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of the wavenumber determine the phase velocity and the attenuation of the layered medium, respectively. Dispersion and the attenuation curves can be obtained by using different periods. The above wavenumbers can be used in the inhomogeneous differential equation; this equation contains the effect of the vertical boundary; solution of this equation determines the displacement at the vertical boundary. The displacements at the left and right sides of the boundary may be used in the energy equation to determine the reflected and the transmitted energy, respectively. The difference between the incident and transmitted energy determines the difference between the wavenumbers at the left and right hand sides of the vertical boundary. Addition of these differences to the wavenumbers of the left hand boundary would result in the formation of the dispersion curve at the right hand boundary.

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ENERGY DISSIPATION OF RAYLEIGH WAVES DUE TO ABSORPTION
ALONG THE PATH BY THE USE OF FINITE ELEMENT METHOD

by

Ali A. Feizpour

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31 July 1979

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ABSTRACT

A normally incident Rayleigh wave may be used for the investigation of a general vertical boundary and the attenuation of a viscoelastic medium. Use of energy conservation and proper boundary conditions produce $2N$ second order differential equations, N being the number of viscoelastic layers in the medium. The homogeneous part of the differential equations can be transformed into an eigenvalue problem by the use of finite element technique; the eigenvalue and eigenvectors of the eigenvalue problem are the wavenumbers and the displacement amplitudes of the viscoelastic layered medium. The real and imaginary parts of the wavenumber determine the phase velocity and the attenuation of the layered medium, respectively. Dispersion and the attenuation curves can be obtained by using different periods. The above wavenumbers can be used in the inhomogeneous differential equation; this equation contains the effect of the vertical boundary; solution of this equation determines the displacement at the vertical boundary. The displacements at the left and right sides of the boundary may be used in the energy equation to determine the reflected and the transmitted energy, respectively. The difference between

the incident and transmitted energy determines the difference between the wavenumbers at the left and right hand sides of the vertical boundary. Addition of these differences to the wavenumbers of the left hand boundary would result in the formation of the dispersion curve at the right hand boundary.

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PREFACE

The attenuation of a surface wave along its path is of interest in many branches of physical science; among the noted are solid mechanics, acoustics, and seismology. The aim of this dissertation is to present a method of Rayleigh wave propagation in a viscoelastic solid constrained by one or two vertical boundaries. The material is arranged to present a summary of viscoelastic behavior of solid, an introduction to the method of finite element for the derivation and solution of governing equations, a discussion on energy conservation in elastic and viscoelastic solids, and presentation of a perturbation theory for the calculation of the phase velocity due to a vertical boundary, followed by a chapter on application of data to the method.

I hope that this dissertation will be a stimulant for interested investigators, to be critical of the material herein, and to be encouraged enough to further test the validity of the theory by using other distinct models. As a reference, the text is supplemented by a relatively vast number of references, which provide a great deal of information for understanding the subject.

I would like to thank Professor E. Herrin for his advice through the course of my research and for his critical opinion of this dissertation. I would also like to thank Doctors B. Mohraz, D. Blackwell, T. Goforth, H. Mack, and W. Peeples, for their criticisms and encouragements.

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LIST OF SYMBOLS

CHAPTER II

σ	Stress in one dimension
σ_{ij}	Stress tensor
σ_v	Two dimensional stress vector normal to a line whose direction cosine is v
ϵ	Strain in one dimension
ϵ_{ij}	Strain tensor
η_v	Coefficient of viscosity
μ	Shear rigidity
τ_σ	Stress relaxation under constant strain
τ_ϵ	Strain relaxation under constant stress
ψ	Energy density
λ	Young modulus
K	Bulk modulus
K_e	Effective bulk modulus
μ_e	Effective rigidity
$\underline{D}$	Matrix moduli 3×3
$\hat{e}$	Strain vector
$\sigma_{ij,j}$	Space derivative of the stress tensor
F_i	Force vector per unit volume
ρ	Density

CHAPTER III

F	Total force
$\underline{K}$	Stiffness matrix
$\hat{U}$	Vector displacements
$\underline{M}$	Mass matrix
$\underline{B}$	Space operating matrix
$\underline{D'}$	Matrix moduli 2 x 3
DZ	Operating matrix in Z direction
$\underline{N}$	Matrix of shape function
$\hat{v}$	Displacement vector
n	Local coordinate of the layer
α	A multiplier
$\underline{V}$	Matrix with columns v
$\hat{P}$	Force at the boundary
$\underline{L}$	Boundary matrix
$\underline{K}_e$	Element stiffness matrix

CHAPTER IV

E	Incident energy
K	Wavenumber
β	Absorption in the x direction
γ	Absorption in the z direction
$\underline{D}_v$	Matrix moduli 2 x 3
E_r	Reflected energy
E_t	Transmitted energy

Q^{-1}	Attenuation
ΔD	Change in matrix moduli
$\Delta \rho$	Change in densities

CHAPTER V

V_p	P wave velocity
V_s	S wave velocity
Q_β	Shear quality factor
Q_α	Compressional quality factor

CHAPTER I

INTRODUCTION

Rayleigh wave amplitude decays as it propagates through the earth. The amount of the decay observed is invariably more than what the theory predicts. This anomaly is due to the loss mechanisms or the energy dissipation in the medium; thus the observed Rayleigh wave amplitude is a function of energy dissipation or attenuation.

Attenuation of Rayleigh wave is due to anelasticity and geometrical discontinuities of the earth. Anelasticity of the earth may be thought of as the imperfection in crystal structure: such as point defects, dislocation across which the atoms are not aligned in accordance with the normal lattice structure, and interatomic bonds at grain boundaries. Geometrical discontinuities, among others (23, 24), cause the distortion of the plane of propagation of Rayleigh waves from the great circle and therefore there is a loss of energy or attenuation.

In a dissipating medium the deformation of a solid is a function of time, temperature and space. Creep phenomenon is a deformation process in which there is a time dependent strain under a constant applied stress and relaxation phenomenon is a time dependent stress under a constant strain. A solid manifesting creep and relaxation phenomena is called viscoelastic. If a deformation is due to discontinuities in the medium or the amplitude decay is only a function of position, the dissipation

of the wave energy is of the hysteretic type (Figure 1), and the amount of this loss must be determined by the average energy dissipation in a hysteresis loop over one cycle of harmonic motion and is a function of displacement amplitude.

To narrow the gap between the observed and predicted Rayleigh wave amplitude the effect of viscoelasticity and geometrical discontinuities must be taken into account. A few workers have studied the viscoelastic behavior of Rayleigh waves for an isotropic homogeneous layered half-space. Press and Healy (94) derived an expression for the absorption coefficient and a dispersion relation assuming complex velocities for shear, longitudinal and Rayleigh waves respectively. King and Sheard (56) used a dissipation function for a general anisotropic medium and derived a dispersion relation for an isotropic layered half-space equivalent to that of Press and Healy (94). Borchardt (15) also formulated a similar theoretical expression for a layered half-space. Auld (8) determined the attenuation of Rayleigh waves due to viscous damping in the medium by using King and Shread's dissipation function (56). He concluded that the Rayleigh wave attenuation is mostly due to shear viscosity of the medium. For observational studies of the dissipation of Rayleigh waves the reader is referred to Anderson, et al (5) and Tryggvason (115).

The effect of geometrical discontinuities on the propagation of Rayleigh waves have been a problem of seismologists for many years. Several studies have been devoted to the description of the field around wedges and corners using approximate analytical techniques: e.g., Lapwood (66), Mal and Knopoff (82), McGarr (85) and Alsop et al (2). Several theoretical studies have also been done in more recent years

with, in most part, the assumption of weak horizontal inhomogeneity on assumption of a small change in elastic parameters. Among the more important are, Babish et al. (9), Kennett (55) and Mukitina et al. (88).

The observed distorted Rayleigh wave through the continental margins or the oceanic ridges has been studied by Capon (19, 20) and by Evernden (31, 32, 33) and the amount of distortion has been approximated analytically by Gjevik (39) who has used ray tracing theory and perturbation theory. The distortion of Rayleigh waves from the great circle has also been studied by Crampin (23, 24) who has stated that the distortion is due to a layer of anisotropic olivine in the upper mantle.

The finite element method has been used, in recent years, in wave propagation problems of seismology. Lysmer (78) and Lysmer and Waas (79) developed a lumped mass method to study the propagation of Rayleigh and shear waves in a homogeneous layered medium. Lysmer and Drake (80) introduced the method of consistent mass matrix of the finite element method to seismology. Drake (28, 29) studied the propagation of Love and Rayleigh waves in a nonhorizontally layered medium. Drake (26) also studied the motion of Rayleigh waves at a continental boundary. Waas (117) improved the method of finite element to a general two dimensional problem and Kausel et al. (54) extended the method to a three dimensional problem in which Rayleigh wave corresponds to a vertical cross section and Love wave corresponds to a horizontal cross section of the medium. Smith (107) applied the method

of finite element to body wave propagation. Finally Segol et al. (105) studied the propagation of a Rayleigh wave being obstructed by trenches.

In this dissertation the finite element method is used to study the attenuation of Rayleigh waves due to viscoelasticity i.e., viscosity, temperature, partial melting, etc., and determine a dissipative factor due to a vertical boundary. The model consists of a heterogeneous medium bounded by two vertical boundaries (Figure 4). These two boundaries are the common boundaries between the heterogeneous medium and two semiinfinite horizontal layered media extending to left and right. For the two viscoelastic layered media a second order differential equation similar to that of a damping oscillator is obtained in which the damping factor is a matrix whose entries are the viscoelastic parameters of the medium. The solution of this equation determines the wavenumbers and the amount of attenuation of Rayleigh waves in the horizontal direction. Use of the boundary conditions at the vertical boundaries results in the equation of motion of the heterogeneous medium whose solution yields the horizontal and vertical displacement of the heterogeneous medium. These displacements and the wavenumbers of the layered regions are used in the energy conservation theorem along with a perturbation theory to determine the amount of absorption due to viscoelasticity of the region and the change in the wavenumbers due to vertical boundaries. The adjusted wavenumbers of the right hand boundary may be used to determine the dispersion curve and the amount of attenuation at the right hand boundary.

CHAPTER II

VISCOELASTICITY

A. Summary

The phenomenon of viscoelasticity is a broad subject in the study of materials in the gaseous, liquid, and solid states. Interested readers are referred to the selected texts on the subject by Bland (13), Christensen (22), Gross (44), Kol'sky (60, 61), Litvitz and Davis (72), Mathson (84) and Zener (122) for theoretical studies, and Anderson (3, 4), Anderson et al. (5, 6), Gordon (41, 42, 43), Hart et al. (45), Jackson and Anderson (50), Liu et al. (74), Magnitsky and Zharkov (81) and Orowan (90) for seismological interest. A brief review for the problem at hand will be presented here.

B. Uniaxial Deformation

Viscoelastic materials combine elastic and viscous effects; therefore, it is possible to model the material of the viscoelastic earth by networks made of simple elastic and viscous elements. The simplest model is a spring and a dashpot in series or parallel and is called a Maxwell or Voigt element, respectively. Mathematically the stress-strain relation for the model is

$$\frac{\sigma}{\eta_v} + \frac{\dot{\sigma}}{\mu} = \dot{\epsilon} \quad (2.1)$$

for the Maxwell element, and

$$\mu \epsilon + \eta_v \dot{\epsilon} = \sigma \quad (2.2)$$

for the Voigt element, where σ is the stress, ϵ is the strain, $\dot{\epsilon}$ the rate of the strain, μ is the elastic modulus of the spring and η is the coefficient of viscosity of the dashpot (Figure 2). A combination of a spring with elastic modulus μ_1 either in series with a Voigt element or in parallel with a Maxwell element will produce a relation

$$\dot{\sigma} \tau_\sigma + \sigma = M_r (\epsilon + \tau_\epsilon \dot{\epsilon}) \quad (2.3)$$

called the standard linear solid, where

$$\tau_\sigma = \frac{\eta_v}{\mu + \mu_1}, \quad \tau_\epsilon = \frac{\eta_v}{\mu}, \quad M_r = \frac{\mu_1 \mu}{\mu_1 + \mu} \quad (2.4)$$

are the stress relaxation time under constant strain, the strain relaxation under constant stress, and the deformation or relaxed elastic modulus, respectively.

The solution of Equation 2.1 for a constant strain results in a decaying exponential function of the form

$$\sigma(t) = \sigma_0 \text{EXP} (- \mu / \eta_v t) \quad (2.5)$$

which is called the relaxation function. Similarly the solution of Equation 2.2 for a constant stress results in an increasing exponential function of the form

$$\epsilon(t) = \frac{\sigma_0}{\mu} (1 - e^{- \mu / \eta_v t}) \quad (2.6)$$

which is called a creep function (Figure 3). The solution of Equation 2.3

yields the relaxation and creep functions of the form

$$\sigma(t) = M_r \epsilon(t) + M_r \frac{\tau_\sigma}{\tau_\sigma} \text{EXP}(-t/\tau_\sigma) \int_{-\infty}^t \text{EXP}(\tau/\tau_\sigma) \dot{\epsilon}(\tau) d\tau$$

and (2.7)

$$\epsilon(t) = \frac{1}{M_r} \sigma(t) + \frac{1}{M_r} \frac{\tau_\sigma}{\tau_\sigma} \text{EXP}(-t/\tau_\sigma) \int_{-\infty}^t \text{EXP}(\tau/\tau_\sigma) \dot{\sigma}(\tau) d\tau$$

Equations 2.7 represent the Boltzmann after-effect. The verbal statement of these equations by Boltzmann is that for all real solids the relation between stress or strain components at any time depends not only on their instantaneous values at that time, but on the whole history of deformation from the time at which the material was formed. It is now possible to represent the viscoelastic behavior of the real material by either a large number of Maxwell elements and springs joined in parallel or a large number of Voigt elements and springs joined in series. The arrangement of the springs represent the molecular position of material and the dashpots represent the perturbation factor for the distortion of molecular positions. Thus the generalized relaxation and creep function for uniaxial deformation is given by

$$\sigma(t) = M_r \epsilon(t) + M_r \sum_{i=1}^N \frac{\tau_{\sigma_i}}{\tau_{\sigma_i}} \text{EXP}(-t/\tau_{\sigma_i}) \int_{-\infty}^t \text{EXP}(\tau/\tau_{\sigma_i}) \dot{\epsilon}(\tau) d\tau$$

and (2.8)

$$\epsilon(t) = \frac{1}{M_r} \sigma(t) + \frac{1}{M_r} \sum_{i=1}^N \frac{\tau_{\sigma_i}}{\tau_{\sigma_i}} \text{EXP}(-t/\tau_{\sigma_i}) \int_{-\infty}^t \text{EXP}(\tau/\tau_{\sigma_i}) \dot{\sigma}(\tau) d\tau$$

Equations 2.8 represent the Boltzmann superposition principle.

C. Three Dimensional Deformation

For a general linear viscoelastic model consider the energy density (67),

$$\psi = 1/2 C_{ijkl} \epsilon_{ij} \epsilon_{kl} \quad (2.9)$$

where ϵ 's are strains and C_{ijkl} , generally complex, are the moduli of the medium. For an anisotropic medium C_{ijkl} contain 21 independent complex coefficients. For an isotropic medium these coefficients reduce to two elastic constants in an elastic medium, whereas, in the viscoelastic medium the two coefficients are not constant and may be a function of time, temperature, position or other thermodynamic parameters. Therefore, in an isotropic viscoelastic medium Equation 2.9 can be written as

$$\begin{aligned} \psi = 1/2 \lambda \delta_{ij} \epsilon_{ij}^2 + \mu \epsilon_{ij}^2 + 1/2 \lambda'(t, T, \bar{x}, \gamma) \delta_{ij} \epsilon_{ij}^2 \\ + \mu'(t, T, \bar{x}, \gamma) \epsilon_{ij}^2 \end{aligned} \quad (2.10)$$

where λ and μ are Lamé constants, λ' and μ' the coefficients which depend on the viscoelastic behavior of the medium. Since Equation 2.10 is a scalar function, the stress field can be obtained by a directional derivative with respect to the strain tensor as

$$\sigma_{ij} = \lambda \delta_{ij} \epsilon_{ii} + 2 \mu \epsilon_{ij} + \lambda'(t, T, \bar{x}, \gamma) \delta_{ij} \epsilon_{ii} + 2 \mu'(t, T, \bar{x}, \gamma) \epsilon_{ij} \quad (2.11)$$

The first two terms in Equation 2.11 represent the familiar form of the stress-strain relation in an isotropic homogeneous medium and the last two terms are due to the viscoelasticity of the medium.

D. Effective Moduli for a Relaxing Medium

For functionals λ' and μ' , the effective moduli of the stressed medium can be obtained by either substitution of Equation 2.11 into Equation 2.7 and integrating it through the limits of integration, or by formation of a relation similar to Equation 2.3, using Equation 2.11; the latter is a more convenient way to choose. For this purpose we may write Equation 2.11 in terms of shear and dilatational components. Using the strain tensor

$$\epsilon_{ij} = (\epsilon_{ij} - 1/3 \delta_{ij} \epsilon_{ii}) + 1/3 \delta_{ij} \epsilon_{ii} , \quad (2.12)$$

Equation 2.11 becomes

$$\begin{aligned} \sigma_{ij} = & 2\mu(\epsilon_{ij} - 1/3 \delta_{ij} \epsilon_{ii}) + K\epsilon_{ij}\delta_{ij} + 2\mu'(\dot{\epsilon}_{ij} - 1/3 \delta_{ij} \dot{\epsilon}_{ii}) \\ & + K'\dot{\epsilon}_{ii}\delta_{ij} \end{aligned} \quad (2.13)$$

where $K = \lambda + 2/3\mu$ is called dilatational or bulk modulus. Separating the stress tensor of Equation 2.13 into the compressional and sheer component, the result, in the form of Equation 2.3, can be written as

$$\begin{aligned} \sigma_{ij} + \tau_v \dot{\sigma}_{ij} &= -K \epsilon_{ij} - \tau_v K_{\infty} \dot{\epsilon}_{ij} \\ \sigma_{ij} + \tau_s \dot{\sigma}_{ij} &= \mu \epsilon_{ij} + \tau_s \mu_{\infty} \dot{\epsilon}_{ij} \end{aligned} \quad (2.14)$$

where τ_v and τ_s are relaxation times, $K_{\infty} = K \tau_s / \tau_v$ and $\mu_{\infty} = \mu \tau_v / \tau_s$. The assumption of a harmonic stress in Equations 2.14 will produce a relation of the form

$$\sigma_{ij}(\omega) = -K_e(\omega) \epsilon_{ij}(\omega) \quad (2.15)$$

$$\sigma_{ij}(\omega) = \mu_e(\omega) \epsilon_{ij}(\omega) \quad (2.16)$$

where

$$\begin{aligned} K_e(\omega) &= K[1 - (1 - \tau_s/\tau_v)\omega^2\tau_v^2/(1 + \omega^2\tau_v^2) \\ &\quad - i\omega(1 - \tau_s/\tau_v)\tau_v/(1 + \omega^2\tau_v^2)] \\ \mu_e(\omega) &= \mu[1 - (1 - \frac{\tau_v}{\tau_s})\omega^2\tau_s^2/(1 + \omega^2\tau_s^2) \\ &\quad - i\omega(1 - \tau_v/\tau_s)\tau_s/(1 + \omega^2\tau_s^2)] \end{aligned} \quad (2.17)$$

Equation 2.17 is identical to Equation 2.16 of Liu et al. (74) which was derived independently. It is clear now that the effective moduli of a viscoelastic wave and therefore the velocity of the wave is a function of frequency which has been claimed by many investigators for body waves and recently by a few for surface waves. Futterman (38) and Lomintz (77) theorized the concept of frequency dependence of the phase velocity of body waves. Kogan (59), Lamb (65), Liu et al. (74), Magnitsky and Zharkov (81), Mason (83) and Strick (109) have studied the frequency dependent velocity of body waves and concluded the validity of Futterman's theory. Most recently a few studies have been devoted to frequency dependent phase velocity of surface wave and the free oscillation of the earth by Liu and Archambeau (73, 75), Park and Rockwell (92), Randall (95) and Kanamori and Anderson (53).

E. Two Dimensional Deformation

The six independent components of the stress tensor in a three dimensional, isotropic medium reduce to three independent components in two dimensions. Therefore, an $x - z$ plane propagation would result in a zero stress in y direction such that

$$\sigma_{xy} = \sigma_{yy} = \sigma_{zy} = 0 \quad (2.18)$$

Starting with the first two terms of Equation 2.11 (for derivation of the equations of motion, the inelastic part of the stress tensor need not be included for the moment),

$$\sigma_{ij} = \lambda \delta_{ij} \epsilon_{ii} + 2\mu \epsilon_{ij} \quad \begin{matrix} i = 1,3 \\ j = 1,3 \end{matrix} \quad (2.19)$$

and the constraint relation, Equation 2.18, the nonzero three independent component of the stress in two dimensional systems are

$$\begin{aligned} \sigma_{11} &= \sigma_{xx} = \lambda \epsilon_{xx} + 2\mu \epsilon_{xx} + \lambda \epsilon_{zz} \\ \sigma_{13} &= \sigma_{xz} = 2\mu \epsilon_{xz} \\ \sigma_{33} &= \sigma_{zz} = \lambda \epsilon_{xx} + 2\mu \epsilon_{zz} + \lambda \epsilon_{zz} \end{aligned} \quad (2.20)$$

where the components of the strain tensor are obtained from the relation

$$\epsilon_{ij} = 1/2 \left(\frac{\partial U_i}{\partial x_j} + \frac{\partial U_j}{\partial x_i} \right) \quad \begin{matrix} i = 1,3 \\ j = 1,3 \end{matrix} \quad (2.21)$$

as

$$\begin{aligned}
\epsilon_{11} &= \epsilon_{xx} = 1/2 \left(\frac{\partial U_x}{\partial x} + \frac{\partial U_x}{\partial x} \right) = \frac{\partial U_x}{\partial x} \\
\epsilon_{13} &= \epsilon_{xz} = 1/2 \left(\frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right) \\
\epsilon_{33} &= \epsilon_{zz} = 1/2 \left(\frac{\partial U_z}{\partial z} + \frac{\partial U_z}{\partial z} \right) = \frac{\partial U_z}{\partial z}
\end{aligned} \tag{2.22}$$

where

U_x and U_z are horizontal and vertical displacements of the propagating plane wave respectively. Substituting Equations 2.22 into Equations 2.20 and expressing the stress components in a vector form, the result is

$$\hat{\sigma} = (\sigma_{xx}, \sigma_{zz}, \sigma_{xz}) = \underline{D} \hat{\epsilon} \tag{2.23}$$

where

$$\underline{D} = \begin{bmatrix} \lambda+2\mu & \lambda & 0 \\ \lambda & \lambda+2\mu & 0 \\ 0 & 0 & \mu \end{bmatrix}, \quad \hat{\epsilon} = \left(\frac{\partial U_x}{\partial x}, \frac{\partial U_z}{\partial z}, \frac{\partial U_z}{\partial x} + \frac{\partial U_x}{\partial z} \right) \tag{2.24}$$

Equation 2.23 is derived for an elastic medium; for an inelastic medium the Lamé constants in Equation 2.23 must be replaced by the effective moduli of Equation 2.17.

F. Wave Equation in Cartesian Coordinates

Using the stress tensor of Equation 2.19 the wave equation is obtained by

$$\sigma_{ij,j} = \frac{\partial}{\partial x_j} (\sigma_{ij}) = F_i \tag{2.25}$$

If the force F_i is assumed to be the inertia force then

$$\frac{\partial}{\partial x_j} (\sigma_{ij}) = \rho \ddot{U}_i \quad (2.26)$$

where ρ is the density and double dot denotes the acceleration.

Assumption of a harmonic plane wave allows a displacement of the form

$$U_i = U_i(x, z) e^{i\omega t} \quad (2.27)$$

where

$$U_i = (U_x, U_z)$$

Substitution of Equations 2.20, 2.22 and 2.27 into Equation 2.26 leads to a two dimensional equation of motion

$$\begin{aligned} -\rho\omega^2 U_x &= (\lambda + \mu) \left(\frac{\partial^2 U_x}{\partial x^2} + \frac{\partial^2 U_z}{\partial x \partial z} \right) + \mu \left(\frac{\partial^2 U_x}{\partial x^2} + \frac{\partial^2 U_x}{\partial z^2} \right) \\ -\rho\omega^2 U_z &= (\lambda + \mu) \left(\frac{\partial^2 U_x}{\partial x \partial z} + \frac{\partial^2 U_z}{\partial z^2} \right) + \mu \left(\frac{\partial^2 U_z}{\partial x^2} + \frac{\partial^2 U_z}{\partial z^2} \right) \end{aligned} \quad (2.28)$$

Since the displacement U_i is a function of x and z , an independent product of the function of x and z may be replaced by $U_i(x, z)$

$$U_x(x, z) = U_x(z)W(x) \quad (2.29)$$

$$U_z(x, z) = U_z(z)W(x)$$

Substitution of Equations 2.29 into Equations 2.28 yields the coupled ordinary second order differential equations

$$\begin{aligned} \mu \frac{d^2 U_x}{dz^2} - iK(\lambda + \mu) \frac{dU_z}{dz} - K^2(\lambda + 2\mu)U_x + \rho\omega^2 U_x &= 0 \\ (\lambda + 2\mu) \frac{d^2 U_z}{dz^2} - iK(\lambda + \mu) \frac{dU_x}{dz} - K^2\mu U_z + \rho\omega^2 U_z &= 0 \end{aligned} \quad (2.30)$$

and

$$\frac{d^2 W}{dx^2} + K^2 W = 0 \quad (2.31)$$

Equation 2.31 has a solution of the form

$$W(x) = e^{-iKx} \quad (2.32)$$

and Equation 2.27 may be expressed by

$$\begin{aligned} U_x &= U_x(z)e^{i(\omega t - Kx)} \\ U_z &= U_z(z)e^{i(\omega t - Kx)} \end{aligned} \quad (2.33)$$

the amplitudes $U_x(z)$ and $U_z(z)$ are obtained by the solution of the differential Equations 2.30.

So far there has not been any restriction on the condition of Equation 2.30 and the amplitudes $U_x(z)$ and $U_z(z)$. Equation 2.30 may be solved for a general plane wave. Our interest is in a solution with proper constraints to produce a Rayleigh wave, a plane wave that propagates in one direction and dissipates in the direction normal to the direction of propagation. In general, these two directions need not be perpendicular; when the medium of propagation is viscoelastic the

direction of propagation and dissipation of Rayleigh wave are not perpendicular. In this case the wave is called a generalized Rayleigh wave and is discussed in more detail in Chapter IV.

To acquire a Rayleigh wave solution, in an N layered medium, from Equation 2.33, we analyze Equation 2.30. For an N layered medium Equation 2.30 constitutes $2N$ second order differential equations. The boundary conditions required for the solution of these equations are continuity of the stresses and displacements at each interfaces, zero stress at free surface, and zero displacement amplitude at a distance far from the surface. The stresses considered are the stress normal to the plane interface and the shear stress on the plane interface; they are given by

$$\sigma_{zz} = \lambda \frac{\partial U_x}{\partial x} + (\lambda + 2\mu) \frac{\partial U_z}{\partial z} \quad (2.34)$$

$$\sigma_{xz} = \mu \left(\frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right)$$

There must be continuity at the interfaces where the Lamé' constants just above and below the interfaces may be different. Thus, the conditions for the stresses are

$$\lambda_1 \frac{\partial U_x}{\partial x} + (\lambda_1 + 2\mu) \frac{\partial U_z}{\partial z} = 0, \quad \mu_1 \left(\frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right) = 0 \quad (2.35)$$

on the surface and

$$\lambda_{j-1} \frac{\partial U_x}{\partial x} + (\lambda_{j-1} + 2\mu_{j-1}) \frac{\partial U_z}{\partial z} = \lambda'_{j-1} \frac{\partial U_x}{\partial x} + (\lambda'_{j-1} + 2\mu_{j-1}) \frac{\partial U_z}{\partial z}$$

$$\mu_{j-1} \left(\frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right) = \mu'_{j-1} \left(\frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right) \quad (2.36)$$

$j = 2, \dots, N$, at the interfaces. Also the conditions for continuity of displacements are

$$U_{x_j} = U'_{x_j} \quad \text{and} \quad U_{z_j} = U'_{z_j} \quad j = 2, \dots, N \quad (2.37)$$

and for Z_{N+1} far enough below the surface

$$U_{x_{N+1}} = 0 \quad \text{and} \quad U_{z_{N+1}} = 0. \quad (2.38)$$

With these boundary conditions the solution of Equation 2.30 will lead to an eigenvalue problem whose solutions are extremely difficult. For this reason we leave further discussion of Equation 2.30 for the next chapter in which we discuss the finite element method.

CHAPTER III

THEORETICAL BACKGROUND OF THE FINITE ELEMENT METHOD

A. Summary

For a detailed review of the method of finite element the reader is referred to the texts by Bathe and Wilson (11), Desai and Abel (25), Norrie and de Vries (89), Richards (97), Segerlind (103) and Zienkiewicz (124) for mathematical and engineering application, and by Lysmer (78), Lysmer and Waas (79), Lysmer and Drake (80), Segol et al. (105) and Waas (117) for seismological applications. A summary of the derivation of the essential equations will be discussed in this chapter.

The finite element technique has been brought to seismology in recent years. It seems to be more successful than the method of finite difference. The basic difference between the two techniques is that in the finite difference method the continuous medium is used to derive a set of differential equations which are replaced by finite difference equations. In the finite element technique the continuous medium is first approximated and the governing differential equations are solved numerically. Therefore, in the finite element technique the basic assumption is to divide the continuous layered medium into a discrete system consisting of, for example, rectangular elements each having four nodal points. The displacement of the structure is equivalent to

the displacement of the nodal points and all the forces between the elements and the external forces are transmitted through the nodal points.

The forces that produce displacements within an element are represented by

$$\hat{F} = \underline{K} \hat{U} \quad (3.1)$$

where $\underline{K}$ is called the stiffness matrix depending on the orientation and properties of the elements. If the inertia force is present Equation 3.1 is written as

$$\hat{F} = (\underline{K} - \omega^2 \underline{M}) \hat{U} \quad (3.2)$$

where $\underline{M}$ is the mass matrix of the element and ω is the frequency of the vibration. Since the number of elements have to be finite the structure must be bounded. For this reason the structure with irregular geometry, I, is joined, along the vertical planes, to the regions, L and R, which extend infinitely to the left and to the right, respectively (Figure 4). Regions L and R consist of horizontal layers which are welded together at their interfaces and may differ in their material properties and thickness. For this configuration the finite element method is used to analyze the motion in the irregular zone I and a discrete theory, which is essentially a one dimensional finite element, is used to analyze the motion of the layered zones. The natural layers of the L and R zones must be subdivided into a larger number of layers so that the nodal points of the elements in the irregular zone at the vertical boundaries coincide with the layer interfaces. The thickness of the

element is determined by the wave length of the S wave in the layered zones (104).

B. Setup of the Method

The irregular region of zone I consists of an assemblage of quadrilateral elements interconnected at their nodal points. A typical element e is defined by its nodes i, j, K and L . The displacements at the element nodes are taken as unknown and the solution within each element is approximated by a shape function $\underline{N}$ that varies linearly along the boundaries, i.e.,

$$\hat{U} = \underline{N}\hat{V} = [N_i, N_j, N_K, N_L] (V_i, V_j, V_K, V_L) \quad (3.3)$$

in which, for a plane wave propagation,

$$\hat{V}_i = (V_1, V_2)i \text{ etc.} \quad (3.4)$$

are the horizontal and vertical displacements at the element's node i , etc., and the matrix $\underline{N}^T$ is the shape function in terms of (ξ, η) , the local coordinates of the element. The component N_i , etc. of the matrix $\underline{N}^T$ is unity at the node i , etc. and is equal to zero at other nodes.

The above displacement is related to the strain by

$$\hat{\epsilon} = \underline{B}\hat{U} \quad (3.5)$$

where

$$\underline{B} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial x} & \frac{\partial}{\partial z} \end{bmatrix}, \quad \hat{U} = (U_x, U_z) \quad (3.6)$$

If the strain energy, or the energy density ψ , defined in Chapter IIB, exists, then as in Chapter IIB, its derivative with respect to the strain tensor is the stress field and is given by

$$\frac{\partial \psi}{\partial \epsilon_{ij}} = \sigma_{ij} \quad (3.7)$$

The elastic part of the stress in Equation 3.7 in terms of the strain is given by

$$\sigma_{ij} = \lambda \epsilon_{ij} \delta_{ij} + 2\mu \epsilon_{ij} \quad (3.8)$$

Equation 3.7 can be written equivalently as

$$\delta \psi = \sigma_{ij} \delta \epsilon_{ij} \quad (3.9)$$

which is the virtual work done by the force F_i per unit of volume and the surface force T_i per unit of area (37). For static equilibrium the expression

$$\int_V \sigma_{ij} \delta \epsilon_{ij} dv = \int_V F_i \delta U_i dv + \int_S T_i \delta U_i ds \quad (3.10)$$

is called the principle of virtual work produced by the virtual displacement δU_i . Equation 3.10 will be used to derive the equation of motion of an element in the irregular region I. If the body force F_i is the inertia force per unit of volume, a set of inhomogeneous differential equations can be derived by rewriting Equation 3.10 as

$$\int_V (\sigma_{ij} \delta \epsilon_{ij} - F_i \delta U_i) dv = \int_S T_i \delta U_i ds \quad (3.11)$$

This equation, with T_i equal to zero can also be used to derive a homogeneous second order differential equation for the layered medium, L and

R of Figure 4, as

$$\int_V (\sigma_{ij} \delta \epsilon_{ij} - F_i U_i) dv = 0. \quad (3.12)$$

The first term of this equation can be written as

$$\begin{aligned} \int_V \sigma_{ij} \delta \epsilon_{ij} dv &= \int_V \sigma_{ij} (\delta U_{ij} + \delta U_{ji}) dv \\ &= - \int_V \delta_{ij,j} \delta U_i dv + \int_S \sigma_{ij} \hat{n}_j \delta U_i ds, \end{aligned} \quad (3.13)$$

using the familiar expression

$$\epsilon_{ij} = 1/2 \left(\frac{\partial U_i}{\partial x_j} + \frac{\partial U_j}{\partial x_i} \right) \quad (3.14)$$

and the Gauss's theorem. Substitution of Equation 3.13 into Equation 2.12 gives

$$\int_V (\sigma_{ij,j} + F_i) \delta U_i dv - \int_S \sigma_{ij} \hat{n}_j \delta U_i ds = 0 \quad (3.15)$$

which for an arbitrary δU_i satisfies the equation of motion

$$\sigma_{ij,j} + F_i = 0 \quad (3.16)$$

and either

$$\delta U_i = 0 \quad (3.17)$$

on the rigid boundary or

$$\sigma_{ij} \hat{n}_j = 0, \quad (3.18)$$

the surface boundary condition.

Equation 3.15 determines the motion of the wave in the horizontal layers L and R. The first integrand of this equation is the general equation of motion and the second integrand is the horizontal boundary condition, i.e., in a plane wave case, the normal stress in the z direction and the shear stress on the plane interfaces. For convenience we write Equation 3.15 in a vector form as

$$\int_V \delta \hat{U}^T \cdot \hat{W} dv - \int_S \delta \hat{U}^T \cdot \sigma \hat{n} ds = 0 \quad (3.19)$$

where W is the equation of motion, and T represents complex conjugate transpose.

C. Rayleigh Wave Motion

The displacement of the medium in the horizontal and vertical directions produced by the propagation of a Rayleigh wave can be represented by (1)

$$\begin{aligned} U_x &= U_x(x, z) e^{i\omega t} \\ U_z &= U_z(x, z) e^{i\omega t} \end{aligned} \quad (3.20)$$

Substitution of this equation into the equations of motion, with no force present, and the separation of variables in the usual way would result in

$$\begin{aligned} U_x(x, z) &= U_x(z) e^{-iKx} \\ U_z(x, z) &= U_z(z) e^{-iKx} \end{aligned} \quad (3.21)$$

Substitution of Equation 3.21 into Equation 3.20 will produce the vector displacement of Equation 3.19 as

$$\hat{U} = (U_x(z), U_z(z))e^{i(\omega t - Kx)} \quad (3.22)$$

The six component stress tensor in three dimensions will reduce to a three component stress vector in two dimensions and is given by

$$\hat{\sigma} = (\sigma_{xx}, \sigma_{zz}, \sigma_{xz}) \quad (3.23)$$

where, from Equation 3.8

$$\begin{aligned} \sigma_{xx} &= (\lambda + 2\mu) \frac{\partial U_x}{\partial x} + \lambda \frac{\partial U_z}{\partial z} \\ \sigma_{zz} &= \lambda \frac{\partial U_x}{\partial x} + (\lambda + 2\mu) \frac{\partial U_z}{\partial z} \\ \sigma_{xz} &= \mu \left(\frac{\partial U_x}{\partial z} + \frac{\partial U_z}{\partial x} \right) \end{aligned} \quad (3.24)$$

Therefore,

$$\hat{\sigma} = \underline{D} \hat{\epsilon} \quad (3.25)$$

where

$$\underline{D} = \begin{bmatrix} \lambda+2\mu & \lambda & 0 \\ \lambda & \lambda+2\mu & 0 \\ 0 & 0 & \mu \end{bmatrix} \quad (3.25a)$$

$$\hat{\epsilon} = \left(\frac{\partial U_x}{\partial x}, \frac{\partial U_z}{\partial z}, \frac{\partial U_z}{\partial x} + \frac{\partial U_x}{\partial z} \right) \quad (3.26)$$

At the interfaces the stress component in the x direction is zero and the surface stress $\hat{\sigma}_v$ in Equation 3.19, and the matrix $\underline{D}$ in this case are

$$\hat{\sigma}_v = (\sigma_{zz}, \sigma_{xz}) , \quad (3.27)$$

$$\underline{D}' = \begin{bmatrix} \lambda & \lambda+2\mu & 0 \\ 0 & 0 & \mu \end{bmatrix} , \quad (3.28)$$

then

$$\hat{\sigma}_v = \underline{D}' \hat{\epsilon} \quad (3.29)$$

respectively. Substituting Equations 3.22, 3.24-3.26 and 3.27- 3.29 into Equation 3.19 and assuming that F_i is the inertia force per unit of volume we have

$$\int_V (\delta \hat{\epsilon}^T \cdot \underline{D}' \hat{\epsilon} - \rho \omega^2 \delta \hat{U}^T \cdot \hat{U}) dx dz - \int_S \delta \hat{U}^T \cdot \underline{D}' \hat{\epsilon} dx \quad (3.30)$$

where the elemental volume dv and surface ds in three dimension reduce to $dx dz$ and dx in two dimensions, respectively. Using Gauss's theorem on the last term of Equation 3.30 we get

$$\int_S \delta \hat{U}^T \cdot \underline{D}' \hat{\epsilon} dx = \int_V DZ^T (\delta \hat{U}^T \cdot \underline{D}' \hat{\epsilon}) dx dz \quad (3.31)$$

where $DZ^T = \frac{\partial}{\partial z} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix}^T$ and T represents transpose. Since $\frac{\partial}{\partial z}$ operates

on a product vectors $\delta \hat{U}$ and $\hat{\epsilon}$ the right hand side of Equation 3.31 expands to

$$\begin{aligned} \int_V \frac{\partial}{\partial z} I^T (\delta \hat{U}^T \cdot \underline{D}' \hat{\epsilon}) dx dz &= \int_V DZ^T \delta \hat{U}^T \cdot (\underline{D}' \hat{\epsilon}) dx dz \\ &+ \int_V \delta \hat{U}^T \cdot \underline{D}' DZ^T \hat{\epsilon}^T dx dz \end{aligned} \quad (3.32)$$

If we rewrite Equation 3.6

$$\hat{\epsilon} = \left(\frac{\partial U_x}{\partial x}, \frac{\partial U_z}{\partial z}, \frac{\partial U_z}{\partial x} + \frac{\partial U_x}{\partial z} \right) = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial x} \end{bmatrix} (U_x, U_z) \quad (3.33)$$

$$\hat{\epsilon}^T = \left(\begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial x} \end{bmatrix} (U_x, U_z) \right)^T \quad (3.34)$$

and substitute Equations 3.32 - 3.34 into Equation 3.30 we have

$$\int (\delta U^T \cdot \underline{H} \hat{U} - DZ^T \hat{U}^T \cdot \underline{H}' \hat{U}) dx dz = 0 \quad (3.35)$$

where

$$\underline{H} = \begin{bmatrix} K^2(\lambda + 2\mu) - \rho\omega^2 & iK\lambda \frac{\partial}{\partial z} \\ iK\mu \frac{\partial}{\partial z} & K^2\mu - \rho\omega^2 \end{bmatrix} \quad (3.36)$$

and

$$\underline{H}' = \begin{bmatrix} -iK\lambda & (\lambda + 2\mu) \frac{\partial}{\partial z} \\ \mu \frac{\partial}{\partial z} & -iK\mu \end{bmatrix} \quad (3.37)$$

Equations 3.35 involves partial derivatives with respect to z of the displacement U in Equation 3.22 which is an implicit function of z . The differential equations obtained in this way will possess the unknowns

in the argument of the exponential functions and thus their solution are nearly impossible. For this reason we use the technique of the finite element to derive equations that may be solved conveniently.

For the layered region we use a one dimensional element, i.e., an element with two nodal points for the two horizontal plane interfaces sharing the j^{th} layer as shown in Figure 5. Let the displacement vector U be approximated by,

$$\hat{U} = \underline{N} \hat{V} \quad (3.38)$$

so that

$$\hat{U}^T = \hat{V}^T \underline{N}^T \quad (3.39)$$

The partial derivative of this with respect to z is

$$\frac{\partial}{\partial z} \hat{U}^T = \hat{V}^T \frac{\partial}{\partial z} \underline{N}^T \quad (3.40)$$

where, from Equation 3.3, for a linear expansion

$$\underline{N} = [N_i, N_j] = 1/2 \begin{bmatrix} 1 - \eta & 0 & 1 + \eta & 0 \\ 0 & 1 - \eta & 0 & 1 + \eta \end{bmatrix} \quad (3.41)$$

If the origin is at the midpoint of the layer then

$$\eta = \frac{z}{h/2} \quad (3.42)$$

$z = -h/2$, $\eta = -1$ at the top of the layer and $z = h/2$, $\eta = 1$ at the bottom of the layer (124) as shown in Figure 5.

Substitution of Equations 3.38-3.40 into Equation 3.35 results in

$$\sum_{\text{Layers}} \int_x \delta \hat{V}^T \left(\int_z K^2 \underline{N}^T \underline{a} \underline{N} + i K \underline{N}^T \underline{c} \frac{\partial}{\partial z} \underline{N} - \rho \omega^2 \underline{N}^T \underline{N} + \frac{\partial}{\partial z} \underline{N}^T \underline{g} \frac{\partial}{\partial z} \underline{N} \right. \\ \left. + i K \frac{\partial}{\partial z} \underline{N}^T \underline{c}^T \underline{N} \right) \hat{V} dx dz = 0 \quad (3.43)$$

where

$$\underline{a} = \begin{bmatrix} \lambda + 2\mu & 0 \\ 0 & \mu \end{bmatrix}, \quad \underline{c} = \begin{bmatrix} 0 & \lambda \\ \mu & 0 \end{bmatrix}, \quad \text{and} \quad \underline{g} = \begin{bmatrix} \mu & 0 \\ 0 & \lambda + 2\mu \end{bmatrix} \quad (3.44)$$

Evaluation of the integral for each layer gives

$$\int_0^h \frac{\partial}{\partial z} \underline{N}^T \underline{g} \frac{\partial}{\partial z} \underline{N} dz = \underline{G}_j$$

$$\int_0^h \underline{N}^T \underline{a} \underline{N} dz = \underline{A}_j \quad (3.45)$$

$$\int_0^h \frac{\partial}{\partial z} \underline{N}^T \underline{c} \underline{N} dz = \underline{C}_j$$

$$\int_0^h \underline{N}^T \underline{N} dz = \underline{M}_j$$

And from Equation 3.42

$$\frac{\partial}{\partial z} = \frac{1}{h/2} \frac{\partial}{\partial n} \quad (3.46)$$

so that

$$\frac{\partial}{\partial z} \underline{N} = \frac{1}{h/2} \frac{\partial}{\partial n} \left(\frac{1}{2} \begin{bmatrix} 1-n & 0 & 1+n & 0 \\ 0 & 1-n & 0 & 1+n \end{bmatrix} \right) = \frac{1}{h} \begin{bmatrix} -1 & 0 & 1 & 0 \\ 0 & -1 & 0 & 1 \end{bmatrix} \quad (3.47)$$

Substitution of Equations 3.41, 3.42 and 3.46 into the first of Equations 3.45 leads to

$$\int_0^h \frac{\partial}{\partial z} \underline{N}^T \underline{q} \frac{\partial}{\partial z} \underline{N} dz = h/2 \int_{-1}^1 \frac{1}{h^2} \begin{bmatrix} -1 & 0 \\ 0 & -1 \\ 1 & 0 \\ 0 & 1 \end{bmatrix} \begin{bmatrix} \mu & 0 \\ 0 & \lambda+2\mu \end{bmatrix} \begin{bmatrix} -1 & 0 & 1 & 0 \\ 0 & -1 & 0 & 1 \end{bmatrix} dn$$

or

$$\underline{G}_j = \frac{1}{h_j} \begin{bmatrix} \mu & 0 & -\mu & 0 \\ 0 & \lambda+2\mu & 0 & -(\lambda+2\mu) \\ -\mu & 0 & \mu & 0 \\ 0 & -(\lambda+2\mu) & 0 & \lambda+2\mu \end{bmatrix} \quad (3.48)$$

Similarly,

$$\underline{A}_j = \frac{h_j}{6} \begin{bmatrix} 2(\lambda+2\mu) & 0 & \lambda+2\mu & 0 \\ 0 & 2\mu & 0 & \mu \\ \lambda+2\mu & 0 & 2(\lambda+2\mu) & 0 \\ 0 & \mu & 0 & 2\mu \end{bmatrix}, \quad \underline{C}_j = 1/2 \begin{bmatrix} 0 & \lambda & 0 & -\lambda \\ \mu & 0 & -\mu & 0 \\ 0 & \lambda & 0 & -\lambda \\ \mu & 0 & -\mu & 0 \end{bmatrix} \quad (3.49)$$

$$\underline{M}_j = \rho_j h_j/6 \begin{bmatrix} 2 & 0 & 1 & 0 \\ 0 & 2 & 0 & 1 \\ 1 & 0 & 2 & 0 \\ 0 & 1 & 0 & 2 \end{bmatrix} \quad (3.50)$$

The j^{th} layer matrices $\underline{A}_j$, $\underline{B}_j$, $\underline{C}_j$ and $\underline{M}_j$ are the same matrices introduced by Lysmer and Drake (80) which were derived originally by Lysmer and Waas (79) by a different method. Addition of these submatrices over the

layers by the method of Lysmer and Drake (80), as shown in Figure 6 for a typical matrix $\underline{A}_j$, will result in

$$\int_x \delta V^T (K^2 \underline{A} - iK\underline{C} + iK\underline{C}^T + \underline{G} - \omega^2 \underline{M}) \hat{V} dx = 0 \quad (3.51)$$

For an arbitrary virtual displacement vector δV^T (37) we may write, since the expression in parentheses is independent of x ,

$$(K^2 \underline{A} - iK\underline{C} + iK\underline{C}^T + \underline{G} - \omega^2 \underline{M}) \hat{V} = 0 \quad (3.52)$$

Equation 3.52 represents a nonlinear eigenvalue problem with eigenvalue K^2 and eigenvector $\underline{V}$ whose solution is usually obtained by the vector iteration method. The analysis of the derivation of the solution of Equation 3.52 will not be presented here but the reader is recommended to refer to the excellent texts by Bathe and Wilson (11), Waas (117), and Zienkiewicz (124) for engineering applications and by Ruhe (100), Papaconstantinov (91), and Wilkinson (120) for theoretical discussions. At a particular frequency, the generalized eigenvalue problem (Equation 3.52) has $4N$ eigenvalues $\sum_i^{2N} K_i^2$ and corresponding eigenvectors $\sum_i^{2N} \hat{V}_i$, N being the number of layers. The eigenvalues may be, real, imaginary or complex.

In the derivation of our eigenvalue problem we assumed that the matrix moduli $\underline{D}$ be real. For a viscoelastic model we may substitute the effective moduli of Equation 2.17 of Chapter II into matrix $\underline{D}$, which is

$$\underline{D} = \begin{bmatrix} \lambda_e + 2\mu_e & \lambda_e & 0 \\ \lambda_e & \lambda_e + 2\mu_e & 0 \\ 0 & 0 & \mu_e \end{bmatrix} \quad (3.53)$$

where

$$\lambda_e = K_e - 2/3 \nu_e \quad (3.54)$$

and subscript e stands for effective or complex moduli. If matrix $\underline{D}$ is real, the motion of the Rayleigh wave in the layers is determined by the number of real wave numbers obtained from the solution of the eigenvalue problem. If matrix $\underline{D}$ is complex the eigenvalue problem has complex eigenvalues. The real part of the eigenvalue determines the phase velocity of the wave in the layers and the imaginary part determines the attenuation of the wave in the horizontal direction. Thus,

$$\text{phase velocity} = \frac{\omega}{\text{REAL}(\text{wavenumber})} \quad (3.55)$$

$$\text{attenuation} = 2 \frac{\text{IMAG}(\text{wavenumber})}{\text{REAL}(\text{wavenumber})} \quad (3.55a)$$

According to the theory of linear differential equation, if each eigenvalue and the corresponding independent eigenvectors determine a solution then a linear combination of these solutions is also a solution. Therefore, if K_i for $i = 1, 2N$ are the eigenvalues and $\hat{V}_i$ for $i = 1, 2N$ are the corresponding eigenvectors the complete motion of the layered region may be obtained by superposition of these solutions as

$$\hat{U} = \sum_i^{2N} \alpha_i \hat{V}_i e^{i(\omega t - K_i x)} \quad (3.56)$$

in which each multiplier α is an amplitude factor specifying the contribution of the mode i to the total motion. At the boundary with no phase change the motion is

$$\hat{U} = \underline{V} \hat{\alpha} \quad (3.57)$$

where $\underline{V}$ is the matrix whose columns are the eigenvectors $\hat{V}_i$. If the displacement $\hat{U}$ in Equation 3.57 is known, the multiplier vector $\hat{\alpha}$ can be solved for as

$$\hat{\alpha} = \underline{V}^{-1} \hat{U} \quad (3.58)$$

D. Vertical Boundary

Let us rewrite Equation 3.11

$$\int_V (\sigma_{ij} \delta \epsilon_{ij} - F_i \delta U_i) dv = \int_S T_i \delta U_i ds \quad (3.59)$$

and substitute Equation 3.13 into it; the result is

$$\int_V (\sigma_{ij,j} + F_i) \delta U_i dv + \int_S (T_i - \sigma_{ij} v) \delta U_i ds = 0 \quad (3.60)$$

This equation can be satisfied, for an arbitrary δU_i , if

$$\sigma_{ij,j} + F_i = 0, \quad (3.61)$$

the equation of motion, and either

$$\delta U_i = 0 \quad (3.62)$$

on the rigid boundary or

$$\int_S (T_i - \sigma_{ij} v) \delta U_i ds = 0, \quad (3.63)$$

the surface boundary condition on the vertical plane. The elemental surface ds in this case is dz , therefore,

$$\int_S T_i \delta U_i dz = \int_S \sigma_{ij} v \delta U_i dz \quad (3.64)$$

On the vertical plane the stress component in the z direction is zero and the surface stress $\sigma_{ij}v$ in Equation 3.64, in vector form, is

$$\hat{\sigma}v = (\sigma_{xx}, \sigma_{xz}) \quad (3.65)$$

and the matrix moduli corresponding to Equation 3.28, in this case is

$$\underline{D}_v = \begin{bmatrix} \lambda+2 & \lambda & 0 \\ 0 & 0 & \mu \end{bmatrix} \quad (3.66)$$

Now the right hand side of Equation 3.63 becomes

$$\int_S \hat{\sigma}v \cdot \delta \hat{U} dz = \int_S \delta \hat{U}^T \cdot \underline{D}_v \hat{\epsilon} dz \quad (3.67)$$

Substituting Equations 3.33, 3.38-3.42, and 3.57 into Equation 3.67, we have

$$\sum_{\text{Layers}} \int_S \hat{\sigma}v \cdot \delta \hat{U} dz = (i \underline{A} \underline{V} \underline{K} - \underline{C} \underline{V}) \hat{\alpha} \delta \hat{V} \quad (3.68)$$

The derivation of this equation takes the same steps that lead to the derivation of Equation 3.52. In Equation 3.68 all matrices and the vector $\hat{\alpha}$ have been defined except for the matrix $\underline{K}$. Matrix $\underline{K}$ is a diagonal matrix whose diagonal elements are the eigenvalues obtained from the solution of the eigenvalue problem. Equation 3.68 represents the force applied at the boundary with the assumption that the finite element region is removed. The reaction forces acting on the finite element region follow then from the equilibrium consideration, i.e., a force equal and opposite to that of Equation 3.68

$$\hat{P} = (i \underline{A} \underline{V} \underline{K} - \underline{C} \underline{V}) \hat{\alpha} \quad (3.69)$$

Substitution of $\hat{\alpha}$ from Equation 3.58 into Equation 3.69 yields

$$\hat{P} = \underline{L} \hat{U} \quad (3.70)$$

where

$$\underline{L} = i \underline{A} \underline{V} \underline{K} \underline{V}^{-1} - C \quad (3.71)$$

is called the stiffness matrix of the vertical boundary.

E. Motion in the Finite Element Region

Rewriting Equation 3.10 with the assumption that the inertia force be F_i , we have, in vector form,

$$\int_V \hat{\sigma} \cdot \hat{\epsilon} \, dv - \rho \omega^2 \int_V \hat{U}^T \cdot \hat{U} \, dx dz = \int_S \hat{U}^T \cdot T \hat{V} \, dz \quad (3.72)$$

The left hand side of Equation 3.72 can be written as

$$\int_V (\delta \hat{\epsilon}^T \cdot \underline{D} \hat{\epsilon} - \rho \omega^2 \delta \hat{U}^T \cdot \hat{U}) \, dx dz = \int_V (\delta \hat{U}^T \underline{B}^T \cdot \underline{D} \underline{B} \hat{U} - \rho \omega^2 \delta \hat{U}^T \cdot \hat{U}) \, dx dz \quad (3.73)$$

where matrix $\underline{B}$ is given by Equation 3.6. Substituting Equations 3.38-3.42 into the expression 3.73, Equation 3.72 becomes

$$\int_V (\delta \hat{V}^T \underline{N} \underline{B}^T \cdot \underline{D} \underline{B} \underline{N}^T \hat{V} - \rho \omega^2 \delta \hat{V}^T \underline{N} \cdot \underline{N}^T \hat{V}) \, dx dz = \pm \int_S \delta \hat{V}^T \underline{N} \cdot T \hat{V} \, dz \quad (3.74)$$

where, from Equation 3.73, for a two dimensional expansion

$$\underline{N} = [N_i, N_j, N_K, N_L] = \begin{bmatrix} \frac{1}{2}n(n-1) & 0 & 1-n^2 & 0 & \frac{1}{2}n(n+1) & 0 \\ 0 & \frac{1}{2}n(n-1) & 0 & 1-n^2 & 0 & \frac{1}{2}n(n+1) \end{bmatrix} \quad (3.75)$$

The integration limits on the right hand side of Equation 3.74 is through the two vertical boundaries on the left and right hand sides of the irregular region and the $\pm$ signs represent the forces that are acting to or away from the finite element region. All in all, there are a maximum of four different forces acting at the vertical boundaries that are determined from the right hand side of Equation 3.74. One of these forces was determined by Equation 3.69. If the incident energy is from the left then there are three forces acting at the vertical boundaries. A force P_L determined by Equation 3.69 due to the incident energy at the left hand boundary, a force P_R due to reflection of energy at the left boundary, and a force P_T due to transmission of energy at the right boundary. Thus, the right hand side of Equation 3.74 is integrated to produce the above three forces, for an arbitrary $\delta \hat{V}^T$. Substitution of Equations 3.6, 3.25a, and 3.75 into Equation 3.74, the first term on the left hand side is integrated, for an arbitrary $\delta \hat{V}^T$, to give

$$\int_V \underline{N} \underline{B}^T \cdot \underline{D} \underline{B} \underline{N}^T dv = \underline{K}_e \quad (3.76)$$

Matrix $\underline{K}_e$ is an 8 by 8 stiffness matrix originating from four nodal points of the element, each node having a horizontal and a vertical force. To obtain the stiffness matrix for all elements Equation 3.76 must be summed over the elements. Thus,

$$\sum_{\text{Elements}} \underline{K}_e = \underline{K} \quad (3.77)$$

Substitution of the contribution from Equations 3.45, 3.69, and 3.77 into Equation 3.74 will get

$$(\underline{K} - \omega^2 \underline{M}) \hat{V} = P_T - P_L - P_R \quad (3.78)$$

or

$$(\underline{K} - \omega^2 \underline{M}) \hat{V} = \underline{I} \hat{V}_T - \underline{L} \hat{V}_L - \underline{R} \hat{V}_R = \underline{I} \hat{V}_T - \underline{L} \hat{V}_L - \underline{R}(\hat{V}_L + \hat{V}_R) + \underline{R} \hat{V}_L \quad (3.79)$$

The vector $\hat{V}$ represents the horizontal and vertical displacements of the nodal points of the irregular region. Vector $\hat{V}_L$ is the known displacements at the left hand boundary, obtained from the solution of the eigenvalue problem. The unknown vectors $\hat{V}_R$, the reflected displacements, and vector $\hat{V}_T$, the right hand boundary displacement can be incorporated into vector $\hat{V}$ as follows. The first $2N$ elements of vector $\hat{V}$ are equal to vector $\hat{V}_L$ and if the vectors $\hat{V}_L + \hat{V}_R$ are substituted for the $2N$ elements of vector $\hat{V}$, then the unknown vector $\hat{V}_R$ can be solved for by

$$\hat{V}_R = \hat{V}_{2N} - \hat{V}_L. \quad (3.80)$$

The last $2N$ elements of vector $\hat{V}$ are equal to vector $\hat{V}_T$. Therefore, Equation 3.79 can be written as

$$(\underline{K} - \omega^2 \underline{M} + \underline{R} - \underline{I}) \hat{V} = (\underline{R} - \underline{L}) \hat{V}_L. \quad (3.81)$$

The unknown vector $\hat{V}$ in this equation can be solved for by the standard method (28, 29).

CHAPTER IV

ENERGY OF A PROPAGATING WAVE

A. Summary

Elasticity theory assumes the medium of propagation to be conservative, a condition in which no energy is lost or changed into other forms. In reality this process is thermodynamically reversible only if it occurs with infinitesimal speed, so that the thermodynamic equilibrium is established in the body at every instant. Stressed body however, is in motion and has finite velocity. The body is not in equilibrium at every instant and, therefore, processes such as stress-induced ordering or thermal displacement of the point defect, change in dislocation through the crystal lattice, and viscous sliding at grain boundaries will take place in it and cause it to return to equilibrium. The existence of these processes has the result that the motion is irreversible, since they require the overcoming of a barrier to initiation. At the end of each cycle of stress it is impossible to reproduce exactly the state of the start; hence, there will be hysteresis effects and energy dissipation or attenuation (Figure 1). Because of these effects the amplitude of the propagating Rayleigh wave falls off somewhat more rapidly than elastic theory predicts. Although attenuation is extremely important in practice, it must still be, in some sense, small, since otherwise waves would not propagate to any appreciable

distance. For large attenuation the propagation process becomes one of the diffusion rather than propagation. Since waves propagate, the assumption of small attenuation keeps the theory of elasticity valid.

The dissipation of energy arises from such a variety of mechanisms that is impossible to discuss them all here. The most effective of all mechanisms fall into the class of viscoelasticity in which the energy of the wave field is taken up by processes such as defect in mobility, diffusion rate, phase change, etc., and is restored partially or completely to the field with a delay. As a result of this delay, the restored energy finds itself partially out of phase with the original wave and destructive interference takes place (114).

B. Energy Flux Vector

The energy flux vector $E_i(t)$ at time t is defined as the rate at which energy leaves the material across an element of area normal to the x_i - axis, measured per unit area. It is given by

$$E_i(t) = - \sigma_{ij}(t) \dot{U}_j(t) \quad (4.1)$$

where $\dot{U}_j$ is the velocity vector created by the application of the stress tensor σ_{ij} , the minus sign is the indication that the energy is leaving the material. For an isotropic, elastic medium, the direction of the energy flow is normal to the wavefront. Equation 4.1 fluctuates with time and a mean energy flux vector is defined as the average of $E_i(t)$ over a cycle. Thus,

$$\bar{E}_i = \frac{\omega}{2\pi} \int_0^{\frac{2\pi}{\omega}} E_i(t) dt \quad (4.2)$$

The energy flux vector, E_i , contains three components. Corresponding to the directions of propagation, i.e., in vector form,

$$E_i = \hat{E} = \underline{\sigma} \cdot \frac{\partial}{\partial t} \hat{U} \quad (4.3)$$

where matrix $\underline{\sigma}$ and vector $\frac{\partial}{\partial t} \hat{U}$ are given by

$$\underline{\sigma} = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix}, \quad \frac{\partial}{\partial t} \hat{U} = \frac{\partial}{\partial t} \begin{bmatrix} U_x \\ U_y \\ U_z \end{bmatrix} \text{EXP} (i(\omega t - K_x x - K_y y - K_z z)) \quad (4.4)$$

For a plane wave propagation in the x direction, vector $\hat{E}$ reduces to a one dimensional energy flux, E , and is given by, setting $i = 1$, and $j = 1, 3$ in Equation 4.1, we have

$$E = \left(\frac{\partial}{\partial t} \hat{U} \right)^T \cdot \hat{\sigma} \quad (4.5)$$

where vectors $\hat{\sigma}$ and $\frac{\partial}{\partial t} \hat{U}$ are given by

$$\hat{\sigma} = (\sigma_{xx}, \sigma_{xz}), \quad \frac{\partial}{\partial t} \hat{U} = \frac{\partial}{\partial t} (U_x, U_z) \text{EXP} (i(\omega t - Kx)) \quad (4.6)$$

the stress vector represents normal and shear stresses on the surface area normal to the direction of propagation. The energy in Equation 4.5

in an elastic medium is normal to the wavefront, i.e., the energy is propagating in the x direction normal to the z axis, only if the propagation constant K is real. Let us analyze the state of vector $\hat{K}$ in general. From Equation 4.4, vector $\hat{K}$ in a plane propagation is

$$\hat{K} = K_x \hat{x} + K_z \hat{z} \quad (4.7)$$

where $\hat{x}$ and $\hat{z}$ represent direction cosines in the x and z directions, respectively. If the matrix moduli $\underline{D}$ in Chapter III contains the effective moduli K_e and μ_e , discussed in Chapter II, Section C, then vector $\hat{K}$ is complex and is given by

$$\hat{K} = \hat{K}_R - i\hat{K}_I \quad (4.7a)$$

Rewriting the vector $\hat{U}$ of Equation 4.4 in two dimensions

$$\hat{U} = \begin{pmatrix} U_x(z) \\ U_z(z) \end{pmatrix} \text{EXP} (i(\omega t - \hat{K} \cdot \hat{X})), \quad (4.8)$$

where

$$\hat{X} = x\hat{x} + z\hat{z}, \quad (4.9)$$

and substituting Equation 4.7 we have

$$\hat{U} = \hat{U}(z) e^{-\hat{K}_I \cdot \hat{X}} e^{i(\omega t - \hat{K}_R \cdot \hat{X})} \quad (4.10)$$

where K_I is called the attenuation vector. To include attenuation from the vector $U(z)$ it is better to write Equation 4.10 as

$$\hat{U} = |\hat{U}(z)| e^{-\hat{K}_I \cdot \hat{X}} e^{i(\omega t - \hat{K}_R \cdot \hat{X})} \quad (4.11)$$

The dot product $\hat{K}_I \cdot \hat{X}$ can be expanded as

$$\hat{K}_I \cdot \hat{X} = \beta X + \gamma z + \text{ARG}(\hat{U}(z)) \quad (4.12)$$

where β and γ represent the components of $\hat{K}_I$ in x and z directions, respectively, and $\text{ARG}(\hat{U}(z))$ is the attenuation factor due to the vector $\hat{U}(z)$. The existence of a surface wave depends on whether vector $\hat{K}_I$ is zero or not.

The attenuation vector $\hat{K}_I$, given by Equation 4.7a, is not, in general, perpendicular to the propagation vector $\hat{K}_R$; the angle θ between the directions of the two vectors is between zero and $\pi/2$, i.e.,

$$0 < \theta \leq \pi/2. \quad (4.13)$$

when $\theta = 0$, $\hat{K}_I = \hat{0}$, and therefore there exists an undamped wave. When $\theta = \pi/2$, the direction of attenuation vector $\hat{K}_I$ is perpendicular to the direction of propagation vector $\hat{K}_R$ and the wave propagates in the direction normal to the surface of constant phase and attenuates in the direction normal to the surface of constant amplitude. This occurs when $\beta = \gamma = 0$ in Equation 4.7a and the attenuation is due to the vector $\hat{U}(z)$; that is, the amplitude will dampout in the z direction. This is the classical Rayleigh wave.

The surface of constant phase may be used to determine the phase velocity of the wave by setting the progressive part of Equation 4.11 equal to a constant, say, $\pi/2$,

$$e^{i(\omega t - \hat{K} \cdot \hat{X})} = e^{i\pi/2} \quad (4.14)$$

or

$$\omega t - \hat{K} \cdot \hat{X} = -i\pi/2 \quad (4.15)$$

Performing the dot product in Equation 4.15 we get,

$$\omega t - K_x X - K_z Z = -i\pi/2. \quad (4.16)$$

Taking the partial derivative of x and z with respect to t in Equation 4.16, we have,

$$\omega - K_x \frac{\partial x}{\partial t} = 0, \quad \omega - K_z \frac{\partial z}{\partial t} = 0 \quad (4.17)$$

Solving for $\frac{\partial x}{\partial t}$ and $\frac{\partial z}{\partial t}$, the phase velocities in the horizontal and vertical directions are given by

$$V_x = \frac{\omega}{K_x}, \quad V_z = \frac{\omega}{K_z} \quad (4.18)$$

If the vector $\hat{K}$ is complex then

$$K_x = K_x + i\beta, \quad K_z = K_z + i\gamma, \quad (4.19)$$

and

$$V_x = \frac{\omega}{K_x + i\beta} = \frac{\omega(K_x - i\beta)}{K_x^2 + \beta^2} = \frac{\omega K_x}{K_x^2 + \beta^2} - i \frac{\beta \omega}{K_x^2 + \beta^2}, \quad (4.20)$$

or approximately

$$V_x = V_x - i\beta V_x = V_x(1 - i\beta), \quad (4.21)$$

for small β such that $\beta^2 \rightarrow 0$. Similarly,

$$V_z = V_z(1 - i\gamma), \quad (4.22)$$

for small γ such that $\gamma^2 \rightarrow 0$. The size of β and γ are ordinarily between .01 and .001 of the K_x and of the K_z , respectively so that the above assumption is valid.

C. Energy Flux in Terms of the Wave-Number and the Amplitude

Let us rewrite Equation 4.5 and integrate it through a period as in Equation 4.2; the result is

$$\bar{E} = \frac{\omega}{2\pi} \int_0^{2\pi} E \, dt = \frac{\omega}{2\pi} \int_0^{2\pi} \left(\frac{\partial}{\partial t} \hat{U} \right)^T \cdot \hat{\sigma} \, dt \quad (4.23)$$

Since most of this study is concerned with the energy and its balance, the analysis of the derivation of the energy relationship seems essential. Thus, we study Equation 4.23 in detail. Consider the dot product $\hat{U}^T \cdot \hat{\sigma}$ which is the energy density normal to the z plane; the energy through that plane is

$$\int_0^h \hat{U}^T \cdot \hat{\sigma} \, dz = \int_0^h \hat{U}^T \cdot \underline{D}_v \hat{\epsilon} \, dz \quad (4.24)$$

where

$$\underline{D}_v = \begin{bmatrix} \lambda + 2\mu & \lambda & 0 \\ 0 & 0 & \mu \end{bmatrix} \quad (4.25)$$

and

$$\hat{\epsilon} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial x} \end{bmatrix} (U_x, U_z) \quad (4.25a)$$

Decomposing the matrix in Equation 4.25 as

$$\begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial x} \end{bmatrix} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & 0 \\ 0 & \frac{\partial}{\partial x} \end{bmatrix} + \begin{bmatrix} 0 & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & 0 \end{bmatrix} \quad (4.26)$$

and substituting this and Equation 4.25 into Equation 4.24 gives

$$\begin{aligned} \int_0^h \hat{U}^T \cdot \underline{D_V} \hat{\epsilon} dz &= \int_0^h \hat{U}^T \cdot \left(\begin{bmatrix} \lambda+2\mu & \lambda & 0 \\ 0 & 0 & \mu \end{bmatrix} \begin{bmatrix} 0 & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & 0 \end{bmatrix} \right. \\ &\quad \left. + \begin{bmatrix} \lambda+2\mu & \lambda & 0 \\ 0 & 0 & \mu \end{bmatrix} \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & 0 \\ 0 & \frac{\partial}{\partial x} \end{bmatrix} \right) \hat{U} dz \\ &= \int_0^h \hat{U}^T \left(\begin{bmatrix} 0 & \lambda \frac{\partial}{\partial z} \\ \mu \frac{\partial}{\partial z} & 0 \end{bmatrix} + \begin{bmatrix} (\lambda+2\mu)(-1K) & 0 \\ 0 & -1K\mu \end{bmatrix} \right) \hat{U} dz \\ \text{or} \\ \int_0^h \hat{U}^T \cdot \underline{D_V} \hat{\epsilon} dz &= \int_0^h \hat{U}^T \left(\begin{bmatrix} 0 & \lambda \\ \mu & 0 \end{bmatrix} \frac{\partial}{\partial z} - 1K \begin{bmatrix} \lambda+2\mu & 0 \\ 0 & \mu \end{bmatrix} \right) \hat{U} dz. \quad (4.27) \end{aligned}$$

Substituting Equations 3.38, 3.40 and 3.42 into Equation 4.27 we

have

$$\int_0^h \hat{U}^T \cdot \underline{D}_V \hat{\epsilon} dz = \int_0^h \hat{V}^T \cdot \underline{N}^T \underline{C} \frac{\partial}{\partial z} \underline{N} \hat{V} dz - iK \int_0^h \hat{V}^T \cdot \underline{N}^T \underline{a} \underline{N} \hat{V} dz \quad (4.28)$$

where

$$\underline{C} = \begin{bmatrix} 0 & \lambda \\ \mu & 0 \end{bmatrix}, \quad \underline{a} = \begin{bmatrix} \lambda+2\mu & 0 \\ 0 & \mu \end{bmatrix} \quad (4.29)$$

From Equation 3.42, if we substitute

$$\frac{\partial}{\partial z} \underline{N} = \frac{1}{h/2} \frac{\partial}{\partial \mu} \quad (4.30)$$

and

$$\frac{\partial}{\partial z} \underline{N} = \frac{1}{h} \begin{bmatrix} -1 & 0 & 1 & 0 \\ 0 & -1 & 0 & 1 \end{bmatrix} \quad (4.31)$$

into Equation 4.28 and changing the variable of integration we have

$$\begin{aligned} \int_0^h \hat{U}^T \cdot \underline{D}_V \hat{\epsilon} dz &= \int_{-1}^1 \hat{V}^T 1/2 \begin{bmatrix} 1-n & 0 \\ 0 & 1-n \\ 1+n & 0 \\ 0 & 1+n \end{bmatrix} \underline{C} \frac{1}{h} \begin{bmatrix} -1 & 0 & 1 & 0 \\ 0 & -1 & 0 & 1 \end{bmatrix} \hat{V} \frac{h}{2} dn \\ &- iK \int_{-1}^1 \hat{V}^T 1/2 \begin{bmatrix} 1-n & 0 \\ 0 & 1-n \\ 1+n & 0 \\ 0 & 1+n \end{bmatrix} \underline{a} \frac{1}{2} \begin{bmatrix} 1-n & 0 & 1+n & 0 \\ 0 & 1-n & 0 & 1+n \end{bmatrix} \hat{V} \frac{h}{2} dn \end{aligned} \quad (4.32)$$

or

$$\int_0^h \hat{U}^T \cdot \underline{D}_V \hat{\epsilon} dz = \int_{-1}^1 \hat{V}^T \cdot \frac{1}{4} \underline{R} \hat{V} dn - iK \int_{-1}^1 \hat{V}^T \frac{1}{8} h \underline{s} \hat{V} dn \quad (4.33)$$

where

$$\underline{R} = \begin{bmatrix} 0 & -\lambda(1-n) & 0 & \lambda(1-n) \\ -\mu(1-n) & 0 & \mu(1-n) & 0 \\ 0 & -\lambda(1+n) & 0 & \lambda(1+n) \\ -\mu(1+n) & 0 & \mu(1+n) & 0 \end{bmatrix} \quad (4.34)$$

and

$$\underline{s} = \begin{bmatrix} (1-n)^2(\lambda+2\mu) & 0 & (1-n)^2(\lambda+2\mu) & 0 \\ 0 & (1-n)^2\mu & 0 & (1-n)^2\mu \\ (1-n)^2(\lambda+2\mu) & 0 & (1+n)^2(\lambda+2\mu) & 0 \\ 0 & (1-n)^2\mu & 0 & (1+n)^2\mu \end{bmatrix} \quad (4.35)$$

Integration of Equation 4.33 yields

$$\int_0^h \hat{U}^T \cdot \underline{D}_V \hat{\epsilon} dz = \hat{V}^T \cdot \underline{C}_j \hat{V} + iK \hat{V}^T \cdot \underline{A}_j \hat{V} = \hat{V}^T \cdot (\underline{C}_j + iK \underline{A}_j) \hat{V} \quad (4.36)$$

for the j^{th} layer. Where

$$\underline{C}_j = \frac{1}{2} \begin{bmatrix} 0 & \lambda & 0 & -\lambda \\ \mu & 0 & -\mu & 0 \\ 0 & \lambda & 0 & -\lambda \\ \mu & 0 & -\mu & 0 \end{bmatrix}, \text{ and } \underline{A}_j = \frac{h_j}{6} \begin{bmatrix} 2(\lambda+2\mu) & 0 & \lambda+2\mu & 0 \\ 0 & 2\mu & 0 & \mu \\ \lambda+2\mu & 0 & 2(\lambda+2\mu) & 0 \\ 0 & \mu & 0 & 2\mu \end{bmatrix} \quad (4.37)$$

Summing up Equation 4.36 over all layers results in

$$\sum_{\text{Layers}} \int_0^h \hat{U}^T \cdot \underline{D}_V \hat{\epsilon} dz = \sum_{j=1}^{2N} \hat{V}^T \cdot (\underline{C}_j + iK \underline{A}_j) \hat{V} = \hat{V}^T \cdot (\underline{C} + iK \underline{A}) \hat{V} \quad (4.38)$$

Recall that

$$\hat{U} = \hat{U}(z)e^{i(\omega t - Kx)}, \text{ and } \hat{U} = \underline{N} \hat{V} \quad (4.39)$$

therefore,

$$\hat{V} = \hat{V}e^{i(\omega t - Kx)} \text{ and } \frac{\partial}{\partial t} \hat{V} = i\omega \hat{V}. \quad (4.40)$$

Substituting Equations 4.38 and 4.40 into Equation 4.23, with a realistic definition, the average energy which is carried across the vertical boundary, by the mode K , is given by

$$\hat{E} = \frac{\omega}{2\pi} \int_0^{2\pi} R(i\omega \hat{V})^T \cdot (\underline{C} + iK\underline{A}) R(\hat{V}) dt \quad (4.41)$$

when matrices $\underline{A}$ and $\underline{C}$ are real, and by

$$E = \frac{\omega}{2\pi} \int_0^{2\pi} R(i\omega \hat{V})^T \cdot (R(\underline{C}) + iKR(\underline{A}))R(\hat{V}) dt \quad (4.42)$$

when matrices $\underline{A}$ and $\underline{C}$ are complex, where R represents the real part.

Now let

$$\hat{V} = (\hat{V}_1 + i\hat{V}_2)e^{i\omega t} = (\hat{V}_1 + i\hat{V}_2)(\cos \omega t + i \sin \omega t) \quad (4.43)$$

Then substituting

$$R(\hat{V}) = \hat{V}_1 \cos \omega t - \hat{V}_2 \sin \omega t,$$

$$R(i\omega \hat{V}) = -\omega(\hat{V}_1 \sin \omega t + \hat{V}_2 \cos \omega t) \quad (4.44)$$

$$\begin{aligned} R(i\omega \hat{V})^T \cdot R(\hat{V}) = & -\omega \hat{V}_1^T \cdot \hat{V}_1 \sin \omega t \cos \omega t - \hat{V}_1^T \cdot \hat{V}_2 \cos^2 \omega t + \\ & \omega \hat{V}_1^T \cdot \hat{V}_2 \sin^2 \omega t + \hat{V}_2^T \cdot \hat{V}_2 \sin \omega t \cos \omega t \end{aligned} \quad (4.45)$$

and

$$\frac{\omega}{2\pi} \int_0^{2\pi} \sin \omega t \cos \omega t dt = 0, \quad \frac{\omega}{2\pi} \int_0^{2\pi} \sin^2 \omega t dt = \frac{\omega}{2\pi} \int_0^{2\pi} \cos^2 \omega t dt = 1/2 \quad (4.46)$$

into Equation 4.41 yields,

$$\hat{E} = \frac{\omega}{2} (\hat{V}_1^T \cdot (\underline{C} + iK\underline{A})\hat{V}_2 - \hat{V}_2^T \cdot (\underline{C} + iK\underline{A})\hat{V}_1), \quad (4.47)$$

where $\hat{V}_1$ and $\hat{V}_2$ are the real and imaginary of the vector $\hat{V}$.

Equation 4.47 expresses the energy carried by the mode K. If the incident energy of the Rayleigh wave across the vertical boundary is in the fundamental mode K, given by Equation 4.47, then the reflected and transmitted energies must be calculated through the modes which are produced by the incident energy. Therefore, if L is the number of different modes produced by the incident energy then the reflected and transmitted energies are given by

$$E_R = \sum_{s=1}^L \frac{\omega}{2} (\hat{V}_{1s}^T \cdot (\underline{C} - iK_s\underline{A})\hat{V}_{2s} - \hat{V}_{2s}^T \cdot (\underline{C} - iK_s\underline{A})\hat{V}_{1s}) \quad (4.48)$$

and

$$E_T = \sum_{s=1}^L \frac{\omega}{2} (\hat{V}_{1s}^T \cdot (\underline{C} + iK_s\underline{A})\hat{V}_{2s} - \hat{V}_{2s}^T \cdot (\underline{C} + iK_s\underline{A})\hat{V}_{1s}), \quad (4.49)$$

respectively.

D. Attenuation

If we substitute the effective moduli of Equation 2.17 into the energy relation Equation 4.47 it produces an additional term called the power loss or dissipated power in the system. This power loss is due to the viscoelastic part of Equation 2.17, and is given by

$$P_D = \frac{\omega}{2} (\hat{V}_1^T \cdot (IM(\underline{C}) + iKIM(\underline{A}))\hat{V}_2 - \hat{V}_2 \cdot (IM(\underline{C}) + iKIM(\underline{A}))\hat{V}_1), \quad (4.48)$$

where IM represents the imaginary part or the viscoelastic part of the matrices $\underline{C}$ and $\underline{A}$. The ratio of the average power loss, Equation 4.48, to the average rate of energy transport, Equation 4.47, is called the attenuation and is denoted by a dimensionless parameter $2\pi Q^{-1}$; thus,

$$2\pi Q^{-1} = \frac{\text{average dissipated power}}{\text{rate of energy transport}} = \frac{P_D}{E}. \quad (4.49)$$

If the complex wavenumber

$$K = K_R + iK_I \quad (4.50)$$

is given, or for a given period, is found from the solution of the eigenvalue problem for the layered region, then the attenuation is given by

$$Q^{-1} = \frac{2K_I}{K_R}. \quad (4.51)$$

Equation 4.49 is general; it can be used for spatial attenuation, i.e., attenuation arising from discontinuities along the path of the wave, or for temporal attenuation, i.e., attenuation arising from time, temperature or other viscoelastic behavior of the medium.

E. Change of Wavenumber at the Vertical Boundary

The existence of reflected energy at the vertical boundary suggests that the phase velocity at the left and the right hand sides of the boundary are different. Since the wave is harmonic then the change in velocity is due to the change in wavenumber. This change must be calculated by means other than from Equation 3.81 whose solution gives the displacements at the vertical boundary.

Auld (7, 8) has used a perturbation theory and has derived a relation to calculate the change in the wavenumber due to various mechanisms. Auld's relation may be revised to be used in seismic waves; the form suitable here may be expressed as

$$\delta K = \omega \frac{\int_z \epsilon^T \cdot \Delta D \hat{\epsilon} - \Delta \rho \omega^2 \hat{U}^T \cdot \hat{U} dz}{4E} \quad (4.52)$$

where

$$D = \begin{bmatrix} \lambda+2\mu & \lambda & 0 \\ \lambda & \lambda+2\mu & 0 \\ 0 & 0 & \mu \end{bmatrix}, \quad (4.53)$$

$$\hat{\epsilon} = \left(\frac{\partial U_x}{\partial x}, \frac{\partial U_z}{\partial z}, \frac{\partial U_z}{\partial x} + \frac{\partial U_x}{\partial z} \right), \quad (4.54)$$

$$\hat{U} = \hat{U}(z) e^{i(\omega t - Kx)}, \quad (4.55)$$

E is given by Equation 4.49 and Δ denotes the contrast between the two different media. Use of Equations 3.33 and 3.34 reduces the numerator of Equation 4.52 to

$$L = \int_z \left(\begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial x} \end{bmatrix} \hat{U} \right)^T \cdot \Delta D \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial x} \end{bmatrix} \hat{U} - \Delta \rho \omega^2 \hat{U}^T \cdot \hat{U} dz. \quad (4.56)$$

Decomposition of the operating matrix into two matrices, depending on x or z only, gives

$$\begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & \frac{\partial}{\partial x} \end{bmatrix} = \begin{bmatrix} \frac{\partial}{\partial x} & 0 \\ 0 & 0 \\ 0 & \frac{\partial}{\partial x} \end{bmatrix} + \begin{bmatrix} 0 & 0 \\ 0 & \frac{\partial}{\partial z} \\ \frac{\partial}{\partial z} & 0 \end{bmatrix} \quad (4.57)$$

and operating on the matrix with the partial derivative with respect to x on the corresponding vectors results in

$$L = \int_z ((\nabla z - iKI)\hat{U})^T \cdot \Delta D (\nabla z - iKI)\hat{U} - \rho \omega^2 \hat{U}^T \cdot \hat{U} dz, \quad (4.58)$$

where matrix ∇z is given by the second term of Equation 4.57, and

$$I = \begin{bmatrix} 1 & 0 \\ 0 & 0 \\ 0 & 1 \end{bmatrix}.$$

Again, by using

$$\hat{U} = \underline{N} \hat{V} \quad (4.59)$$

$$\underline{N} = \frac{1}{2} \begin{bmatrix} 1-n & 0 & 1+n & 0 \\ 0 & 1-n & 0 & 1+n \end{bmatrix}, \quad -1 \leq n \leq 1 \quad (4.60)$$

$$n = \frac{z}{h/2}, \quad \frac{\partial}{\partial z} = \frac{1}{h/2} \frac{\partial}{\partial n} \quad 0 \leq z \leq h \quad (4.61)$$

$$\frac{\partial}{\partial z} \underline{N} = \frac{1}{h} \begin{bmatrix} -1 & 0 & 1 & 0 \\ 0 & -1 & 0 & 1 \end{bmatrix}, \quad (4.62)$$

Equation 4.58 becomes (for the moment we set $\Delta D = D$, $\Delta \rho = \rho$)

$$\begin{aligned}
L = \int_{-1}^1 (\hat{V}^T \cdot \left(\frac{1}{h} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 1 \\ -1 & 0 & 1 & 0 \end{bmatrix} - iK/2 \begin{bmatrix} 1-\eta & 0 & 1+\eta & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 1-\eta & 0 & 1+\eta \end{bmatrix} \right)^T \underline{D} \\
\left(\frac{1}{h} \begin{bmatrix} 0 & 0 & 0 & 0 \\ 0 & -1 & 0 & 1 \\ -1 & 0 & 1 & 0 \end{bmatrix} - iK/2 \begin{bmatrix} 1-\eta & 0 & 1+\eta & 0 \\ 0 & 0 & 0 & 0 \\ 0 & 1-\eta & 0 & 1+\eta \end{bmatrix} \right) \hat{V} \\
- \rho \frac{\omega^2}{4} \hat{V}^T \cdot \begin{bmatrix} (1-\eta)^2 & 0 & 1-\eta^2 & 0 \\ 0 & (1-\eta)^2 & 0 & 1-\eta^2 \\ 1-\eta^2 & 0 & (1+\eta)^2 & 0 \\ 0 & 1-\eta^2 & 0 & (1+\eta)^2 \end{bmatrix} \hat{V} h/2 d\eta. \quad (4.63)
\end{aligned}$$

After performing the transpose and corresponding multiplications,

Equation 4.63 becomes

$$\begin{aligned}
L = \int_{-1}^1 (\hat{V}^T \cdot \left(\frac{1}{h^2} \begin{bmatrix} \mu & 0 & -\mu & 0 \\ 0 & \lambda+2\mu & 0 & -(\lambda+2\mu) \\ -\mu & 0 & \mu & 0 \\ 0 & -(\lambda+2\mu) & 0 & \lambda+2\mu \end{bmatrix} \right. \\
\left. - i \frac{K}{2h} \begin{bmatrix} 0 & -\mu(1-\eta) & 0 & -\mu(1+\eta) \\ -\lambda(1-\eta) & 0 & -\lambda(1+\eta) & 0 \\ 0 & \mu(1-\eta) & 0 & \mu(1+\eta) \\ \lambda(1-\eta) & 0 & \lambda(1+\eta) & 0 \end{bmatrix} \right) \hat{V} h/2 d\eta.
\end{aligned}$$

$$\begin{aligned}
& + i \frac{K}{2h} \begin{bmatrix} 0 & -\lambda(1-\eta) & 0 & \lambda(1-\eta) \\ -\mu(1-\eta) & 0 & \mu(1-\eta) & 0 \\ 0 & -\lambda(1+\eta) & 0 & \lambda(1+\eta) \\ -\mu(1+\eta) & 0 & \mu(1+\eta) & 0 \end{bmatrix} \\
& + \frac{K^2}{4} \begin{bmatrix} (\lambda+2\mu)(1-\eta)^2 & 0 & (\lambda+2\mu)(1-\eta)^2 & 0 \\ 0 & \mu(1-\eta)^2 & 0 & \mu(1-\eta)^2 \\ (\lambda+2\mu)(1-\eta)^2 & 0 & (\lambda+2\mu)(1+\eta)^2 & 0 \\ 0 & \mu(1-\eta)^2 & 0 & \mu(1+\eta)^2 \end{bmatrix} \hat{V} \\
& - \rho \frac{\omega^2}{4} \hat{V}^T \cdot \begin{bmatrix} (1-\eta)^2 & 0 & 1-\eta^2 & 0 \\ 0 & (1-\eta)^2 & 0 & 1-\eta^2 \\ 1-\eta^2 & 0 & (1+\eta)^2 & 0 \\ 0 & 1-\eta^2 & 0 & (1+\eta)^2 \end{bmatrix} \hat{V} h/2 d\eta \quad (4.64)
\end{aligned}$$

Now, the integrations of

$$\int_{-1}^1 (1-\eta) d\eta = \int_{-1}^1 (1+\eta) d\eta = 2, \quad \int_{-1}^1 (1-\eta)^2 d\eta = \int_{-1}^1 (1+\eta)^2 d\eta = \frac{8}{3}, \quad (4.65)$$

and

$$\int_{-1}^1 (1-\eta^2) d\eta = \frac{4}{3},$$

reduces Equation 4.64 into

$$L = \hat{V}^T \cdot (1/2 \underline{G}_j + iK\underline{F}_j + K^2\underline{A}_j - \omega^2\underline{M}_j) \hat{V} \quad (4.66)$$

where matrices $\underline{G}_j$, $\underline{A}_j$, and $\underline{M}_j$ are given by Equations 3.48 - 3.50 and

$$\underline{F}_j = \frac{1}{2} \begin{bmatrix} 0 & -(\lambda-\mu) & 0 & (\lambda+\mu) \\ \lambda-\mu & 0 & \lambda+\mu & 0 \\ 0 & -(\lambda+\mu) & 0 & \lambda-\mu \\ -(\lambda+\mu) & 0 & -(\lambda-\mu) & 0 \end{bmatrix} \cdot \quad (4.67)$$

Since j represents a typical layer, the sum of Equation 4.66 over all layers, when substituted into Equation 4.52 gives, considering now ΔD and ΔP ,

$$\delta K = \omega \frac{\hat{V}^T \cdot (1/2 \Delta \underline{G} + iK \Delta \underline{F} + K^2 \Delta \underline{A} - \omega^2 \Delta \underline{M}) \hat{V}}{4E} \quad (4.68)$$

When calculated, this δK may be added to the wave number of the left hand side of the boundary; the phase velocity of the right hand side of the boundary can be calculated by the expression,

$$V_x = \frac{\omega}{K + \delta K} \quad (4.69)$$

For different periods the dispersion curve of the desired medium can be computed.

Equation 4.68 may also be used for the determination of attenuation.

For this purpose if

$$\delta K = \delta K_R + i\delta K_I \quad (4.70)$$

then

$$\delta K_I = \omega \frac{\hat{V}^T (1/2 \Delta \underline{G}_I + iK \Delta \underline{F}_I + K^2 \Delta \underline{A}_I) \hat{V}}{4E} \quad (4.71)$$

and the attenuation is given by

$$Q^{-1} = \frac{2\delta K_I}{\delta K} \quad (4.72)$$

where I stands for the imaginary part. Since attenuations due to various mechanisms are additive, Equation 4.72 may be added to the attenuation obtained due to viscoelastic behavior.

CHAPTER V

APPLICATION OF THE METHOD

A. A Model

To apply a model to the theories discussed in the last chapters, we chose Herrin's models (49) of the Canadian Shield and Basin and Range provinces for the elastic part of the medium. The Basin and Range province is used in the left layered region and the Canadian Shield is used in the right layered region; the order is reversible. For the irregular region we chose different combinations of the Basin and Range province and the Canadian Shield. The data given in Herrin's model are thicknesses, densities, compressional velocities, and shear velocities of the two layered media. To account for the anelasticity in the model a complex part for the compressional velocities and shear velocities may be assumed. Since the elastic moduli are obtained by using the expressions

$$\lambda = \rho(V_p^2 - 2V_s^2) \quad (5.1)$$

$$\mu = \rho V_s^2, \quad (5.2)$$

where, λ and μ are the elastic moduli of the medium, and V_p and V_s are the real parts of compressional and shear velocities, respectively, then the effective moduli, or complex moduli, as given in Equation 2.17, may be obtained by

$$\lambda_e = \rho(V_p^2 - 2V_s^2) \quad (5.3)$$

$$\mu_e = \rho V_s^2 \quad (5.4)$$

for complex velocities. For this reason, in our model, any viscoelastic behavior of the medium may be incorporated in the imaginary parts of the compressional and shear velocities. Thus, for the viscoelastic part of the medium in this paper, three different sets of data are used: A small enough (.1 percent of the real part) imaginary parts for compressional and shear velocities; a starting constant shear quality factor, Q_β , for the Basin and Range province and a starting constant Q_β for the Canadian Shield; and finally the starting Q_β model of Lee and Solomon (69a), whose Q_β of Western U. S. and East-Central U. S. are used in Basin and Range provinces and Canadian Shield, respectively.

When Q_β is given, the imaginary parts of V_p and V_s are determined by the following expressions. From

$$Q_\beta = \frac{\text{REAL}(V_s)}{2 \text{IMAG}(V_s)} \quad (5.5)$$

$$V_p = \text{REAL}(V_p) + i \text{IMAG}(V_p), \quad (5.6)$$

$$V_s = \text{REAL}(V_s) + i \text{IMAG}(V_s), \quad (5.7)$$

$$\text{IMAG}(V_s) = \frac{\text{REAL}(V_s)}{2Q_\beta}. \quad (5.8)$$

And from (69a),

$$Q_\alpha = \frac{3}{4} \left(\frac{V_p}{V_s} \right)^2 Q_\beta, \quad (5.9)$$

we have

$$\text{IMAG}(V_p) = \frac{\text{REAL}(V_p)}{2Q_\alpha}. \quad (5.10)$$

Both regions of the model are assumed each to contain 15 layers with a total depth of 250 kilometers; this depth includes the crust and upper mantle so that any partial melting zone in the upper mantle may be studied. The irregular region I is divided into 15 by 15 network of elements whose total number of elements and total number of nodal points of the elements are N^2 and $N(N + 1)$, respectively, where N is the number of layers. The slightly adjusted Herrin model for the above geometry is given in Tables 1 and 2. The integer N in Table 2 represents the number of elements, in the finite element region, that have the same property. The numbers assigned to the integer N in Tables 2 and 2A are completely arbitrary.

B. Real Data

A set of periods from 9 to 60 seconds along with the data of Table 1, may be used as an input for the solution of the eigenvalue problem, Equation 3.52, in the layered regions. The fundamental solution of the eigenvalue problem corresponding to each period is the largest eigenvalue or the largest wavenumber selected from the $2N$ eigenvalues. The phase velocity corresponding to each period is given by

$$\text{PHASE VELOCITY} = \frac{\text{OMEGA}}{\text{WAVENUMBER}}, \quad (5.11)$$

for the given period and corresponding frequency.

The computer program RALEE used to calculate the unknown of the medium is the revised form of the program RALEEM written by Drake (29) in 1972. To test the program RALEE we used Lysmer and Drake's (80) data of the Sierra Nevada and calculated the phase velocities of Rayleigh waves for a range of periods of 6 to 10 seconds and the horizontal and vertical amplitudes for a period of 6 seconds. The agreement is excellent for the phase velocities and good for the amplitudes; but the small difference in the amplitudes may be due to scale arbitrariness. The phase velocities are given in Table 16 and the amplitudes are shown in Figure 16.

The wavenumbers and the phase velocities calculated for the two layered systems of the Basin and Range province and the Canadian shields with the data of Table 1 and the periods of 9 to 60 seconds are also given in Tables 3 and 4, and the corresponding dispersion curves are shown in Figures 7 and 8.

To test the effect of layering we reduced the number of layers from 15 to 3 and assumed two layers for the crust and a deep layer for the upper mantle. The computed Rayleigh wave phase velocities are somewhat higher than those of the 15 layered case. The difference may be due somewhat to the averaging data of 15 layers into 3 layers but it is mostly due to the large dimensions of the elements as compared to the wavelength of the shear wave (104).

To test the computer program RALEE for the irregular zone I in the real case, we first assumed the three zones L, I and R of Figure 5 to have the same properties as the Basin and Range provinces and calculated the reflected and transmitted energies. For the period range of

10 to 80 seconds the sum of the reflected and transmitted energies is very close to incident energy. These data are shown in Table 5.

Second, we set the boundary between the Basin and Range province and the Canadian Shield at the right hand side of the zone L, i.e., we let the zone L be the Basin and Range province and zones I and R be the Canadian Shield. Third, we set the boundary at the left hand side of the zone R, i.e., we let the zones L and I be the Basin and Range province and zone R be the Canadian Shield. Fourth, we set the boundary at the middle of zone I, i.e., we let the zone L and one-half of the zone I be the Basin and Range province and one-half of the zone I and zone R be the Canadian shield. Finally, we let the zones L and R be the Basin and Range province and the Canadian shield, respectively, but gradually changed the parameters of the zone I from the parameters of the Basin and Range province into the parameters of the Canadian shield. The sum of the reflected and transmitted energies determined for each of the above models in the real case, was found to be almost equal to the incident energy. The transmitted energy in all of the above models, in the real case, is more than 99% except for a few period of 18 to 25, in the second case, in which for each period the reflected energy is the highest, i.e., from .05% to 2.2%. On the other hand, in the third case, the reflected energy is the smallest, i.e., from .007% to .06%. Therefore, the decrease in reflected energy is from left to right with respect to the position of the vertical boundary; nevertheless, the process is reversible. The values for the above cases are given in Tables 6, 6A, 6B, and 6C, respectively.

For the determination of the phase velocity in the irregular region, in the real case, the boundary displacements, determined by Equation 3.81, are used in Equation 4.68; the left boundary displacements are replaced by the vector $\hat{V}^T$ and the right boundary displacements are replaced by the vector $\hat{V}$ in Equation 4.68. Substitution of the resulting δK into Equation 4.69 determines the fundamental mode phase velocity. Repetition of the use of Equations 4.68 and 4.69, corresponding to the number of distinct periods gives the phase velocities of the irregular region for each case mentioned in the previous paragraph. The dispersion curves for each case are shown in Figures 9, 9A, 9B, 9C, and 9D, respectively.

C. Complex Data

The geometry of the model remains the same in the viscoelastic case. Therefore, the real parts of the complex V_p and V_s velocities do not change in the complex case. Since thicknesses, densities, and periods are assumed to be real in the elastic and viscoelastic medium, they will not be repeated again in this section. Thus, for the complex case of the model we assigned three different sets of data to the imaginary parts of the complex V_p and V_s velocities. First, the imaginary parts of the V_p and V_x were assumed to be .1 percent of their corresponding real parts. Second, a constant shear quality factor for each zone of L and R was assumed and the corresponding compressional quality factor Q_α and the imaginary parts of V_p and V_s were calculated. Third, we used the Q_β model of Lee and Solomon (69a) and calculated the corresponding compressional quality factor Q_α and the imaginary

parts of V_p and V_s . The imaginary parts of V_p and V_s for the three cases are shown in Tables 7, 8, and 9.

For the same number of periods that was used in the real case, the values, given in Tables 7, 8, and 9 are used as the input for the solution of complex eigenvalue problem, Equation 3.52, in the layered regions. The fundamental solution of the complex eigenvalue problem corresponding to each period is the largest eigenvalue with a negative imaginary part, selected from 2N eigenvalues. The phase velocity and the attenuation corresponding to each period are given by

$$\text{PHASE VELOCITY} = \frac{\text{OMEGA}}{\text{REAL (WAVENUMBER)}} \quad (5.11a)$$

and

$$\text{ATTENUATION} = \frac{2 \times \text{IMAG (WAVENUMBER)}}{\text{REAL (WAVENUMBER)}}, \quad (5.12a)$$

respectively. The real and imaginary parts of wavenumbers, the phase velocities and the attenuations, corresponding to Tables 7, 8, and 9 are given in Tables 10 - 15 and the corresponding attenuation curves and the dispersion curves are shown in Figures 10 - 15.

For the complex part of the irregular region I we used the imaginary parts for V_p and V_s velocities, obtained from the Q_b of Lee and Solomon (69a), in all cases described for the real case in section B. In each case the sum of the reflected and transmitted energies is considerably less than the incident energy; but the difference is remarkably in accordance with the given Q_b value with respect to the depth where the energy is computed. The attenuated percentage of reflected and transmitted energies are given in Tables 5, 6, 6A, 6B, and 6C.

For consistency of the result, we asked H. Mack (80a) to input our complex data in his computer program VERES. He calculated the transmitted Rayleigh wave energy for the case where the medium is considered to have the parameters of Basin and Range province. His result agrees well with higher periods, fluctuates somewhat with the intermediate periods, and agrees with periods of 13 - 15 seconds as shown in Table 5. For periods lower than 13 seconds his program is not applicable.

The determination of the phase velocity of the irregular region, in the complex case, follows the same procedure of the last subsection; however, since the change in the phase velocities between the real and the complex cases are very small the dispersion curves would look the same as the ones in the real case. Thus, the repetition is not necessary.

Finally, for a check on the result we used the data of Lee and Solomon (69a, pages 77 and 83) in our 15 layered systems and computed the phase velocities and the attenuations for East-Central and Western North America. The result is compared well with their result which is obtained by inversion. The phase velocities and the attenuations are given in Tables 17 and 18 and the corresponding dispersion and attenuation curves are shown in Figures 17, 18, 19, and 20.

CHAPTER VI

SUMMARY AND CONCLUSION

The anomalous decay of surface wave amplitude may be due to viscoelasticity and geometrical discontinuities of the medium. In this paper the path of a Rayleigh wave was assumed to be through the Basin and Range provinces and the Canadian Shield. The two regions were treated as elastic and inelastic; and the surface of intersection between them was considered to be an approximation to a plane vertical boundary.

With the plane vertical boundary removed, the two regions, as a layered system, were studied as separate elastic and viscoelastic media. The Rayleigh wave phase velocities, amplitudes, and the attenuations of the two regions, with the technique of finite element, were computed for a range of periods. The change in phase velocities, in each region due to viscoelasticity in three different cases of data, were from .01 percent to 0.5 percent of elastic phase velocities, an almost negligible amount. Consequently, the dispersion curves, with the range of periods of 9 - 60 seconds, are almost similar in elastic and viscoelastic cases in all three zones. On the other hand, the amplitudes and attenuations, in three cases of different viscoelastic parameters in the layered system, are in accordance with the input data and agree well with the very limited published data. Though small, the difference in amplitude and attenuation due to viscoelasticity appear to be significant and is

in accordance with the amount of imaginary parts of V_p and V_s velocities with respect to the depth of the layers. The result of the use of the model of Lee and Solomon (69a) also confirms the above result, in addition to being in excellent agreement with their result.

The reflected and transmitted energies through the vertical boundary are functions of position of the vertical boundary with respect to the two media of L and R, in the real case. They are in addition to the above condition, functions of attenuation or the Q_β as well, in the viscoelastic case. The reflection of energy, although small in all cases, decreases as the vertical boundary, in our model, moves from left to right; but the process is completely reversible. The highest reflection of energy (.04% to 2.2%) occurs where the vertical boundary is at the right hand side of the zone L so that the zones I and R both have the parameters of the Canadian shield and the zone L has the Basin and Range parameters. The one position of the vertical boundary where the reflected energy has the intermediate value (.04% to .5%) is in the middle of the zone I; the reflected energy in the real and complex cases are almost equal; the sum of the reflected and transmitted energies, in the real case are almost exactly equal to the incident energy and the transmitted energy, in the complex case is the highest.

Due to the fact that the reflected energy, in all of the models tested, is very small, it may be concluded that for a normal incidence of Rayleigh wave at a vertical boundary between the Continental provinces the mode conversion is too small to measure in the real crust. Due to the computer core limitation the number of sublayers and the size of the elements may be somewhat large compared to the wavelength of the shear wave.

TABLE 1

HERRIN'S MODEL OF BASIN AND RANGE PROVINCES
AND CANADIAN SHIELD, SLIGHTLY ADJUSTED TO
FIT THE FIFTEEN LAYERED MODEL

Basin and Range				Canadian Shield			
Depth (KM)	Density (G/CM ³)	V _p (KM/SEC)	V _s (KM/SEC)	Depth (KM)	Density (G/CM ³)	V _p (KM/SEC)	V _s (KM/SEC)
0-14	2.640	6.000	3.540	0-14	2.610	5.800	3.470
14-19	2.640	6.000	3.560	14-19	2.620	6.700	3.740
19-20	2.640	6.700	3.600	19-20	2.620	6.700	3.740
20-28	2.640	6.695	3.595	20-28	2.620	6.700	3.740
28-30	2.640	6.650	3.550	28-30	2.620	6.700	3.740
30-34	2.900	7.900	4.430	30-34	2.620	7.000	3.820
34-50	2.900	7.900	4.425	34-50	2.900	8.050	4.620
50-60	3.420	7.899	4.420	50-60	2.900	8.050	4.620
60-80	3.400	7.880	4.390	60-80	2.900	8.050	4.620
80-90	3.400	7.860	4.355	80-90	2.920	8.100	4.540
90-100	3.392	7.820	4.320	90-100	3.400	8.400	4.660
100-120	3.382	7.678	4.182	100-120	3.500	8.500	4.500
120-140	3.380	7.730	4.220	120-140	3.500	8.500	4.500
140-150	3.400	7.800	4.270	140-150	3.500	8.500	4.500
150-200	3.400	7.890	4.430	150-200	3.500	8.500	4.500

APPENDIX A

TABLE 2

DATA OF TABLE 1 FOR THE FINITE ELEMENT REGION;
N REPRESENTS THE NUMBER OF ELEMENTS HAVING
THE SAME PROPERTY (REAL CASE)

Depth (KM)	N	Density (KM/CM ³)	N	V _p (KM/SEC)	N	V _s (KM/SEC)	N
		2.64	15	6.00	15	3.54	15
0	14	2.61	1	5.80	1	3.47	1
		2.64	14	6.00	14	3.56	14
14	14	2.62	2	6.70	2	3.74	2
		2.64	13	6.70	13	3.60	13
19	14	2.62	4	6.70	4	3.74	2
		2.64	11	6.69	11	3.59	13
20	14	2.62	6	6.70	6	3.74	4
		2.64	9	6.66	9	3.55	11
38	14	2.62	8	6.70	8	3.74	6
		2.90	7	7.90	7	4.43	9
30	14	2.62	10	7.00	10	3.82	8
		2.90	5	7.90	5	4.42	7
34	14	2.90	12	8.05	12	4.62	10
		3.42	3	7.90	3	4.42	5
50	14	2.90	14	8.05	14	4.62	12
		3.40	1	7.88	1	4.39	3
120	14	2.90	11	8.05	11	4.62	14
		3.40	4	7.86	4	4.36	1
140	14	3.40	13	8.10	13	4.62	11
		2.92	2	7.82	2	4.32	4
150	14	3.50	13	8.40	13	4.62	13
		3.38	2	7.68	2	4.18	2
250	14	3.50	13	8.50	13	4.62	13
		3.38	2	7.73	2	4.22	2
		3.50	13	8.50	13	4.54	13
		3.40	2	7.80	2	4.27	2
		3.50	13	8.13	13	4.66	13
		3.40	2	7.85	2	4.43	2

TABLE 2a

DATA FROM TABLE 2, WITH LEE AND SOLOMON'S DATA, FOR
THE FINITE ELEMENT REGION IN COMPLEX CASE

V_p (KM/SEC)			V_s (KM/SEC)		
REAL (V_p)	IMAG (V_p) ($\times 10^{-2}$)	N	REAL (V_s)	IMAG (V_s) ($\times 10^{-2}$)	N
6.00	0.40	15	3.54	6.00	15
5.80	0.14	1	3.47	0.17	1
6.00	0.40	14	3.56	6.00	14
6.70	0.14	2	3.74	0.18	2
6.70	0.40	13	3.60	6.00	13
6.70	0.14	4	3.74	0.18	2
6.69	0.20	11	3.59	0.23	13
6.70	0.14	6	3.74	0.18	4
6.66	0.20	9	3.55	0.23	11
6.70	0.14	8	3.74	0.18	6
7.90	0.20	7	4.43	0.28	9
7.00	0.14	10	3.82	0.18	8
7.90	0.20	5	4.42	0.28	7
8.05	0.17	12	4.62	0.23	10
7.90	0.20	3	4.42	0.28	15
8.05	0.17	14	4.62	0.23	12
7.88	0.20	1	4.39	0.28	3
8.05	0.17	11	4.62	0.28	14
7.86	9.00	4	4.36	13.00	1
8.10	0.17	13	4.62	0.23	11
7.82	9.00	2	4.32	13.00	4
8.40	0.16	13	4.62	0.23	13
7.68	9.00	2	4.18	12.00	2
8.50	0.16	13	4.62	0.23	13
7.73	9.00	2	4.22	12.00	2
8.50	3.60	13	4.54	0.23	13
7.80	9.00	2	4.27	13.00	2
8.13	3.60	13	4.66	5.40	13
7.85	5.00	2	4.30	7.00	2

TABLE 3

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES
AND CORRESPONDING PHASE VELOCITIES OF THE
LEFT LAYERS (REAL CASE)

Wavenumbers (KM^{-1})			Phase Velocity (KM/SEC)		
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode
0.2009	0.1618	0.1589	3.475	4.315	4.394
0.1806	0.1453	0.1420	3.478	4.323	4.423
0.1497	0.1207	0.1168	3.496	4.337	4.481
0.1179	0.0960	--	3.551	4.363	--
0.0962	0.0792	--	3.627	4.402	--
0.0853	--	--	3.681	--	--
0.0765	--	--	3.732	--	--
0.0662	--	--	3.796	--	--
0.0541	--	--	3.865	--	--
0.0460	--	--	3.902	--	--
0.0400	--	--	3.924	--	--
0.0354	--	--	3.940	--	--
0.0318	--	--	3.955	--	--
0.0262	--	--	3.997	--	--
0.0221	--	--	4.066	--	--

TABLE 4

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES
AND CORRESPONDING PHASE VELOCITIES OF THE
RIGHT LAYERS (REAL CASE)

Wavenumbers (KM^{-1})			Phase Velocity (KM/SEC)		
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode
0.2002	0.1545	0.1533	3.487	4.522	4.553
0.1799	0.1368	0.1321	3.493	4.537	4.590
0.1490	0.1150	--	3.514	4.552	--
0.1174	0.0915	--	3.569	4.577	--
0.0958	0.0757	--	3.645	4.614	--
0.0849	--	--	3.700	--	--
0.0761	--	--	3.755	--	--
0.0656	--	--	3.830	--	--
0.0533	--	--	3.928	--	--
0.0449	--	--	3.996	--	--
0.0388	--	--	4.045	--	--
0.0342	--	--	4.084	--	--
0.0305	--	--	4.120	--	--
0.0250	--	--	4.198	--	--
0.0208	--	--	4.308	--	--

TABLE 5

DISTINCT PERIODS AND PERCENTAGE OF REFLECTED AND TRANSMITTED ENERGIES PASSING THROUGH THE VERTICAL BOUNDARY (THE CASE WHERE ALL THREE ZONES HAVE THE BASIN AND RANGE PROPERTIES) THE LAST TWO COLUMNS ARE THE RESULT OF H. MACK

Period Sec	Real		Complex		H. Mack (Complex)	
	Reflected Energy	Transmitted Energy	Reflected Energy	Transmitted Energy	Period Sec	Transmitted Energy
10	.0011	99.9954	.0073	84.2120		
12	.0054	99.9945	.0112	88.2821	13.8	87.5
15	.0007	99.9994	.0017	92.5926	14.2	94.0
18	.0010	99.9989	.0024	95.3736	15.1	99.4
20	.0005	99.9988	.0018	96.4662	20.5	99.8
22	.0004	100.0000	.0011	97.0742	22.3	96.6
25	.0001	99.9996	.0002	97.2623	25.6	93.6
30	.0001	99.9996	.0055	96.6856	28.4	93.4
35	.0001	99.9996	.0165	96.1306	32.0	94.0
40	.0001	99.9997	.0186	95.8209	39.4	92.0
45	.0001	99.9997	.0155	95.7497	42.7	94.7
50	.0001	100.0000	.0135	95.7523	46.5	96.0
60	.0001	99.9998	.0122	95.8963	51.2	95.3
70	.0001	99.9996	.0223	96.0832		
80	.0001	100.0000	.0001	96.1831		

TABLE 6

DISTINCT PERIODS AND PERCENTAGE OF REFLECTED AND
TRANSMITTED ENERGIES PASSING THROUGH THE
VERTICAL BOUNDARY (THE CASE WHERE THE
VERTICAL BOUNDARY IS AT THE RIGHT
HAND SIDE OF THE ZONE L

Period Sec	Complex		Real	
	Reflected Energy	Transmitted Energy	Reflected Energy	Transmitted Energy
10	.1795	99.1060	.2581	99.7302
12	.0733	99.3311	.0551	99.9465
15	.1209	99.3781	.1396	99.8880
18	2.2780	97.1127	1.9161	98.9499
20	.3447	99.2575	.3268	99.6688
22	1.0353	98.5935	1.0766	98.9225
25	.9698	98.6958	1.0128	98.9857
30	.3175	99.3742	.3549	99.6445
35	.4085	99.2819	.4607	99.5385
40	.1525	99.5212	.1645	99.8983
45	.1209	99.4230	.1318	99.8678
50	.0832	99.4036	.0892	99.9100
60	.0555	99.3044	.0586	99.9413
70	.1309	99.1301	.1081	99.8914
80	.0454	99.0328	.0439	99.9560

TABLE 6A

DISTINCT PERIODS AND PERCENTAGE OF REFLECTED AND
TRANSMITTED ENERGIES PASSING THROUGH THE
VERTICAL BOUNDARY (THE CASE WHERE THE
VERTICAL BOUNDARY IS AT THE LEFT
HAND SIDE OF THE ZONE R)

Period Sec	Real		Complex	
	Reflected Energy	Transmitted Energy	Reflected Energy	Transmitted Energy
10	.0127	99.9840	.0259	84.2332
12	.0071	99.9923	.0123	88.2958
15	.0048	99.9943	.0061	92.5954
18	.0344	99.9659	.0342	95.3359
20	.0145	99.9848	.0137	96.4257
22	.0438	99.9541	.0406	97.0042
25	.0497	99.9490	.0468	97.1677
30	.0344	99.9655	.0417	96.6803
35	.0653	99.9338	.0690	96.1376
40	.0125	99.9872	.0240	95.8200
45	.0109	99.9888	.0206	95.6108
50	.0106	99.9895	.0162	95.6320
60	.0118	99.9879	.0149	95.8032
70	.1074	99.8923	.0241	95.9536
80	.0283	99.9718	.0235	96.1427

TABLE 6B

DISTINCT PERIODS AND PERCENTAGE OF REFLECTED AND
TRANSMITTED ENERGIES PASSING THROUGH THE
VERTICAL BOUNDARY (THE CASE WHERE THE
VERTICAL BOUNDARY IS AT THE
MIDDLE OF THE ZONE I)

Period Sec	Real		Complex	
	Reflected Energy	Transmitted Energy	Reflected Energy	Transmitted Energy
10	.1410	99.8621	.1414	91.7903
12	.0710	99.9305	.0583	93.9755
15	.0537	99.9441	.0373	96.2030
18	.9384	99.0587	.8457	96.3639
20	.1468	99.8515	.1430	98.0320
22	.5724	99.4253	.5377	97.9097
25	.2310	99.7686	.2317	98.3317
30	.1871	99.7836	.1913	98.1774
35	.2673	99.7323	.2732	97.8066
40	.3002	99.7002	.3195	97.5468
45	.2449	99.7547	.2573	97.4167
50	.2027	99.7973	.2126	97.4084
60	.1677	99.8321	.1750	97.6243
70	.4799	99.5201	.4220	97.1232
80	.6412	99.9581	.0414	97.5714

TABLE 6C

DISTINCT PERIODS AND PERCENTAGE OF REFLECTED AND
TRANSMITTED ENERGIES PASSING THROUGH THE
VERTICAL BOUNDARY (THE CASE WHERE THE
POSITION OF THE VERTICAL BOUNDARY
GRADUALLY CHANGES)

Period Sec	Real		Complex	
	Reflected Energy	Transmitted Energy	Reflected Energy	Transmitted Energy
10	.0177	99.9815	.0383	87.122
12	.0119	99.9876	.0165	90.5295
15	.0092	99.9906	.0142	94.0460
18	.0207	99.9767	.0266	96.2619
20	.0073	99.9912	.0080	97.1105
22	.0134	99.9845	.0084	97.5348
25	.0271	99.9707	.0226	97.5493
30	.0278	99.9713	.0114	96.9659
35	.0577	99.9379	.0218	96.3299
40	.0266	99.9728	.0081	95.9103
45	.0140	99.9856	.0065	95.7905
50	.0111	99.9886	.0077	95.7517
60	.0113	99.9879	.0107	95.8399
70	.1399	99.9598	.0582	95.8825
80	.0209	99.9790	.0193	96.0392

TABLE 7

STARTING COMPLEX V_D AND V_S VELOCITIES OF THE
LAYERED SYSTEMS, THE IMAGINARY PART
BEING .1% OF THE REAL PART

Left Layers				Right Layers			
V_D		V_S		V_D		V_S	
REAL	IMAG ($\times 10^{-2}$)	REAL	IMAG ($\times 10^{-2}$)	REAL	IMAG ($\times 10^{-2}$)	REAL	IMAG ($\times 10^{-2}$)
6.000	0.600	3.540	0.354	5.800	0.580	3.470	0.347
6.000	0.600	3.560	0.356	6.700	0.670	3.740	0.374
6.700	0.670	3.600	0.360	6.700	0.670	3.740	0.374
6.695	0.670	3.595	0.360	6.700	0.670	3.740	0.374
6.650	0.665	3.550	0.355	6.700	0.670	3.740	0.374
7.900	0.790	4.430	0.443	7.000	0.700	3.820	0.382
7.900	0.790	4.425	0.443	8.050	0.805	4.620	0.462
7.899	0.790	4.420	0.442	8.050	0.805	4.620	0.462
7.880	0.788	4.390	0.439	8.050	0.805	4.620	0.462
7.860	0.786	4.355	0.435	8.100	0.810	4.540	0.454
7.820	0.782	4.320	0.432	8.400	0.840	4.660	0.466
7.678	0.768	4.182	0.418	8.500	0.850	4.500	0.450
7.730	0.773	4.220	0.422	8.500	0.850	4.500	0.450
7.800	0.780	4.278	0.428	8.500	0.850	4.500	0.450
7.890	0.789	4.430	0.443	8.500	0.850	4.500	0.450

TABLE 8

THE CONSTANT Q_B , AND CORRESPONDING Q_A , AND THE
CORRESPONDING IMAGINARY PARTS OF V_P AND V_S
OF THE LAYERED REGIONS

Left Layers ($Q_B = 400$)			Right Layers ($Q_B = 800$)		
Q_A	IMAG (V_P) ($\times 10^{-2}$)	IMAG (V_S) ($\times 10^{-2}$)	Q_A	IMAG (V_P) ($\times 10^{-2}$)	IMAG (V_S) ($\times 10^{-2}$)
881	0.34	0.40	1676	0.15	0.19
875	0.34	0.40	1925	0.15	0.23
1039	0.32	0.40	1925	0.15	0.23
1040	0.32	0.40	1925	0.15	0.23
1050	0.40	0.40	1925	0.17	0.24
954	0.40	0.40	2014	0.22	0.29
960	0.40	0.40	1820	0.22	0.29
955	0.40	0.40	1820	0.22	0.29
916	0.40	0.40	1820	0.22	0.28
977	0.40	0.40	1909	0.21	0.29
980	0.40	0.40	1950	0.21	0.28
1011	0.40	0.40	2140	0.21	0.28
1006	0.40	0.40	2140	0.21	0.28
997	0.40	0.40	2140	0.21	0.28
951	0.40	0.40	2140	0.21	0.28

TABLE 9

THE Q_B MODEL OF LEE AND SOLOMON AND CORRESPONDING Q_a ,
AND CORRESPONDING IMAGINARY PARTS OF V_p AND V_s
OF THE LAYERED REGIONS

Left Layers				Right Layers			
Q_B	Q_a	IMAG (V_p) ($\times 10^{-2}$)	IMAG (V_s) ($\times 10^{-2}$)	Q_B	Q_a	IMAG (V_p) ($\times 10^{-2}$)	IMAG (V_s) ($\times 10^{-2}$)
312	672	0.40	6.00	1000	2095	0.14	0.17
312	664	0.40	6.00	1000	2400	0.14	0.18
312	810	0.40	6.00	1000	2400	0.14	0.18
770	2000	0.20	0.23	1000	2400	0.14	0.18
770	2026	0.20	0.23	1000	2400	0.14	0.18
770	1836	0.20	0.23	1000	2518	0.14	0.19
770	1836	0.24	0.28	1000	2277	0.17	0.23
770	1836	0.24	0.28	1000	2277	0.17	0.23
770	1825	0.24	0.28	1000	2277	0.17	0.25
17	40	9.00	13.00	1000	2436	0.17	0.23
17	40	9.00	13.00	1000	2675	0.16	0.23
17	42	9.00	12.00	1000	2675	0.16	0.23
17	43	9.00	12.00	1000	2675	0.16	0.23
17	43	9.00	13.00	44	117	3.60	5.00
34	80	5.00	7.00	44	117	3.60	5.00

TABLE 10

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES,
AND CORRESPONDING PHASE VELOCITIES AND ATTENUATIONS
OF THE LEFT LAYERS (THE CASE WHERE THE IMAGINARY
PART OF THE INPUT VELOCITY IS .1%
OF ITS REAL PART)

Wavenumber (KM^{-1})			Phase Velocity (KM/SEC)			Attenuation ($\times 10^{-2}$)
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode
0.2009	0.1618	0.1589	3.475	4.315	4.394	0.199
0.1806	0.1453	0.1421	3.478	4.323	4.423	0.202
0.1497	0.1207	0.1207	3.497	4.337	4.481	0.208
0.1180	0.0960	--	3.551	4.363	--	0.218
0.0962	--	--	3.627	--	--	0.226
0.0853	--	--	3.681	--	--	0.228
0.0765	--	--	3.732	--	--	0.227
0.0662	--	--	3.796	--	--	0.224
0.0542	--	--	3.866	--	--	0.215
0.0460	--	--	3.902	--	--	0.209
0.0400	--	--	3.924	--	--	0.207
0.0355	--	--	3.940	--	--	0.206
0.0318	--	--	3.955	--	--	0.208
0.0262	--	--	3.997	--	--	0.216
0.0221	--	--	4.066	--	--	0.230

TABLE 11

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES,
AND CORRESPONDING PHASE VELOCITIES AND ATTENUATIONS
OF THE RIGHT LAYERS (THE CASE WHERE THE IMAGINARY
PART OF THE INPUT VELOCITY IS .1%
OF ITS REAL PART)

Wavenumber (KM^{-1})			Phase Velocity			Attenuation ($\times 10^{-2}$)
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode
0.2002	0.1544	0.1532	3.487	4.522	4.553	0.202
0.1799	0.1385	0.1369	3.493	4.537	4.590	0.204
0.1490	0.1150	--	3.514	4.552	--	0.209
0.1174	0.0915	--	3.569	4.578	--	0.218
0.0958	0.0757	--	3.644	4.613	--	0.226
0.0849	--	--	3.700	--	--	0.229
0.0761	--	--	3.755	--	--	0.230
0.0656	--	--	3.830	--	--	0.229
0.0533	--	--	3.928	--	--	0.224
0.0449	--	--	3.996	--	--	0.219
0.0388	--	--	4.045	--	--	0.216
0.0342	--	--	4.084	--	--	0.215
0.0305	--	--	4.120	--	--	0.217
0.0249	--	--	4.198	--	--	0.225
0.0208	--	--	4.308	--	--	0.243

TABLE 12

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES,
AND CORRESPONDING PHASE VELOCITIES AND
ATTENUATIONS OF THE LEFT LAYERS
(THE CASE WHERE Q_B IS CONSTANT)

Wavenumber (KM^{-1})			Phase Velocity			Attenuation ($\times 10^{-2}$)
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode
0.2009	0.1618	0.1589	3.475	4.316	4.394	0.2053
0.1806	0.1453	0.1420	3.478	4.323	4.423	0.2053
0.1497	0.1207	--	3.497	4.337	--	0.2065
0.1179	0.0960	--	3.551	4.363	--	0.2092
0.0962	--	--	3.627	--	--	0.2092
0.0853	--	--	3.681	--	--	0.2068
0.0765	--	--	3.732	--	--	0.2026
0.0662	--	--	3.797	--	--	0.1951
0.0542	--	--	3.866	--	--	0.1848
0.0460	--	--	3.903	--	--	0.1794
0.0400	--	--	3.924	--	--	0.1772
0.0354	--	--	3.939	--	--	0.1768
0.0318	--	--	3.955	--	--	0.1781
0.0262	--	--	3.997	--	--	0.1836
0.0221	--	--	4.067	--	--	0.1940

TABLE 13

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES,
AND CORRESPONDING PHASE VELOCITIES AND
ATTENUATIONS OF THE RIGHT LAYERS
(THE CASE WHERE Q_B IS CONSTANT)

Wavenumber (KM^{-1})			Phase Velocity			Attenuation ($\times 10^{-2}$)
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode
0.2002	0.1544	0.1533	3.487	4.522	4.553	0.1071
0.1799	0.1385	0.1369	3.493	4.537	4.590	0.1081
0.1490	0.1150	--	3.514	4.552	--	0.1108
0.1174	0.0915	--	3.569	4.578	--	0.1160
0.0958	0.0757	--	3.645	4.614	--	0.1205
0.0849	--	--	3.700	--	--	0.1221
0.0761	--	--	3.755	--	--	0.1226
0.0656	--	--	3.830	--	--	0.1216
0.0533	--	--	3.928	--	--	0.1185
0.0449	--	--	3.996	--	--	0.1159
0.0388	--	--	4.045	--	--	0.1146
0.0342	--	--	4.084	--	--	0.1142
0.0305	--	--	4.120	--	--	0.1148
0.0249	--	--	4.198	--	--	0.1187
0.0208	--	--	4.308	--	--	0.1270

TABLE 14

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES,
AND CORRESPONDING PHASE VELOCITIES AND ATTENUATIONS
OF THE LEFT LAYERS (THE CASE OF
LEE AND SOLOMON'S MODEL)

Wavenumber (KM^{-1})			Phase Velocity			Attenuation ($\times 10^{-2}$)
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode
0.2008	0.1613	0.1591	3.476	4.329	4.389	2.1463
0.1806	0.1449	0.1421	3.479	4.335	4.419	2.0031
0.1497	0.1205	--	3.498	4.345	--	1.7263
0.1179	0.0959	--	3.552	4.368	--	1.3356
0.0962	--	--	3.628	--	--	1.0034
0.0853	--	--	3.682	--	--	0.8519
0.0765	--	--	3.733	--	--	0.7852
0.0662	--	--	3.797	--	--	0.8549
0.0541	--	--	3.867	--	--	1.2567
0.0460	--	--	3.903	--	--	1.7255
0.0400	--	--	3.925	--	--	2.1302
0.0354	--	--	3.941	--	--	2.4612
0.0318	--	--	3.956	--	--	2.7363
0.0262	--	--	3.999	--	--	3.1892
0.0221	--	--	4.068	--	--	3.6047

TABLE 15

FUNDAMENTAL, AND FIRST AND SECOND HIGHER MODE EIGENVALUES
AND CORRESPONDING PHASE VELOCITIES AND ATTENUATIONS
OF THE RIGHT LAYERS (THE CASE OF
LEE AND SOLOMON'S MODEL)

Wavenumber (KM^{-1})			Phase Velocity			Attenuation ($\times 10^{-2}$)
Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode
0.2002	0.1543	0.1534	3.487	4.524	4.552	0.0890
0.1799	0.1383	0.1370	3.493	4.541	4.586	0.0891
0.1490	0.1149	--	3.514	4.554	--	0.0901
0.1173	0.0915	--	3.569	4.579	--	0.0928
0.0958	0.0756	--	3.645	4.615	--	0.0958
0.0849	--	--	3.700	--	--	0.0973
0.0761	--	--	3.755	--	--	0.0986
0.0656	--	--	3.830	--	--	0.1012
0.0533	--	--	3.928	--	--	0.1176
0.0449	--	--	3.997	--	--	0.1623
0.0388	--	--	4.045	--	--	0.2346
0.0342	--	--	4.084	--	--	0.3248
0.0305	--	--	4.120	--	--	0.4256
0.0249	--	--	4.198	--	--	0.6517
0.0208	--	--	4.308	--	--	0.9238

TABLE 16

THE SIERRA NEVADA MODEL OF LYSMER AND DRAKE
AND CALCULATED PHASE VELOCITIES USING
LAYERED SYSTEMS

Thickness	Compressional Velocity	Shear Velocity	Density	Period	Phase Velocity	
					Lysmer & Drake	This Study
6.0	5.60	3.10	2.67	6	3.065	3.064
4.5	6.00	3.40	2.70	7	3.105	3.104
5.0	6.10	3.40	2.80	8	3.148	3.147
15.0	6.90	3.90	2.80	9	3.191	3.191
20.0	7.90	4.39	3.30	10	3.236	3.237

TABLE 17
LEE AND SOLOMON'S PHASE VELOCITY

Period	Eastern United States			Western United States		
	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode
10	3.532	4.468	4.521	3.379	4.454	4.657
12	3.559	4.474	4.543	3.420	4.511	4.758
15	3.620	4.483	4.577	3.489	4.578	4.980
18	3.697	4.494	4.618	3.562	4.658	5.150
20	3.752	4.502	4.652	3.611	4.723	5.232
22	3.805	4.512	4.691	3.657	4.799	--
25	3.874	4.528	4.763	3.720	4.918	--
30	3.958	4.566	4.926	3.802	5.090	--
35	4.006	4.616	--	3.860	5.209	--
45	4.047	--	--	3.941	--	--
50	4.054	--	--	3.975	--	--
60	4.061	--	--	4.044	--	--
70	4.064	--	--	4.116	--	--
80	4.070	--	--	4.190	--	--

TABLE 18
LEE AND SOLOMON'S ATTENUATION ($\times 10^{-2}$)

Period	Eastern United States			Western United States		
	Funda- mental Mode	1st Higher Mode	2nd Higher Mode	Funda- mental Mode	1st Higher Mode	2nd Higher Mode
10	.0903	2.1459	1.7652	.2516	.9800	4.6070
12	.0933	2.1146	1.6871	.2348	.1986	4.1221
15	.0958	2.0682	1.6130	.2125	2.9181	3.9127
18	.0977	2.0344	1.6255	.2017	3.4195	3.7966
20	.0982	2.0239	1.6693	.2122	3.6140	3.6374
22	.0981	2.0230	1.7286	.2482	3.7146	--
25	.0983	2.0344	1.8300	.3657	3.7300	--
30	.1089	2.0665	2.0030	.7190	3.6524	--
35	.1446	2.0970	--	1.1676	3.6649	--
45	.2848	2.1568	--	2.0141	--	--
50	.3737	2.2036	--	2.3602	--	--
60	.5602	2.3752	--	2.8826	--	--
70	.7409	--	--	3.1902	--	--
80	.9083	--	--	3.3239	--	--

APPENDIX B

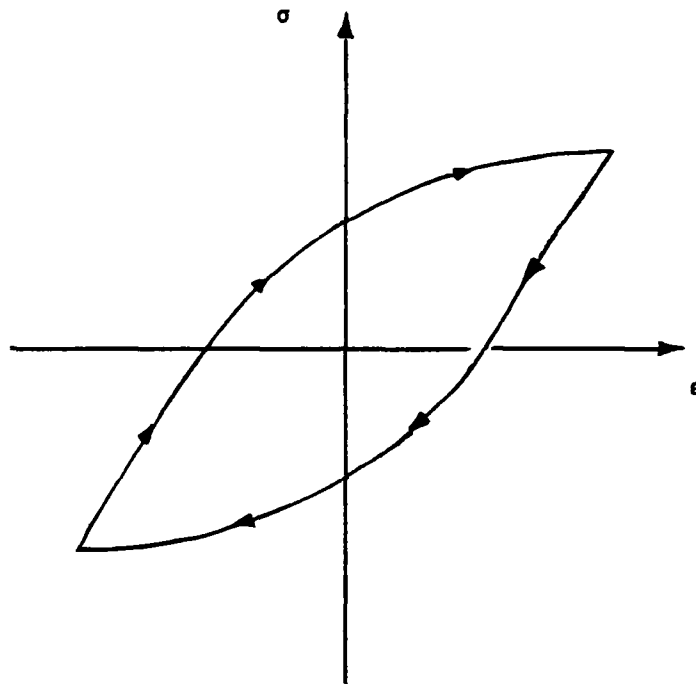
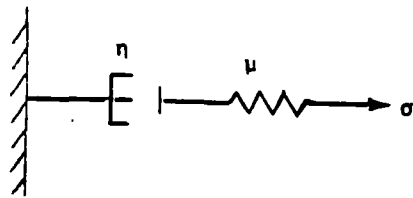
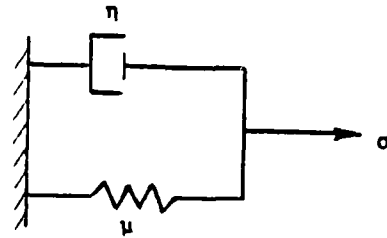


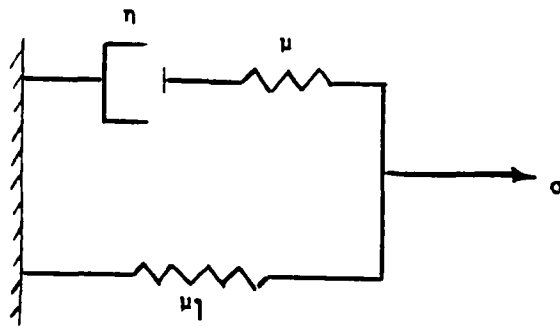
Figure 1. HYSTERETIC EFFECT OF THE VISCOELASTIC SOLID



MAXWELL ELEMENT

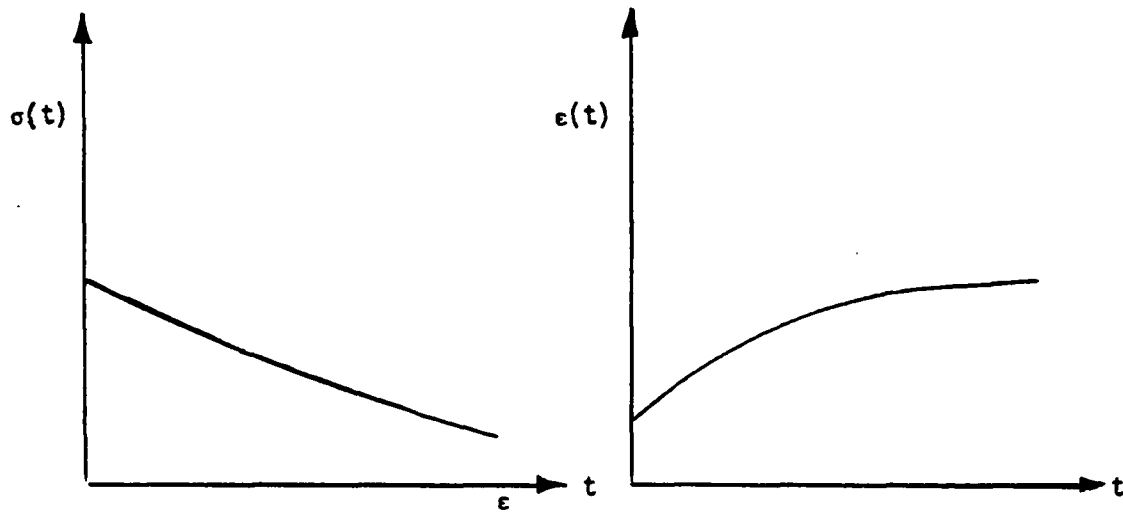


VOIGT ELEMENT



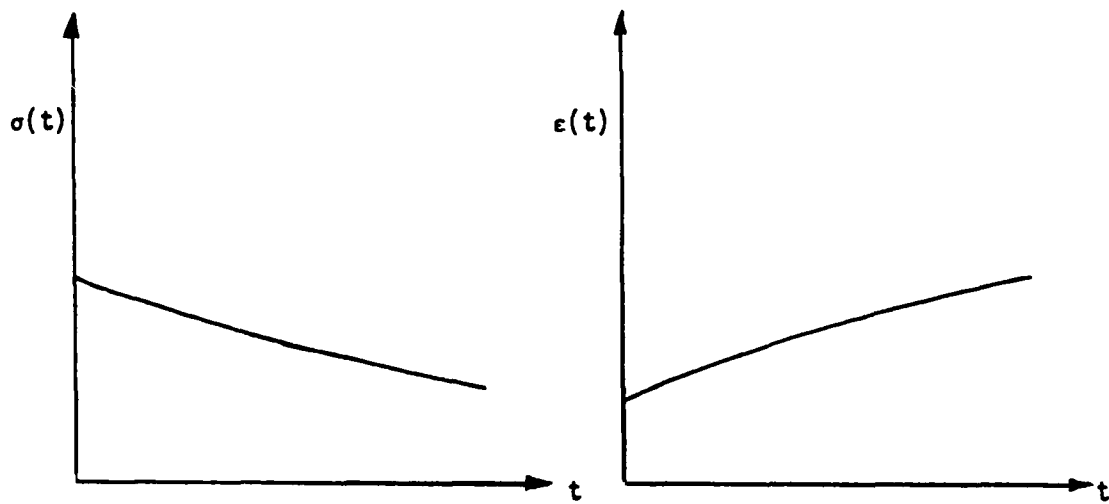
STANDARD LINEAR SOLID

Figure 2. ELEMENTAL MODEL OF VISCOELASTIC SOLID



(a) RELAXATION FUNCTION

(b) CREEP FUNCTION



(c) RELAXATION FUNCTION

(d) CREEP FUNCTION

FIGURE 3. (a) and (b) SOLUTIONS OF MAXWELL AND VOIGT ELEMENTS,
(c) and (d) SOLUTIONS OF STANDARD LINEAR SOLID

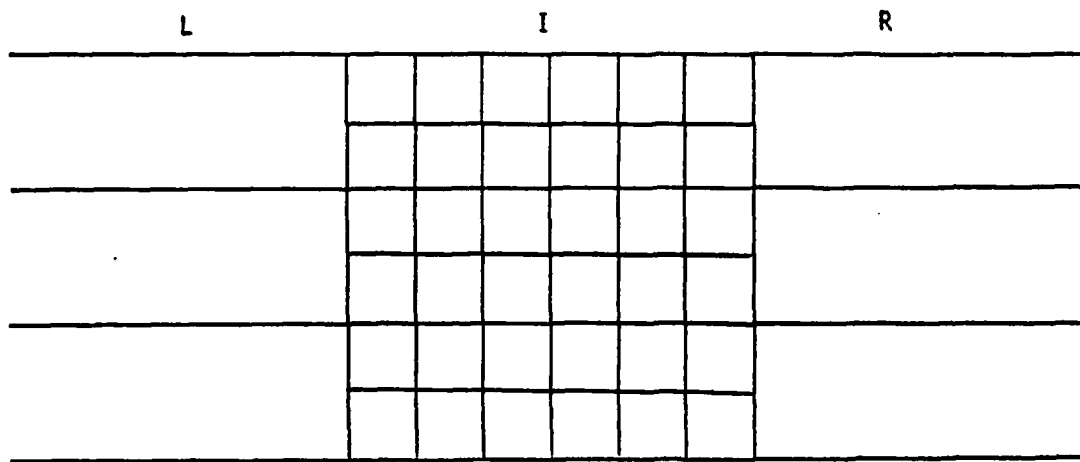


Figure 4. A FINITE ELEMENT NET, BOUNDED BY TWO SEMI-INFINITE HORIZONTALLY LAYERED SYSTEM

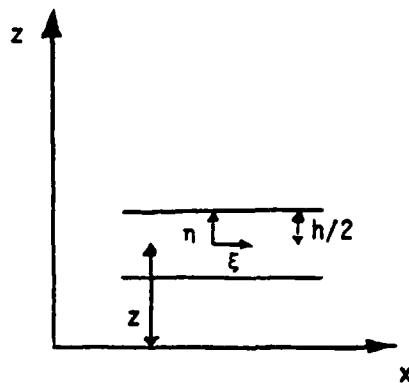


Figure 5. COORDINATES OF AN ELEMENT, η AND ξ ARE THE LOCAL COORDINATES OF THE ELEMENT

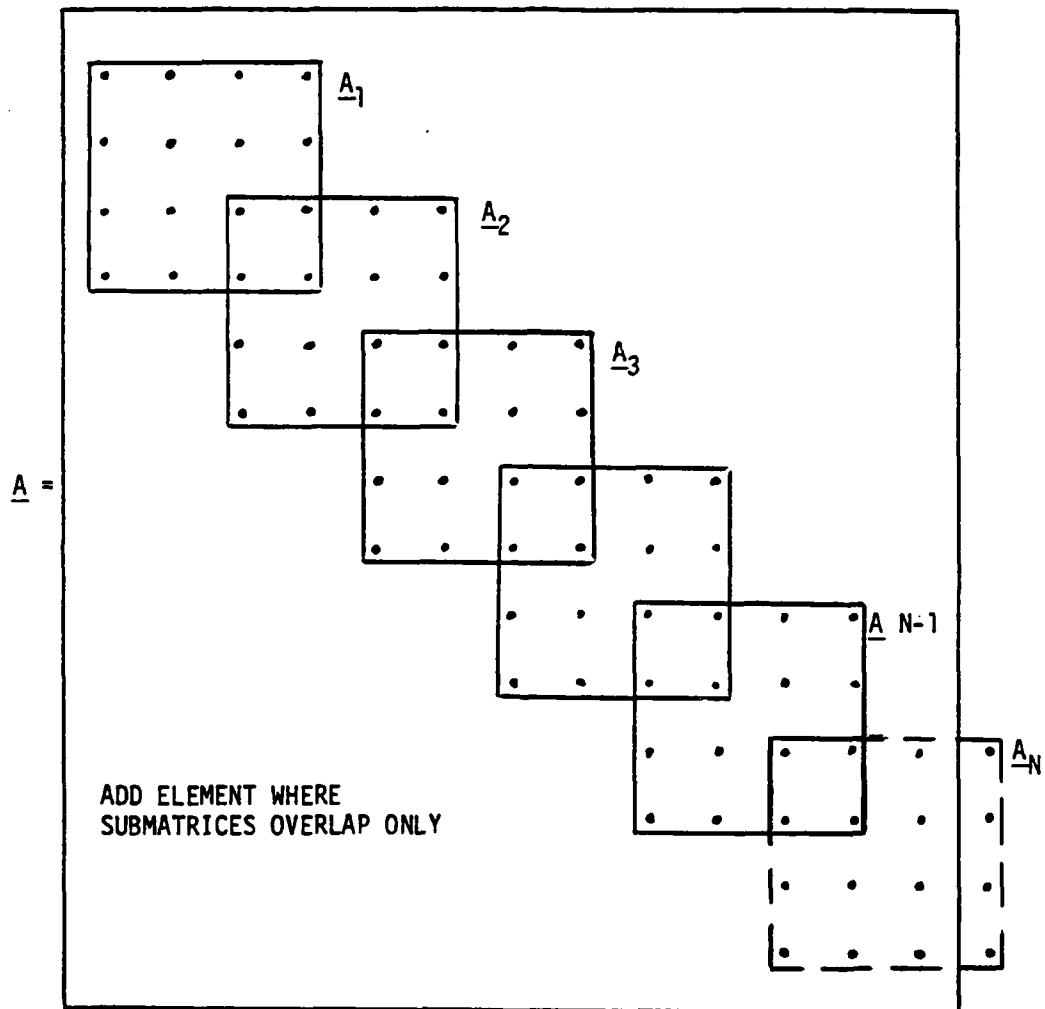


Figure 6. STRUCTURE OF MATRICES $\underline{A}$, $\underline{C}$, $\underline{G}$, AND $\underline{M}$

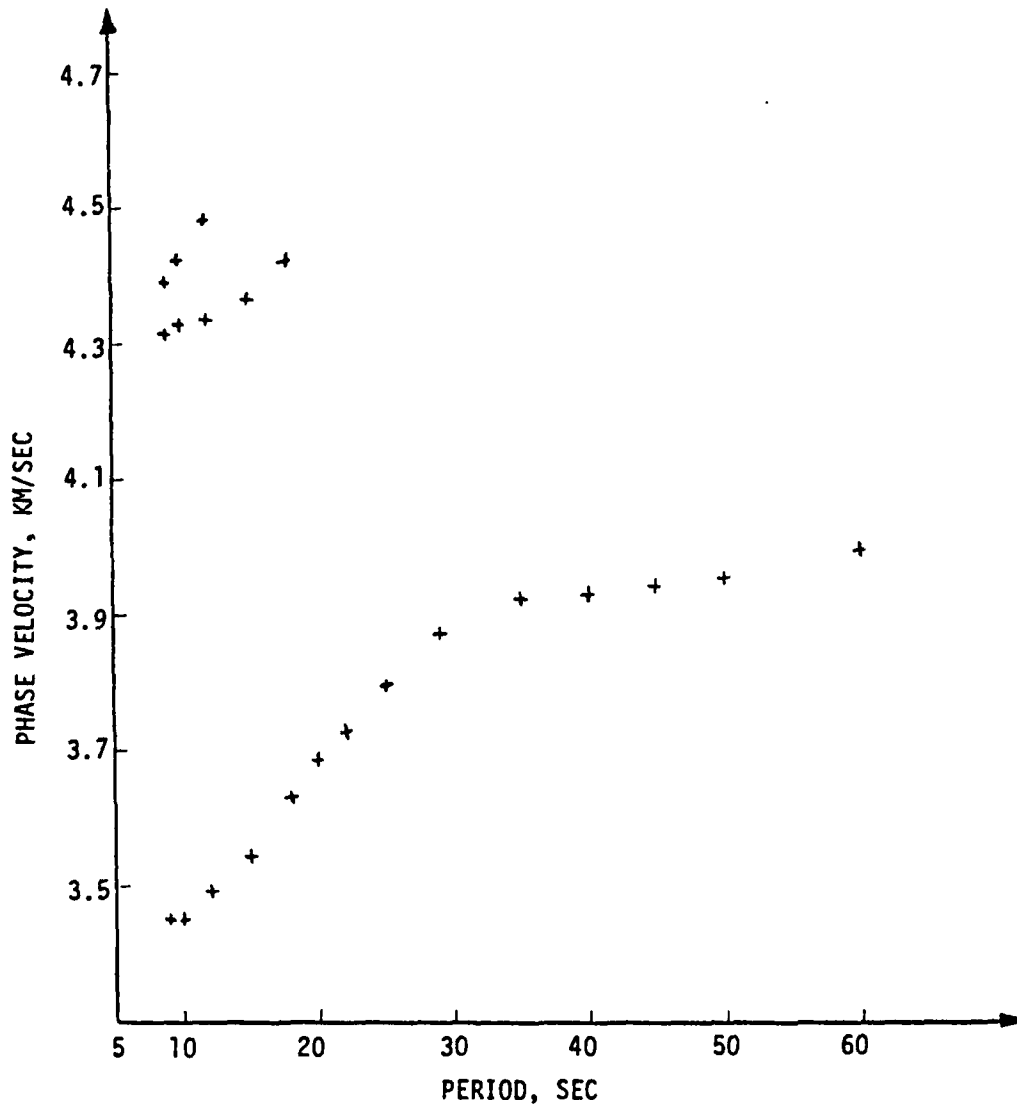


FIGURE 7. FUNDAMENTAL AND FIRST AND SECOND HIGHER MODE RAYLEIGH WAVE PHASE VELOCITIES, BASIN AND RANGE PROVINCES

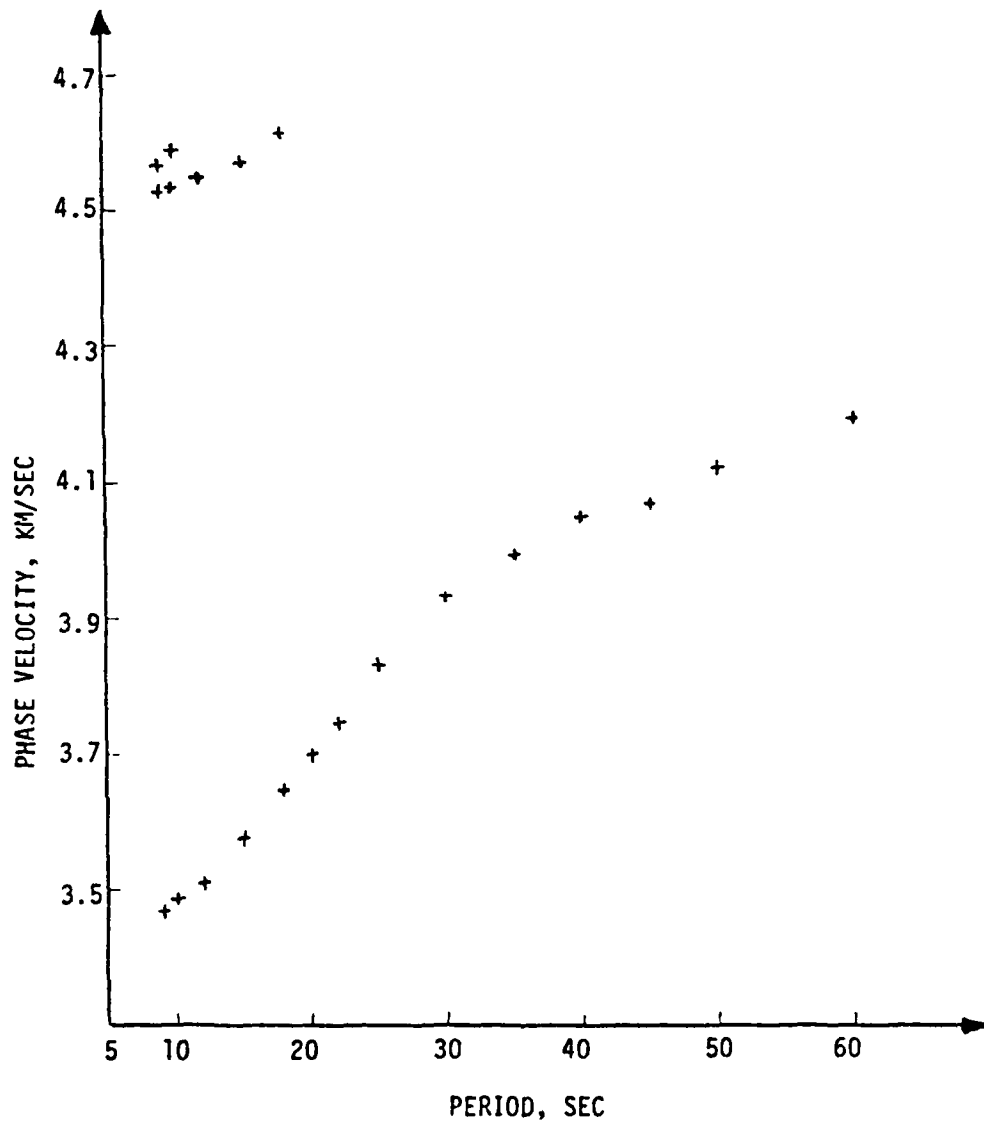


Figure 8. FUNDAMENTAL AND FIRST AND SECOND HIGHER MODE RAYLEIGH WAVE PHASE VELOCITIES, CANADIAN SHIELD

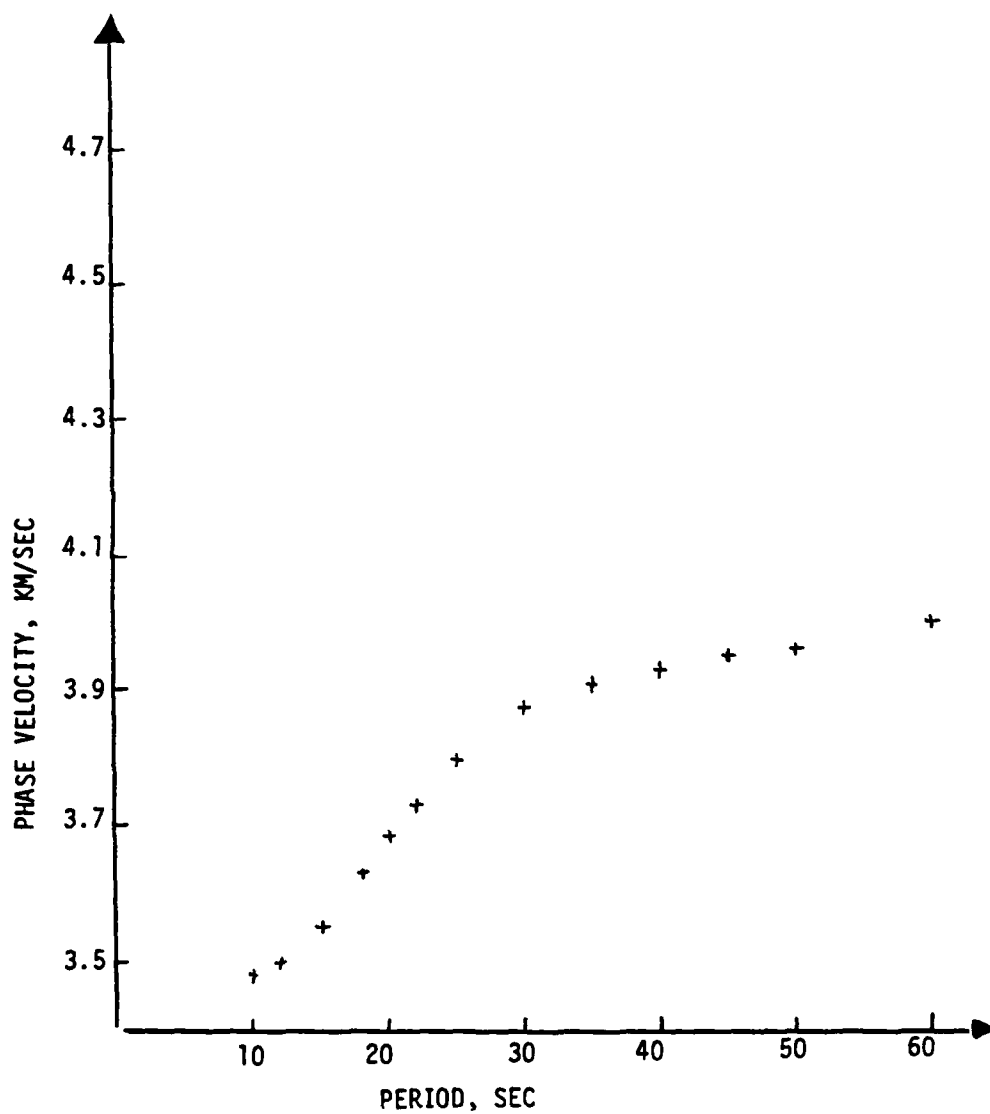


Figure 9. FUNDAMENTAL RAYLEIGH WAVE PHASE VELOCITIES OF ZONE I
(THE CASE WHERE ALL THREE ZONES ARE ASSUMED TO BE THE
BASIN AND RANGE PROVINCE.)

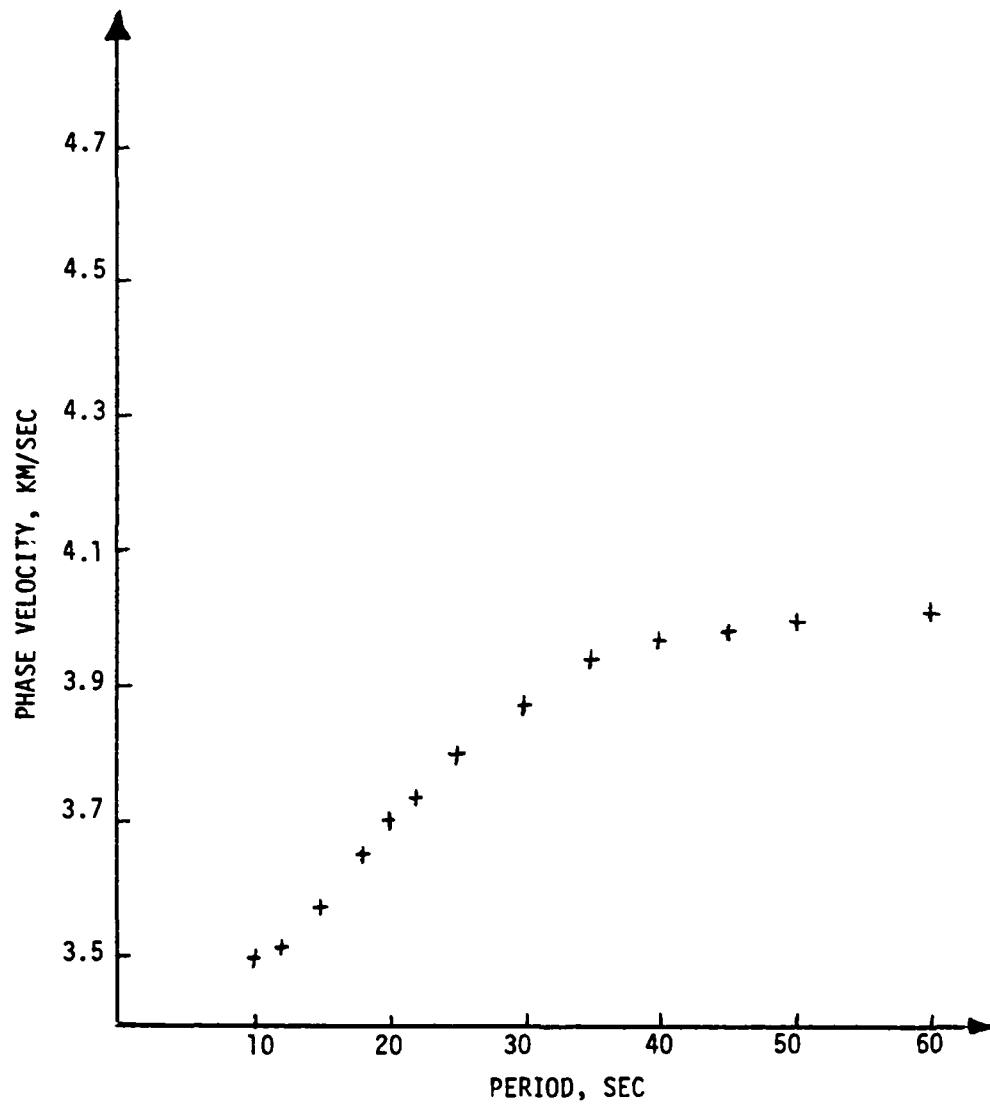


Figure 9A. FUNDAMENTAL RAYLEIGH WAVE PHASE VELOCITIES OF ZONE I
(THE CASE WHERE THE VERTICAL BOUNDARY IS AT THE RIGHT
HAND SIDE OF THE ZONE L).

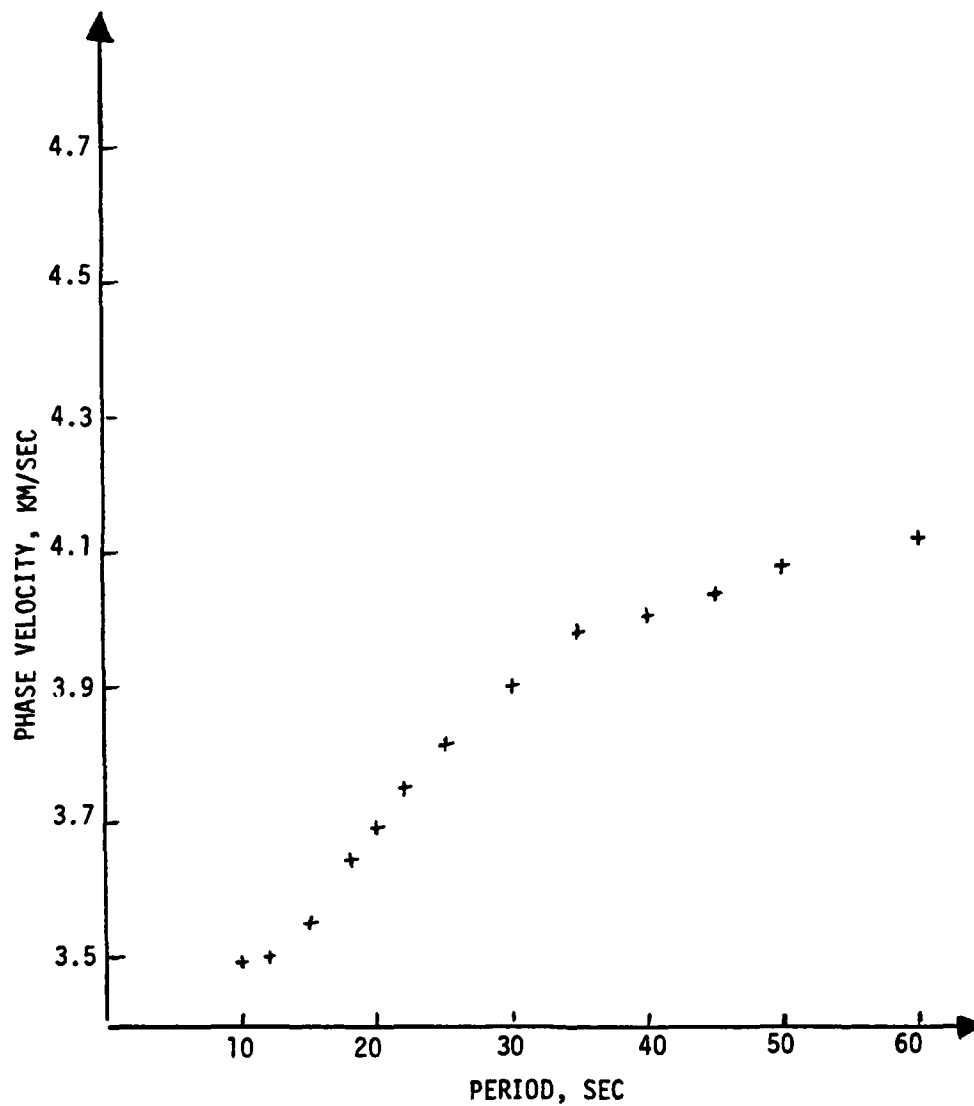


Figure 9B. FUNDAMENTAL RAYLEIGH WAVE PHASE VELOCITIES OF ZONE I
(THE CASE WHERE THE VERTICAL BOUNDARY IS AT THE LEFT
HAND SIDE OF THE ZONE R).

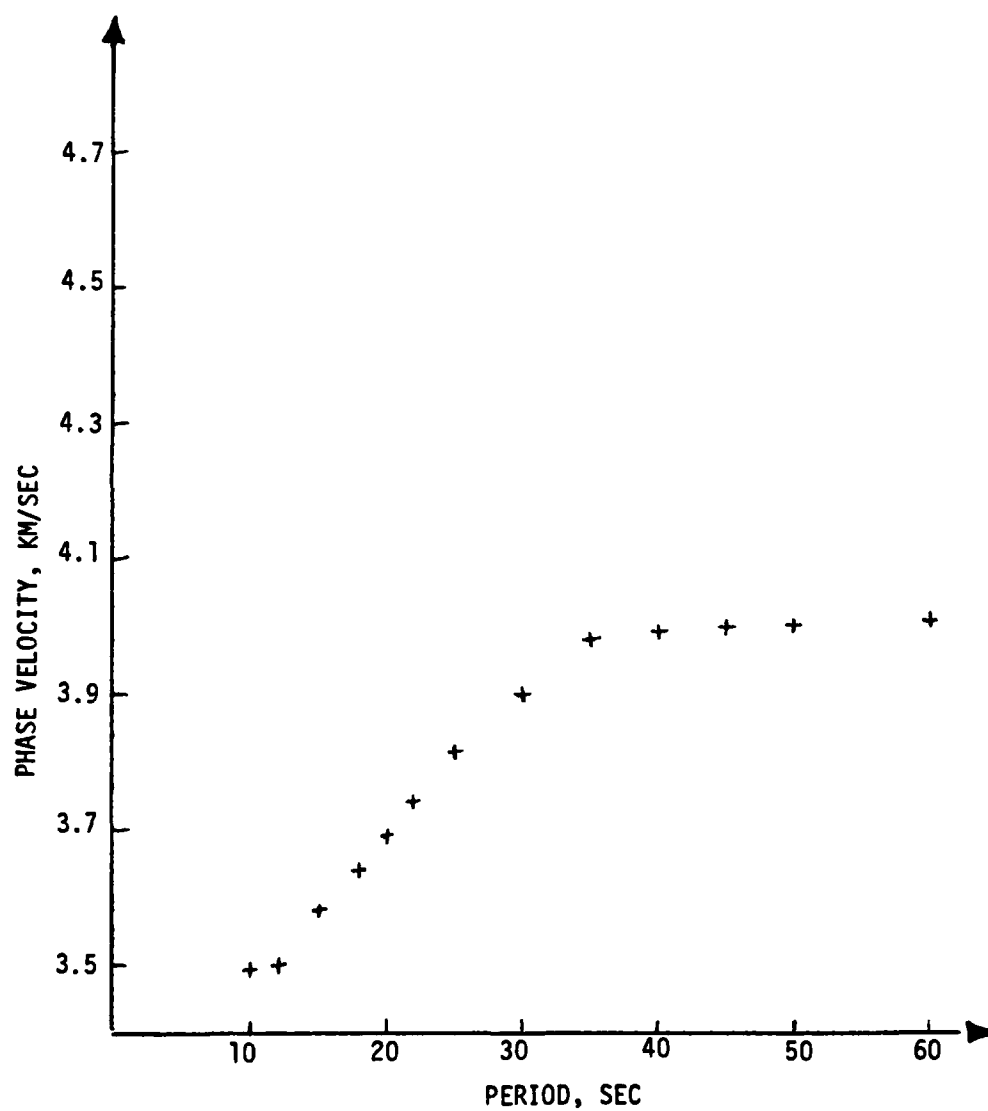


Figure 9C. FUNDAMENTAL RAYLEIGH WAVE PHASE VELOCITIES OF ZONE I
(THE CASE WHERE THE VERTICAL BOUNDARY IS AT THE MIDDLE
OF ZONE I).

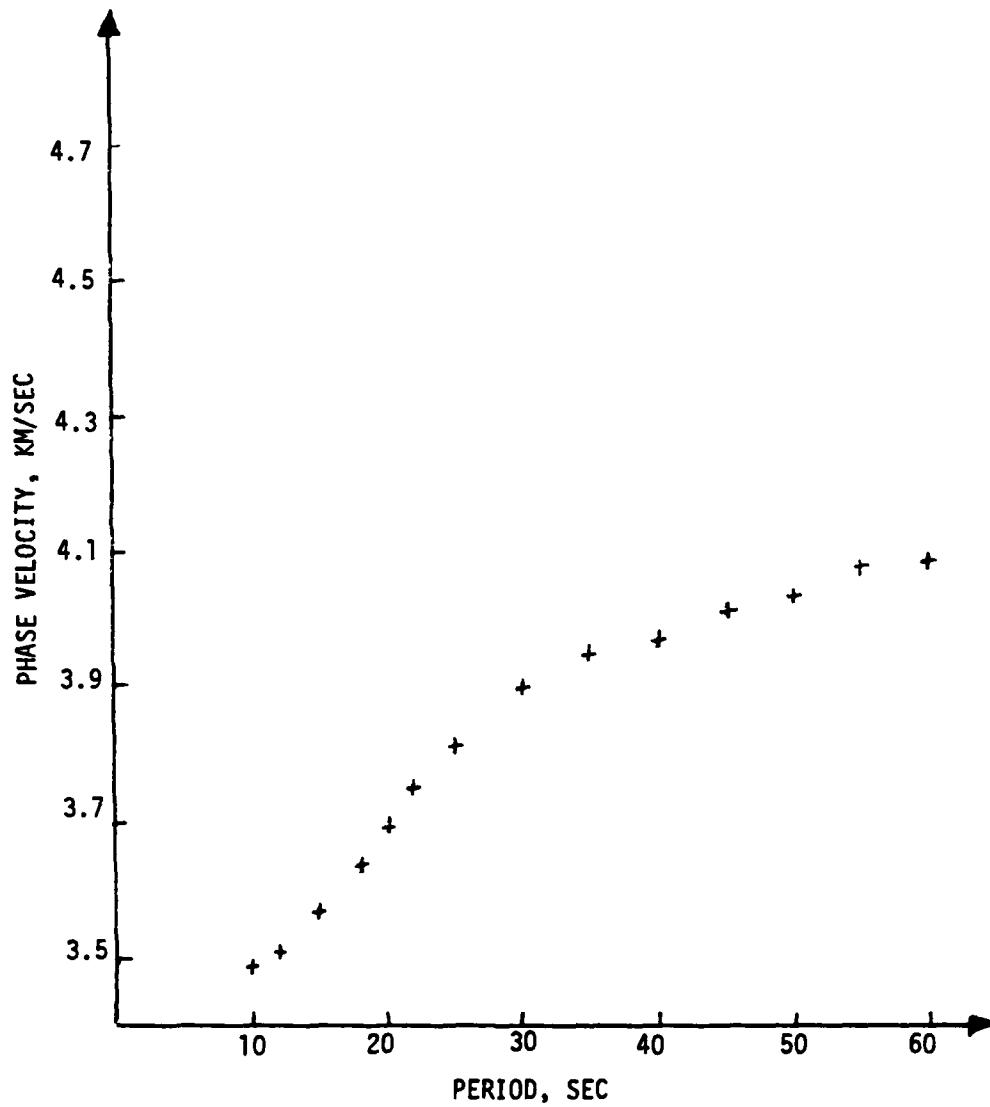


Figure 9D. FUNDAMENTAL RAYLEIGH WAVE PHASE VELOCITIES OF ZONE I
(THE CASE WHERE THE POSITION OF VERTICAL BOUNDARY
GRADUALLY CHANGES).

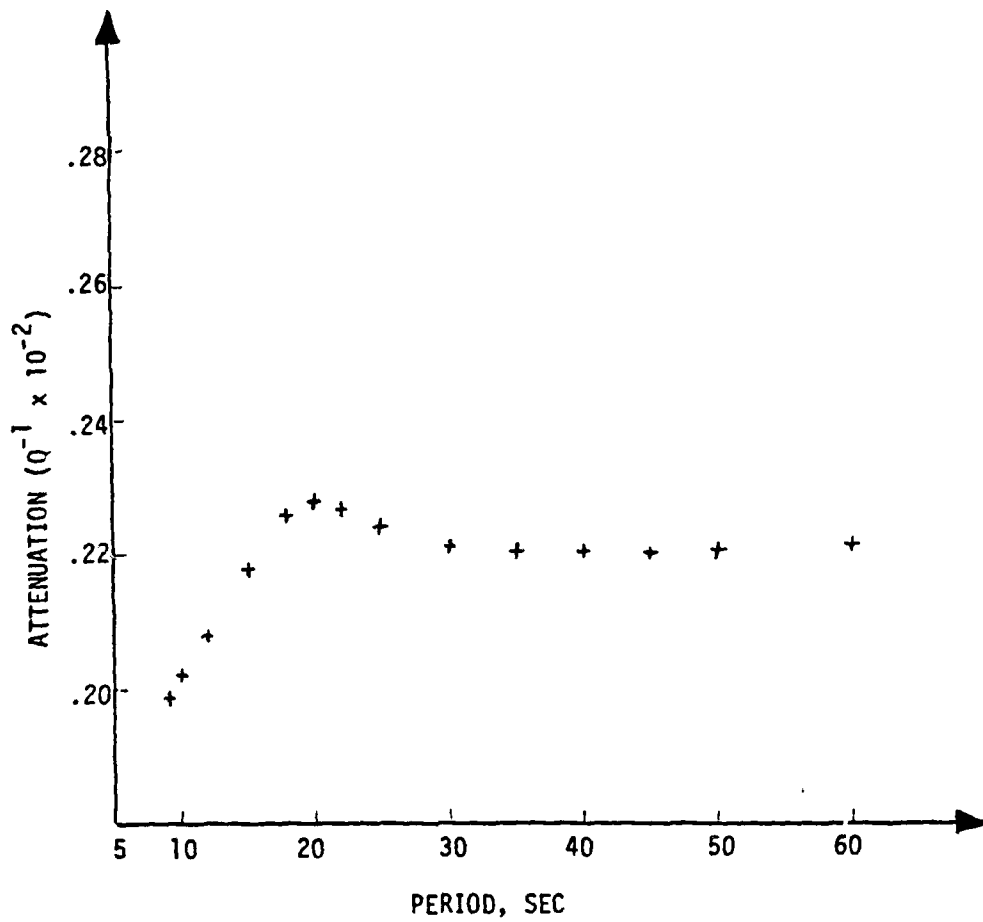


Figure 10. RAYLEIGH WAVE ATTENUATION, BASIN AND RANGE PROVINCES.
(The case where the imaginary part of the input velocity
is .1% of its real part)

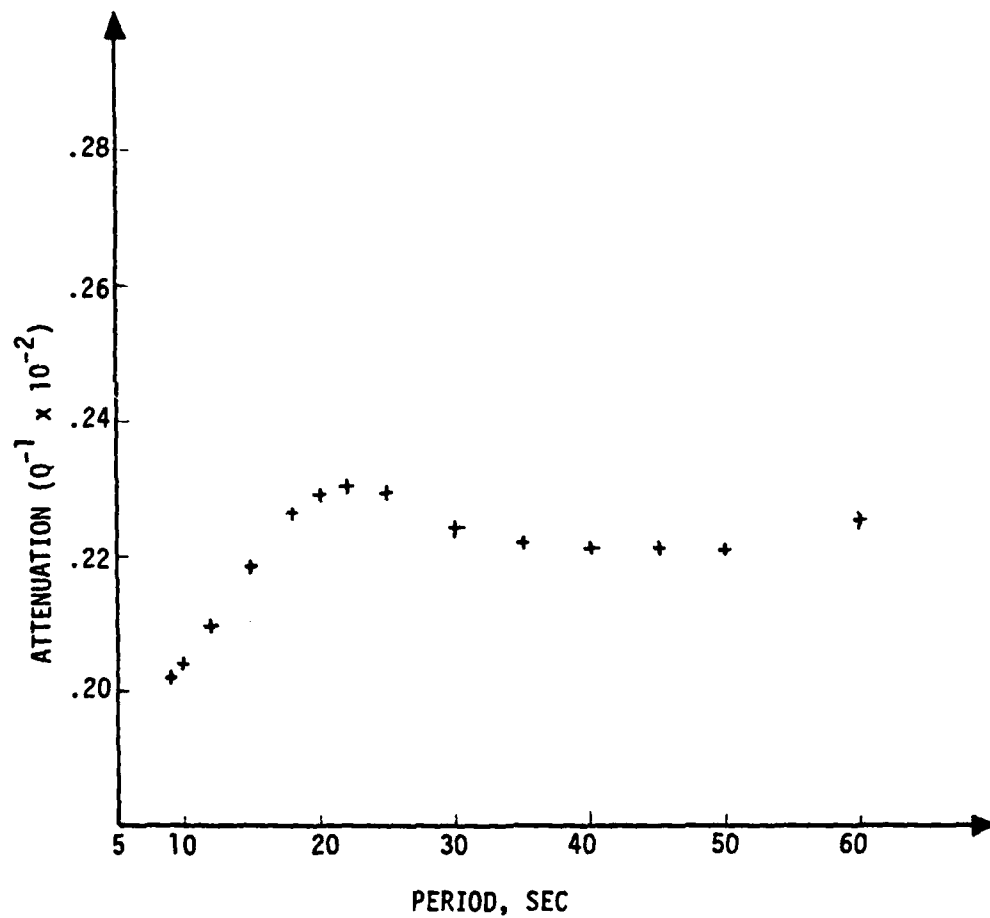


Figure 11. RAYLEIGH WAVE ATTENUATION, CANADIAN SHIELD.
(The case where the real part of the input velocity is .1% of its real part)

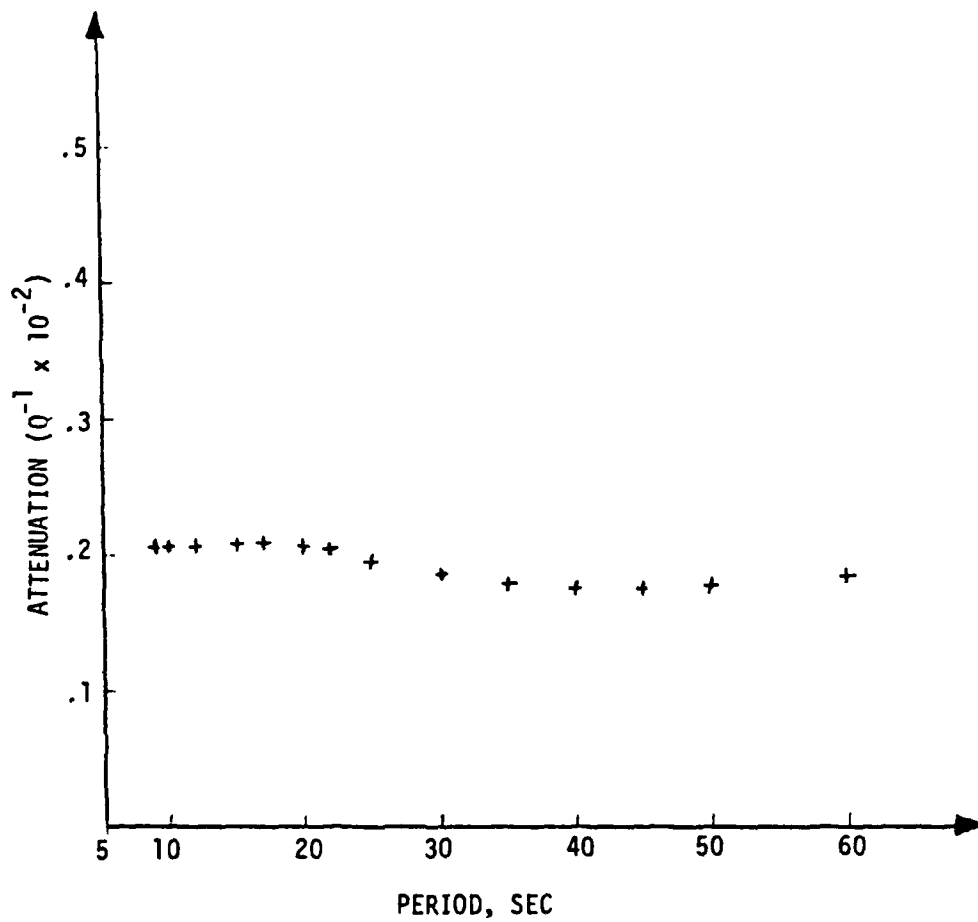


Figure 12. RAYLEIGH WAVE ATTENUATION, BASIN AND RANGE PROVINCES
(The case where Q_β is constant)

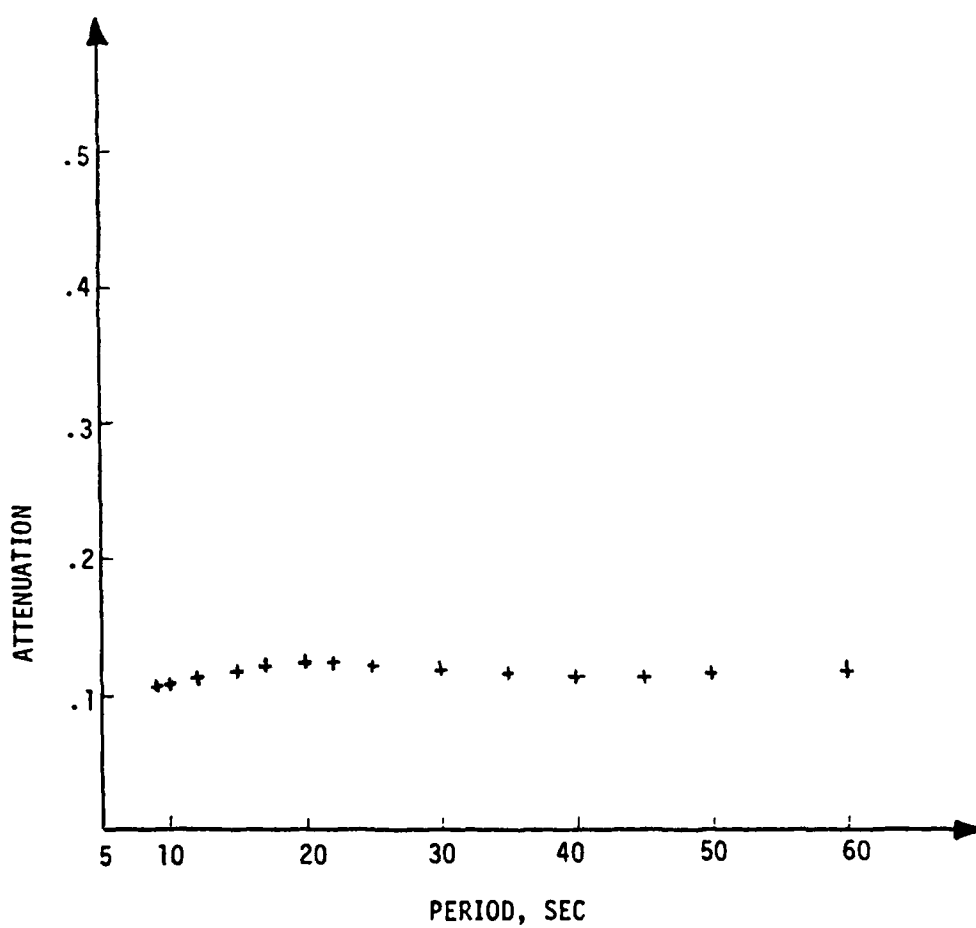


Figure 13. RAYLEIGH WAVE ATTENUATION, CANADIAN SHIELD
(The case where Q_b is constant)

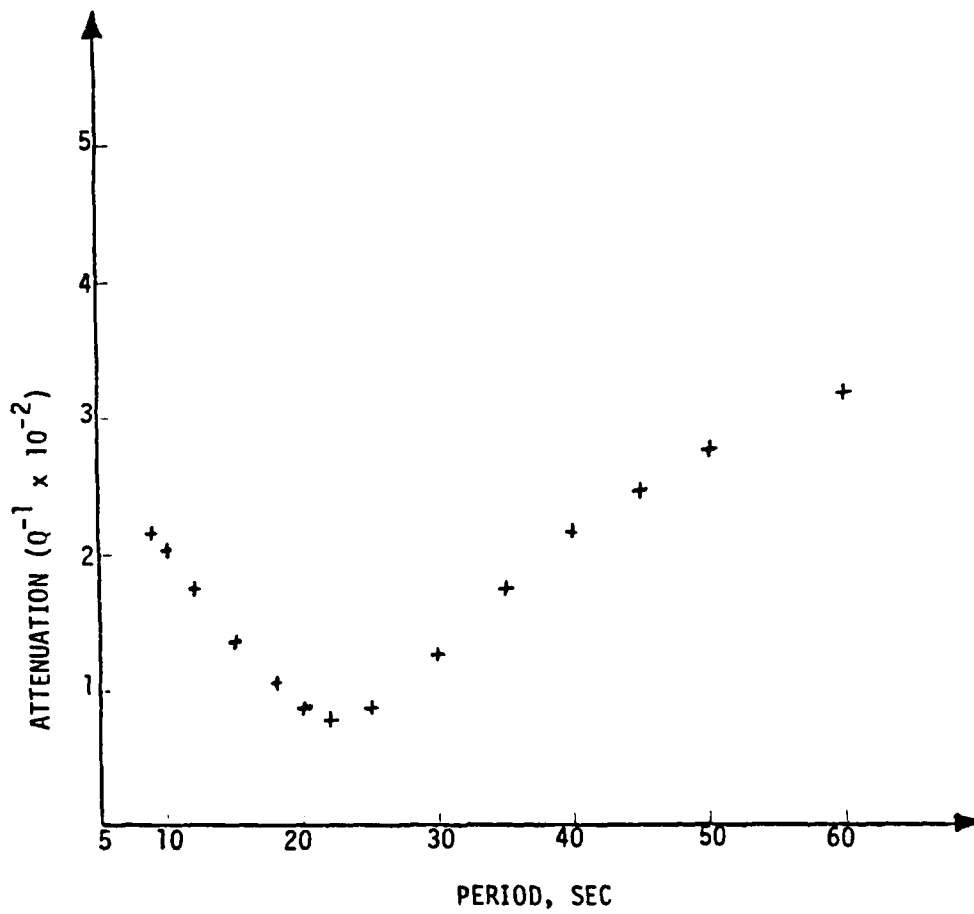


Figure 14. RAYLEIGH WAVE ATTENUATION, BASIN AND RANGE PROVINCES
(The case of Lee and Solomon's data)

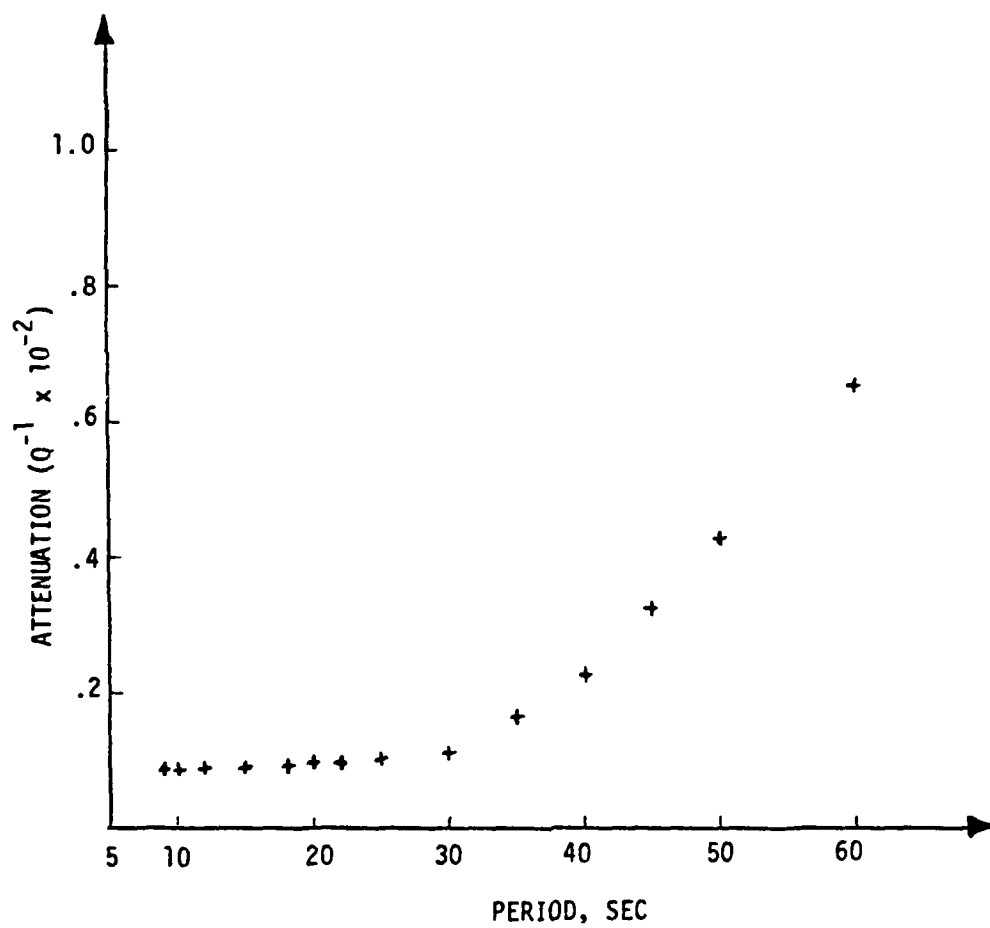


Figure 15. RAYLEIGH WAVE ATTENUATION, CANADIAN SHIELD
(The case of Lee and Solomon's data)

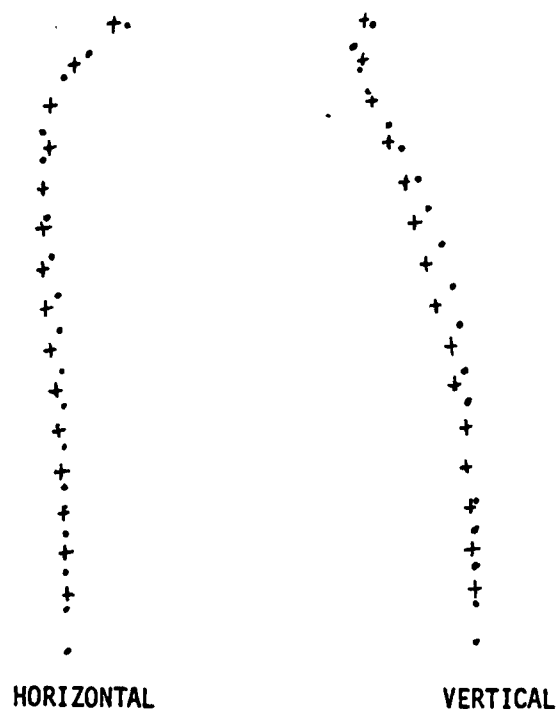


Figure 16. HORIZONTAL AND VERTICAL AMPLITUDES FOR THE FUNDAMENTAL RAYLEIGH MODE OF SIERRA NEVADA FOR A PERIOD OF 6 SECONDS. THE CIRCLES ARE FROM LYSMER AND DRAKE (80).

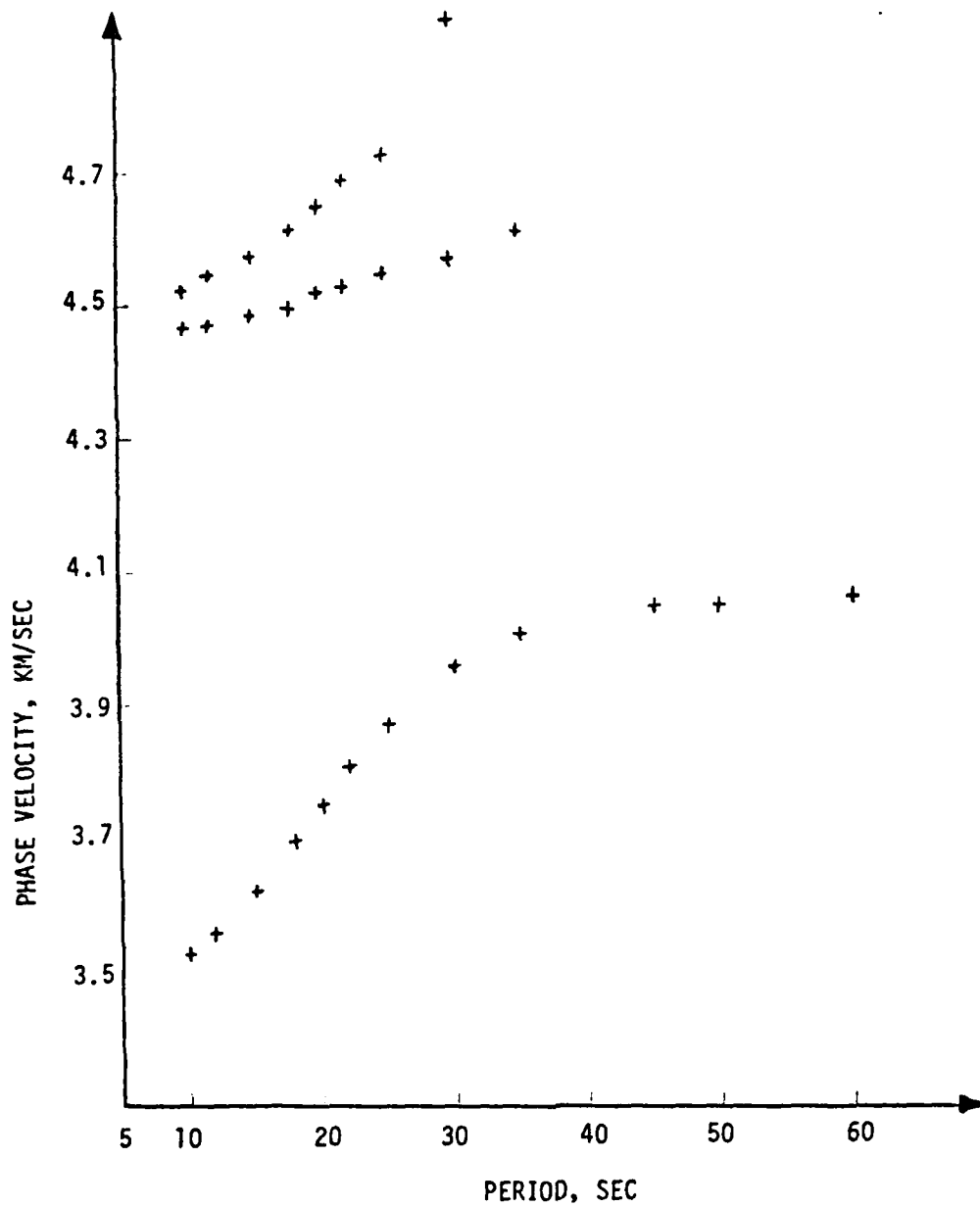


Figure 17. FUNDAMENTAL AND FIRST AND SECOND HIGHER MODE PHASE VELOCITIES, EASTERN UNITED STATES. (Lee and Solomon's model)

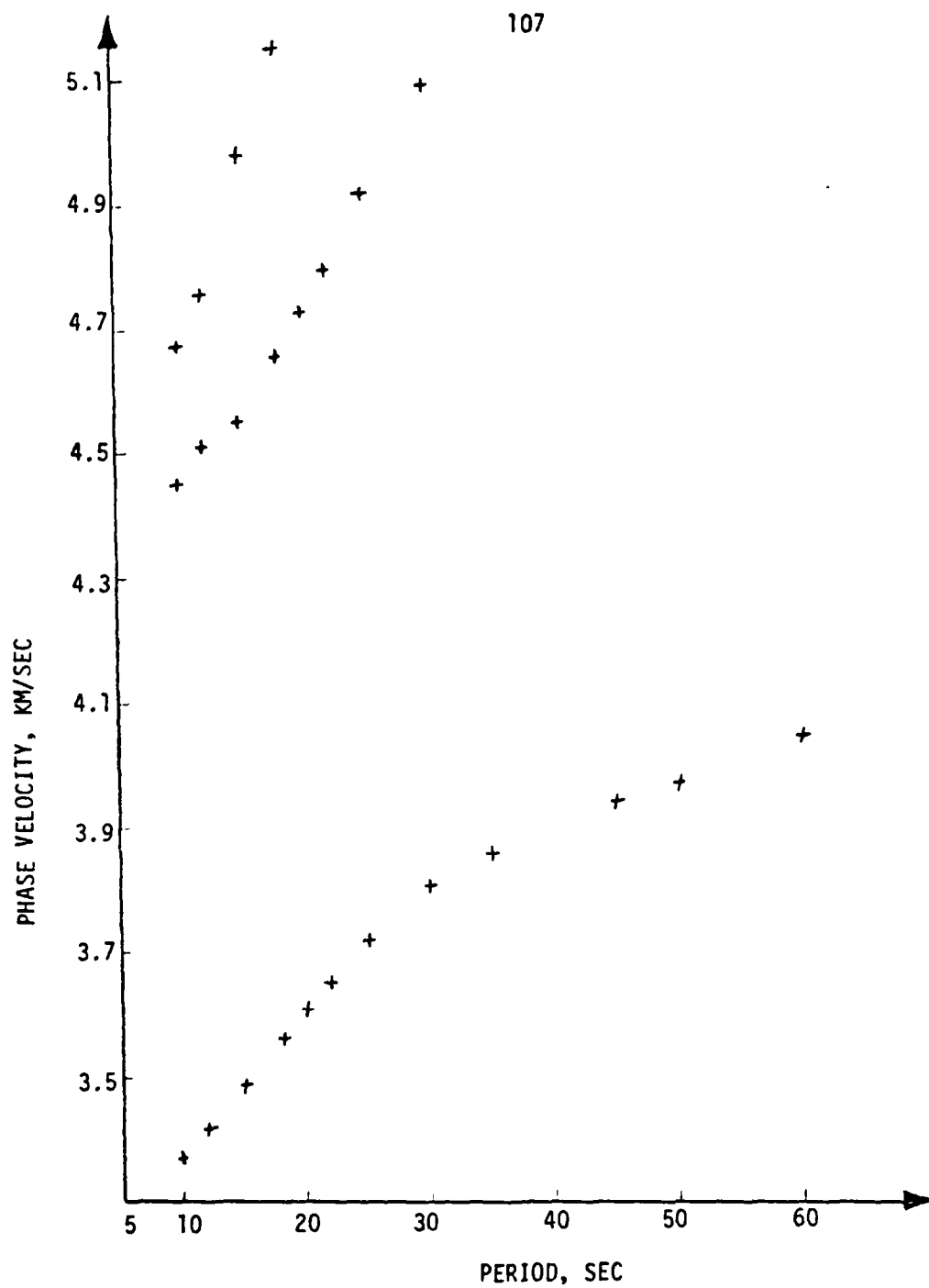


Figure 18. FUNDAMENTAL AND FIRST AND SECOND HIGHER MODE PHASE VELOCITIES, WESTERN UNITED STATES. (Lee and Solomon's model)

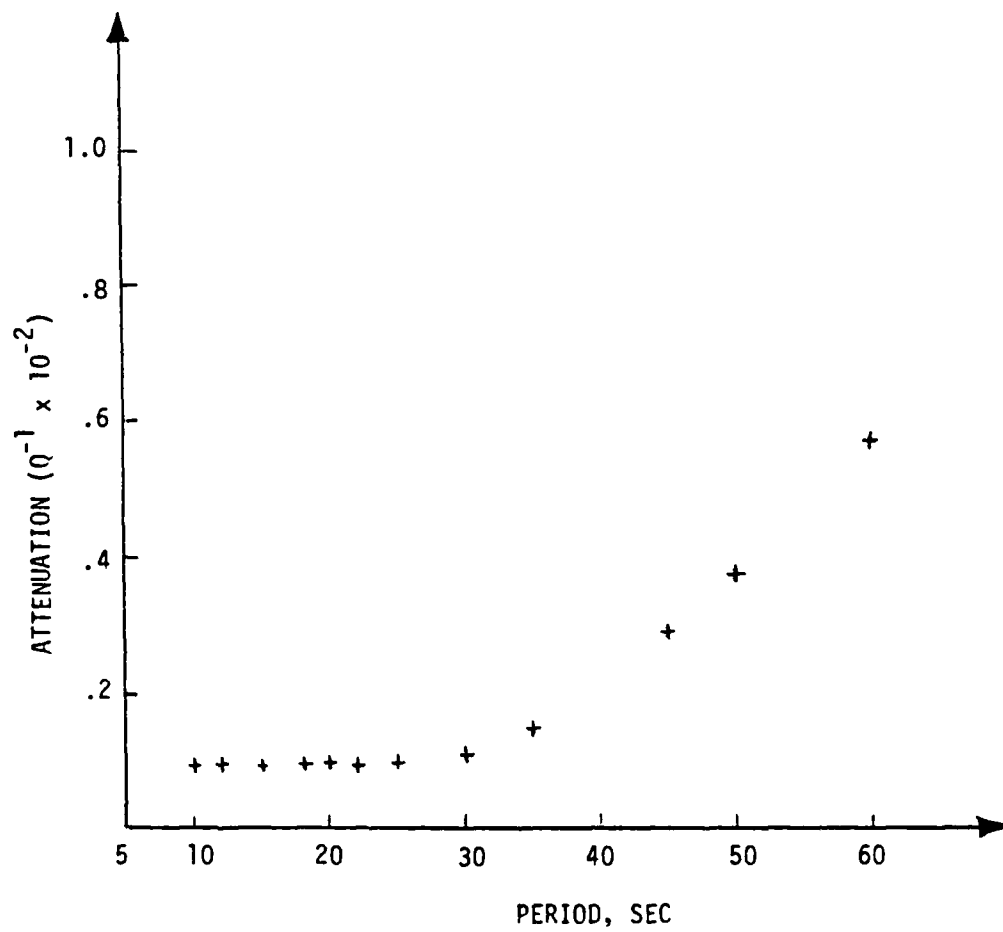


Figure 19. RAYLEIGH WAVE ATTENUATION, EASTERN UNITED STATES
(Lee and Solomon's model)

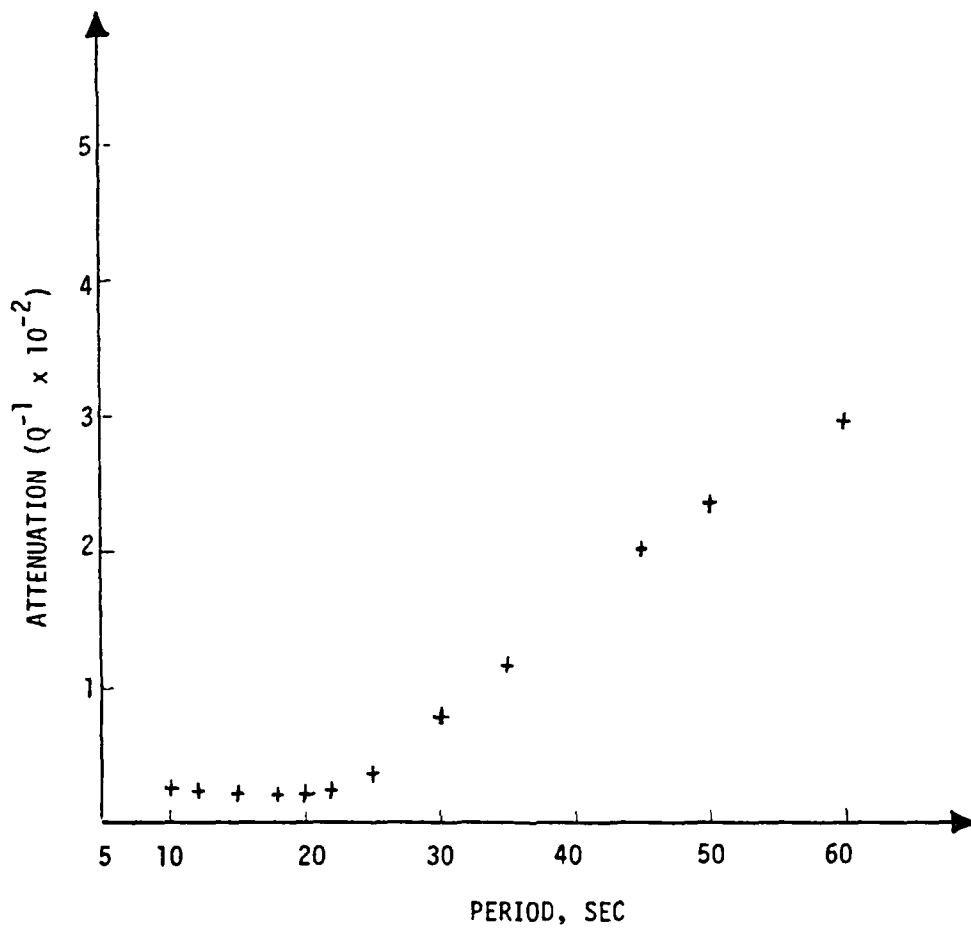


Figure 20. RAYLEIGH WAVE ATTENUATION, WESTERN UNITED STATES
(Lee and Solomon's model)

APPENDIX C

PROGRAM RALEE

This program is a revised form of the Program Raleem, written by L. A. Drake in 1972. The program is designed to analyze the motion of harmonic surface waves in a linearly elastic or viscoelastic layered medium. The layered medium is subdivided into a finite element Region I and one or two semiinfinite regions L and R. The two or three regions are joined together at the finite element boundaries.

The program consists of the main program RALEE, and 11 subroutines: EIGVEC, BOUNDA, SECEVA, SOLVE, ORTHOG, SETUP, INVERTC, MODAL, STIFFER, WRDSC AND BANSOL.

The function of the main and subroutines are as follows.

RALEE: Reads in the input data, assembles the stiffness matrices of the finite element regions and calculates amplitude and energies at the vertical boundaries.

EIGVEC: Computes the entries of the mass and elastic or viscoelastic matrices of the layered systems.

BOUNDA: Computes boundary matrices.

SECEVA: Constructs the eigenvalue problem, and solves it.

SOLVE: Solves an eigenvector corresponding to the eigenvalue, found in SECEVA.

ORTHOG: Finds an eigenvector normal to the one that has already been found.

SETUP: Decomposes the matrix $\underline{V}$, used in the boundary matrix of the subroutine, BOUNDA, into a vector; rewrites this vector into its real and imaginary parts.

INVERTC: Inverts a matrix.

MODAL: This subroutine is used only in the elastic case and selects the compatible eigenvalues.

STIFFER: Computes the stiffness matrix of an element.

WRDSC: Writes vector $\hat{B}$ and the matrix $\underline{A}$ of equation $\underline{A}\hat{X} = \hat{B}$ on Tape 2.

BANSOL: Solves equation $\underline{A}\hat{X} = \hat{B}$.

INPUT DATA

Read In:

I. Layered System

- A. Period (F10.5) (one card).
- B. Number of column and width of the finite element region (I10, 7F10.5) (one card).
- C. Number of elements having the same depth, young modulus, rigidity and width (6 I10) (one card).
- D. Number of natural layers and depth of the top of the first layer (I10, 7F10.5) (one card).
- E. Number, thickness, compressional and shear velocities and density of the sublayers (I10, 7F10.5) (N cards, $N \leq 15$).
Step D and E may be repeated once if there are two layered systems.

II. Finite Element Region

- A. Depth to corner of the element and the number of elements having the same depth (4(F10.5, I10)) (Number of cards needed varies in this and the next two cases. For example, there

are four values of depth on each card and each value multiplies by the number of the element having the same depth. Addition of these values plus the values of the following cards must add up to be the number of elements, N^2).

- B. Moduli of the elements and the number of elements having the same moduli (2(2 F10.5, I10)).
- C. Width of the elements and the number of elements having the same width (4(F10.5, I10)).

A typical Read In data, described above, is given at the end of the program RALEE.

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```

      DO 739 J=1,M
      K=K+1
739  R(I,J)=43(K)
      DO 17 I=1,N
      DO 17 J=1,M
      HL(I,J)=R(I,J)*(HL(I,J)**2-2.*G(I,J)**2)
17  G(I,J)=R(I,J)*G(I,J)**2
      HV(NZ,1)=HV(N,1)+4(N,1)
      HV(NZ,MZ)=4V(NZ,1)
      DO 19 J=2,M
      HV(NZ,J)=HV(NZ,1)
19  HH(J)=(J-1)*44/M
      HH(1)=J.
      HH(MZ)=M4
      PRINT 230
20  FORMAT(*OX COORDINATES OF COLUMNS*)
      PRINT 22,(HH(J),J=1,MZ)
22  FORMAT(1X,21=6.2)
      PRINT 230
230  FORMAT(*DEPTH TO CORNERS OF ELEMENTS*)
      DO 24 I=1,NZ
      PRINT 22,(HV(I,J),J=1,MZ)
      PRINT 225
225  FORMAT(*LAMBDA OF ELEMENTS*)
      DO 25 I=1,N
      PRINT 22,(HL(I,J),J=1,M)
      PRINT 210
210  FORMAT(*SHEAR MODULUS OF ELEMENTS*)
      DO 21 I=1,N
      PRINT 22,(G(I,J),J=1,M)
      PRINT 220
220  FORMAT(*DENSITY OF ELEMENTS*)
      DO 23 I=1,N
      PRINT 22,(P(I,J),J=1,M)
      OMS=OMEGA*OMEGA
C 5FN 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99
      GENERATE EQUIVALENT BOUNDARY MATRIX
      DO 57 I=1,N
      FA(I)=H(I,2)
      HLA(I)=44(I,2)
      GA(I)=F(I,2)
      FA(I)=S(I,2)
57  GA=SIGVEC(HA,HLA,GA,FA,N)
      PRINT 1001
1001  FORMAT(*EIGENVALUES OF TRANSMITTED MODES*)
      PRINT 59,E
59  FORMAT(1X,1CE13.6)
      FF=E(IR)*(0.01.)
      DO 361 I=1,N3
      IF(REAL(E(I)).EQ.0.) GO TO 361
      VV=OMEGA*FA(E(I))
      Q=414AG(E(I))/REAL(E(I))
      IF(Q.EQ.0.) GO TO 365
      Q=Q**2.
      PRINT 53,Q
369  PRINT 59,VV
351  CONTINUE
      IS=IR

```

```

DO 61 J=1,NP
FA(J)=V(J,IR)
51 FT(J)=E(J)
PRINT 59.(V(I,IR),I=1,NP)
LR=1
CALL BOUNDA(HA,PA,HLA,GA,LR,SD,W,FC,FF)
P1=J.
P2=0.
DO 723 I=1,NP
PB(I)=0.
PC(I)=1.
DO 745 J=1,NP
745 PB(I)=PB(I)+W(I,J)*PA(J)
PC(I)=PC(I)+AIMAG(W(I,J))*PA(J)
723 P1=1+CONJG(PA(I))*PB(I)
P2=P2+CONJG(PA(I))*PC(I)
P1=OMEGA/2.*AIMAG(P1)
Q=P2/P1
PRINT 59.Q
DO 201 I=1,NP
DO 201 J=1,NP
Z(I,J)=W(I,J)
CT(I,J)=C(I,J)
201 VB(I,J)=V(I,J)
C GENERATE EQUIVALENT BOUNDARY MATRIX
DO 56 I=1,N
HA(I)=H(I,1)
HLA(I)=HM(I,1)
GA(I)=F(I,1)
56 HIA(I)=S(I,1)
CALL FIGVEC(HA,HLA,GA,FA,N)
PRINT 1003
1003 FORMAT(*EIGENVALUES OF INCIDENT MODES*)
PRINT 59.E
FC=E(IR)*(0.,1.)
DO 352 I=1,NP
IF(REAL(E(I)).LE.0.) GO TO 352
VV=OMEGA/REAL(F(I))
Q=AIMAG(E(I))/REAL(E(I))
IF(Q.EQ.0.) GO TO 359
Q=P2.
PRINT 59.Q
359 PRINT 59.VV
352 CONTINUE
IN=I.
DO 222 I=1,NP
PB(I)=V(I,IR)
222 PRINT 59.(BR(I),I=1,NP)
C TRANSPLACEMENT MATRIX FOR REFLECTED MODES
DO 63 I=2,NP,2
DO 61 J=1,NP
61 V(I,J)=-V(I,J)
LR=0
CALL BOUNDA(HA,PA,HLA,GA,LR,SD,W,FC,FF)
P1=J.
P2=0.
DO 735 I=1,NP

```

```

      PR(I)=0.
      PR(I)=0.
      755 J=1,NP
      PR(I)=PR(I)+A(I,J)*BR(J)
      755 PR(I)=PR(I)+A1MAG(X(I,J))*BR(J)
      735 PR1=PR1+CONJUG(BR(I))*PR(I)
      PR2=PR2+CONJUG(BR(I))*PR(I)
      PR1=PR1+PR2/2.*A1MAG(P1)
      PR2=PR2/P1
      PRINT 99,0
      DO 51 I=1,NP
      DO 51 J=1,NP
      C(K,J)=C(I,J)
      51 C(K,L)=C(I,J)
      DO 202 I=1,NP
      DO 202 J=1,NP
      212 VA(I,J)=V(I,J)
      DO 11 I=1,NP
      11 BR(I)=BR(I)*100.
      DO 91 I=2,NP,2
      K=I-1 J=2,NP,2
      L=J-1
      V(I,J)=-C(I,J)
      V(K,J)=C(K,J)
      V(K,L)=-C(K,L)
      31 V(I,L)=C(I,L)
      CALCULATE STIFFNESS AND CONSISTENT MASS MATRICES OF SLOPING STRUCTURE
      NH=NN+NN
      DO 27 I=1,NH
      C(I)=0.
      DO 27 J=1,NH
      27 A(I,J)=0.
      READ 2
      NUMBER=K=U
      LCOUNT=0
      IV=0
      DO 170 J=1,M
      DO 170 I=1,N
      IV=IV+2
      IF(IV-NH) 85,85,190
      190 IV=IV-NH
      C 190 IF A BLOCK OF EQUATIONS (64) ON TAPE 2
      190 READ 2 SC(LCOUNT,NR)
      95 U=IV-1
      LU=IV+1
      LV=LU+1
      JU=IV+N
      JV=JU+1
      KU=JV+1
      KV=KV+1
      Y(1,1)=HH(J)
      Y(2,1)=HH(J+1)
      Y(3,1)=HH(J+1)
      Y(4,1)=HH(J)
      Y(1,2)=IV(I,J)
      Y(2,2)=IV(I,J+1)

```

```

Y(3,2)=HV(I+1,J+1)
Y(4,2)=HV(I+1,J)
CAL=STIFFR(Y,HL(I,J),C(I,J),R(I,J),DMS,AK)
IF(1.EJ.N)GO TO 199
A(IJ,NP+3)=A(IJ,NP+3)+AK(1,5)
A(IJ,NP+4)=A(IJ,NP+4)+AK(1,6)
A(IJ,3)=A(IJ,3)+AK(1,7)
A(IJ,4)=A(IJ,4)+AK(1,8)
A(IV,NP+2)=A(IV,NP+2)+AK(2,5)
A(IV,NP+3)=A(IV,NP+3)+AK(2,5)
A(IV,2)=A(IV,2)+AK(2,7)
A(IV,3)=A(IV,3)+AK(2,8)
A(JJ,3)=A(JJ,3)+AK(3,5)
A(JJ,4)=A(JJ,4)+AK(3,6)
A(JV,2)=A(JV,2)+AK(4,5)
A(JV,3)=A(JV,3)+AK(4,6)
A(KU,1)=A(KU,1)+AK(5,5)
A(KU,2)=A(KU,2)+AK(5,6)
A(KV,1)=A(KV,1)+AK(5,6)
A(LJ,1)=A(LJ,1)+AK(7,7)
A(LJ,2)=A(LJ,2)+AK(7,8)
A(LJ,NP-1)=A(LJ,NP-1)+AK(3,7)
A(LJ,NP)=A(LJ,NP)+AK(4,7)
A(LJ,NP+1)=A(LJ,NP+1)+AK(5,7)
A(LJ,NP+2)=A(LJ,NP+2)+AK(6,7)
A(LV,1)=A(LV,1)+AK(8,8)
A(LV,NP-2)=A(LV,NP-2)+AK(3,8)
A(LV,NP-1)=A(LV,NP-1)+AK(4,8)
A(LV,NP)=A(LV,NP)+AK(5,8)
A(LV,NP+1)=A(LV,NP+1)+AK(6,8)
197 A(IJ,1)=A(IJ,1)+AK(1,1)
A(IJ,2)=A(IJ,2)+AK(1,2)
A(IJ,NP+1)=A(IJ,NP+1)+AK(1,3)
A(IJ,NP+2)=A(IJ,NP+2)+AK(1,4)
A(IV,1)=A(IV,1)+AK(2,2)
A(IV,NP)=A(IV,NP)+AK(2,3)
A(IV,NP+1)=A(IV,NP+1)+AK(2,4)
A(JJ,1)=A(JJ,1)+AK(3,3)
A(JJ,2)=A(JJ,2)+AK(3,4)
A(JV,1)=A(JV,1)+AK(4,4)
-2=I+1
-1=I2-1
IF(J.GT.1)GO TO 141
NF=NP-I1+1
NI=NP-I2+1
DO 140 J1=1,NF
NG=I1+J1-1
140 A(IJ,J1)=A(IJ,J1)+C(I1,NG)
DO 150 J1=1,NI
NG=I2+J1-1
150 A(IV,J1)=A(IV,J1)+C(I2,NG)
B(IJ)=J.
B(IV)=J.
DO 71 K=1,NP
B(IJ)=B(IJ)+(C(IJ,K)-V(IJ,K))*BR(K)
71 B(IV)=B(IV)+(C(IV,K)-V(IV,K))*BR(K)
160 IF(J.LT.4)GO TO 170

```

```

      NF=NP-I1+1
      NI=NP-I2+1
      DO 145 J1=1,NF
      NG=I1+J1-1
145  A(JJ,J1)=A(JJ,J1)-CT(I1,NG)
      DO 155 J1=1,NI
      NG=I2+J1-1
155  A(JV,J1)=A(JV,J1)-CT(I2,NG)
170  CONTINUE
180  CALL WRDESC(LCOUNT,NR)
      IF(LCOUNT-NR)195,315,330
315  LCOUNT=LCOUNT+1
330  PRINT 146,NUMB,K
146  FORMAT(*NO. OF BLOCKS OF EQUATIONS =*,I4)
      LCO=LCOUNT-1
      PRINT 147, LCO
147  FORMAT(* NO. OF EQUATIONS =*,I5)
C SOLVE THE EQUATIONS WRITTEN ON PAGE 2
      CALL SANSOL
      PRINT 601
601  FORMAT(*MAXIMUM (COMPLEX) DISPL. OF CORNER POINTS*)
      MN=M*NO
      DO 630 K=1,NR
630  K1(K)=K
      PRINT 602,(K1(I),B(I),J=1,NR)
602  FORMAT(4(I3,2F13.6))
C COMPUTE AMPLITUDES AND PHASE LEADS OF REFLECTED MODES
      DO 41 I=1,NP
41  E(I)=B(I)-BR(I)
      PRINT 2016
2016  FORMAT(*O REFLECTED MODE DISPLACEMENT*)
      PRINT 59,(B(I),I=1,NP)
      DO 508 I=1,NP
      AA(I)=0.
      DO 508 K=1,NP
608  AA(I)=AA(I)+VA(I,K)*B(K)
      DO 603 I=1,NP
603  AMP(I)=ABS(AA(I))
      PHASE(I)=ATAN2(AIMAG(AA(I)),REAL(AA(I)))
      PRINT 604
604  FORMAT(*O AMPLITUDES OF REFLECTED MODES*)
      PRINT 59,AMP
      PRINT 606
606  FORMAT(*O PHASE LEADS OF REFLECTED MODES (AT L-1. END OF SLOPING
2 STRUCTURE)*)
      PRINT 59,PHASE
      HJ=10000./REAL(E(TW))
      DO 614 I=1,IR
614  E(I,1)=AMP(I)**2/REAL(E(I))
      PRINT 620,HJ
620  FORMAT(*O INCIDENT ENERGY =*,E13.6)
      PRINT 621
621  FORMAT(*O REFLECTED ENERGY*)
      PRINT 59,(P(I,1),I=1,IR)
      MN1=MN+1
      J=J
      DO 618 I=MN1,NR

```

```

      J=J+1
819      ES(J)=B(I)
      PRINT 2017
2017      FORMAT(*O RIGHT BOUNDARY MODE DISPLACEMENT*)
      PRINT 59.(SS(I),I=1,NP)
      DO 612 I=1,NP
      AA(I)=J.
      DO 612 K=1,NP
612      AA(I)=AA(I)+VB(I,K)*B(K+MN)
      DO 613 I=1,NP
      AMP(I)=CABS(AA(I))
613      PHASE(I)=ATAN2(AIMAG(AA(I)),REAL(AA(I)))
      PRINT 605
605      FORMAT(*AMPLITUDES OF TRANSMITTED MODES*)
      PRINT 59,AMP
      PRINT 607
607      FORMAT(*PHASE LEADS OF TRANSMITTED MODES (AT R.H. END OF SLOPING
2      STRUCTURE)*)
      PRINT 59,PHASE
      DO 511 I=1,NP
      PA(I)=(REAL(ET(I))*WM+PHASE(I))/OMEGA
511      VV=WM/PA(I)
      PRINT 59,VV
      DO 615 I=1,IS
615      P(I,2)=AMP(I)**2/REAL(ET(I))
      PRINT 622
622      FORMAT(*O TRANSMITTED ENERGY*)
      PRINT 59.(P(I,2),I=1,IS)
      P1=0.
      P2=0.
      P3=0.
      P4=0.
      P5=0.
      P6=0.
      DO 47 I=1,NP
      PA(I)=0.
      PB(I)=0.
      PC(I)=0.
      DO 49 J=1,NF
      PA(I)=PA(I)+A(I,J)*BR(J)
      PB(I)=PB(I)+A(I,J)*B(J)
49      PC(I)=PC(I)+Z(I,J)*BF(J)
      P1=P1+REAL(BR(I))*AIMAG(PA(I))
      P2=P2+AIMAG(BR(I))*REAL(PA(I))
      P3=P3+REAL(B(I))*AIMAG(PB(I))
      P4=P4+AIMAG(B(I))*REAL(PB(I))
      P5=P5+REAL(BS(I))*AIMAG(PC(I))
47      P6=P6+AIMAG(BS(I))*REAL(PC(I))
      EI=(P1-P2)*.5*OMEGA
      EP=(P3-P4)*.5*OMEGA
      ETT=(P5-P6)*.5*OMEGA
      EI=A35(EI)
      EP=A35(EP)
      ETT=A35(ETT)
      PRINT 3333
3333      FORMAT(*O INCIDENT ENERGY*)
      PRINT 59,EI

```

```

PRINT 4444
4444  FORMAT(*O REFLECTED ENERGY*)
PRINT 59,ER
PRINT 5555
5555  FORMAT(*O TRANSMITTED ENERGY*)
PRINT 59,ET1
F1=EI+ET1
F2=0.
DO 307 I=1,NP
PC(I)=0.
DO 405 J=1,NP
405  PC(I)=PC(I)+(SD(I,J)-FD(I,J))*BS(J)
307  P2=P2+CONJG(BR(I))*PC(I)
PRINT 2050
2050  FORMAT(*O CHANGE IN WAVENUMBER BETWEEN TWO BOUNDARIES*)
DK=P2*OMEGA/(4*F1)
PRINT 59,DK
ET1=ET1+EI
ET2=ET1-EI
DK=ET2/ET1*E(IR)
PRINT 59,DK
STOP
END

```

```

1      SUBROUTINE EIGVEC(H,HL,G,P,N)
2      DIMENSION H(15),Z(15)
3      COMPLEX E,E0,D,Z1,Z2,U1,U2,V1,V2,V,A3
4      COMPLEX AL,A1,AV,CH,CV,BH,BV,AH0,BH0,CH0,AV0,CV0
5      COMPLEX A1,A3,B2,B4,C1,C3,Z1,Z2
6      COMPLEX HL(15),G(15)
7      COMPLEX N,NP,N1,N2,N3,CMS,IF,IC,II,A1(30),A3(30),B2(30),B4(30),C1
8      2(30),C3(30),S1(30),S2(30),LD(30),MV3(30),E(20),R1(30),R2(30),J1(30),
9      3,U2(30),V1(30),V2(30),EOLD(30),V(30,30),AP(120)
10     DO 9 I=1,NP
11     EOLD(I)=0.
12     AH0=J.
13     AV0=J.
14     BH0=0.
15     CH0=J.
16     CV0=J.
17     I=J
18     DO 22 J=1,N
19     H3=H(J)/3.
20     AL=HL(J)+G(J)+G(J)
21     CM=Z(J)+H3*OM3
22     AH=A1+H3
23     AV=G(J)+H3
24     CH=G(J)/H(J)-CM
25     CV=AL/H(J)-CM
26     BH=(G(J)-H(J))*0.5
27     BV=(G(J)+HL(J))*0.5
28     I=I+1
29     A1(I)=AH+AH0
30     A3(I)=A1*0.5
31     B2(I)=BH-BH0
32     B4(I)=BV
33     C1(I)=CH+CH0
34     C3(I)=-CH-1.5*CM
35     I=I+1
36     A1(I)=AV+AV0
37     A3(I)=AV*0.5
38     B2(I)=-BV
39     B4(I)=0.
40     C1(I)=CV+CV0
41     C3(I)=-CV-1.5*CM
42     AH0=AH
43     AV0=AV
44     BH0=BH
45     CH0=CH
46     CV0=CV
47     C 22 SOLVE THE NON-LINEAR EIGENVALUE PROBLEM,  $(E^2 + A + E^3 + C)V = 0$ 
48     CALL SECEVA
49     X=ATMAG(AL)
50     IF(X.EQ.0.0) CALL MODAL
51     100 END

```

```

SUBROUTINE BOUNDX(H,R,HL,G,LR,W1,W,FC,FF)
DIMENSION H(15),R(15)
COMMON X E,EOLD,K1,K2,U1,U2,V1,V2,V,A,P,C,D,AB,FC,FF
COMPLEX X A1,A3,S2,S4,C1,C3,S1,S2
COMPLEX HL(15),S(15),WA(30,30),WD(30,30),WC(30,30),WB(30,30)
A(30,30)
COMPLEX AL,A4,AV,BH,BV,CH,CV,AHO,AVO,BHO,CHO,YT,XJ,CVC,
YU,YI,YJ,OK,CK,CL
COMMON ND,NP,N1,N2,N3,NMS,IP,IC,II,A1(30),A3(30),B2(30),B4(30),C1(
30),C3(30),S1(30),S2(30),LO(30),NVR(30),E(30),P1(30),P2(30),M1(3
),U2(30),V1(30),V2(30),EOLD(30),V(30,30),A2(12,1)
COMMON BAN,RG/N1,MN,NUMBLK,R(65),A(FA,FA),C(30,30)
DO 10 J=1,NP
C(1,J)=V(1,J)*A1(1)+V(2,J)*A3(1)
C(2,J)=V(2,J)*A1(2)+V(3,J)*A3(2)
C(4,1,J)=V(N1,J)*A1(N1)+V(N3,J)*A3(N3)
C(NP,J)=V(NP,J)*A1(NP)+V(N2,J)*A3(N2)
DO 10 I=3,N2
C(I,J)=V(I,J)*A1(I)+V(I+2,J)*A3(I)+V(I-2,J)*A3(I-2)
DO 20 J=1,NP
IF(LR.EQ.0.1)GO TO 12
C(J)=(C(J)+(0.,1.))
GO TO 13
C(J)=-C(J)*(0.,1.)
DO 20 I=1,NP
C(I,J)=C(I,J)*D
CALL SETUP(V,ND)
DO 30 I=1,NP
DO 30 J=I,NP
V1(J)=C.
DO 30 K=1,NP
V1(J)=V1(J)+C(I,K)*V(K,J)
DO 40 J=I,NP
C(I,J)=V1(J)
XI=C.
YI=-.
M=J
DO 50 I=2,N2,2
K=I-1
M=M+1
XJ=.5*HL(M)
YJ=.5*G(M)
C(K,I)=C(K,I)+XI-XJ
J=K+3
C(K,J)=C(K,J)+XJ
J=I+1
C(I,J)=C(I,J)+YJ
XI=XJ
YI=YJ
M=M+1
C(N1,NP)=C(N1,NP)+YI-.5*HL(M)
M=
AHO=C.
AVO=C.
BHO=C.
BVO=C.
C40=C.

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```

CVD=0.
XI=0.
YI=J.
DO 11 I=2,N2,2
K=I-1
H=1+I
H3=H(M)/3
AL=HL(M)+G(M)+G(M)
C4=R(M)+H3*OM5
A4=AL*H3
AV=G(M)*H3
B4=.5*HL(M)
BV=.5*G(M)
C4=G(M)/H(M)-CM
CV=AL/H(M)-CM
YJ=-(HL(M)-G(M))*0.5
YJ=(HL(M)+G(M))*0.5
H(K,K)=AH+AH0
H1(K,K)=AH+AH0
H2(K,K)=CH+CH0
H3(K,I)=(YJ-XI)
H(K,I)=BH-BH
H3(K,I)=BH-BH
H(K,I+1)=AH*.5
H1(K,I+1)=AH*.5
H2(I+1,K)=AH*.5
H3(K,I+1)=-CH-1.5*CM
H3(I+1,K)=-CH-1.5*CM
H3(K,K+3)=YJ
H(K,K+3)=BH
H3(K,K+3)=BH
H(I,I)=AV+AV0
H1(I,I)=AV+AV0
H2(I,I)=CV+CV0
H3(I,I+1)=YJ
H(I,I+1)=BV
H3(I,I+1)=BV
H(I,I+2)=AV*.5
H1(I,I+2)=AV*.5
H2(I+2,I)=AV*.5
H1(I+2,I)=AV*.5
H3(I,I+2)=-CV-1.5*CM
H3(I+2,I)=-CV-1.5*CM
AH0=AH
AV0=AV
BH0=BH
BV0=BV
C40=C4
CVD=CV
YI=YJ
XI=XJ
M=M+1
H3=H(M)/3
AL=HL(M)+G(M)+G(M)
C4=R(M)+H3*OM5
A4=AL*H3

```

```

AV=G(M)*H3
BV=.5*G(M)
B4=.5*HL(M)
C4=G(M)/H(M)-CM
CV=AL/H(M)-CM
XJ=-(HL(M)-G(M))*0.5
YJ=(HL(M)+G(M))*0.5
W(N1,N1)=A1+A40
WA(N1,N1)=AH+AH0
WC(N1,N1)=CH+CH0
WJ(N1,NP)=(X1-XJ)
W(N1,NP)=B40-.5*HL(M)
W3(N1,NP)=B40-.5*HL(M)
W(NP,NP)=AV+AV0
WA(NP,NP)=AV+AV0
WC(NP,NP)=CV+CV0
DO 410 I=1,NP
DO 410 J=1,NP
W3(J,I)=W3(I,J)
WJ(J,I)=WJ(I,J)
PRINT 9,FC,FF
29 FORMAT(1X,10(13.6)
IF(LR.EQ.1) GO TO 3032
DO 411 J=1,NP
DO 411 I=1,NP
W(I,J)=W(I,J)*FC
411 WJ(I,J)=WJ(I,J)*FC
CK=FC*FC
DO 412 J=1,NP
DO 412 I=1,NP
412 WA(I,J)=WA(I,J)*CK+WJ(I,J)+WC(I,J)
GO TO 3035
3032 CONTINUE
CK=FF*FF
DO 409 J=1,NP
DO 409 I=1,NP
W(I,J)=W(I,J)*(-FF)
WJ(I,J)=WJ(I,J)*(-FF)
409 WA(I,J)=WA(I,J)*CK
DO 3035 I=1,NP
DO 3035 J=1,NP
3035 WA(I,J)=WA(I,J)+WJ(I,J)+WC(I,J)
3035 END

```

```

SUBROUTINE SECEVA
  COMPLEX E, ED, D, P1, P2, U1, U2, V1, V2, V, A3, EV, EVS, CE, CD, DEV
  COMPLEX A1, A3, B1, B3, C1, C3, D1, D3, E1, E3, F1, F3
  COMPLEX W(30,30)
  COMMON ND, NP, N1, N2, N3, CMS, IF, IC, IL, A1(30), A3(30), B2(30), B4(30), C1(
230), C3(30), S1(30), S2(30), LO(30), MVB(30), E(30), R1(30), R2(30), J1(30)
3, U2(30), V1(30), V2(30), FOLD(30), V(30,30), AB(120)
  DATA EPS/1.0E-7/, EPS2/1.0E-11/
  NN1=NP-1
  NN2=NP*2
  DO 700 N=1, NN1
    MVB(N)=N+3*N
    I=N+1
700  MVB(I)=1+2*N
    MVB(NN1)=NN1+N
    XN=0.
    DO 15 J=1, NP
      X=C1(J)/A1(J)
15    XN=XN+ABS(X)
    XN=SQRT(XN/NP)
    C=CMPLX(XN, XN+XN)
    DO 102 N=1, NP
      LO(N)=0
      EOLD(N)=0.
      F1(N)=(0.,0.)
      F2(N)=(0.,0.)
      F1(N)=1.
      S2(N)=0
102  V1(N)=S1(N)
      V2(N)=S2(N)
      EV=XN/NP
      X1=AIMAG(A1)
      IF(X1.NE.0.0) GO TO 800
      DO 108 N=1, NP
        S2(N)=1.
108  V2(N)=S2(N)
      EV=1.
      DO 900 N=1, NN1, 2
        MVB(N)=N+3*N
        I=N+1
900  MVB(I)=I+2*N
        MVB(NN1)=NN1+N
        K=0
        MC=0
        IM=0
        PRINT 2000
        PRINT 2007
400  K=K+1
        IF(X1.NE.0.0) GO TO 80
        IF(EOLD(K).EQ.0.0) GO TO 110
        EV=EOLD(K)*(1.,0.0001)
        GO TO 87
110  X=CABS(EV)/2.
        EV=CMPLX(X, X)
87  CD=1.
88  DO 112 N=1, NP
      U1(N)=C1(N)*V1(N)

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112 U2(N)=A1(N)*V2(N)
DO 113 N=1,N2
L=N+2
U1(N)=U1(N)+C3(N)*V1(L)
U1(L)=U1(L)+C3(N)*V1(N)
U2(N)=U2(N)+A3(N)*V2(L)
113 U2(L)=U2(L)+A3(N)*V2(N)
IF(X1.EQ.C.C) GO TO A9
IF(IM.EQ.1) GO TO B5
IM=0
CD=1.
X=CABS(EV)/2.
EV=CMPLX(X,X+X)
GO TO B9
85 EV=EV*(1.,.0001)
IM=2
CD=1.
IK=0
DO 200 IT=1,20
EVS=EV**2
DO 120 N=1,N2
V2(N)=(U1(N)-U2(N)*EV)*CD
NQ=N+ND
NR=NQ+ND
NS=NR+ND
AB(N) = EVS*A1(N)+C1(N)
AB(NQ) = EV*B2(N)
AB(NS) = EVS*A3(N)+C3(N)
120 AB(NS) = EV*B4(N)
CE=(0.,0.)
CALL SOLVE
TO 130 N=1,N2
V1(N)=(V2(N)-V1(N)*CD)/EV
130 CE=CE-U1(N)*V1(N)+U2(N)*V2(N)
CE=CE+CD
DO 132 N=1,N2
U1(N)=C1(N)*V1(N)
132 U2(N)=A1(N)*V2(N)
DO 133 N=1,N2
L=N+2
U1(N)=U1(N)+C3(N)*V1(L)
U1(L)=U1(L)+C3(N)*V1(N)
U2(N)=U2(N)+A3(N)*V2(L)
133 U2(L)=U2(L)+A3(N)*V2(N)
CD=(0.,0.)
DO 140 N=1,N2
140 CD=CD-U1(N)*V1(N)+U2(N)*V2(N)
DEV=CE/CD
EV=EV+DEV
CD=1./CSQRT(CD)
C=DEV/EV
X=ABS(REAL(C))+ABS(AIMAG(C))
IF(IX.EQ.1) GO TO 300
IF(X.LT.EPS) IK=1
200 CONTINUE
PRINT 2020
STOP

```

```

15      300 E(K)=EV
      EOLJ(K)=EV
      X=ABS(REAL(EV))
      Y=ABS(AIMAG(EV))
      Z=X+Y
20      IF(Y/Z.LT.EPS2) L0(K)=1
      IF(X/Z.LT.EPS2) L0(K)=2
      DO 310 N=1,NP
      V1(N)=V1(N)*Z
      V2(N)=V2(N)*Z
25      V(N,K)=V1(N)
      U1(N)= U1(N)*Z
      U2(N)= U2(N)*Z
      310 IF(L0(K).NE.1) GO TO 316
      X=(V1(1)**2+V1(2)**2)*FV
      IF(X.GT.0.) GO TO 316
      E(K)=-E(K)
      DO 315 N=2,NP,2
      315 V(N,K)=-V(N,K)
      316 IF(K.EQ.0) GO TO 335
      CE=(J.,0.)
      DO 330 N=1,NP
      330 CE=CE-R1(N)*U1(N)+R2(N)*U2(N)
      X=ABS(REAL(CE))+ABS(AIMAG(CE))
      PRINT 2008,K,IT,E(K),JEV,X
      IF(X.LT.EPS) GO TO 335
      PRINT 2021
      STOP
335 DO 340 N=1,NP
      R1(N)=R1(N)+V1(N)
      340 R2(N)=R2(N)+V2(N)
      IF(X1.NE.0.0) GO TO 450
      J=L0(K)
      CALL ORTHOG(J)
      DO 445 N=1,NP
      445 V1(N)=S1(N)
      V2(N)=S2(N)
      MC=MC+4
      IF(L0(K).NE.J) MC=MC-2
      IF(MC.EQ.NN2) GO TO 100
      GO TO 460
450 C=0.
      J=0.
      DO 342 N=2,NP,2
      M=N-1
      C=C-U1(M)*S1(M)+U2(N)*S2(N)
      342 U=J-U1(N)*S1(N)+U2(M)*S2(M)
      C=C+2.
      J=J*2.
      DO 392 N=2,NP,2
      M=N-1
      65 S1(M)=S1(M)-C*V1(M)
      S1(N)=S1(N)-C*V1(N)
      S2(M)=S2(M)-C*V2(M)
      S2(N)=S2(N)-C*V2(N)
      70 V1(M)=S1(M)
      V1(N)=S1(N)

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```

392 V2(M)=V2(M)
V2(N)=S2(N)
DO 345 N=1,NP
75 V1(N)=S1(N)
345 V2(N)=S2(N)
31 IF (LO(K)+I4.EQ.1) IM=1
MC=MC+1
IF (MC.EQ.NP) GO TO 350
90 GO TO 400
350 X=0.
C=(J.-1.)
DO 610 N=1,NP
X=AIMAG(E(N))
IF (X.LE.J.J) GO TO 600
E(N)=-E(N)
C=-C
600 DO 610 J=2,NP,2
90 610 V(J,N)=V(J,N)*C
I=1
530 IF (I.GT.NP) GO TO 550
X=0.
DO 540 J=I,NP
AC=REAL(E(L))
IF (X.EQ.J.J) X=AC
IF (AC.GT.X) GO TO 540
J=L
X=AC
540 CONTINUE
C=E(I)
E(I)=E(J)
E(J)=C
DO 535 L=1,NP
C=V(L,I)
V(L,I)=V(L,J)
15 535 V(L,J)=C
535 I=I+1
GO TO 530
10 550 CONTINUE
K=J
DO 656 I=1,NP
AD=REAL(E(I))
IF (AD.LT.0.0) GO TO 666
15 656 K=K+1
CONTINUE
IS=K
IP=IS
IN=0
DO 550 I=1,NP
AD=REAL(E(I))
IF (AD.LT.0.0) GO TO 620
20 550 IN=IN+1
S1(IN)=E(I)
DO 556 L=1,NP
25 556 W(L,IN)=V(L,I)
GO TO 560
620 IP=IP+1
S1(IP)=E(I)

```

```

30      DO 557 L=1,NP
557      W(L,IP)=V(L,I)
561      CONTINUE
      DO 555 I=1,NP
555      DO 555 J=1,NP
      V(J,I)=W(J,I)
      F(I)=S1(I)
2001      FORMAT( / 43X,204 OUTPUT FROM SEDEVA )
2007      FORMAT( /124 NO. NO.DF,20X,10HEIGENVAL,JE,27X,154LAST CORRECT!
1.12X,14HDEVIATION FROM/
2.7X,5HITER.,.5X,9HREAL PART,14X,13HIMAG. PART,15X,9HREAL PART
3.3X,13HIMAG. PART,8X,14HORTHOGONALITY /)
2008      FORMAT( 14,15,5X,2E23,15,4X,2E12.5,4X,2E12.5)
2010      FORMAT( /68H NORM OF SEIDJA VECTOR AFTER THE INITIAL VECTOR V1
1 ONE-ELEMENTS /62H HAS BEEN ORTHOGONALIZED WITH RESPECT TO ALL
2E EIGENVECTORS, E18.5)
45 2020      FORMAT(43H1 FAILURE TO CONVERGE IN 20 ITERATION STEPS )
2021      FORMAT(68H1 FAILURE TO FIND AN EIGENVECTOR ORTHOGONAL TO THE ONE
1 FOUND BEFORE )
100      END

```

```

1  SUBROUTINE SOLVE
   COMPLEX E, ED, J, R1, R2, U1, U2, V1, V2, V, AB, C
   COMPLEX A1, A3, B2, B4, C1, C3, J1, J2
   COMMON ND, NP, N1, N2, N3, CMS, IF, IC, II, A1(30), A3(30), B2(30), B4(30), C
2  30), C3(30), S1(30), S2(30), L3(30), MVB(30), E(30), R1(30), R2(30), J1(3
3  3), U2(30), V1(30), V2(30), FOLD(30), V(30,30), AB(120)
   DO 100 N=1, N1
      I=N
      J=N+ND
      M=MVB(N)
      DO 50 L=J, M, ND
         C=AB(L)/AB(N)
         I=I+1
         J1=I
         DO 30 K=L, M, ND
15          AB(J1)=AB(J1)-C*AB(K)
30          J1=J1+ND
         AB(L)=C
50          V2(I)=V2(I)-C*V2(N)
100         V2(N)=V2(N)/AB(N)
      N=NP
      V2(N)=V2(N)/AB(N)
      DO 250 K=1, N1
         N=N-1
         I=N
         J=N+ND
         M=MVB(N)
         DO 250 L=J, M, ND
            I=I+1
250          V2(N)=V2(N)-AB(L)*V2(I)
      END

```

```

1      SUBROUTINE ORTHOC(J)
2      DIMENSION E(0), R1, R2, U1, U2, V1, V2, V, A3, C4, C2
3      DIMENSION A1, A3, B2, B4, C1, C3, A1, B2
4      DIMENSION ND, NP, N1, N2, N3, C43, TR, IC, TI, A1(30), A3(30), B2(30), B4(30), C
5      DIMENSION C3(30), S1(30), S2(30), L3(30), MVB(30), E(30), R1(30), R2(30), J1(3
6      DIMENSION U2(30), V1(30), V2(30), FOLD(30), V(30,30), A3(120)
7      C4=0.
8      C2=0.
9      DO 10 N=2, NP, 2
10     T=N-1
11     C4=C4-U1(M) *S1(M)+J2(N) *S2(N)
12     C2=C2-U1(N) *S1(N)+J2(M) *S2(M)
13     C4=C4*2.
14     C2=C2*2.
15     IF (J.NE.0) GO TO 25
16     DO 20 N=2, NP, 2
17     T=N-1
18     X=C4*V1(M)
19     S1(M)=S1(M)-X-X
20     X=C2*V1(N)
21     S1(N)=S1(N)-X-X
22     X=C2*V2(M)
23     S2(M)=S2(M)-X-X
24     X=C4*V2(N)
25     C2(N)=S2(N)-X-X
26     GO TO 40
27     DO 30 N=2, NP, 2
28     T=N-1
29     S1(M)=S1(M)-C4*V1(M)
30     S1(N)=S1(N)-C2*V1(M)
31     S2(M)=S2(M)-C2*V2(M)
32     S2(N)=S2(N)-C4*V2(N)
33     END

```

```

SUBROUTINE SETUP(V,N)
  COMPLEX F(300),V(30,3)
  REAL RF(300),CF(300),RFI(300),CFI(300)
  N3=N*N
  I=3
  L=3
12  L=L+1
  DO 11 J=1,N
    I=I+1
    F(I)=V(L,J)
    IF(L.LT.N) GO TO 12
    PRINT 59,I
59  FORMAT(I5)
    DO 10 J=1,15
      CF(J)=REAL(F(J))
      CFI(J)=AIMAG(F(J))
10  CONTINUE
    CALL INVERTC(RF,CF,N,RFI,CFI)
    DO 2 J=1,15
      F(J)=COMPLEX(RFI(J),CFI(J))
      I=J
2  L=L+1
  DO 25 J=1,N
    I=I+1
    V(L,J)=F(I)
    IF(L.LT.N) GO TO 25
  RETURN
  END

```

```

SUBROUTINE INVERTC(AAR,AAI,NP,AR,AT)
  REAL AAR(1,1),AAI(1,1),AR(1,1),AT(1,1)
  REAL AMAXR,AMAXI,KK,XI
  DIMENSION ISPACE(3,32)
  NN=NP*NP
  DO 15 I=1,NP
    ISPACE(3,I)=0
  DO 16 J=1,NN,NP
    AK(I,J)=AAR(I,J)
    AT(I,J)=AAI(I,J)
  CONTINUE
15  CONTINUE
  DO 140 I=1,NP
    L41XR=0.
    L41XI=0.
    XR=0.
  DO 50 J=1,NP
    IF(ISPACE(3,J)-1) 20,60,20
    KK=J
    DO 50 K=1,NN,NP
    KK=KK+1
    DO 30 K1=KK,KK
    IF(ISPACE(3,K1)-1) 40,50,30
    CONTINUE
    IF(AR(J,K)+AR(J,K)+AI(J,K)+AT(J,K)-XP) 50,45,45
    IROW=J
    ICC=KK
    L41XR=AR(J,K)
    L41XI=AI(J,K)
    XR=AMAXR+AMAXR+AMAXI+AMAXI
    CONTINUE
    CONTINUE
    ICOLUM=ICC
    ISPACE(3,ICOLUM)=ISPACE(3,ICOLUM)+1
    IF(IROW-ICOLUM) 70,90,70
    DO 80 L=1,NN,NP
    XR=AR(IROW,L)
    XI=AI(IROW,L)
    L41(IROW,L)=AR(ICOLUM,L)
    L41(IROW,L)=AI(ICOLUM,L)
    L41(ICOLUM,L)=XR
    L41(ICOLUM,L)=XI
    ISPACE(1,I)=IROW
    ISPACE(2,I)=ICOLUM
    IC=(ICOLUM-1)*NP+1
    AR(ICOLUM,IC)=1.0
    AI(ICOLUM,IC)=0.0
    DO 101 L=1,NN,NP
    XR=AMAXR+AMAXR+AMAXI+AMAXI
    XI=AR(ICOLUM,L)
    AR(ICOLUM,L)=(XI*AMAXR+AI(ICOLUM,L)*AMAXI)/XR
    AI(ICOLUM,L)=(AI(ICOLUM,L)*AMAXR-XI*AMAXI)/XR
    CONTINUE
    DO 130 L1=1,NP
    IF(L1-ICOLUM) 110,170,110
    XR=AR(L1,IC)
    XI=AI(L1,IC)

```

```

      AT(L1,I)=J.0
      AL(L1,IC)=1.0
      DO 120 L=1,NP,NP
      AP(L1,L)=AR(L1,L)-AR(ICOLUM,L)*XR+AT(ICOLUM,L)*XI
120  AI(L1,L)=AI(L1,L)-AR(ICOLUM,L)*XI+AT(ICOLUM,L)*XR
      CONTINUE
130  CONTINUE
140  CONTINUE
      DO 170 I=1,NP
      C=NP+1-I
      IF (ISPACE(1,L)-ISPACE(2,L)) 150,170,150
150  JROW=ISPACE(1,L)
      JCOLUM=ISPACE(2,L)
      JR=(JROW-1)*NP+1
      JC=(JCOLUM-1)*NP+1
      DO 160 K=1,NP
      XR=AR(K,JR)
      XI=AI(K,JR)
      AR(K,JR)=AR(K,JC)
      AI(K,JR)=AI(K,JC)
      AR(K,JC)=XR
160  AI(K,JC)=XI
170  CONTINUE
      END

```

```

1      SUBROUTINE MODAL
2      COMPLEX E, EC, D, P1, P2, H1, U2, V1, V2, V, A3, C
3      COMPLEX A1, A3, B2, B4, C1, C3, I1, I2
4      COMMON /N/, NP, N1, N2, N3, CM5, I5, I7, TI, A1(30), A3(30), B2(30), B4(30), C1(30), C3(30), S1(30), S2(30), L3(30), MVR(30), E(30), R1(30), P2(30), J1(30), U2(30), V1(30), V2(30), FOLD(30), V(30,30), A3(120)
5      K=NP
6      N=N+1
7      C=(0.,-1.)
8      I=I3(N)+1
9      I=(I-2)/3, 2(,40
10     V=AIMAG(E(N))
11     F=(V,LT,0.) GOTO 12
12     V(N)=CONJG(E(N))
13     DO 11 J=1,NP
14     V(J,N)=CONJG(V(J,N))
15     F(K)=-CONJG(E(N))
16     F(K)=0
17     DO 13 J=1,NP
18     V(J,K)=CONJG(V(J,N))
19     DO 14 J=2,NP,2
20     V(J,K)=-V(J,K)*C
21     K=K-1
22     GOTO 50
23     EV=REAL(E(N))
24     E(N)=EV
25     X=ABS(REAL(V(1,N)))+ABS(REAL(V(2,N)))
26     IF(X.GT.1.E-12) GOTO 34
27     DO 33 L=1,NP
28     V(L,N)=AIMAG(V(L,N))
29     GOTO 50
30     DO 35 L=1,NP
31     V(L,N)=REAL(V(L,N))
32     GOTO 50
33     EV=AIMAG(E(N))
34     F=(V,LT,0.) GOTO 41
35     F(N)=EV
36     C=-C
37     E(N)=C*V(X(0.,EV))
38     DO 51 J=2,NP,2
39     V(J,N)=V(J,N)*C
40     IF(F(J,N).NE.2) GOTO 130
41     X=ABS(REAL(V(1,N)))+ABS(REAL(V(2,N)))
42     IF(X.GT.1.E-12) GOTO 64
43     DO 53 L=1,NP
44     V(L,N)=AIMAG(V(L,N))
45     GOTO 100
46     DO 55 L=1,NP
47     V(L,N)=REAL(V(L,N))
48     I=(N-K)/3, 101, 101
49     I=0
50     I=0
51     DO 105 J=1,NP
52     IF(L(J).EQ.0) I=I+1
53     IF(L(J).EQ.2) I=I+1
54     CONTINUE

```

```

IR=NP-IC-II
I=1
11c IF(I.GT.IR) GOTO 130
X=0.
DO 120 L=I,NP
IF(LJ(L).NE.1) GOTO 120
AC=ABS(REAL(E(L)))
65 IF(X.EQ.0.) X=AC
IF(AC.GT.X) GOTO 120
J=L
X=AC
12c CONTINUE
L=LJ(I)
LO(I)=LO(J)
LO(J)=L
C=E(I)
E(I)=E(J)
E(J)=C
DO 125 L=1,NP
C=V(L,I)
125c V(L,I)=V(L,J)
V(L,J)=C
T=T+1
GOTO 110
13c IF(I.GT.IR+IC) GOTO 150
X=0.
DO 140 L=I,NP
IF(LJ(L).NE.0) GOTO 140
AC=REAL(E(L))
75 IF(X.EQ.0.) X=AC
IF(AC.GT.X) GOTO 140
J=L
X=AC
14c CONTINUE
L=LJ(I)
LO(I)=LO(J)
LO(J)=L
C=E(I)
E(I)=E(J)
E(J)=C
DO 135 L=1,NP
C=V(L,I)
V(L,I)=V(L,J)
135c V(L,J)=C
I=I+1
GOTO 130
15c IF(I.GE.NP) GOTO 170
X=0.
DO 160 L=I,NP
AC=ABS(AIMAG(E(L)))
85 IF(X.EQ.0.) X=AC
IF(AC.GT.X) GOTO 160
J=L
X=AC
15c CONTINUE
L=LJ(I)
LO(I)=LO(J)

```

```
5      LD(J)=L  
      C=E(I)  
      E(I)=E(J)  
      E(J)=C  
      DO 155 I=1,NP  
10      C=V(L,I)  
      V(L,I)=V(L,J)  
155  V(L,J)=C  
      T=T+1  
15      GOTO 150  
170  END
```

```

1      SUBROUTINE STIFF(Y,H1,G1,RH,HF,AK)
2      DIMENSION Y(4,2),E(3),AJ(2,2),Z(2,4),B(4),W(3),A(3,8)
3      1,A4(4,4),AN(6,8)
5      COMPLEX C(3,3),CA(3,8),AK(8,1),AT(3,3,9),F(3),G1
6      E(1)=-.77459565024149
7      E(2)=0.
8      W(1)=.5555555555555555
9      W(2)=.3558868999999999
10     E(3)=-E(1)
11     W(3)=W(1)
12     L=1
13     DO 4 IA=1,3
14     DO 4 JA=1,3
15     Z(1,1)=-1+E(IA)
16     Z(1,2)=1-E(IA)
17     Z(1,3)=1+E(IA)
18     Z(1,4)=-1-E(IA)
19     Z(2,1)=-1+E(JA)
20     Z(2,2)=1-E(JA)
21     Z(2,3)=1+E(JA)
22     Z(2,4)=-1-E(JA)
23     B(1)=Z(2,1)*Z(1,1)/4.
24     B(2)=-Z(2,2)*Z(1,2)/4.
25     B(3)=Z(2,3)*Z(1,3)/4.
26     B(4)=-Z(2,4)*Z(1,4)/4.
27     DO 1 I=1,2
28     DO 1 J=1,2
29     AJ(I,J)=0.
30     DO 1 K=1,4
31     1 AJ(I,J)=AJ(I,J)+Z(I,K)*Y(K,J)/4.
32     DET=AJ(1,1)*AJ(2,2)-AJ(1,2)*AJ(2,1)
33     AJ1=AJ(1,1)
34     AJ(1,1)=AJ(2,2)/DET
35     AJ(1,2)=-AJ(1,2)/DET
36     AJ(2,1)=-AJ(2,1)/DET
37     AJ(2,2)=AJ1/DET
38     DO 2 JJ=1,4
39     JJ=J+J
40     DO 2 I=1,2
41     A(I,JJ-1)=0.
42     A(I,JJ)=0.
43     DO 21 K=1,2
44     A(1,JJ-1)=A(1,JJ-1)+AJ(1,K)*Z(K,J)/4.
45     21 A(2,JJ)=A(2,JJ)+AJ(2,K)*Z(K,J)/4.
46     A(3,JJ-1)=A(2,JJ)
47     A(3,JJ)=A(1,JJ-1)
48     DO 11 I=1,3
49     DO 11 J=1,3
50     11 C(I,J)=0.
51     CC(1,1)=H1+G1+G1
52     CC(2,2)=H1+G1+G1
53     CC(1,2)=H1
54     CC(2,1)=H1
55     CC(3,3)=G1
56     DO 3 I=1,3
57     DO 3 J=1,6
58     CA(I,J)=0.

```

```

DO 3 K=1,3
3 CA(I,J)=CA(I,J)+C(I,K)*A(K,J)
DO 4 J=1,9
DO 5 I=1,8
DO 6 K=1,3
AN(I,J)=0.
DO 7 J=1,3
37 CA(I,J)=AK(I,J)+A(K,I)*CA(K,I)
DO 8 I=1,4
DO 9 J=1,4
A(I,J)=W(I)*B(I)*B(J)
DO 10 I=1,4
DO 11 J=1,4
JJ=JJ+I
AN(I,J)=1. JJ-1)=AM(I,J)
DO 12 I=1,9
DO 13 J=1,9
17 AT(I,J)=(AK(I,J)-AN(I,J))*E
DO 14 I=1,8
DO 15 J=1,9
DO 16 K=1,3
P(K)=3*(K-1)
DO 17 M=1,3
7 P(K)=P(K)+W(M)*AT(I,J,MQ)
AK(I,J)=J
DO 18 M=1,3
AK(I,J)=AK(I,J)+W(M)*P(M)
END

```

```

1  SUBROUTINE WRDSC(LCOUNT,NR)
COMMON/5ANAS/NN,MM,NUMBLK,B(68),A(68,34),C(30,30)
COMP-EX A,B,C
NUMBLK=NUMBLK+1
N=1
DO 310 I1=1,NN
LCOUNT=LCOUNT+1
IF(LCOUNT-N) 320,320,330
320 NZ=NN+I1
E(I1)=3(NZ)
E(NZ)=0.
DO 310 J1=1,MM
310 A(I1,J1)=A(NZ,J1)
330 A(NZ,J1)=0.
END

```

```

SUBROUTINE BAN3OL
  DO 140 N/BANARS/VN.M, NUMBLK, B(59), A(59, 34), C(30, 30)
  COMPLEX A,B,C,D
  NL=NN+1
  NH=NN+NN
  N=1
  N=2
  NB=1
  GO TO 150
100 N5=N3+1
  DO 125 M=1, NV
  NM=NM+N
  B(N)=B(NM)
  B(NM)=0.0
  DO 125 M=1, M4
  B(N,M)=A(NM,M)
125 A(NM,M)=J.0
  IF (NUMBLK-NB) 150, 200, 150
150 A(2)(5(NK), (A(NK,M), M=1, 11), NK=N, NM)
  (NB) 200, 100, 200
200 DO 300 N=1, NN
  IF (A(N,1)) 225, 300, 225
225 B(N)=B(N)/A(N,1)
  DO 275 L=2, M4
  B(N,L)=A(N,L)/A(N,1)
230 B=A(N,L)/A(N,1)
  I=N+L-1
  J=1
  DO 250 K=L, M4
  J=J+1
250 A(I,J)=A(I,J)-J*A(N,K)
  B(I)=B(I)-A(N,L)*B(N)
  A(N,L)=0
275 CONTINUE
300 CONTINUE
  IF (NUMBLK-NB) 375, 400, 375
375 A(1)(B(N), (A(N,M), M=2, M1), N=1, NV)
  GO TO 100
400 DO 450 M=1, NV
  NM=NM+1-M
  DO 425 K=2, M4
  L=N+K-1
425 B(N)=B(N)-A(N,K)*B(L)
  NM=NM+NN
  B(NM)=B(N)
450 A(NM,N3)=B(N)
  NB=N3-1
  IF (NB) 475, 500, 475
475 PACKSPACE 1
  READ (1) (B(N), (A(N,M), M=2, M1), N=1, NV)
  BACKSPACE 1
  GO TO 400
500 K=1
  DO 600 N3=1, NUMBLK
  DO 500 N=1, NV
  NM=NM+NV
  K=K+1

```


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