FLD(I,J) = EXP(LAMBL(I)*X(JELT))
2790 FGD(I,J) = EXP(LAMBL(I)*(L-X(JELT)))
2800 CONTINUE
2810 FORM ANS
2820 DO 710 J=1,4
2830 WORK(J) = PSI(J)
2840 CALL MULT(FLD,4,4,WORK,4,1,WORK(5))
2850 CALL MULT(HL,14,4,WORK(5),4,1,ANS)
2860 DO 720 J=1,4
2870 WORK(J) = PSI(J+4)
2880 CALL MULT(FGD,4,4,WORK,4,1,WORK(5))

```

```

2890      DO 730 I=6,11
2900      DO 730 J=1,4
2910      HL(I,J) = -HL(I,J)
2920      DO 740 J=1,4
2930      HL(13,J) = -HL(13,J)
2940      CALL MULT(HL,14,4,WORK(5),4,1,WORK(10))
2950      DO 750 I=1,14
2960      ANS(I) = ANS(I)+WORK(I+9)
2970      DO 760 I=6,11
2980      DO 760 J=1,4
2990      HL(I,J) = -HL(I,J)
3000      DO 770 J=1,4
3010      HL(13,J) = -HL(13,J)
3020      DO 780 I=1,14
3030      DO 780 J=1,6
3040      TEMP(I,J) = 0.0
3050      DO 790 I=1,5
3060      TEMP(I,1) = 1.0
3070      TEMP(I,2) = X(JELT)
3080      TEMP(I,4) = U1(I)
3090      TEMP(I,5) = U1(I)*X(JELT)
3100      TEMP(I,6) = 0.5*TEMP(I,5)*X(JELT)+U2(I)
3110      DO 800 I=1,5
3120      TEMP(I+5,2) = KA(I,I)
3130      CALL MULT(KA,5,5,U1,5,1,A23)
3140      DO 810 I=1,5
3150      AB = A23(I)
3160      TEMP(I+5,5) = AB
3170      TEMP(I+5,6) = AB*X(JELT)
3180      DO 820 I=11,14
3190      TEMP(I,1-8) = 1.0
3200      DO 830 I=11,13
3210      TEMP(I,1-7) = X(JELT)
3220      TEMP(11,5) = X(JELT)*X(JELT)*0.5
3230      TEMP(12,6) = TEMP(11,5)
3240      TEMP(11,6) = TEMP(11,5)*X(JELT)/3.0

```

```

DO 840 I=1,6
840 WORK(I) = PSI(I+8)
CALL MULT(TEMP,14,6,WORK,6,1,WORK(10))
DO 850 I=1,14
850 ANS(I) = ANS(I)+WORK(I+9)
C
FORM QXJ
DO 860 I=1,4
860 WORK(I) = TBAR(I)*ANS(12)
DO 870 I=1,5
870 A23(I) = ANS(I)
CALL LTMULT(DELTA0,5,4,A23,5,1,WORK(10))
DO 880 I=1,4
880 WORK(I) = WORK(I)-WORK(I+9)
CALL MULT(KETA,4,4,WORK,4,1,QXJ)
C
FORM VXJ
CALL MULT(VQ,5,4,QXJ,4,1,A23)
DO 910 I=1,5
910 A52(I) = XKI(I)*ANS(14)
DO 920 I=1,5
920 VXJ(I) = A23(I)-A52(I)
C
FORM MXJ
DO 930 I=1,5
930 MXJ(I) = XKI(I)*ANS(13)
J1 = JELT-1
C
OUTPUT DESIRED MATRICES.
935 FORMAT(1H1)
WRITE(6,940) J1
940 FORMAT(1H /3H X(,I5,1H))
WRITE(6,950) (ANS(I),I=1,5)
950 FORMAT(1H /3H U ,1P5E16.6)
WRITE(6,960) (ANS(I),I=6,10)
960 FORMAT(1H /3H H ,1P5E16.6)
WRITE(6,970) (ANS(I),I=11,14)
970 FORMAT(1H /5H VEE ,1PE16.6,7H THETA ,1PE16.6,6H BETA ,1PE16.6,5H P
1H1 ,1PE16.6)
WRITE(6,980) (QXJ(I),I=1,4)
3250
3260
3270
3280
3290
3300
3310
3320
3330
3340
3350
3360
3370
3380
3390
3400
3410
3420
3430
3440
3450
3460
3470
3480
3490
3500
3510
3520
3530
3540
3550
3560
3570
3580
3590
3600

```

AD-A044 595

DOUGLAS AIRCRAFT CO LONG BEACH CALIF
THE DETERMINATION OF DEFLECTION AND STRESS DISTRIBUTION FOR A T--ETC(U)
NOV 76 P H DENKE, J B HOFFMAN

F/G 1/3

F33615-75-C-3105

UNCLASSIFIED

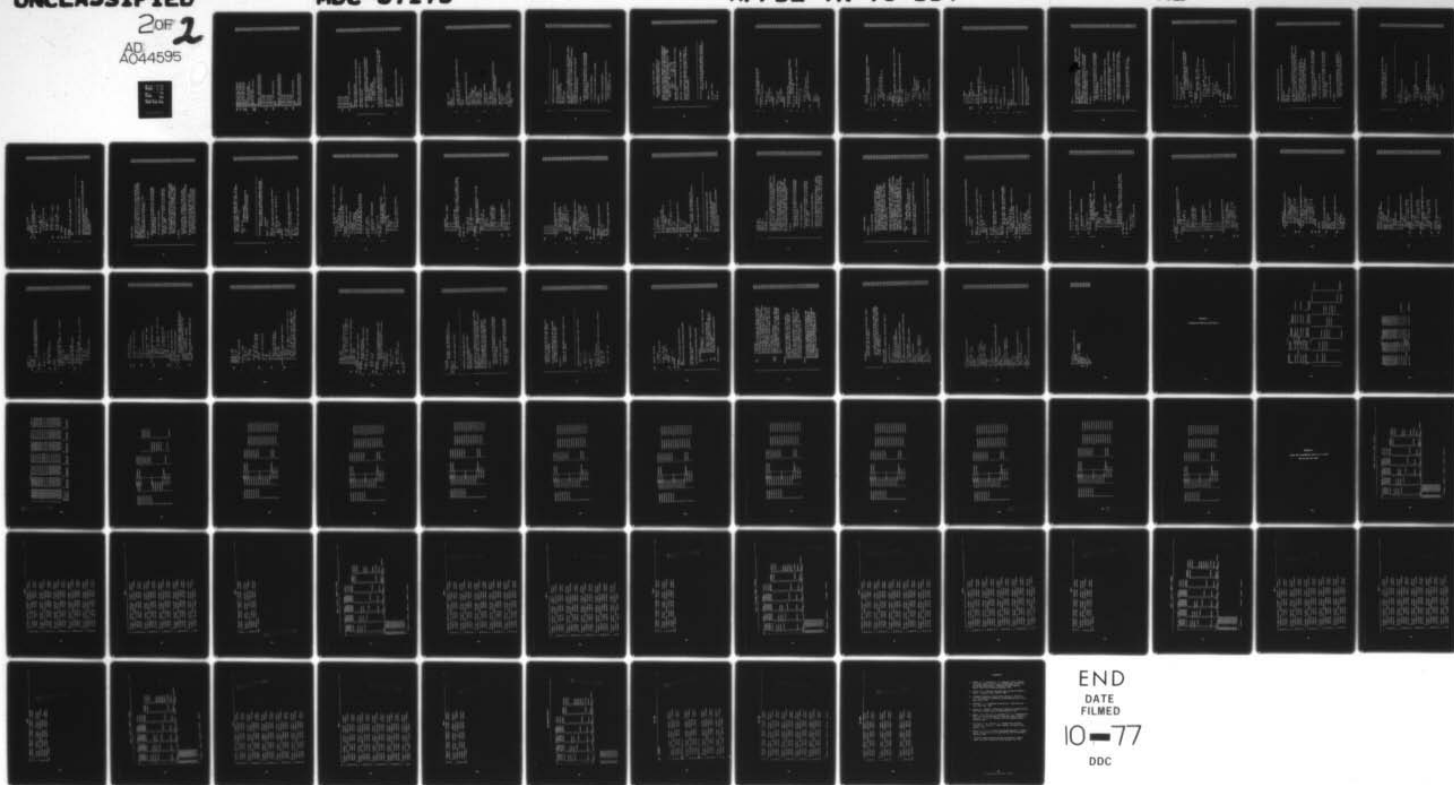
MDC-J7173

AFFDL-TR-76-114

NL

2 OF 2
AD A044595

1



END
DATE
FILMED

10-77

DDC

```

980 FORMAT(1H /5H QXJ ,1P4E16.6)
3620 WRITE(6,990) (VXJ(I),I=1,5)
3630 FORMAT(1H /5H VXJ ,1P5E16.6)
3640 WRITE(6,995) (MXJ(I),I=1,5)
3650 FORMAT(1H /5H MXJ ,1P5E16.6)
3660 CONTINUE
3670 FORM EI(EFF) AND OUTPUT.
3680 EIEFF = P*L**3/(12.0*ANS(11))
3690 WRITE(6,1100) EIEFF
3700 FORMAT(1H /9H EI(EFF) ,1PE16.6)
3710 STOP
3720 END
3730 SUBROUTINE MULT(A,N1,M1,B,N2,M2,C)
3740 DIMENSION A(N1,M1),B(N2,M2),C(N1,M2)
3750 DO 100 I=1,N1
3760 DO 100 J=1,M2
3770 C(I,J) = 0.0
3780 DO 200 I=1,N1
3790 DO 200 J=1,M2
3800 DO 200 K=1,M1
3810 C(I,J) = C(I,J)+A(I,K)*B(K,J)
3820 RETURN
3830 END
3840 SUBROUTINE TMULT(A,N1,M1,B,N2,M2,C)
3850 DIMENSION A(N1,M1),B(N2,M2),C(N1,N2)
3860 DO 100 I=1,N1
3870 DO 100 J=1,N2
3880 C(I,J) = 0.0
3890 DO 200 I=1,N1
3900 DO 200 J=1,N2
3910 DO 200 K=1,M1
3920 C(I,J) = C(I,J)+A(I,K)*B(K,J)
3930 RETURN
3940 END
3950 SUBROUTINE LTMULT(A,N1,M1,B,N2,M2,C)
3960 DIMENSION A(N1,M1),B(N2,M2),C(M1,M2)

```

C

```

3970 DO 100 I=1,M1
3980 DO 100 J=1,M2
3990 100 C(I,J) = 0.0
4000 DO 200 I=1,M1
4010 DO 200 J=1,M2
4020 DO 200 K=1,N1
4030 200 C(I,J) = C(I,J)+A(K,I)*B(K,J)
4040 RETURN
4050 END
4060 SUBROUTINE SLEQ(A,B,N,IER)
4070 SLEQ FROM THE COMPUTER CENTER LIBRARY OF 6600 ROUTINES
4080
4090 SIMULTANEOUS SOLUTION OF LINEAR EQUATIONS, AX = B.
4100 CALL SLEQ(A,B,N,IER)
4110
4120 A(N,N) MATRIX OF COEFFICIENTS, DESTROYED IN COMPUTATION.
4130 B(N) VECTOR OF ORIGINAL CONSTANTS, SOLUTION VECTOR X
4140 RETURNED HERE.
4150 N NUMBER OF EQUATIONS.
4160 IER IF 0, NORMAL SOLUTION
4170 IF 1, MATRIX A IS SINGULAR
4180
4190 IF NO PIVOT CAN BE FOUND EXCEEDING A TOLERANCE OF 0.0, THE MATRIX
4200 IS CONSIDERED SINGULAR AND IER IS SET TO 1. THIS TOLERANCE CAN
4210 BE MODIFIED BY REPLACING THE FIRST STATEMENT.
4220
4230 DIMENSION A(N,N),B(N)
4240 TOL = 0.
4250 IER = 0
4260 DO 8 J=1,N
4270 J1 = J+1
4280
4290 SEARCH FOR MAXIMUM COEFFICIENT IN COLUMN J.
4300 PIVOT = 0.
4310 DO 2 I=J,N
4320 IF(ABS(PIVOT)-ABS(A(I,J))) 1,2,2

```

4330
4340
4350
4360
4370
4380
4390
4400
4410
4420
4430
4440
4450
4460
4470
4480
4490
4500
4510
4520
4530
4540
4550
4560
4570
4580
4590
4600
4610
4620
4630
4640
4650
4660
4670
4680

```
1 PIVOT = A(I,J)
  IMAX = I
2 CONTINUE
  WRITE(6,100) PIVOT
C
C IF PIVOT LESS THAN TOLERANCE (SINGULAR MATRIX) EXIT.
  IF (ABS(PIVOT)-TOL) 3,3,4
3 IER = 1
  RETURN
C
C INTERCHANGE ROWS AND DIVIDE BY PIVOT.
4 DO 5 K=J,N
  TEMP = A(J,K)
  A(J,K) = A(IMAX,K)
  A(IMAX,K) = TEMP
5 A(J,K) = A(J,K)/PIVOT
  TEMP = B(IMAX)
  B(IMAX) = B(J)
  B(J) = TEMP/PIVOT
C
C ELIMINATE NEXT VARIABLE
  IF (J-N) 6,9,6
6 J1 = J+1
  DO 8 IROW=J1,N
  DO 7 JCOL=J1,N
7 A(IROW,JCOL) = A(IROW,JCOL) - A(IROW,J)*A(J,JCOL)
8 B(IROW) = B(IROW) - B(J)*A(IROW,J)
C
C BACK SUBSTITUTION
9 DO 10 K=2,N
  I = N+1-K
  I1 = I+1
  DO 10 J=I1,N
10 B(I) = B(I) - A(I,J)*B(J)
100 FORMAT(7H PIVOT ,1PE16.6)
  RETURN
```

```

4690
4700
4710
4720
4730
4740
4750
4760
4770
4780
4790
4800
4810
4820
4830
4840
4850
4860
4870
4880
4890
4900
4910
4920
4930
4940
4950
4960
4970
4980
4990
5000
5010
5020
5030
5040

END
-----
SUBROUTINE BALANC(NM,N,A,LOW,IGH,SCALE)
INTEGER I,J,K,L,M,N,JJ,NM,IGH,LOW,IEXC
REAL A(NM,N),SCALE(N)
REAL C,F,G,R,S,B2,RADIX
REAL ABS
LOGICAL NOCONV

THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE BALANCE,
NUM. MATH. 13, 293-304(1969) BY PARLETT AND REINSCH.
HANDBOOK FOR AUTO. COMP., VOL. II-LINEAR ALGEBRA, 315-326(1971).
(REFERENCE 7)
THIS SUBROUTINE BALANCES A REAL MATRIX AND ISOLATES
EIGENVALUES WHENEVER POSSIBLE.

ON INPUT-

NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
DIMENSION STATEMENT.

N IS THE ORDER OF THE MATRIX.

A CONTAINS THE INPUT MATRIX TO BE BALANCED.

ON OUTPUT-

A CONTAINS THE BALANCED MATRIX.

LOW AND IGH ARE TWO INTEGERS SUCH THAT A(I,J)
IS EQUAL TO ZERO IF
(1) I IS GREATER THAN J AND

```



```

C ***** IN-LINE PROCEDURE FOR ROW AND
C COLUMN EXCHANGE *****
20 SCALE(M) = J
   IF (J .EQ. M) GO TO 50
C
   DO 30 I = 1, L
     F = A(I,J)
     A(I,J) = A(I,M)
     A(I,M) = F
   30 CONTINUE
C
   DO 40 I = K, N
     F = A(J,I)
     A(J,I) = A(M,I)
     A(M,I) = F
   40 CONTINUE
C
   50 GO TO (80,130), IEXC
   ***** SEARCH FOR ROWS ISOLATING AN EIGENVALUE
   AND PUSH THEM DOWN *****
80 IF (L .EQ. 1) GO TO 280
   L = L - 1
   ***** FOR J=L STEP -1 UNTIL 1 DO -- *****
100 DO 120 JJ = 1, L
     J = L + 1 - JJ
C
   DO 110 I = 1, L
     IF (I .EQ. J) GO TO 110
     IF (A(J,I) .NE. 0.0) GO TO 120
   110 CONTINUE
C
     M = L
     IEXC = 1
     GO TO 20
   120 CONTINUE
C

```

```

5410
5420
5430
5440
5450
5460
5470
5480
5490
5500
5510
5520
5530
5540
5550
5560
5570
5580
5590
5600
5610
5620
5630
5640
5650
5660
5670
5680
5690
5700
5710
5720
5730
5740
5750
5760

```

```

C      GO TO 140
C      ***** SEARCH FOR COLUMNS ISOLATING AN EIGENVALUE
C      AND PUSH THEM LEFT *****
C      130 K = K + 1
C      140 DO 170 J = K, L
C          DO 150 I = K, L
C              IF (I.EQ. J) GO TO 150
C              IF (A(I,J) .NE. 0.0) GO TO 170
C          150 CONTINUE
C          M = K
C          IEXC = 2
C          GO TO 20
C      170 CONTINUE
C      ***** NOW BALANCE THE SUBMATRIX IN ROWS K TO L *****
C      DO 180 I = K, L
C      180 SCALE(I) = 1.0
C      ***** ITERATIVE LOOP FOR NORM REDUCTION *****
C      190 NOCONV = .FALSE.
C          DO 270 I = K, L
C              C = 0.0
C              R = 0.0
C              DO 200 J = K, L
C                  IF (J .EQ. I) GO TO 200
C                  C = C + ABS(A(J,I))
C                  R = R + ABS(A(I,J))
C              200 CONTINUE
C              G = R / RADIX
C              F = 1.0
C              S = C + R
C              IF (C .GE. G) GO TO 220
C          270

```

```

5770
5780
5790
5800
5810
5820
5830
5840
5850
5860
5870
5880
5890
5900
5910
5920
5930
5940
5950
5960
5970
5980
5990
6000
6010
6020
6030
6040
6050
6060
6070
6080
6090
6100
6110
6120

```

```

6130      F = F * RADIX
6140      C = C * B2
6150      GO TO 210
6160      G = R * RADIX
6170      IF (C .LT. G) GO TO 240
6180      F = F / RADIX
6190      C = C / B2
6200      GO TO 230
6210      ***** NOW BALANCE *****
6220      IF ((C + R) / F .GE. 0.95 * S) GO TO 270
6230      G = 1.0 / F
6240      SCALE(I) = SCALE(I) * F
6250      NOCONV = .TRUE.
6260
6270      DO 250 J = K, N
6280      A(I,J) = A(I,J) * G
6290
6300      DO 260 J = 1, L
6310      A(J,I) = A(J,I) * F
6320
6330      CONTINUE
6340
6350      IF (NOCONV) GO TO 190
6360
6370      LOW = K
6380      IGH = L
6390      RETURN
6400      ***** LAST CARD OF BALANC *****
6410      END
6420
6430      -----
6440      SUBROUTINE BALBAK(NM,N,LOW,IGH,SCALE,M,Z)
6450      INTEGER I,J,K,M,N,II,NM,IGH,LOW
6460      REAL SCALE(N),Z(NM,M)
6470
6480

```



```

-----
C      IF (IGH .EQ. LOW) GO TO 120
C
C      DO 110 I = LOW, IGH
C      S = SCALE(I)
C      ***** LEFT HAND EIGENVECTORS ARE BACK TRANSFORMED
C      IF THE FOREGOING STATEMENT IS REPLACED BY
C      S=1.0/SCALE(I). *****
C      DO 100 J = 1, M
C      Z(I,J) = Z(I,J) * S
C
C      100 CONTINUE
C      *****- FOR I=LOW-1 STEP -1 UNTIL 1.
C      IGH+1 STEP 1 UNTIL N DO -- *****
C      120 DO 140 II = 1, N
C      I = II
C      IF (I .GE. LOW .AND. I .LE. IGH) GO TO 140
C      IF (I .LT. LOW) I = LOW - II
C      K = SCALE(I)
C      IF (K .EQ. I) GO TO 140
C
C      DO 130 J = 1, M
C      S = Z(I,J)
C      Z(I,J) = Z(K,J)
C      Z(K,J) = S
C      130 CONTINUE
C
C      140 CONTINUE
C
C      RETURN
C      ***** LAST CARD OF BALBAK *****
C      END
-----

```

C	SUBROUTINE ELMHES(NM,N,LOW,IGH,A,INT)	7210
		7220
	INTEGER I,J,M,N,LA,NM,IGH,KPI,LOW,MM1,MP1	7230
	REAL A(NM,N)	7240
	REAL X,Y	7250
C	REAL ABS	7260
	INTEGER INT(IGH)	7270
C		7280
C	THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE ELMHES,	7290
C	NUM. MATH. 12, 349-368(1968) BY MARTIN AND WILKINSON.	7300
C	HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 339-358(1971).	7310
C	(REFERENCE 7)	7320
C	GIVEN A REAL GENERAL MATRIX, THIS SUBROUTINE	7330
C	REDUCES A SUBMATRIX SITUATED IN ROWS AND COLUMNS	7340
C	LOW THROUGH IGH TO UPPER HESSENBERG FORM BY	7350
C	STABILIZED ELEMENTARY SIMILARITY TRANSFORMATIONS.	7360
C		7370
C	ON INPUT-	7380
C		7390
C	NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL	7400
C	ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM	7410
C	DIMENSION STATEMENT.	7420
C		7430
C	N IS THE ORDER OF THE MATRIX.	7440
C		7450
C	LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING	7460
C	SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED,	7470
C	SET LOW=1, IGH=N.	7480
C		7490
C	A CONTAINS THE INPUT MATRIX.	7500
C		7510
C	ON OUTPUT-	7520
C		7530
C	A CONTAINS THE HESSENBERG MATRIX. THE MULTIPLIERS	7540
C	WHICH WERE USED IN THE REDUCTION ARE STORED IN THE	7550
C	REMAINING TRIANGLE UNDER THE HESSENBERG MATRIX.	7560

7570
7580
7590
7600
7610
7620
7630
7640
7650
7660
7670
7680
7690
7700
7710
7720
7730
7740
7750
7760
7770
7780
7790
7800
7810
7820
7830
7840
7850
7860
7870
7880
7890
7900
7910
7920

INT CONTAINS INFORMATION ON THE ROWS AND COLUMNS
INTERCHANGED IN THE REDUCTION.
ONLY ELEMENTS LOW THROUGH IGH ARE USED.

C
C
C
C
C
C
C
C
C

LA = IGH - 1
KPI = LOW + 1
IF (LA .LT. KPI) GO TO 200

DO 180 M = KPI, LA
MM1 = M - 1
X = 0.0
I = M

C

DO 100 J = M, IGH
IF (ABS(A(J,MM1)) .LE. ABS(X)) GO TO 100
X = A(J,MM1)
I = J

C

100 CONTINUE

C

INT(M) = I
IF (I .EQ. M) GO TO 130

***** INTERCHANGE ROWS AND COLUMNS OF A *****

DO 110 J = MM1, N
Y = A(I,J)
A(I,J) = A(M,J)
A(M,J) = Y

C

110 CONTINUE

C

DO 120 J = 1, IGH
Y = A(J,I)

```

7930 A(J,I) = A(J,M)
7940 A(J,M) = Y
7950
7960
7970
7980
7990
8000
8010
8020
8030
8040
8050
8060
8070
8080
8090
8100
8110
8120
8130
8140
8150
8160
8170
8180
8190
8200
8210
8220
8230
8240
8250
8260
8270
8280

120 CONTINUE
C ***** END INTERCHANGE *****
130 IF (X .EQ. 0.0) GO TO 180
MP1 = M + 1
C
DO 160 I = MP1, IGH
Y = A(I,MM1)
IF (Y .EQ. 0.0) GO TO 160
Y = Y / X
A(I,MM1) = Y
C
DO 140 J = M, N
A(I,J) = A(I,J) - Y * A(M,J)
C
DO 150 J = 1, IGH
A(J,M) = A(J,M) + Y * A(J,I)
C
160 CONTINUE
C
180 CONTINUE
C
200 RETURN
C ***** LAST CARD OF ELMHES *****
END
-----
SUBROUTINE HQR(NM,N,LOW,IGH,H,WR,WI,IERR)
INTEGER I,J,K,L,M,N,EN,LL,MM,NA,NM,IGH,ITS,LOW,MP2,ENM2,IERR
REAL H(NM,N),WR(N),WI(N)
REAL P,Q,R,S,T,W,X,Y,ZZ,MACHEP
REAL SQRT,ABS,SIGN
INTEGER MINO
C

```

C	LOGICAL NOTLAS	8290
C		8300
C	THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE HQR.	8310
C	NUM. MATH. 14, 219-231(1970) BY MARTIN, PETERS, AND WILKINSON.	8320
C	HANDBOOK FOR AUTO. COMP., VOL. II-LINEAR ALGEBRA, 359-371(1971).	8330
C	(REFERENCE 7)	8340
C	THIS SUBROUTINE FINDS THE EIGENVALUES OF A REAL	8350
C	UPPER HESSENBERG MATRIX BY THE QR METHOD.	8360
C		8370
C	ON INPUT -	8380
C		8390
C	NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL	8400
C	ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM	8410
C	DIMENSION STATEMENT.	8420
C		8430
C	N IS THE ORDER OF THE MATRIX.	8440
C		8450
C	LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING	8460
C	SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED,	8470
C	SET LOW=1, IGH=N.	8480
C		8490
C	H CONTAINS THE UPPER HESSENBERG MATRIX. INFORMATION ABOUT	8500
C	THE TRANSFORMATIONS USED IN THE REDUCTION TO HESSENBERG	8510
C	FORM BY ELMHES OR ORTHES, IF PERFORMED, IS STORED	8520
C	IN THE REMAINING TRIANGLE UNDER THE HESSENBERG MATRIX.	8530
C		8540
C	ON OUTPUT -	8550
C		8560
C	H HAS BEEN DESTROYED. THEREFORE, IT MUST BE SAVED	8570
C	BEFORE CALLING HQR IF SUBSEQUENT CALCULATION AND	8580
C	BACK TRANSFORMATION OF EIGENVECTORS IS TO BE PERFORMED.	8590
C		8600
C	WR AND WI CONTAIN THE REAL AND IMAGINARY PARTS,	8610
C	RESPECTIVELY, OF THE EIGENVALUES. THE EIGENVALUES	8620
C	ARE UNORDERED EXCEPT THAT COMPLEX CONJUGATE PAIRS	8630
C	OF VALUES APPEAR CONSECUTIVELY WITH THE EIGENVALUE	8640


```

C          FOR L=EN STEP -1 UNTIL LOW DO -- *****
70 DO 80 LL = LOW, EN
   L = EN + LOW - LL
   IF (L.EQ. LOW) GO TO 100
   IF (ABS(H(L,L-1)) .LE. MACHEP * (ABS(H(L-1,L-1))
     X   + ABS(H(L,L)))) GO TO 100
80 CONTINUE
C ***** FORM SHIFT *****
100 X = H(EN,EN)
   IF (L.EQ. EN) GO TO 270
   Y = H(NA,NA)
   W = H(EN,NA) * H(NA,EN)
   IF (L.EQ. NA) GO TO 280
   IF (ITS.EQ. 30) GO TO 1000
   IF (ITS.NE. 10 .AND. ITS.NE. 20) GO TO 130
C ***** FORM EXCEPTIONAL SHIFT *****
   T = T + X
C
C   DO 120 I = LOW, EN
120 H(I,I) = H(I,I) - X
C
C   S = ABS(H(EN,NA)) + ABS(H(NA,ENM2))
   X = 0.75 * S
   Y = X
   W = -0.4375 * S * S
130 ITS = ITS + 1
C ***** LOOK FOR TWO CONSECUTIVE SMALL
   SUB-DIAGONAL ELEMENTS.
C   FOR M=EN-2 STEP -1 UNTIL L DO -- *****
   DO 140 MM = L, ENM2
   M = ENM2 + L - MM
   ZZ = H(M,M)
   R = X - ZZ
   S = Y - ZZ
   P = (R * S - W) / H(M+1,M) + H(M,M+1)
   Q = H(M+1,M+1) - ZZ - R - S

```

```

9010
9020
9030
9040
9050
9060
9070
9080
9090
9100
9110
9120
9130
9140
9150
9160
9170
9180
9190
9200
9210
9220
9230
9240
9250
9260
9270
9280
9290
9300
9310
9320
9330
9340
9350
9360

```

9370
9380
9390
9400
9410
9420
9430
9440
9450
9460
9470
9480
9490
9500
9510
9520
9530
9540
9550
9560
9570
9580
9590
9600
9610
9620
9630
9640
9650
9660
9670
9680
9690
9700
9710
9720

```
R = H(M+2,M+1)
S = ABS(P) + ABS(Q) + ABS(R)
P = P / S
Q = Q / S
R = R / S
IF (M .EQ. L) GO TO 150
IF (ABS(H(M,M-1)) * (ABS(Q) + ABS(R)) .LE. MACHEP * ABS(P)
  * (ABS(H(M-1,M-1)) + ABS(ZZ) + ABS(H(M+1,M+1)))) GO TO 150
X
140 CONTINUE
C
150 MP2 = M + 2
C
DO 160 I = MP2, EN
  H(I,I-2) = 0.0
  IF (I .EQ. MP2) GO TO 160
  H(I,I-3) = 0.0
160 CONTINUE
C
***** DOUBLE OR STEP INVOLVING ROWS L TO EN AND
  COLUMNS M TO EN *****
DO 260 K = M, NA
  NOTLAS = K .NE. NA
  IF (K .EQ. M) GO TO 170
  P = H(K,K-1)
  Q = H(K+1,K-1)
  R = 0.0
  IF (NOTLAS) R = H(K+2,K-1)
  X = ABS(P) + ABS(Q) + ABS(R)
  IF (X .EQ. 0.0) GO TO 260
  P = P / X
  Q = Q / X
  R = R / X
170 S = SIGN(SQRT(P*P+Q*Q+R*R),P)
  IF (K .EQ. M) GO TO 180
  H(K,K-1) = -S * X
  GO TO 190
180 IF (L .NE. M) H(K,K-1) = -H(K,K-1)
```

```

190 P = P + S
X = P / S
Y = Q / S
ZZ = R / S
Q = Q / P
R = R / P
C ***** ROW MODIFICATION *****
DO 210 J = K, EN
P = H(K,J) + Q * H(K+1,J)
IF (.NOT. NOTLAS) GO TO 200
P = P + R * H(K+2,J)
H(K+2,J) = H(K+2,J) - P * ZZ
H(K+1,J) = H(K+1,J) - P * Y
H(K,J) = H(K,J) - P * X
200 CONTINUE
C
C J = MINO(EN,K+3)
***** COLUMN MODIFICATION *****
DO 230 I = L, J
P = X * H(I,K) + Y * H(I,K+1)
IF (.NOT. NOTLAS) GO TO 220
P = P + ZZ * H(I,K+2)
H(I,K+2) = H(I,K+2) - P * R
H(I,K+1) = H(I,K+1) - P * Q
H(I,K) = H(I,K) - P
220 CONTINUE
C
230 CONTINUE
C
260 CONTINUE
C
C GO TO 70
***** ONE ROOT FOUND *****
270 WR(EN) = X + T
WI(EN) = 0.0
EN = NA
GO TO 60
C ***** TWO ROOTS FOUND *****

```

```

9730
9740
9750
9760
9770
9780
9790
9800
9810
9820
9830
9840
9850
9860
9870
9880
9890
9900
9910
9920
9930
9940
9950
9960
9970
9980
9990
10000
10010
10020
10030
10040
10050
10060
10070
10080

```

```

280 P = (Y - X) / 2.0
    Q = P * P + W
    ZZ = SQRT(ABS(Q))
    X = X + T
    IF (Q .LT. 0.0) GO TO 320
    ***** REAL PAIR *****
    ZZ = P + SIGN(ZZ,P)
    WR(NA) = X + ZZ
    WR(EN) = WR(NA)
    IF (ZZ .NE. 0.0) WR(EN) = X - W / ZZ
    WI(NA) = 0.0
    WI(EN) = 0.0
    GO TO 330
    ***** COMPLEX PAIR *****
320 WR(NA) = X + P
    WR(EN) = X + P
    WI(NA) = ZZ
    WI(EN) = -ZZ
    EN = ENM2
330 GO TO 60
    ***** SET ERROR -- NO CONVERGENCE TO AN
    ***** EIGENVALUE AFTER 30 ITERATIONS *****
    C IERR = EN
    C 1000 RETURN
    C ***** LAST CARD OF HQR *****
    C END
    C -----
    C SUBROUTINE HQR2(NM,N,LOW,IGH,H,WR,WI,Z,IERR)
    C INTEGER I,J,K,L,M,N,EN,II,JJ,LL,MM,NA,NM,NN,
    X IGH,ITS,LOW,MP2,ENM2,IERR
    REAL H(NM,N),WR(N),WI(N),Z(NM,N)
    REAL P,Q,R,S,T,W,X,Y,RA,SA,VI,VR,ZZ,NORM,MACHEP
    REAL SQRT,ABS,SIGN
10090
10100
10110
10120
10130
10140
10150
10160
10170
10180
10190
10200
10210
10220
10230
10240
10250
10260
10270
10280
10290
10300
10310
10320
10330
10340
10350
10360
10370
10380
10390
10400
10410
10420
10430
10440

```

C INTEGER MINO 10450
 LOGICAL NOTLAS 10460
 COMPLEX Z3 10470
 COMPLEX CMLPX 10480
 REAL T3(2) 10490
 EQUIVALENCE (Z3,T3(1)) 10500
 10510
 C THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE HQR2, 10520
 NUM. MATH. 16, 181-204(1970) BY PETERS AND WILKINSON. 10530
 HANDBOOK FOR AUTO. COMP., VOL.II-LINEAR ALGEBRA, 372-395(1971). 10540
 (REFERENCE 7) 10560
 C THIS SUBROUTINE FINDS THE EIGENVALUES AND EIGENVECTORS 10560
 OF A REAL UPPER HESSENBERG MATRIX BY THE QR METHOD. THE 10570
 EIGENVECTORS OF A REAL GENERAL MATRIX CAN ALSO BE FOUND 10580
 IF ELMHES AND ELTRAN OR ORTHES AND ORTRAN HAVE 10590
 BEEN USED TO REDUCE THIS GENERAL MATRIX TO HESSENBERG FORM 10600
 AND TO ACCUMULATE THE SIMILARITY TRANSFORMATIONS. 10610
 10620
 C ON INPUT- 10630
 10640
 C NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL 10650
 ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM 10660
 DIMENSION STATEMENT. 10670
 10680
 C N IS THE ORDER OF THE MATRIX. 10690
 10700
 C LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING 10710
 SUBROUTINE BALANC. IF BALANC HAS NOT BEEN USED, 10720
 SET LOW=1, IGH=N. 10730
 10740
 C H CONTAINS THE UPPER HESSENBERG MATRIX. 10750
 10760
 C Z CONTAINS THE TRANSFORMATION MATRIX PRODUCED BY ELTRAN 10770
 AFTER THE REDUCTION BY ELMHES, OR BY ORTRAN AFTER THE 10780
 REDUCTION BY ORTHES, IF PERFORMED. IF THE EIGENVECTORS 10790
 OF THE HESSENBERG MATRIX ARE DESIRED, Z MUST CONTAIN THE 10800


```

C          THE RELATIVE PRECISION OF FLOATING POINT ARITHMETIC.
C          *****
C          MACHEP = 2.**(-47)
C
C          IERR = 0
C          ***** STORE ROOTS ISOLATED BY BALANC *****
C          DO 50 I = 1, N
C             IF (I .GE. LOW .AND. I .LE. IGH) GO TO 50
C             WR(I) = H(I,I)
C             WI(I) = 0.0
C          50 CONTINUE
C
C          EN = IGH
C          T = 0.0
C          ***** SEARCH FOR NEXT EIGENVALUES *****
C          60 IF (EN .LT. LOW) GO TO 340
C             ITS = 0
C             NA = EN - 1
C             ENM2 = NA - 1
C             ***** LOOK FOR SINGLE SMALL SUB-DIAGONAL ELEMENT
C             FOR L=EN STEP -1 UNTIL LOW DO -- *****
C          70 DO 80 LL = LOW, EN
C             L = EN + LOW - LL
C             IF (L .EQ. LOW) GO TO 100
C             IF (ABS(H(L,L-1)) .LE. MACHEP * (ABS(H(L-1,L-1))
C                 + ABS(H(L,L)))) GO TO 100
C             X
C          80 CONTINUE
C             ***** FORM SHIFT *****
C          100 X = H(EN,EN)
C             IF (L .EQ. EN) GO TO 270
C             Y = H(NA,NA)
C             W = H(EN,NA) * H(NA,EN)
C             IF (L .EQ. NA) GO TO 280
C             IF (ITS .EQ. 30) GO TO 1000
C             IF (ITS .NE. 10 .AND. ITS .NE. 20) GO TO 130

```

```

11170
11180
11190
11200
11210
11220
11230
11240
11250
11260
11270
11280
11290
11300
11310
11320
11330
11340
11350
11360
11370
11380
11390
11400
11410
11420
11430
11440
11450
11460
11470
11480
11490
11500
11510
11520

```

```

C ***** FORM EXCEPTIONAL SHIFT *****
T = T + X
C DO 120 I = LOW, EN
H(I,I) = H(I,I) - X
C S = ABS(H(EN,NA)) + ABS(H(NA,ENM2))
X = 0.75 * S
Y = X
W = -0.4375 * S * S
ITS = ITS + 1
C ***** LOOK FOR TWO CONSECUTIVE SMALL
SUB-DIAGONAL ELEMENTS.
FOR M=EN-2 STEP -1 UNTIL L DO -- *****
DO 140 MM = L, ENM2
M = ENM2 + L - MM
ZZ = H(M,M)
R = X - ZZ
S = Y - ZZ
P = (R * S - W) / H(M+1,M) + H(M,M+1)
Q = H(M+1,M+1) - ZZ - R - S
R = H(M+2,M+1)
S = ABS(P) + ABS(Q) + ABS(R)
P = P / S
Q = Q / S
R = R / S
IF (M.EQ. L) GO TO 150
IF (ABS(H(M,M-1)) * (ABS(Q) + ABS(R)) .LE. MACHEP * ABS(P)
X * (ABS(H(M-1,M-1)) + ABS(ZZ) + ABS(H(M+1,M+1)))) GO TO 150
140 CONTINUE
C 150 MP2 = M + 2
C DO 160 I = MP2, EN
H(I,I-2) = 0.0
IF (I .EQ. MP2) GO TO 160

```

```

11530
11540
11550
11560
11570
11580
11590
11600
11610
11620
11630
11640
11650
11660
11670
11680
11690
11700
11710
11720
11730
11740
11750
11760
11770
11780
11790
11800
11810
11820
11830
11840
11850
11860
11870
11880

```

```

11890
11900
11910
11920
11930
11940
11950
11960
11970
11980
11990
12000
12010
12020
12030
12040
12050
12060
12070
12080
12090
12100
12110
12120
12130
12140
12150
12160
12170
12180
12190
12200
12210
12220
12230
12240

160      H(I,I-3) = 0.0
          CONTINUE
C          ***** DOUBLE QR STEP INVOLVING ROWS L TO EN AND
C          COLUMNS M TO EN *****
          DO 260 K = M, NA
            NOTLAS = K .NE. NA
            IF (K .EQ. M) GO TO 170
            P = H(K,K-1)
            Q = H(K+1,K-1)
            R = 0.0
            IF (NOTLAS) R = H(K+2,K-1)
            X = ABS(P) + ABS(Q) + ABS(R)
            IF (X .EQ. 0.0) GO TO 260
            P = P / X
            Q = Q / X
            R = R / X
            S = SIGN(SQRT(P*P+Q*Q+R*R),P)
            IF (K .EQ. M) GO TO 180
            H(K,K-1) = -S * X
            GO TO 190
          170      IF (L .NE. M) H(K,K-1) = -H(K,K-1)
          180      P = P + S
          190      X = P / S
                Y = Q / S
                ZZ = R / S
                Q = Q / P
                R = R / P
C          ***** ROW MODIFICATION *****
          DO 210 J = K, N
            P = H(K,J) + Q * H(K+1,J)
            IF (.NOT. NOTLAS) GO TO 200
            P = P + R * H(K+2,J)
            H(K+2,J) = H(K+2,J) - P * ZZ
            H(K+1,J) = H(K+1,J) - P * Y
            H(K,J) = H(K,J) - P * X
          200      CONTINUE
          210

```

```

12250
12260
12270
12280
12290
12300
12310
12320
12330
12340
12350
12360
12370
12380
12390
12400
12410
12420
12430
12440
12450
12460
12470
12480
12490
12500
12510
12520
12530
12540
12550
12560
12570
12580
12590
12600

C      J = MINO(EN,K+3)
C      ***** COLUMN MODIFICATION *****
      DO 230 I = 1, J
        P = X * H(I,K) + Y * H(I,K+1)
        IF (.NOT. NOTLAS) GO TO 220
        P = P + ZZ * H(I,K+2)
        H(I,K+2) = H(I,K+2) - P * R
        H(I,K+1) = H(I,K+1) - P * Q
        H(I,K) = H(I,K) - P
      CONTINUE
C      ***** ACCUMULATE TRANSFORMATIONS *****
      DO 250 I = LOW, IGH
        P = X * Z(I,K) + Y * Z(I,K+1)
        IF (.NOT. NOTLAS) GO TO 240
        P = P + ZZ * Z(I,K+2)
        Z(I,K+2) = Z(I,K+2) - P * R
        Z(I,K+1) = Z(I,K+1) - P * Q
        Z(I,K) = Z(I,K) - P
      CONTINUE
C      CONTINUE
C      CONTINUE
C      GO TO 70
C      ***** ONE ROOT FOUND *****
      H(EN,EN) = X + T
      WR(EN) = H(EN,EN)
      WI(EN) = 0.0
      EN = NA
      GO TO 60
C      ***** TWO ROOTS FOUND *****
      P = (Y - X) / 2.0
      Q = P * P + W
      ZZ = SORT(ABS(Q))
      H(EN,EN) = X + T
      X = H(EN,EN)

```

```

12610 H(NA,NA) = Y + T
12620 IF (Q .LT. 0.0) GO TO 320
12630 ***** REAL PAIR *****
12640 ZZ = P + SIGN(ZZ,P)
12650 WR(NA) = X + ZZ
12660 WR(EN) = WR(NA)
12670 IF (ZZ .NE. 0.0) WR(EN) = X - W / ZZ
12680 WI(NA) = 0.0
12690 WI(EN) = 0.0
12700 X = H(EN,NA)
12710 R = SQRT(X*X+ZZ*ZZ)
12720 P = X / R
12730 Q = ZZ / R
12740 ***** ROW MODIFICATION *****
12750 DO 290 J = NA, N
12760   ZZ = H(NA,J)
12770   H(NA,J) = Q * ZZ + P * H(EN,J)
12780   H(EN,J) = Q * H(EN,J) - P * ZZ
12790 CONTINUE
12800 ***** COLUMN MODIFICATION *****
12810 DO 300 I = 1, EN
12820   ZZ = H(I,NA)
12830   H(I,NA) = Q * ZZ + P * H(I,EN)
12840   H(I,EN) = Q * H(I,EN) - P * ZZ
12850 CONTINUE
12860 ***** ACCUMULATE TRANSFORMATIONS *****
12870 DO 310 I = LOW, IGH
12880   ZZ = Z(I,NA)
12890   Z(I,NA) = Q * ZZ + P * Z(I,EN)
12900   Z(I,EN) = Q * Z(I,EN) - P * ZZ
12910 CONTINUE
12920
12930 GO TO 330
12940 ***** COMPLEX PAIR *****
12950 WR(NA) = X + P
12960 WR(EN) = X + P

```

```

12970 WI(NA) = ZZ
12980 WI(EN) = -ZZ
12990 EN = ENM2
13000 GO TO 60
13010 ***** ALL ROOTS FOUND, BACKSUBSTITUTE TO FIND
13020 VECTORS OF UPPER TRIANGULAR FORM *****
13030 NORM = 0.0
13040 K = 1
13050 DO 360 I = 1, N
13060
13070 DO 350 J = K, N
13080 NORM = NORM + ABS(H(I,J))
13090
13100 K = I
13110 K = I
13120 K = I
13130 K = I
13140 IF (NORM .EQ. 0.0) GO TO 1001
13150 ***** FOR EN=N STEP -1 UNTIL 1 DO -- *****
13160 DO 800 NN = 1, N
13170 EN = N + 1 - NN
13180 P = WR(EN)
13190 Q = WI(EN)
13200 NA = EN - 1
13210 IF (Q) 710, 600, 800
13220 ***** REAL VECTOR *****
13230 M = EN
13240 H(EN, EN) = 1.0
13250 IF (NA .EQ. 0) GO TO 800
13260 ***** FOR I=EN-1 STEP -1 UNTIL 1 DO -- *****
13270 DO 700 II = 1, NA
13280 I = EN - II
13290 W = H(I,I) - P
13300 R = H(I,EN)
13310 IF (M .GT. NA) GO TO 620
13320

```

```

13330 DO 610 J = M, NA
13340 R = R + H(I,J) * H(J,EN)
13350
13360 IF (WI(I) .GE. 0.0) GO TO 630
13370 ZZ = W
13380 S = R
13390 GO TO 700
13400 M = I
13410 IF (WI(I) .NE. 0.0) GO TO 640
13420 T = W
13430 IF (W .EQ. 0.0) T = MACHEP * NORM
13440 H(I,EN) = -R / T
13450 GO TO 700
13460
13470
13480
13490
13500
13510
13520
13530
13540
13550
13560
13570
13580
13590
13600
13610
13620
13630
13640
13650
13660
13670
13680

610 DO 610 J = M, NA
C R = R + H(I,J) * H(J,EN)
620 IF (WI(I) .GE. 0.0) GO TO 630
ZZ = W
S = R
GO TO 700
M = I
630 IF (WI(I) .NE. 0.0) GO TO 640
T = W
IF (W .EQ. 0.0) T = MACHEP * NORM
H(I,EN) = -R / T
GO TO 700
C ***** SOLVE REAL EQUATIONS *****
640 X = H(I,I+1)
Y = H(I+1,I)
Q = (WR(I) - P) * (WR(I) - P) + WI(I) * WI(I)
T = (X * S - ZZ * R) / Q
H(I,EN) = T
IF (ABS(X) .LE. ABS(ZZ)) GO TO 650
H(I+1,EN) = (-R - W * T) / X
GO TO 700
H(I+1,EN) = (-S - Y * T) / ZZ
700 CONTINUE
C ***** END REAL VECTOR *****
GO TO 800
C ***** COMPLEX VECTOR *****
710 M = NA
C ***** LAST VECTOR COMPONENT CHOSEN IMAGINARY SO THAT
C EIGENVECTOR MATRIX IS TRIANGULAR *****
C IF (ABS(H(EN,NA)) .LE. ABS(H(NA,EN))) GO TO 720
H(NA,NA) = Q / H(EN,NA)
H(NA,EN) = -(H(EN,EN) - P) / H(EN,NA)
GO TO 730
720 Z3 = CMPLX(0.0, H(NA,EN)) / CMPLX(H(NA,NA) - P, Q)
H(NA,NA) = T3(1)

```

```

13690
13700
13710
13720
13730
13740
13750
13760
13770
13780
13790
13800
13810
13820
13830
13840
13850
13860
13870
13870
13870
13880
13890
13910
13920
13930
13940
13950
13960
13970
13980
13990
14000
14010
14020
14030
14040

H(NA,EN) = T3(2)
H(EN,NA) = 0.0
H(EN,EN) = 1.0
ENM2 = NA - 1
IF (ENM2 .EQ. 0) GO TO 800

C
DO 790 II = 1, ENM2
  I = NA - II
  W = H(I,I) - P
  RA = 0.0
  SA = H(I,EN)

C
DO 760 J = M, NA
  RA = RA + H(I,J) * H(J,NA)
  SA = SA + H(I,J) * H(J,EN)
CONTINUE

760
IF (WI(I) .GE. 0.0) GO TO 770
ZZ = W
R = RA
S = SA
GO TO 790
M = I
770
IF (WI(I) .NE. 0.0) GO TO 780
Z3 = CMPLX(-RA,-SA) / CMPLX(W,Q)
H(I,NA) = T3(1)
H(I,EN) = T3(2)
GO TO 790

C ***** SOLVE COMPLEX EQUATIONS *****
780
X = H(I,I+1)
Y = H(I+1,I)
VR = (WR(I) - P) * (WR(I) - P) + WI(I) * WI(I) - Q * Q
VI = (WR(I) - P) * 2.0 * Q
IF (VR .EQ. 0.0 .AND. VI .EQ. 0.0) VR = MACHEP * NOR1
  * (ABS(W) + ABS(Q) + ABS(X) + ABS(Y) + ABS(ZZ))
Z3 = CMPLX(X*R-ZZ*RA+Q*SA,X*S-ZZ*SA-Q*RA) / CMPLX(VR,VI)
X

```

```

14050 H(I,NA) = T3(1)
14060 H(I,EN) = T3(2)
14070 IF (ABS(X) .LE. ABS(ZZ) + ABS(Q)) GO TO 785
14080 H(I+1,NA) = (-RA - W * H(I,NA) + Q * H(I,EN)) / X
14090 H(I+1,EN) = (-SA - W * H(I,EN) - Q * H(I,NA)) / X
14100 GO TO 790
14110 Z3 = CMPLX(-R-Y*H(I,NA), -S-Y*H(I,EN)) / CMPLX(ZZ,Q)
14120 H(I+1,NA) = T3(1)
14130 H(I+1,EN) = T3(2)
14140
14150
14160
14170
14180
14190
14200
14210
14220
14230
14240
14250
14260
14270
14280
14290
14300
14310
14320
14330
14340
14350
14360
14370
14380
14390
14400

785
C ***** END COMPLEX VECTOR *****
790 CONTINUE
C ***** END BACK SUBSTITUTION.
C ***** VECTORS OF ISOLATED ROOTS *****
C DO 840 I = 1, N
C IF (I .GE. LOW .AND. I .LE. IGH) GO TO 840
C
C DO 820 J = I, N
C Z(I,J) = H(I,J)
C
C 840 CONTINUE
C ***** MULTIPLY BY TRANSFORMATION MATRIX TO GIVE
C ***** VECTORS OF ORIGINAL FULL MATRIX.
C ***** FOR J=N STEP -1 UNTIL LOW DO -- *****
C DO 880 JJ = LOW, N
C J = N + LOW - JJ
C M = MINO(J,IGH)
C
C DO 880 I = LOW, IGH
C ZZ = 0.0
C
C DO 860 K = LOW, M
C ZZ = ZZ + Z(I,K) * H(K,J)
C
C Z(I,J) = ZZ
C 880 CONTINUE

```

```

14410
14420
14430
14440
14450
14460
14470
14480
14490
14500
14510
14520
14530
14540
14550
14560
14570
14580
14590
14600
14610
14620
14630
14640
14650
14660
14670
14680
14690
14700
14710
14720
14730
14740
14750
14760

C      GO TO 1001
C      ***** SET ERROR -- NO CONVERGENCE TO AN
C      EIGENVALUE AFTER 30 ITERATIONS *****
C      1000 IERR = EN
C      1001 RETURN
C      ***** LAST CARD OF HQR2 *****
C      END
C      -----
C      SUBROUTINE ELTRAN(NM,N,LOW,IGH,A,INT,Z)
C      INTEGER I,J,N,KL,MM,MP,NM,IGH,LOW,MPT
C      REAL A(NM,IGH),Z(NM,N)
C      INTEGER INT(IGH)
C
C      THIS SUBROUTINE IS A TRANSLATION OF THE ALGOL PROCEDURE ELMTRANS,
C      NUM, MATH, 16, 181-204(1970) BY PETERS AND WILKINSON,
C      HANDBOOK FOR AUTO. COMP., VOL. II-LINEAR ALGEBRA, 372-395(1971).
C      (REFERENCE 7)
C      THIS SUBROUTINE ACCUMULATES THE STABILIZED ELEMENTARY
C      SIMILARITY TRANSFORMATIONS USED IN THE REDUCTION OF A
C      REAL GENERAL MATRIX TO UPPER HESSENBERG FORM BY ELMHES,
C      ON INPUT-
C
C      NM MUST BE SET TO THE ROW DIMENSION OF TWO-DIMENSIONAL
C      ARRAY PARAMETERS AS DECLARED IN THE CALLING PROGRAM
C      DIMENSION STATEMENT,
C      N IS THE ORDER OF THE MATRIX,
C      LOW AND IGH ARE INTEGERS DETERMINED BY THE BALANCING
C      SUBROUTINE BALANC, IF BALANC HAS NOT BEEN USED,
C      SET LOW=1, IGH=N,

```

14770
 14780
 14790
 14800
 14810
 14820
 14830
 14840
 14850
 14860
 14870
 14880
 14890
 14900
 14910
 14920
 14930
 14940
 14950
 14960
 14970
 14980
 14990
 15000
 15010
 15020
 15030
 15040
 15050
 15060
 15070
 15080
 15090
 15100
 15110
 15120

```

C C A CONTAINS THE MULTIPLIERS WHICH WERE USED IN THE
C C REDUCTION BY ELMHES IN ITS LOWER TRIANGLE
C C BELOW THE SUBDIAGONAL,
C C
C C INT CONTAINS INFORMATION ON THE ROWS AND COLUMNS
C C INTERCHANGED IN THE REDUCTION BY ELMHES,
C C ONLY ELEMENTS LOW THROUGH IGH ARE USED.
C C
C C ON OUTPUT -
C C
C C Z CONTAINS THE TRANSFORMATION MATRIX PRODUCED IN THE
C C REDUCTION BY ELMHES,
C C
C C -----
C C ***** INITIALIZE Z TO IDENTITY MATRIX *****
C C DO 80 I = 1, N
C C DO 60 J = 1, N
C C Z(I,J) = 0.0
C C
C C Z(I,I) = 1.0
C C 80 CONTINUE
C C
C C KL = IGH - LOW - 1
C C IF (KL .LT. 1) GO TO 200
C C ***** FOR MP=IGH-1 STEP -1 UNTIL LOW+1 DO -- *****
C C DO 140 MM = 1, KL
C C MP = IGH - MM
C C MP1 = MP + 1
C C
C C DO 100 I = MP1, IGH

```



```

16210
16220
16230
16240
16250
16260
16270
16280
16290
16300
16310
16320
16330
16340
16350
16360
16370
16380
16390
16400
16410
16420
16430
16440
16450
16460
16470
16480
16490
16500
16510
16520
16530
16540
16550
16560

      K = 1
4 IF (EIGVAL(2,K).NE.0.0) GO TO 6
  DO 5 J=1,M
    EIGVEC(2,J,K) = 0.0
    EIGVEC(1,J,K) = A(J,K)
5 CONTINUE
  GO TO 8
6 DO 7 J=1,M
  EIGVEC(2,J,K) = A(J,K+1)
  EIGVEC(2,J,K+1) = -A(J,K+1)
  EIGVEC(1,J,K) = A(J,K)
  EIGVEC(1,J,K+1) = A(J,K)
7 CONTINUE
  K = K+1
8 K = K+1
  IF (K.LE.N) GO TO 4
  GO TO 12
9 CALL HQR(M,N,LOW,IGH,A,EIGVAL,EIGVAL(N+1),IND)
  DO 10 I=1,N
    WORK(I) = EIGVAL(N+I)
10 CONTINUE
  DO 11 I=1,N
    EIGVAL(2,N-I+1) = WORK(N-I+1)
    EIGVAL(1,N-I+1) = EIGVAL(N-I+1)
11 CONTINUE
  RETURN
12 DO 15 J=1,N
  CMOD = 0.0
  DO 13 I=1,N
  B = CMPLX(EIGVEC(1,I,J),EIGVEC(2,I,J))
  TEMP = CABS(B)
  IF(TEMP.LE.CMOD) GO TO 13
  K = I
  CMOD = TEMP
13 CONTINUE
  DENOM = CMPLX(EIGVEC(1,K,J),EIGVEC(2,K,J))

```

16570
16580
16590
16600
16610
16620
16630
16640
16550
16660
16670

```
DO 14 I=1,N
  B = CMPLX(EIGVEC(1,I,J),EIGVEC(2,I,J))
  B = B/DENOM
  EIGVEC(1,I,J) = REAL(B)
  EIGVEC(2,I,J) = AIMAG(B)
14 CONTINUE
15 CONTINUE
  RETURN
20 IND = N + 1
  RETURN
  END
```

APPENDIX C
INTERMEDIATE MATRICES (FOR CASE A)

1.22747019E+06	M=SUM=L	MATRIX	-1.45002334E+05
1.70995429E+05			1.58123755E+04
-1.34825576E+06			1.12802423E+04
2.72271204E+06			5.01848180E+06
-3.10745802E+05			3.05860007E+04
-4.16471809E+01			1.00000000E+00
-5.20229292E+01			9.28344458E+01
4.2118011E+02			6.42045207E+01
6.23174609E+02			2.91970812E+02
1.00000000E+00			6.19370934E+01
-3.62433035E+03			2.14154621E+03
1.24044444E+03			1.49070922E+04
-4.54701452E+04			1.02405701E+05
1.65110425E+04			7.08992724E+07
1.50460807E+01			7.04546734E+01
1.00000000E+06			8.50000000E+04
-1.247500000E+06			0.
6.25409300E+02			1.52454002E+03
-2.41080776E+01			8.83955224E+01
			1.23395522E+00

BEST AVAILABLE COPY

1.0000000E+00	F-SUR=L (0)	1.0000000E+00	0.	0.	0.
1.0000000E+00	F-SUR=O (0)	7.00346734E-01	0.	0.	0.
1.0000000E+01	F-SUR=Q (0)	-2.41088774E-01	0.	0.	0.25495800E+02
1.0000000E+00	Y-SUR=AU (0)	1.03959224E-01	0.	0.	1.13427469E+03
1.0000000E+00		5.33955224E-01	0.	0.	1.34000070E+03
1.0000000E+00		0.03955224E-01	0.	0.	1.5245002E+03
1.0000000E+00		1.2339522E+00	0.	0.	0.
0.0	Y-SUR=AM (0)	0.	-2.41088774E+05	0.	0.
0.0		0.	1.96361940E+04	0.	0.
0.0		0.	4.53961940E+04	0.	0.
0.0		0.	7.51361940E+04	0.	0.
0.0		0.	1.00000194E+05	0.	0.
0.0	Y-SUR=AV (0)	0.	0.	0.	0.
0.0	Y-SUB=A TMEAL (0)	1.00000000E+00	0.	0.	0.
0.0	Y-SUR=A BETA (0)	0.	1.00000000E+00	0.	0.
0.0	Y-SUR=A PHI (0)	0.	0.	0.	1.00000000E+00

0.37041932E-01	F-SUR=L (1)	9.66105974E-01	-1.20522388E-01	-3.01509970E-02
2.01773047E-01	F-SUR=G (1)	7.33194062E-01	9.19776119E-02	6.25514403E+02
1.70000000E+00	Y-SUR=AU (1)	-2.41044774E-01	2.66977612E-01	1.13634143E+03
1.70000000E+00	F-SUR=L (1)	1.63955224E-01	4.41977612E-01	1.34099119E+03
1.70000000E+00	F-SUR=G (1)	5.33955224E-01	6.16977612E-01	1.52471464E+03
1.70000000E+00	Y-SUR=AU (1)	8.63955224E-01	-2.41044774E+05	-1.20522388E+05
0.0	F-SUR=L (1)	0.	1.56361940E+04	7.41409701E+03
0.0	F-SUR=G (1)	0.	4.53461940E+04	2.26930970E+04
0.0	Y-SUR=AU (1)	0.	7.51361940E+04	3.75467970E+04
0.0	F-SUR=L (1)	0.	1.04486194E+05	5.24430970E+04
0.0	F-SUR=G (1)	5.00000000E+01	1.25000000E-01	2.04333333E-02
0.0	Y-SUR=AU (1)	1.00000000E+00	5.00000000E-01	1.25000000E-01
0.0	F-SUR=L (1)	0.	1.00000000E+00	5.00000000E-01
0.0	F-SUR=G (1)	0.	0.	1.00000000E+00
0.0	Y-SUR=AU (1)	0.	0.	0.

50659600E+01	F-SUR=L (3)	7.74093310E+01	0.17193337E+01	9.0172489E+01	-3.6156716E+01	-2.7117337E+01
20794387E+01	F-SUR=G (3)	5.50196390E+01	6.24344907E+01	7.85566917E+01	2.7593203E+01	0.2570275E+02
10000000E+00	Y-SUR=AU (3)	1.5000000E+00	0.	-2.4104476E+01	0.0093203E+01	1.14487519E+03
10000000E+00		1.5000000E+00	0.	1.6395522E+01	1.3259124E+00	1.38167515E+03
10000000E+00		1.5000000E+00	0.	5.3395522E+01	1.0509328E+00	1.5259446E+03
10000000E+00		1.5000000E+00	0.	0.6395522E+01	1.0509328E+00	1.5259446E+03
10000000E+00	Y-SUR=AM (3)	1.5000000E+00	0.	1.23394522E+00	-2.4104476E+01	-3.6156716E+01
07		1.0000000E+06	0.	0.	1.5636194E+04	2.34542910E+04
07		0.5000000E+04	0.	0.	0.5386194E+04	0.80792910E+04
07		0.5000000E+04	0.	0.	7.5136194E+04	1.12702291E+05
07		0.5000000E+04	0.	0.	1.04486194E+05	1.57329291E+05
07	Y-SUR=AV (3)	0.5000000E+04	0.	0.	1.12500000E+00	5.62500000E+01
07	Y-SUR=AV (3)	0.	1.00000000E+00	1.50000000E+00	1.50000000E+00	1.50000000E+00
07	Y-SUR=AV (3)	0.	0.	1.00000000E+00	1.00000000E+00	1.50000000E+00
07	Y-SUR=AV (3)	0.	0.	0.	1.00000000E+00	1.50000000E+00
07	Y-SUR=AV (3)	0.	0.	0.	0.	1.00000000E+00
07	Y-SUR=AV (3)	0.	0.	0.	0.	1.00000000E+00
07	Y-SUR=AV (3)	0.	0.	0.	0.	1.00000000E+00

0.1002249E+01	F=SUB=L (5)	0.4163337E-01	-6.0261190E-01	-7.5326092E-01
0.1002249E+01	F=SUB=L (5)	0.4163337E-01	0.5000000E+01	0.2607069E+02
0.1002249E+01	F=SUB=L (5)	0.4163337E-01	1.3346840E+00	1.1350433E+03
1.0000000E+00	Y=SUB=AU (5)	-2.4104477E-01	2.2098840E+00	1.3836030E+03
1.0000000E+00	0.	1.8395224E-01	3.0848440E+00	1.5284165E+03
1.0000000E+00	0.	9.3395224E-01	-2.4104477E+05	-4.0261190E+05
1.0000000E+00	0.	0.8395224E-01	1.5636194E+04	3.0000465E+04
1.0000000E+00	0.	1.2339522E+00	4.5386194E+04	1.1346548E+05
0.	Y=SUB=AM (5)	0.	7.5136194E+04	1.6740045E+05
0.	0.	0.	1.0488019E+05	2.6221548E+05
0.	Y=SUB=AV (5)	0.	3.1250000E+00	2.6041647E+00
0.	Y=SUB=A YMETAL (5)	2.5000000E+00	2.5000000E+00	3.1250000E+00
0.	Y=SUB=A BETA (5)	1.0000000E+00	1.0000000E+00	2.5000000E+00
0.	Y=SUB=A PHI (5)	0.	0.	1.0000000E+00
0.	0.	0.	0.	0.

37.44013480E-01	P-SUR-L (6)	0.13104698E-01	-7.23158328E-01	-1.08270149E+00
47.90960725E-01	S-SUR-0 (6)	0.71140080E-01	5.51849672E-01	8.26323407E+02
17.00000000E+00	Y-SUR-AU (6)	-2.41044774E-01	1.60186367E+00	1.13467749E+03
17.00000000E+00	0.	1.43955224E-01	2.65186367E+00	1.80485450E+03
17.00000000E+00	0.	5.31955224E-01	3.70186367E+00	1.53011322E+03
17.00000000E+00	0.	8.43955224E-01	-2.41044774E+03	-7.23158328E+03
17.00000000E+00	Y-SUR-AM (6)	1.23395222E+00	0.	0.
07	0.	0.	1.54361940E+04	4.80045421E+04
07	0.	0.	4.53861940E+04	1.36144542E+05
07	0.	0.	7.51361940E+04	2.254004542E+05
07	0.	0.	1.0486194E+05	3.14454542E+05
07	Y-SUR-AV (6)	3.00000000E+00	4.50000000E+00	4.50000000E+00
07	0.	1.00000000E+00	3.00000000E+00	4.50000000E+00
07	Y-SUB-A THETA (6)	0.	1.00000000E+00	3.00000000E+00
07	0.	0.	0.	1.00000000E+00
07	Y-SUB-A BETA (6)	0.	0.	0.
07	0.	0.	0.	0.
07	Y-RUR-A PHI (6)	0.	0.	0.
07	0.	0.	0.	0.

2.61963174E+01	F-SUR-L (7)	7.85546017E+01	-8.43656716E+01	-1.47639925E+00
5.50196399E-01	F-SUR-G (7)	9.01724292E+01	6.43683284E+01	4.2662254E+02
7.74093114E-01	Y-SUR-AU (7)	-2.41944774E+01	1.86884328E+00	1.13754516E+03
3.50000000E+00	0.	1.83955224E+01	3.09384328E+00	1.34629492E+03
3.50000000E+00	0.	5.33955224E+01	4.31844328E+00	1.5321149E+03
3.50000000E+00	0.	8.83955224E+01	-2.41044776E+05	-8.43656716E+05
3.50000000E+00	Y-SUR-AM (7)	1.23395522E+00	1.56361940E+04	5.47244791E+04
1.00000000E+06	0.	0.	4.53861940E+04	1.58451679E+05
8.50000000E+04	0.	0.	7.51361940E+04	2.62874679E+05
8.50000000E+04	0.	0.	1.04486194E+05	3.67101679E+05
8.50000000E+04	Y-SUR-AV (7)	0.	6.12500000E+00	7.14583331E+00
0.	Y-SUR-A TME TA (7)	3.50000000E+00	3.50000000E+00	6.12500000E+00
0.	0.	1.00000000E+00	1.00000000E+00	3.50000000E+00
0.	Y-SUR-A BETA (7)	0.	1.00000000E+00	3.50000000E+00
0.	0.	0.	0.	1.00000000E+00
0.	Y-SUR-A PHI (7)	0.	0.	1.00000000E+00
0.	0.	0.	0.	1.00000000E+00

27	2.41088361E+01	P-SUR=L (R)	7.58021259E+01	-9.64179104E+01	-1.92835821E+00
28	7.00688780E+01	F-SUR=G (R)	9.33390000E+01	7.35820806E+01	8.26987450E+02
29	1.00000000E+00	V-SUR=AU (B)	-2.41044776E+01	2.13582090E+00	1.13854633E+03
30	1.00000000E+00	0.	1.81955224E+01	3.53582090E+00	1.38795234E+03
31	1.00000000E+00	0.	5.33955224E+01	4.9352090E+00	1.53443206E+03
32	1.00000000E+00	0.	6.83955224E+01	-2.41044776E+05	-9.64179104E+05
33	1.00000000E+00	0.	1.23395522E+00	1.56361940E+04	6.25447741E+04
01	0.	0.	0.	4.53881940E+04	1.81544776E+05
02	0.	0.	0.	7.51361940E+04	3.00544776E+04
03	0.	0.	0.	1.04888194E+05	4.19544776E+05
04	0.	0.	4.00000000E+00	8.00000000E+00	1.00000000E+01
05	0.	0.	1.00000000E+00	4.00000000E+00	8.00000000E+00
06	0.	0.	0.	1.00000000E+00	4.00000000E+00
07	0.	0.	0.	0.	4.00000000E+00
08	0.	0.	0.	0.	1.00000000E+00
09	0.	0.	0.	0.	1.00000000E+00
10	0.	0.	0.	0.	1.00000000E+00
11	0.	0.	0.	0.	1.00000000E+00
12	0.	0.	0.	0.	1.00000000E+00
13	0.	0.	0.	0.	1.00000000E+00
14	0.	0.	0.	0.	1.00000000E+00
15	0.	0.	0.	0.	1.00000000E+00
16	0.	0.	0.	0.	1.00000000E+00
17	0.	0.	0.	0.	1.00000000E+00
18	0.	0.	0.	0.	1.00000000E+00
19	0.	0.	0.	0.	1.00000000E+00
20	0.	0.	0.	0.	1.00000000E+00
21	0.	0.	0.	0.	1.00000000E+00
22	0.	0.	0.	0.	1.00000000E+00
23	0.	0.	0.	0.	1.00000000E+00
24	0.	0.	0.	0.	1.00000000E+00
25	0.	0.	0.	0.	1.00000000E+00
26	0.	0.	0.	0.	1.00000000E+00
27	0.	0.	0.	0.	1.00000000E+00
28	0.	0.	0.	0.	1.00000000E+00
29	0.	0.	0.	0.	1.00000000E+00
30	0.	0.	0.	0.	1.00000000E+00
31	0.	0.	0.	0.	1.00000000E+00
32	0.	0.	0.	0.	1.00000000E+00
33	0.	0.	0.	0.	1.00000000E+00
34	0.	0.	0.	0.	1.00000000E+00
35	0.	0.	0.	0.	1.00000000E+00
36	0.	0.	0.	0.	1.00000000E+00
37	0.	0.	0.	0.	1.00000000E+00
38	0.	0.	0.	0.	1.00000000E+00
39	0.	0.	0.	0.	1.00000000E+00
40	0.	0.	0.	0.	1.00000000E+00
41	0.	0.	0.	0.	1.00000000E+00
42	0.	0.	0.	0.	1.00000000E+00
43	0.	0.	0.	0.	1.00000000E+00
44	0.	0.	0.	0.	1.00000000E+00
45	0.	0.	0.	0.	1.00000000E+00
46	0.	0.	0.	0.	1.00000000E+00
47	0.	0.	0.	0.	1.00000000E+00
48	0.	0.	0.	0.	1.00000000E+00
49	0.	0.	0.	0.	1.00000000E+00
50	0.	0.	0.	0.	1.00000000E+00
51	0.	0.	0.	0.	1.00000000E+00
52	0.	0.	0.	0.	1.00000000E+00
53	0.	0.	0.	0.	1.00000000E+00
54	0.	0.	0.	0.	1.00000000E+00
55	0.	0.	0.	0.	1.00000000E+00
56	0.	0.	0.	0.	1.00000000E+00
57	0.	0.	0.	0.	1.00000000E+00
58	0.	0.	0.	0.	1.00000000E+00
59	0.	0.	0.	0.	1.00000000E+00
60	0.	0.	0.	0.	1.00000000E+00
61	0.	0.	0.	0.	1.00000000E+00
62	0.	0.	0.	0.	1.00000000E+00
63	0.	0.	0.	0.	1.00000000E+00
64	0.	0.	0.	0.	1.00000000E+00
65	0.	0.	0.	0.	1.00000000E+00
66	0.	0.	0.	0.	1.00000000E+00
67	0.	0.	0.	0.	1.00000000E+00
68	0.	0.	0.	0.	1.00000000E+00
69	0.	0.	0.	0.	1.00000000E+00
70	0.	0.	0.	0.	1.00000000E+00
71	0.	0.	0.	0.	1.00000000E+00
72	0.	0.	0.	0.	1.00000000E+00
73	0.	0.	0.	0.	1.00000000E+00
74	0.	0.	0.	0.	1.00000000E+00
75	0.	0.	0.	0.	1.00000000E+00
76	0.	0.	0.	0.	1.00000000E+00
77	0.	0.	0.	0.	1.00000000E+00
78	0.	0.	0.	0.	1.00000000E+00
79	0.	0.	0.	0.	1.00000000E+00
80	0.	0.	0.	0.	1.00000000E+00
81	0.	0.	0.	0.	1.00000000E+00
82	0.	0.	0.	0.	1.00000000E+00
83	0.	0.	0.	0.	1.00000000E+00
84	0.	0.	0.	0.	1.00000000E+00
85	0.	0.	0.	0.	1.00000000E+00
86	0.	0.	0.	0.	1.00000000E+00
87	0.	0.	0.	0.	1.00000000E+00
88	0.	0.	0.	0.	1.00000000E+00
89	0.	0.	0.	0.	1.00000000E+00
90	0.	0.	0.	0.	1.00000000E+00
91	0.	0.	0.	0.	1.00000000E+00
92	0.	0.	0.	0.	1.00000000E+00
93	0.	0.	0.	0.	1.00000000E+00
94	0.	0.	0.	0.	1.00000000E+00
95	0.	0.	0.	0.	1.00000000E+00
96	0.	0.	0.	0.	1.00000000E+00
97	0.	0.	0.	0.	1.00000000E+00
98	0.	0.	0.	0.	1.00000000E+00
99	0.	0.	0.	0.	1.00000000E+00
100	0.	0.	0.	0.	1.00000000E+00

BEST AVAILABLE COPY

2.01993027E-01	F-SUR=L (9)	7.33190062E+01	-1.00000000E+00	-2.00000000E+00
4.63852559E+01	F-SUR=0 (9)	8.27798307E-01	0.27798307E-01	0.27798307E-01
9.18186930E-01	F-SUR=0 (9)	2.40279051E+00	2.40279051E+00	1.13968090E+03
4.50000000E+00	V-SUR=AU (9)	3.97779051E+00	3.97779051E+00	1.38983075E+03
4.50000000E+00	0.	5.45279051E+00	5.45279051E+00	1.53705421E+03
4.50000000E+00	0.	-2.81044776E+05	-2.81044776E+05	-1.00000000E+00
4.50000000E+00	0.	1.56361940E+04	1.56361940E+04	7.03620731E+04
4.50000000E+00	0.	4.53861940E+04	4.53861940E+04	2.00217073E+04
4.50000000E+00	0.	7.51361940E+04	7.51361940E+04	3.38112073E+05
4.50000000E+00	0.	1.04806194E+05	1.04806194E+05	4.71007073E+05
1.00000000E+00	Y-SUR=AV (9)	1.01250000E+01	1.01250000E+01	1.51075000E+01
0.	Y-SUR=ALPHA (9)	4.50000000E+00	4.50000000E+00	1.01250000E+01
0.	Y-SUR=ALPHA (9)	1.00000000E+00	1.00000000E+00	1.01250000E+01
0.	Y-SUR=ALPHA (9)	0.	1.00000000E+00	4.50000000E+00
0.	Y-SUR=ALPHA (9)	0.	0.	1.00000000E+00
0.	Y-SUR=ALPHA (9)	0.	0.	1.00000000E+00

15.0000000E+00	F-SUB-L (10)	7.08346738E-01	-1.2052238E+00	-3.01305970E+00
16.0000000E+00	F-SUB-G (10)	1.00000000E+00	9.19776119E-01	8.27705249E+02
17.0000000E+00	Y-SUB-AU (10)	-2.41044774E-01	2.66977612E+00	1.14004913E+03
18.0000000E+00	0.	1.83955224E-01	4.41977612E+00	1.21193114E+03
19.0000000E+00	0.	5.33955224E-01	6.16977612E+00	1.53994486E+03
20.0000000E+00	0.	4.83955224E-01	-2.41044774E+05	-1.2052238E+04
21.0000000E+00	Y-SUB-AM (10)	1.23395522E+00	1.56361940E+04	7.8180701E+04
02.	1.00000000E+06	0.	4.33881940E+04	2.26930970E+05
03.	8.50000000E+04	0.	7.51361940E+04	3.75680970E+05
04.	8.50000000E+04	0.	1.04886194E+05	5.24430970E+05
05.	8.50000000E+04	0.	1.25000000E+01	2.08333333E+01
06.	Y-SUB-AV (10)	5.00000000E+00	5.00000000E+00	1.25000000E+01
07.	0.	1.00000000E+00	1.00000000E+00	5.00000000E+00
08.	Y-SUB-A THETA (10)	0.	0.	0.
09.	Y-SUB-A BETA (10)	0.	0.	0.
10.	Y-SUB-A PHI (10)	0.	0.	0.

APPENDIX D
OUTPUT FOR ILLUSTRATIVE CASE A, B, C, AND D
AND FOR THE TEST BEAM

CASE A

M	-0.988101E-02	-0.766906E-02	-1.92937E-03	3.091331E-03	1.018038E-01
VPE	0.	THETA	-0.551115E-17	BETA	3.062296E-08
PMI	-1.920000E-04				
NYJ	-1.970272E-15	-0.117707E-15	1.620926E-16	-7.051330E-15	
VYJ	1.600000E-01	0.500000E-02	0.500000E-02	0.500000E-02	0.400000E-02
WYJ	3.210800E-01	1.709071E-01	1.709071E-01	1.709071E-01	1.779971E-01
X(1)					
U	-0.600010E-08	-0.60007E-07	-1.113675E-08	2.251000E-08	1.109972E-06
M	-0.033333E-02	-0.219557E-02	-1.029517E-03	3.690235E-03	1.006002E-01
VPE	0.430591E-05	THETA	1.700000E-04	BETA	2.056672E-04
PMI	-1.711782E-04				
NYJ	1.125515E-02	4.304150E-02	4.322022E-02	0.285449E-02	
VYJ	1.037111E-01	0.301000E-02	0.609900E-02	0.609103E-02	0.111302E-02
WYJ	2.063093E-01	1.308943E-01	1.308943E-01	1.308943E-01	1.308943E-01
X(2)					
U	-0.070011E-08	-1.000100E-06	-2.110001E-08	0.270813E-08	2.000070E-06
M	-0.451720E-02	-7.702093E-02	-1.553133E-03	3.139095E-03	1.50097E-01
VPE	1.020973E-08	THETA	2.971771E-04	BETA	2.101732E-04
PMI	-1.556290E-04				
NYJ	3.710505E-02	7.910500E-02	7.908205E-02	7.649495E-02	
VYJ	1.513407E-01	0.293990E-02	0.777990E-02	0.700022E-02	7.020296E-02
WYJ	1.702770E-01	0.081630E-02	0.491630E-02	0.481630E-02	0.081630E-02
X(3)					
U	-1.213901E-07	-1.057231E-06	-2.911223E-08	5.000070E-08	2.055003E-06
M	-5.109921E-02	-5.502965E-02	-1.125000E-03	2.276101E-03	1.075300E-01
VPE	3.150700E-04	THETA	3.053066E-04	BETA	1.592010E-04
PMI	-1.000025E-04				
NYJ	0.020000E-02	0.700320E-02	0.030102E-02	0.600010E-02	
VYJ	1.231891E-01	0.233300E-02	0.000000E-02	0.000000E-02	7.610230E-02
WYJ	1.160300E-01	0.100333E-02	0.100333E-02	0.100333E-02	0.100333E-02
X(4)					
U	-1.000010E-07	-1.700030E-06	-3.020230E-08	0.915732E-08	3.500070E-06
M	-0.009720E-02	-2.000301E-02	-5.000200E-04	1.100000E-03	5.000000E-02
VPE	0.021007E-08	THETA	0.371201E-04	BETA	0.097172E-05
PMI	-1.000035E-04				
NYJ	0.070303E-02	1.100000E-01	1.100000E-01	1.000000E-01	1.000000E-01
VYJ	1.161616E-01	0.197710E-02	0.000000E-02	0.000000E-02	7.000000E-02
WYJ	0.910010E-02	1.035770E-02	3.035770E-02	3.035770E-02	3.035770E-02

CASE A

```

X( 9)
U -1.491028E-07 -1.789893E-06 -3.595931E-08 7.267924E-08 3.506917E-06
W -6.771472E-12 -2.100510E-13 -2.078330E-13 3.161915E-13 3.466132E-13
VEE 7.466274E-08 THETA 4.582276E-08 BETA 1.566672E-17 PMI -1.362289E-08
X( 8)
U 9.688888E-02 1.147791E-01 1.159756E-01 1.135482E-01
VUJ 1.169495E-01 8.185945E-02 8.923076E-02 8.909739E-02 7.460041E-02
WUJ 1.305956E-14 6.935766E-15 6.935766E-15 6.935766E-15 6.935766E-15
X( 4)
U -1.280410E-07 -1.708038E-06 -3.420239E-08 6.913732E-08 3.380974E-06
W 2.466722E-02 2.460301E-02 3.909254E-04 -1.196684E-03 -5.606667E-02
VEE 9.068422E-08 THETA 4.571261E-08 BETA -6.857172E-05 PMI -1.385355E-04
X( 7)
U 9.870335E-02 1.104667E-01 1.116108E-01 1.029788E-01
VUJ 1.181818E-01 8.197714E-02 8.909505E-02 8.896459E-02 7.499304E-02
WUJ -8.718310E-02 -3.035727E-02 -3.035727E-02 -3.035727E-02 -3.035727E-02
X( 7)
U -1.213801E-07 -1.572515E-06 -2.811225E-08 5.888679E-08 2.854815E-06
W 3.397821E-02 3.502985E-02 1.129589E-03 -2.276101E-03 -1.074568E-01
VEE 1.19772E-03 THETA 3.053066E-08 BETA -1.592614E-04 PMI -1.488625E-08
X( 6)
U 4.206622E-02 9.740329E-02 9.836162E-02 9.460218E-02
VUJ 1.231818E-01 8.233366E-02 8.808492E-02 8.808008E-02 7.618238E-02
WUJ -1.160345E-01 -6.160335E-02 -6.160335E-02 -6.160335E-02 -6.160335E-02
X( 8)
U 8.478011E-08 -1.066100E-06 -2.116841E-08 8.278813E-08 2.889070E-06
W 7.851728E-02 7.702895E-02 1.535135E-03 -3.139285E-03 -1.409997E-01
VEE 1.569958E-03 THETA 2.971771E-08 BETA -2.181735E-04 PMI -1.556288E-08
X( 9)
U 3.716505E-02 7.516594E-02 7.588209E-02 7.443493E-02
VUJ 1.319897E-01 8.293946E-02 8.777992E-02 8.768822E-02 7.828296E-02
WUJ -1.988792E-01 -8.881638E-02 -8.881638E-02 -8.881638E-02 -8.881638E-02
X( 9)
U -8.689614E-08 -9.648407E-07 -1.113675E-08 2.251040E-08 1.189912E-06
W 9.033333E-02 9.219557E-02 1.828917E-03 -3.498023E-03 -1.804668E-01
VEE 1.688855E-03 THETA 1.700808E-08 BETA -2.956672E-08 PMI -1.711722E-04
WUJ 8.124113E-02 8.130814E-02 8.134202E-02 8.128949E-02

```

CASE A

VUJ 1.03711E+01 0.30180E+02 0.65890E+02 0.65013E+02 0.11130E+02
 WUJ -8.00303E+01 -1.30893E+01 -1.30893E+01 -1.30893E+01 -1.30893E+01
 VC 10)
 U 1.93094E-17 2.77550E-17 3.06947E-18 -5.20417E-18 -1.38779E-17
 W 0.94410E+02 0.78090E+02 1.92307E+03 -3.89133E+03 -1.91008E+01
 VZE 1.932051E+03 THETA 0. BETA -3.86229E+08 PHI -1.02000E+08
 BUJ -3.60992E+15 1.77287E+14 0.33174E+15 0.33174E+15
 WUJ 1.40000E+01 0.50000E+02 0.50000E+02 0.50000E+02 0.50000E+02
 WUJ -3.51850E+01 -1.70987E+01 -1.70987E+01 -1.70987E+01 -1.70987E+01
 P1(ESP) 3.30780E+03

BEST AVAILABLE COPY

BEST AVAILABLE COPY

CASE A

M 00.009379E=01 00.097110E=01 -7.902772E=02 2.140342E=01 1.072663E=00
VPE -0.453115E=17 TMEYA -0.103336E=17 BETA 5.354590E=00 PHI -1.020000E=04
GVJ 00.076823E=13 -2.056719E=10 -3.355823E=13 1.373908E=14
VVJ 1.400000E=01 0.500000E=02 0.500000E=02 0.500000E=02 0.500000E=02 0.500000E=02
WVJ 0.620825E=01 2.459063E=01 2.459063E=01 2.459063E=01 2.459063E=01 2.459063E=01
V(1)
U -1.462524E=06 -1.392023E=05 -1.370002E=06 3.725642E=06 2.077079E=05
W 00.307907E=01 -7.090929E=01 -7.067957E=02 2.037555E=01 1.551240E=00
VPE 9.290808E=00 TMEYA 6.2498113E=00 BETA 3.299032E=00 PHI -1.154021E=00
GVJ 0.237823E=02 1.560651E=01 1.623069E=01 1.077438E=01
VVJ 1.008378E=01 0.127238E=02 9.102584E=02 0.908567E=02 0.959731E=02
WVJ 2.709903E=01 1.460907E=01 1.460907E=01 1.460907E=01 1.460907E=01 1.460907E=01
V(2)
U 00.741618E=06 -2.499118E=05 -2.508000E=06 7.070531E=06 5.377203E=05
W -7.429132E=01 -0.083005E=01 -0.338398E=02 1.722098E=01 1.261020E=00
VPE 1.017710E=03 TMEYA 1.033795E=03 BETA 1.029854E=00 PHI -7.120126E=05
GVJ 1.350820E=01 2.070575E=01 2.070575E=01 2.000418E=01
VVJ 0.619810E=02 7.932439E=02 9.497356E=02 9.246659E=02 0.031203E=02
WVJ 1.608212E=01 0.503627E=02 0.503627E=02 0.503627E=02 0.503627E=02 0.503627E=02
V(3)
U -3.720805E=06 -3.513893E=05 -3.563511E=06 9.706826E=06 7.276832E=05
W 00.955124E=01 -0.232616E=01 -0.308358E=02 1.202404E=01 0.799777E=01
VPE 3.509596E=03 TMEYA 1.253797E=03 BETA 1.065091E=00 PHI -0.653331E=05
GVJ 1.647703E=01 2.972951E=01 3.106370E=01 2.743103E=01
VVJ 0.701620E=02 7.035966E=02 9.659480E=02 9.372169E=02 5.006987E=02
WVJ 0.875738E=02 0.715207E=02 0.715207E=02 0.715207E=02 0.715207E=02 0.715207E=02
V(4)
U -0.331640E=06 -0.060222E=05 -0.178251E=06 1.158899E=05 0.439106E=05
W -7.752933E=01 -2.161712E=01 -2.372479E=02 0.499538E=02 4.501560E=01
VPE 4.926080E=03 TMEYA 1.560828E=03 BETA 0.730022E=05 PHI -4.007066E=05
GVJ 1.402720E=01 3.220891E=01 3.378695E=01 2.956157E=01
VVJ 3.740602E=02 7.792840E=02 9.762716E=02 9.420208E=02 5.206041E=02
WVJ 1.016855E=02 2.090800E=02 2.090800E=02 2.090800E=02 2.090800E=02 2.090800E=02

BEST AVAILABLE COPY

CASE A

U	0.53809E+06	-0.271381E+05	-0.380059E+06	1.196655E+05	0.053795E+03
M	1.115552E+12	2.03089E+10	-2.131628E+14	2.664555E+14	1.021045E+13
VE	7.411232E+03	THETA	1.001990E+03	BETA	3.035766E+10
UJ	1.050636E+01	3.301294E+01	3.461603E+01	3.022100E+01	
UJ	3.480000E+02	7.780810E+02	9.744351E+02	9.449369E+02	9.118269E+02
UJ	2.529005E+15	1.343999E+15	1.343999E+15	1.343999E+15	1.343999E+15
U	0.331640E+06	-0.000222E+05	-0.478251E+06	1.130099E+05	0.055196E+03
M	2.732933E+01	2.161712E+01	2.372079E+02	-0.497938E+02	-0.501560E+01
VE	0.640335E+03	THETA	1.368820E+03	BETA	-4.730022E+05
UJ	1.602720E+01	3.228091E+01	3.378605E+01	2.958137E+01	
UJ	3.740002E+02	7.782000E+02	9.742716E+02	9.429200E+02	9.206041E+02
UJ	-5.041685E+02	-2.094020E+02	-2.094020E+02	-2.094020E+02	-2.094020E+02
U	-5.720440E+06	-5.515893E+05	-5.565311E+06	9.704826E+06	7.274832E+05
M	0.359129E+01	0.232616E+01	0.568356E+02	-1.242404E+01	-0.799772E+01
VE	1.167210E+02	THETA	1.293797E+03	BETA	-1.065091E+04
UJ	1.687703E+01	2.972951E+01	3.106570E+01	2.743103E+01	
UJ	4.701628E+02	7.035086E+02	9.659000E+02	9.372169E+02	9.488967E+02
UJ	-0.875736E+02	-0.715207E+02	-0.715207E+02	-0.715207E+02	-0.715207E+02
U	-2.741615E+06	-2.599116E+05	-2.590006E+06	7.070331E+06	3.377203E+05
M	7.689132E+01	0.033805E+01	0.316390E+02	-1.722098E+01	-1.261878E+00
VE	1.546474E+02	THETA	1.033795E+03	BETA	-1.029050E+04
UJ	1.350820E+01	2.670579E+01	2.570765E+01	2.300818E+01	
UJ	0.619310E+02	7.938495E+02	9.057356E+02	9.246659E+02	6.031203E+02
UJ	-1.408212E+01	-0.533627E+02	-0.533627E+02	-0.533627E+02	-0.533627E+02
U	-1.409952E+06	-1.392023E+05	-1.370042E+06	3.725682E+06	2.877078E+05
M	0.302907E+01	7.486692E+01	7.487457E+02	-2.037555E+01	-1.551248E+00
VE	1.460302E+02	THETA	6.008115E+04	BETA	-3.249032E+04
UJ	4.427933E+02	1.080817E+01	1.083069E+01	1.077438E+01	

BEST AVAILABLE COPY

CASE A

VUJ 1.004874E+01 0.127234E+02 9.102564E+02 0.908567E+02 0.959731E+02
WUJ -P.340903E+01 -1.460907E+01 -1.460907E+01 -1.460907E+01 -1.460907E+01
YI 10)
U 2.414024E+10 2.081608E+17 1.734723E+17 1.778092E+17 0.673617E+10
W 0.009379E+01 0.097114E+01 7.902772E+02 -2.180342E+01 -1.672643E+00
VGE 1.522296E+02 THEYA 2.775598E+17 BETA -5.58590E+08 PHI -1.020000E+08
QUJ -1.810876E+15 0.628266E+15 6.774943E+15 1.373988E+14
VUJ 1.600000E+01 0.500000E+02 0.400000E+02 0.300000E+02 0.200000E+02
WUJ -0.628032E+01 -2.459063E+01 -2.459063E+01 -2.459063E+01 -2.459063E+01
FI(GPF) 0.237992E+03

BEST AVAILABLE COPY

CASE B

W	0.039160E+02	0.000124E+02	2.139297E+02	1.760417E+01	2.270349E+06		
VEE	-1.736732E+17	THETA	6.071932E+18	BETA	1.073308E+03	PHI	-2.374710E+06
QUJ	1.950093E+16	6.249341E+15	7.409649E+15	0.050073E+19			
VUJ	2.633936E+03	2.475360E+01	2.475360E+01	2.633936E+03	1.032686E+12		
WUJ	1.100566E+02	1.118112E+00	1.118112E+00	1.100566E+02	0.283868E+12		
XC	1)						
U	-0.060670E+08	-1.037444E+08	6.192286E+09	1.576393E+07	9.052286E+07		
W	-0.575090E+02	-0.319360E+02	2.019494E+02	1.667293E+01	1.084533E+04		
VEE	0.076734E+06	THETA	0.568588E+06	BETA	0.020092E+06	PHI	-2.249930E+06
QUJ	1.007802E+02	2.166616E+02	1.041753E+02	6.057493E+09			
VUJ	3.093973E+03	2.425097E+01	2.440304E+01	3.963220E+03	0.050217E+11		
WUJ	0.348722E+03	0.775096E+01	0.775096E+01	0.303722E+03	6.901293E+12		
XC	2)						
U	-1.699194E+07	-3.604349E+08	7.402908E+09	2.070803E+07	1.798017E+06		
W	-7.040036E+02	-7.007088E+02	1.700066E+02	1.995666E+01	1.997044E+06		
VEE	1.030670E+03	THETA	1.008477E+03	BETA	6.224123E+06	PHI	-2.154007E+06
QUJ	1.017692E+02	3.022123E+02	3.419029E+02	4.906304E+09			
VUJ	3.440712E+03	2.388041E+01	2.425339E+01	4.240746E+03	4.060916E+11		
WUJ	6.003909E+03	6.403661E+01	6.403661E+01	6.003909E+03	0.003467E+12		
XC	3)						
U	-2.291855E+07	-5.013303E+08	1.008740E+09	4.074015E+07	2.314292E+06		
W	-4.610792E+02	-5.091200E+02	1.223033E+02	9.007982E+02	9.303655E+09		
VEE	3.002754E+03	THETA	2.204380E+03	BETA	4.103086E+06	PHI	-2.087473E+06
QUJ	3.901874E+02	4.980914E+02	4.449581E+02	4.642071E+09			
VUJ	3.491669E+03	2.362130E+01	2.410030E+01	4.743049E+03	5.039307E+11		
WUJ	4.953222E+03	4.276129E+01	4.276129E+01	4.953222E+03	3.164049E+12		
XC	4)						
U	-2.608999E+07	-8.009099E+08	1.274269E+09	4.770908E+07	2.649240E+06		
W	-2.029282E+02	-2.011017E+02	6.387011E+03	5.197977E+02	4.694622E+09		
VEE	6.177789E+03	THETA	2.911260E+03	BETA	2.039333E+06	PHI	-2.040116E+06
QUJ	2.048824E+02	5.602749E+02	5.062231E+02	4.696814E+09			
VUJ	5.431902E+03	2.360734E+01	2.402074E+01	5.055929E+03	5.079218E+11		
WUJ	2.041009E+03	2.124299E+01	2.124299E+01	2.041009E+03	1.571000E+12		

CASE B

VI 5) U -2.416735E-07 -6.168920E-08 1.340594E-08 5.00698E-07 2.761124E-06
 W -8.610928E-14 6.110648E-13 -2.010036E-12 1.527467E-13 2.646787E-21
 VGE 8.754974E-08 TMEYA 2.613125E-05 BETA -1.807554E-19 PHI -2.038030E-06
 ORJ 2.463402E-02 5.912992E-02 5.265187E-02 4.701608E-09
 VRJ 1.807988E-03 2.341638E-01 2.399173E-01 5.152930E-03 5.093735E-11
 WJ 2.104802E-16 -1.974610E-14 -1.974610E-14 -2.104802E-16 -1.004283E-25
 VI 6) U -2.608093E-07 -5.669099E-09 1.276262E-08 4.770908E-07 2.649240E-06
 W 2.924262E-02 2.911017E-02 -6.387011E-03 -5.197577E-02 -4.676682E-09
 VGE 1.155162E-04 TMEYA 2.511260E-05 BETA -2.039323E-06 PHI -2.086116E-06
 ORJ 2.408242E-02 5.687492E-02 5.062231E-02 4.608014E-09
 VRJ 1.434390E-03 2.346746E-01 2.402078E-01 5.055292E-03 5.074218E-11
 WJ -2.861909E-03 -2.124295E-01 -2.124295E-01 -2.261949E-03 -1.571848E-12
 VI 7) U -2.291955E-07 -8.013893E-08 1.088740E-08 4.076015E-07 2.316242E-06
 W 5.619798E-02 5.991240E-02 -1.223033E-02 -9.087982E-02 -9.363655E-09
 VGE 1.371120E-06 TMEYA 2.204380E-05 BETA -6.105084E-06 PHI -2.887873E-06
 ORJ 2.501974E-02 4.886914E-02 4.809361E-02 4.442071E-09
 VRJ 1.691405E-03 2.362130E-01 2.410830E-01 4.763069E-03 5.035307E-11
 WJ -4.533222E-03 -4.276129E-01 -4.276129E-01 -4.533222E-03 -3.164098E-12
 VI 8) U -1.675194E-07 -3.664349E-08 7.942098E-09 2.979803E-07 1.758417E-06
 W 7.800836E-02 7.807086E-02 -1.700668E-02 -1.395048E-01 -1.397049E-08
 VGE 1.567528E-04 TMEYA 1.688477E-05 BETA -6.224122E-06 PHI -2.154807E-06
 ORJ 1.017585E-02 3.622123E-02 3.615829E-02 6.966304E-09
 VRJ 3.640712E-03 2.388085E-01 2.425339E-01 4.288746E-03 6.960916E-11
 WJ -6.903590E-03 -6.803461E-01 -6.803461E-01 -6.903590E-03 -4.803461E-12
 VI 9) U -4.846690E-08 -1.037944E-08 6.192286E-09 1.576393E-07 9.452246E-07
 W 9.575090E-02 9.319360E-02 -2.019094E-02 -1.667295E-01 -1.688333E-08
 VGE 1.901628E-04 TMEYA 9.368594E-06 BETA -6.424092E-06 PHI -2.748934E-06
 ORJ 1.489458E-02 3.166614E-09 1.011741E-02 4.476934E-09

BEST AVAILABLE COPY

CASE B

VUJ 1.003872E+03 2.428047E+01 2.446304E+01 3.503202E+03 0.454217E+11
WUJ -0.303222E+03 -0.775062E+01 -0.775062E+01 -0.303222E+03 -0.401293E+12
V(10)
U 1.102622E+10 -1.130412E+10 -3.794700E+10 0.336002E+19 -0.073617E+10
W 0.030100E+02 0.000324E+02 -2.139207E+02 -1.740017E+01 -2.278349E+08
VGE 1.751392E+04 THETA 2.002005E+10 BETA -1.073300E+05 PSI -2.378714E+00
QUJ 8.059074E+15 1.565516E+14 -1.774622E+14 7.267222E+17
VUJ 2.633952E+03 2.073660E+01 2.073660E+01 2.633952E+03 1.032646E+12
WUJ -1.100566E+02 -1.110112E+00 -1.110112E+00 -1.100566E+02 -0.203060E+12
PI(EP) 2.370058E+09

BEST AVAILABLE COPY

CASE C

W 02.712233E-02 -1.000120E-02 4.672257E-02 3.414436E-07 5.193776E-07
VEE 01.110223E-16 THETA 6.938002E-18 BETA 6.080009E-05 PHI -2.835767E-05
04J 2.250100E-15 -7.893570E-15 7.401114E-16 -3.442507E-16
V4J 2.464223E-02 4.731129E-01 2.424027E-03 1.229542E-10 1.229542E-10
W4J 5.871602E-02 1.136002E+00 5.822109E-03 2.452201E-10 2.452201E-10
X1 1)
U -1.266000E-08 -3.200200E-08 0.492600E-07 5.069000E-06 6.177517E-06
W -2.450000E-02 -1.000000E-02 4.400113E-02 2.820000E-07 4.280730E-07
VEE 7.000032E-06 THETA 2.720776E-05 BETA 4.814030E-05 PHI -2.401260E-05
04J 4.000322E-03 1.016000E-02 3.450210E-07 2.101525E-07
V4J 2.220501E-02 4.690270E-01 3.135300E-03 1.000231E-08 4.192016E-09
W4J 4.660220E-02 9.020066E-01 4.623020E-03 2.340372E-10 2.340372E-10
X1 2)
U -2.506236E-08 -6.143000E-08 6.406516E-07 7.277000E-06 1.101000E-05
W -2.140007E-02 -1.540003E-02 3.600506E-02 2.160397E-07 3.220400E-07
VEE 2.426302E-05 THETA 4.030070E-05 BETA 3.603336E-05 PHI -2.439267E-05
04J 1.003100E-02 1.790113E-02 3.322303E-07 2.127401E-07
V4J 2.007000E-02 4.670722E-01 3.601910E-03 1.106450E-08 4.280114E-09
W4J 3.070025E-02 6.720091E-01 3.447605E-03 1.740155E-10 1.740155E-10
X1 3)
U -5.270003E-08 -8.005600E-08 1.162775E-06 9.586555E-06 1.480027E-05
W -1.030232E-02 -1.100000E-02 2.607000E-02 1.435007E-07 2.150010E-07
VEE 5.400073E-05 THETA 6.336100E-05 BETA 2.393657E-05 PHI -2.800003E-05
04J 1.366007E-02 2.352202E-02 3.560027E-07 2.143100E-07
V4J 2.396120E-02 4.657200E-01 4.060007E-03 1.110300E-08 4.273155E-09
W4J 2.300131E-02 4.665000E-01 2.200035E-03 1.160035E-10 1.160035E-10
X1 4)
U -3.000732E-08 -9.000355E-08 1.361000E-06 1.070010E-05 1.657700E-05
W -4.010000E-03 -5.770000E-03 1.370000E-02 7.205500E-06 1.070000E-07
VEE 4.400000E-05 THETA 7.230010E-05 BETA 1.193000E-05 PHI -2.301070E-05
04J 1.550726E-02 2.603000E-02 3.993370E-07 2.155700E-07
V4J 2.900100E-02 4.640000E-01 4.290020E-03 1.125000E-06 4.290010E-06
W4J 1.101000E-02 2.227000E-01 1.101000E-03 5.700030E-11 5.700030E-11

BEST AVAILABLE COPY

CASE C

V(9)
U -0.03297E+00 -1.03310E-07 1.42000E+06 1.14037E+03 1.72750E-05
W 2.25922E+12 -1.56310E-13 -3.03757E+13 -2.42251E-19 -2.40027E-19
VEE 1.856001E+04 THETA 7.552937E-05 BETA -2.374000E-1A PHI -2.305760E-05
QVJ 1.62002E+02 2.793761E+02 3.60210E+07 2.159311E-07
VUJ 2.30637E+02 4.64342E+01 4.376731E+03 1.12705E+08 4.29436E+09
WUJ -2.480031E-18 -4.600291E-14 -2.40232E+16 -1.24055E-23 -1.24055E-23
V(A)
U -0.00073E+00 -0.00035E+00 1.36149E+06 1.007861E+05 1.65770E-05
W 0.011000E+03 3.772303E-03 -1.37855E-02 -7.20556E+08 -1.07406E-07
VEE 1.62075E+04 THETA 7.230610E-05 BETA -1.103066E-05 PHI -2.391678E-05
QVJ 1.55972E+02 2.60395E+02 3.593370E+07 2.19570E-07
VUJ 2.30610E+02 4.64000E+01 4.29942E+03 1.12549E+08 4.292010E+09
WUJ -1.15170E+02 -2.22746E+01 -1.16165E+03 -5.788013E+11 -5.788013E+11
V(7)
U -3.270003E+00 -0.00560E+00 1.16277E+06 9.86655E+06 1.44027E-05
W 1.43022E+02 1.10800E+02 -2.64704E+02 -1.435907E+07 -2.154610E-07
VEE 1.07157E+04 THETA 4.33810E-05 BETA -2.33363E-05 PHI -2.400073E-05
QVJ 1.16607E+02 2.33220E+02 3.56602E+07 2.14518E-07
VUJ 2.30612E+02 4.63729E+01 4.04900E+03 1.11830E+08 4.27313E+09
WUJ -2.309131E+02 -4.65596E+01 -2.26893E+03 -1.16063E+10 -1.16063E+10
V(A)
U -2.35623E+00 -0.14340E+00 0.406516E+07 7.27780E+06 1.10109E-05
W 2.14000E+02 1.94800E+02 -3.64650E+02 -2.14039E-07 -3.22304E-07
VEE 2.25552E+04 THETA 4.03807E-05 BETA -3.60533E-05 PHI -2.43020E-05
QVJ 1.00310E+02 1.79911E+02 3.52230E+07 2.12740E-07
VUJ 2.00707E+02 4.67472E+01 3.68191E+03 1.10649E+08 4.24011E+09
WUJ -3.67802E+02 -4.72609E+01 -3.44760E+03 -1.74615E+10 -1.74615E+10
V(0)
U -1.26600E+00 -3.24920E+00 0.40280E+07 4.06000E+06 6.17731E-06
W 2.55000E+02 1.94800E+02 -6.40113E+02 -2.82800E-07 -4.28073E-07
VEE 2.44500E+04 THETA 2.72977E-05 BETA -4.63495E-05 PHI -2.40124E-05
QVJ 9.08000E+03 1.01000E+02 3.45021E+07 8.10193E-07

BEST AVAILABLE COPY

CASE C

VHJ 2.22851E+02 4.69274E+01 3.13330E+03 1.08923E+08 6.19201E+09
MHJ -8.66822E+02 -9.02088E+01 -8.62342E+03 -2.36837E+10 -2.36837E+10
H(10)
H 1.12757E+17 -8.33098E+18 -2.00168E+17 1.04034E+17 -2.00168E+17
M 2.71222E+02 1.98012E+02 -8.67229E+02 -3.81403E+07 -3.19377E+07
VGE 2.91618E+08 THETA -6.79389E-18 BETA -6.08868E-09 PHJ -2.93976E-03
QHJ 3.82837E+15 5.78296E+15 -8.29645E+16 .91922E+16
VHJ 2.66823E+02 8.73112E+01 2.62882E+03 1.22952E+10 1.22952E+10
MHJ -9.07368E+02 -1.13602E+00 -5.82230E+03 -2.93281E+10 -2.93281E+10
PI(EP) 2.06951E+08

BEST AVAILABLE COPY

CASE D

M	-1.82032E=01	-2.801195E=01	1.959910E=01	2.561508E=01	1.185610E=07
VE	0.	THETA	1.730723E=18	BETA	9.835109E=05 PHI
NUJ	-1.683736E=19	2.206009E=18	-2.493098E=14	1.830133E=16	-6.299134E=05
VUJ	5.200295E=02	2.230991E=01	2.230991E=01	1.316903E=03	5.200290E=12
WUJ	8.189924E=02	3.403268E=01	3.403268E=01	2.056190E=03	8.195920E=12
XI	1)				
U	-8.015285E=08	-7.717303E=07	5.607600E=07	3.171340E=06	5.076000E=06
M	-1.710210E=01	-2.510100E=01	1.803379E=01	2.300781E=01	9.950902E=00
VE	1.106193E=09	THETA	4.192601E=05	BETA	7.030342E=05 PHI
NUJ	4.170633E=02	1.040062E=01	6.047740E=02	6.246814E=08	-6.020230E=05
VUJ	4.309131E=02	2.109066E=01	2.153793E=01	3.407757E=03	2.165400E=10
WUJ	5.073205E=02	2.096906E=01	2.096906E=01	1.473992E=03	5.073205E=12
XI	2)				
U	-1.480708E=07	-1.433210E=06	1.065824E=06	5.967510E=06	9.897617E=06
M	-1.422623E=01	-2.080045E=01	1.536823E=01	1.400804E=01	7.600079E=09
VE	3.006793E=09	THETA	7.106518E=05	BETA	4.644449E=05 PHI
NUJ	7.144010E=02	1.773002E=01	1.010600E=01	4.759021E=08	-5.065200E=05
VUJ	3.661941E=02	2.026908E=01	2.100972E=01	4.071600E=03	2.612550E=10
WUJ	8.037041E=02	1.715702E=01	1.715702E=01	1.012813E=03	8.037041E=12
XI	3)				
U	-2.293502E=07	-1.000459E=06	1.455624E=06	8.126940E=06	1.313262E=05
M	-1.012270E=01	-1.478005E=01	1.095650E=01	1.390710E=01	5.220390E=08
VE	4.006713E=05	THETA	9.102166E=05	BETA	3.030013E=05 PHI
NUJ	9.110091E=02	2.251700E=01	1.260390E=01	5.073719E=08	-3.341001E=05
VUJ	3.200022E=02	1.974534E=01	2.063650E=01	5.772700E=03	2.560700E=10
WUJ	2.329010E=02	1.073129E=01	1.073129E=01	6.338700E=04	2.629010E=12
XI	4)				
U	-2.680133E=07	-2.312163E=06	1.701930E=06	9.463536E=06	1.510219E=05
M	-9.267565E=02	-7.605033E=02	5.607070E=02	7.205925E=02	2.639740E=06
VE	1.200015E=06	THETA	1.021600E=08	BETA	1.457310E=05 PHI
NUJ	1.020033E=01	2.320907E=01	1.410700E=01	5.203000E=08	-2.000070E=05
VUJ	3.000022E=02	1.905513E=01	2.001973E=01	6.200307E=03	2.600761E=10
WUJ	1.210000E=02	9.101000E=02	5.101000E=02	3.000000E=04	1.210000E=12

BEST AVAILABLE COPY

CASE D

U	9.812197E-07	-2.422208E-06	1.766070E-06	9.995910E-06	1.575329E-05
M	9.769963E-13	-1.402140E-13	-5.322071E-14	-2.486900E-14	-5.509714E-21
VPE	1.817831E-08	TMYA	1.057867E-08	BETA	1.626303E-10
QUJ	1.061740E-01	2.607472E-01	1.456284E-01	3.297292E-08	
VUJ	2.428337E-02	1.936227E-01	2.034860E-01	6.426612E-03	2.672621E-10
WUJ	1.359233E-16	5.759824E-16	5.759824E-16	3.400058E-16	1.552253E-26
YI	4)				
U	-9.880133E-07	-2.312163E-06	1.701930E-06	9.483336E-06	1.510219E-05
M	5.287568E-02	7.649633E-02	-5.687070E-02	-7.205929E-02	-2.639740E-08
VPE	2.340747E-08	TMYA	1.021669E-08	BETA	-1.497338E-05
QUJ	1.024933E-01	2.420597E-01	1.410744E-01	5.243680E-08	
VUJ	3.068302E-02	1.495513E-01	2.041973E-01	6.268387E-03	2.664761E-10
WUJ	-1.214499E-02	-9.161406E-02	-9.161406E-02	-3.046808E-04	-1.214499E-12
YI	5)				
U	-2.293932E-07	-1.080599E-06	1.459624E-06	8.126405E-06	1.313262E-05
M	1.012274E-01	1.478089E-01	-1.095650E-01	-1.394710E-01	-5.224394E-08
VPE	2.826491E-08	TMYA	9.102166E-05	BETA	-3.030015E-05
QUJ	9.110491E-02	2.251784E-01	1.280394E-01	5.073719E-08	
VUJ	3.240422E-02	1.976554E-01	2.063656E-01	5.772270E-03	2.568704E-10
WUJ	-9.525010E-02	-1.073124E-01	-1.073124E-01	-6.358746E-04	-2.525010E-12
YI	6)				
U	-1.488708E-07	-1.453210E-06	1.069324E-06	5.967910E-06	9.897617E-06
M	1.422622E-01	2.084049E-01	-1.538823E-01	-1.069844E-01	-7.689497E-08
VPE	3.236803E-08	TMYA	7.146514E-05	BETA	-8.866449E-05
QUJ	7.144810E-02	1.779842E-01	1.010668E-01	4.799021E-08	
VUJ	3.661391E-02	2.026906E-01	2.100972E-01	6.871666E-03	2.412954E-10
WUJ	-4.037041E-02	-1.715742E-01	-1.715742E-01	-1.012613E-03	-4.037041E-12
YI	7)				
U	-4.913293E-08	-7.717303E-07	5.667469E-07	3.171349E-06	5.874899E-06
M	1.710214E-01	2.914144E-01	-1.843379E-01	-2.380781E-01	-9.050492E-08
VPE	3.989832E-08	TMYA	4.192681E-05	BETA	-7.050342E-05
QUJ	6.178633E-02	1.048849E-01	6.047744E-02	4.440810E-08	

BEST AVAILABLE COPY

CASE D

VTJ 0.509131E+02 2.109066E+01 2.159793E+01 3.407797E+03 2.165000E+10
WTJ -9.079899E+02 -2.406996E+01 -2.406996E+01 -1.473992E+03 -5.079285E+12
XC 10)
U 2.001556E+10 -2.100000E+10 -0.743309E+10 2.002069E+10 -0.073617E+10
W 1.020321E+01 2.001199E+01 -1.000102E+01 -2.361998E+01 -1.109010E+07
VGE 3.639002E+00 THETA 0. BETA -9.035100E+09 PHI -6.200150E+03
QVJ 0.063030E+15 1.132997E+10 -3.232007E+10 3.009007E+10
VTJ 5.200299E+02 2.230991E+01 2.230991E+01 1.316093E+03 5.200297E+12
WTJ -0.109020E+02 -3.003200E+01 -3.003200E+01 -2.056100E+03 -0.109020E+12
P3(EAP) 1.432300E+00

BEST AVAILABLE COPY

TEST BEAM

2.240313E+01
 1.020446E+00

VI 0) 1.461251E+17 4.934889E+16 -9.673617E+19 4.336809E+14
 " 0. 1.008949E+05 -1.230382E+00 1.230393E+00 1.008949E+06 2.073345E+06
 W 0.274453E+13 THERA 1.387779E+17 BETA 1.002049E+08 PMI -2.466210E+09
 VPE 0.470272E+15 1.050911E+14 2.491247E+16 -1.063646E+16
 VUJ 2.472068E+09 6.974508E+01 2.580546E+03 1.292791E+10 1.292791E+10
 VUJ 1.411213E+08 3.501644E+00 1.415106E+02 9.204200E+10 9.204200E+10

ABSOLUTE TON SMALL
 PDB# NUMBER 115 DETECTED BY F10

VI 1) 0.231846E+08 -7.083089E+06 4.111853E+05 7.002086E+05 1.027233E+04
 " 0. 1.172240E+05 -1.190237E+00 1.190245E+00 1.633356E+06 2.075048E+06
 W 2.431997E+08 THERA 2.007155E+06 BETA 1.440908E+08 PMI -2.398277E+09
 VPE 1.440932E+06 4.200017E+02 4.031249E+07 2.374533E+07
 VUJ 1.411422E+07 4.817400E+01 4.022622E+03 1.348619E+08 5.104068E+08
 VUJ 1.409316E+08 2.725008E+00 1.390904E+02 7.083624E+10 7.083624E+10

ABSOLUTE TON SMALL
 PDB# NUMBER 115 DETECTED BY F10

VI 2) 0.448801E+08 -1.336303E+05 7.751077E+05 1.459877E+04 1.087440E+04
 " 0. 0.448801E+08 -0.435277E+01 0.435277E+01 1.200970E+06 1.404911E+06
 W 0.470819E+08 THERA 5.002649E+08 BETA 1.068061E+08 PMI -2.196338E+09
 VPE 1.750372E+06 1.300094E+01 5.051053E+07 2.855281E+07
 VUJ 0.401122E+08 4.706164E+01 1.250235E+02 1.502501E+08 5.634508E+08
 VUJ 1.024064E+08 1.004241E+00 1.017508E+02 5.159614E+10 5.159614E+10

VI 3) 0.401170E+08 -1.023122E+04 1.057418E+04 1.037402E+04 2.450473E+04
 " 0. 0.401170E+08 -0.405004E+01 0.405004E+01 0.405004E+07 1.087747E+06
 W 1.002033E+03 THERA 4.115074E+08 BETA 6.990170E+04 PMI -2.067418E+09
 VPE 1.002033E+03 1.002033E+03 1.002033E+03 1.002033E+03 1.002033E+03

BEST AVAILABLE COPY

TEST BEAM

VUJ	4.63508E+08	6.63284E+01	1.93368E+02	1.637261E+08	4.00207E+09		
VUJ	4.976782E+09	1.29871E+00	6.686100E+03	3.369993E+10	3.369993E+10		
Vr	a)						
"	-2.78899E+06	-2.12008E+09	1.239400E+08	2.235537E+08	2.017742E+08		
"	-2.78899E+06	-3.40659E+01	3.50077E+01	4.48004E+07	5.09008E+07		
VPE	3.22642E+03	THEYA	7.514820E+08	MEYA	3.033372E+09	PHI	-1.00872E+05
VUJ	1.419510E+08	1.00872E+01	5.694215E+07	3.140610E+07			
VUJ	4.67882E+08	4.59114E+01	1.68984E+02	1.722582E+08	6.103277E+08		
VUJ	3.12131E+09	6.60584E+01	3.203162E+03	1.668772E+10	1.668772E+10		
Vr	a)						
"	-4.00821E+08	-2.23807E+09	1.20612E+08	2.335831E+08	2.00111E+08		
"	6.68979E+20	-2.90381E+13	-6.75015E+18	-5.75982E+20	-6.47397E+20		
VPE	6.40118E+03	THEYA	7.514820E+08	BEYA	-1.408063E+18	PHI	-1.071003E+05
VUJ	1.40075E+08	2.00802E+01	5.70925E+07	3.10090E+07			
VUJ	4.60881E+08	6.57761E+01	1.740533E+02	1.743780E+08	6.25852E+08		
VUJ	-1.94061E+22	-2.02071E+18	-1.58770E+16	-6.03410E+24	-6.83810E+24		
Vr	a)						
"	-4.94822E+08	-2.12008E+09	1.23580E+08	2.235537E+08	2.017742E+08		
"	2.78899E+06	3.40659E+01	-3.50077E+01	-4.48004E+07	-5.09008E+07		
VPE	9.02014E+03	THEYA	7.514820E+08	BEYA	-3.033372E+09	PHI	-1.00872E+05
VUJ	1.419510E+08	1.00872E+01	5.694215E+07	3.140610E+07			
VUJ	4.67882E+08	4.59114E+01	1.68984E+02	1.722582E+08	6.103277E+08		
VUJ	-3.12131E+09	-6.60584E+01	-3.203162E+03	-1.668772E+10	-1.668772E+10		
Vr	a)						
"	-4.00817E+08	-1.42312E+09	1.057810E+08	1.037092E+08	2.050473E+08		
"	4.67882E+08	6.60584E+01	-6.05102E+01	-6.790468E+07	-1.08774E+06		
VPE	7.137502E+03	THEYA	6.761587E+08	MEYA	-6.090170E+09	PHI	-2.06718E+05
VUJ	1.40889E+08	1.78121E+01	5.46021E+07	3.04093E+07			
VUJ	4.05884E+08	4.63284E+01	1.93368E+02	1.637261E+08	4.00207E+09		
VUJ	-4.70872E+09	-1.29871E+00	-6.686100E+03	-3.369993E+10	-3.369993E+10		
Vr	a)						
"	-4.00881E+08	-1.42312E+09	7.78107E+09	1.458477E+08	1.40748E+08		

BEST AVAILABLE COPY

TEST BEAM

U -1.00001E+00 -1.13633E+03 7.75107E+03 1.43307E+00 1.00700E+00
W 0.49187E+06 0.43502E+01 0.01500E+01 01.20007E+00 01.00001E+06
VPE 0.10200E+03 1.4E+00 0.00000E+00 0.00000E+00 0.00000E+00
VUJ 1.70000E+00 1.30000E+01 0.01000E+07 2.00000E+07
VUJ 0.00100E+00 0.70000E+01 1.20000E+02 1.00000E+00 0.00000E+00
VUJ -1.00000E+00 01.00000E+00 -1.01000E+02 0.15000E+10 0.15000E+10
VF 0)
U -2.00000E+00 -7.00000E+06 0.11000E+05 7.00000E+05 1.00000E+00
W 1.10000E+05 1.10000E+00 -1.10000E+00 -1.00000E+00 -2.00000E+00
VPE 0.00000E+03 1.4E+00 2.00000E+00 0.00000E+00 0.00000E+00
VUJ 1.00000E+00 0.20000E+02 0.01000E+07 2.00000E+07
VUJ 1.01000E+07 0.01000E+01 0.00000E+03 1.00000E+00 0.00000E+00
VUJ -1.00000E+00 -2.00000E+00 -1.00000E+02 -7.00000E+10 -7.00000E+10
VF 10)
U 1.00000E+10 1.00000E+10 -1.00000E+17 -3.00000E+10 -9.00000E+10
W 1.00000E+05 1.00000E+00 -1.00000E+00 -1.00000E+00 -2.00000E+00
VPE 0.10000E+03 1.4E+00 0.00000E+10 0.00000E+00 0.00000E+00
VUJ 0.00000E+10 0.00000E+15 -2.10000E+10 1.00000E+10
VUJ 0.00000E+00 0.00000E+01 2.00000E+03 1.00000E+10 1.00000E+10
VUJ 01.00000E+00 -1.00000E+00 -1.00000E+02 -9.00000E+10 -9.00000E+10
VF 100) 2.00000E+00

REFERENCES

1. Rhodes, G. F. and Denke, P. H., "Damping, Static, Dynamic and Impact Characteristics of Laminated Beams Typical of Windshield Construction," (Douglas Aircraft Company) Report AFFDL-TR-76-156 dated November 1976.
2. Allen, H. G., "Analysis and Design of Structural Sandwich Panels," Pergamon Press, Oxford, 1969.
3. "Advanced Composites Design Guide; Volume II, Analysis," Air Force Materials Laboratory, Wright-Patterson Air Force Base, Ohio, 1973.
4. Plantema, F. J., "Sandwich Construction," John Wiley and Sons, New York, 1966.
5. Kaplan, W., "Ordinary Differential Equations," Addison-Wesley Publishing Company; Reading, Massachusetts, 1967.
6. Denke, P. H., Eide, G. R., and Morris, R. C., "A Mathematical Model for Aircraft Windshield System Bird Impact Analysis," Volumes I, II and III, (Douglas Aircraft Company) Report AFFDL-TR-(TBD).
7. Wilkinson, J. H., Reinsch, C., "Handbook for Automatic Computation," Volume 2 - Linear Algebra, Springer-Verlag, Germany, 1971.
8. Smith, B. T., et al, "Matrix Eigensystem Routines - EISPACK Guide," Second Edition, Springer-Verlag, Berlin-Heidelberg, New York, 1976.
9. "Computer Aided Structural Design Users Manual," Report Number DAC 33447, Douglas Aircraft Company, June 1970.