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NEW YORK STATE DEPT OF ENVIRONMENTAL CONSERVATION ALBANY F/G 13/13
NATIONAL DAM SAFETY PROGRAM. COBLESKILL UPPER RESERVOIR DAM (IN--ETC(U)
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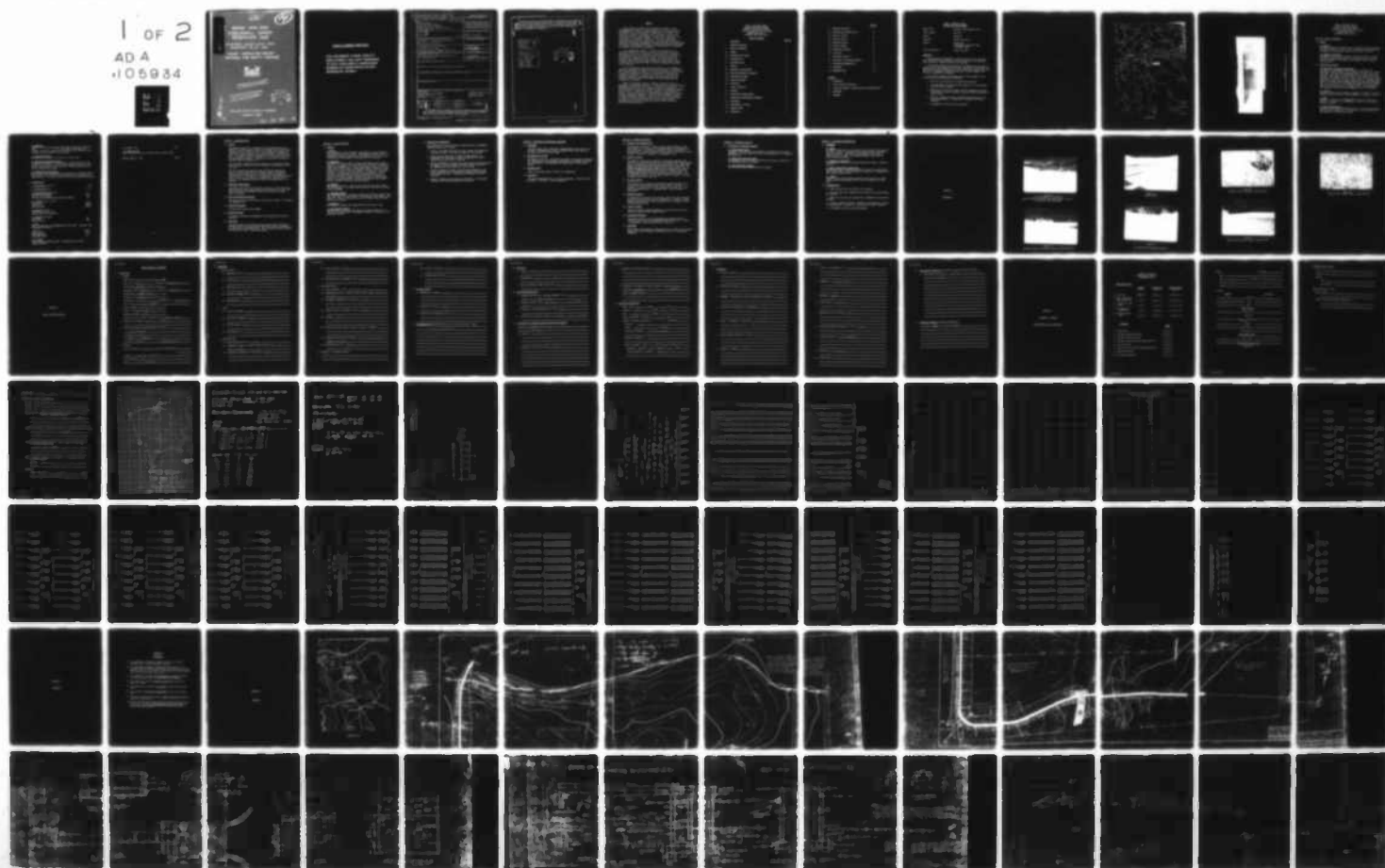
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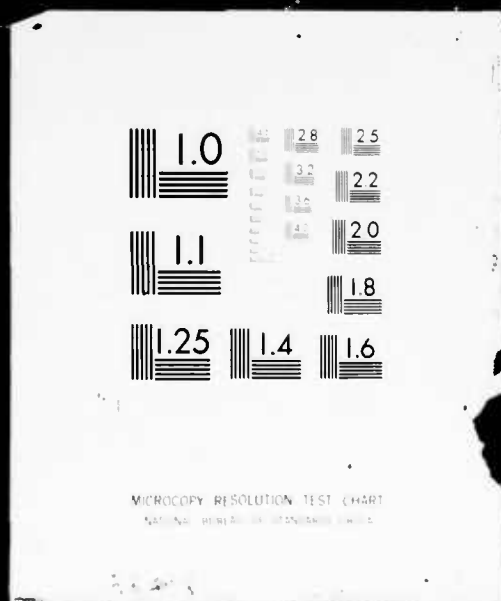
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LEVEL II



MOHAWK RIVER BASIN
COBLESKILL UPPER
RESERVOIR DAM

SCHOHARIE COUNTY, NEW YORK
INVENTORY NO. N.Y. 656

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM



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NEW YORK DISTRICT CORPS OF ENGINEERS
FEBRUARY, 1980

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National Dam Safety Program. Cobleskill Upper Reservoir Dam (Inventory Number NY 656). Mohawk River Basin, Schoharie County, New York. Phase I Inspection Report.

(Entered)

1. TITLE (and Subtitle)

Phase I Inspection Report
Cobleskill Reservoir Dam No. 1:
Mohawk River Basin, Schoharie County, N.Y.
Inventory No. 656

7. AUTHOR(s)

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Cobleskill Reservoir Dam No. 1
Schoharie County
Mohawk River Basin

20. ABSTRACT (Continue on reverse side if necessary and identify by block number)

This report provides information and analysis on the physical condition of the dam as of the report date. Information and analysis are based on visual inspection of the dam by the performing organization.

The examination of documents and visual inspection of the Cobleskill Upper Reservoir Dam and appurtenant structures did not reveal conditions which constitute a hazard to human life or property.

PAGE

2. GOVT ACCESSION NO.

AD-A105 934

HEAD INSTRUCTIONS
BEFORE COMPLETING FORM

3. RECIPIENT'S CATALOG NUMBER

5. TYPE OF REPORT & PERIOD COVERED
Phase I Inspection Report
National Dam Safety Program

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The discharge capacity of the spillway is inadequate for all storms in excess of 57% of the Probable Maximum Flood. During the 1/2 PMF event, the maximum water surface elevation will be 0.33 feet below the top of dam. The dam will be overtopped by 0.28 feet during the full PMF; therefore, the spillway is assessed as "Inadequate".

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PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through frequent inspections can unsafe conditions be detected and only through continued care and maintenance can these conditions be prevented or corrected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM
COBLESKILL UPPER RESERVOIR DAM
I.D. No. NY 656 DEC No. 174A-3138B
MOHAWK RIVER BASIN
SCHOHARIE COUNTY, NEW YORK

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PHASE I INSPECTION REPORT
NATIONAL DAM SAFETY PROGRAM

Name of Dam: Cobleskill Upper Reservoir Dam
State Located: New York
County: Schoharie
Watershed: Mohawk River Basin
Stream: Smith Brook
(tributary of Cobleskill Creek
and Mohawk River)
Date of Inspection: October 30, 1980

ASSESSMENT

The examination of documents and visual inspection of the Cobleskill Upper Reservoir Dam and appurtenant structures did not reveal conditions which constitute a hazard to human life or property.

The discharge capacity of the spillway is inadequate for all storms in excess of 57% of the Probable Maximum Flood. During the 1/2 PMF event, the maximum water surface elevation will be 0.33 feet below the top of dam. The dam will be overtopped by 0.28 feet during the full PMF; therefore, the spillway is assessed as "Inadequate".

The following problems were observed which require remedial action within one year of notification to the owner:

1. Joints in the spillway must be cleaned and recaulked.
2. The voids under the riprap spillway channel must be investigated and backfilled or grouted.
3. Remove the trees and brush growth from the embankment and around the spillway. Provide a program of periodic cutting and mowing of these surfaces.
4. Provide a program of periodic inspection and maintenance of the dam and appurtenances. Document this information for future reference.
5. An emergency action plan must be developed.

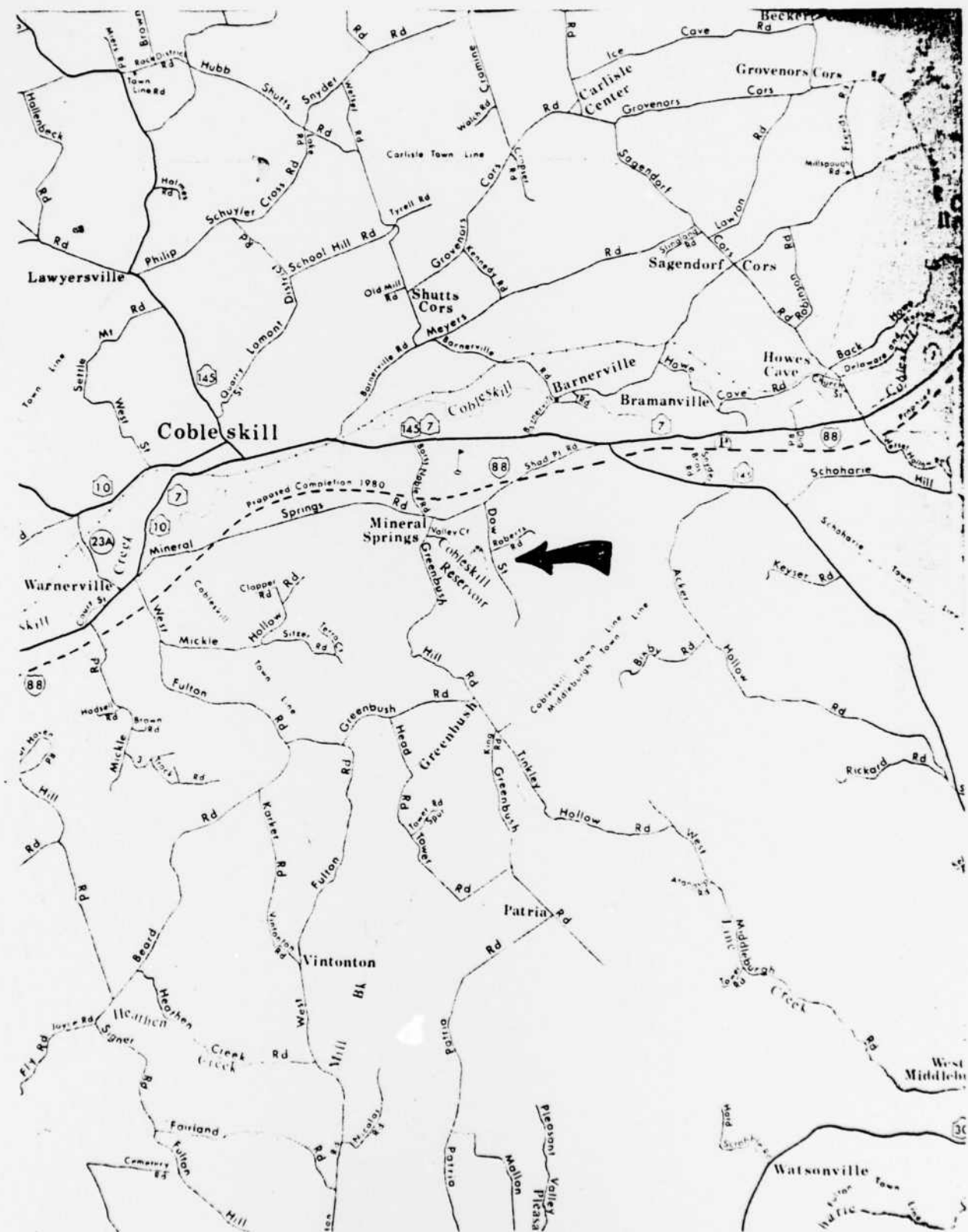
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JUL 28 1981



VICINITY MAP

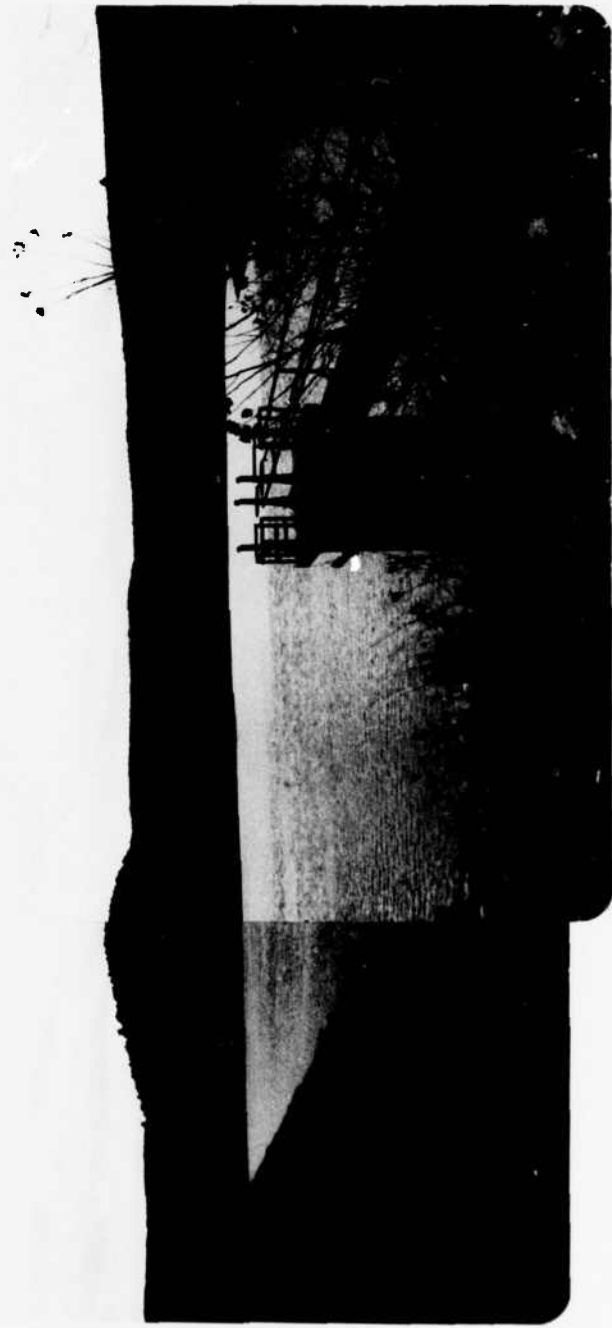


Photo #1
Overview of Cobleskill Upper Reservoir Dam

PHASE I INSPECTION REPORT
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COBLESKILL UPPER RESERVOIR DAM
I.D. No. NY 656 DEC No. 174A - 3138B
MOHAWK RIVER BASIN
SCHOHARIE COUNTY, NEW YORK

SECTION 1: PROJECT INFORMATION

1.1 GENERAL

a. Authority

The Phase I inspection reported herein was authorized by the Department of the Army, New York District, Corps of Engineers, to fulfill the requirements of the Nation Dam Inspection Act, Public Law 92-367.

b. Purpose of Inspection

Evaluation of the existing conditions of the subject dam to identify deficiencies and hazardous conditions, determine if they constitute hazards to human life and property and recommend remedial measures where necessary.

1.2 DESCRIPTION OF PROJECT

a. Description of Dam and Appurtenances

The Cobleskill Upper Reservoir Dam consists of a 2200 feet long homogenous earth embankment, with a compacted core trench to an impervious layer. One leg of the embankment follows the shore line of the Cobleskill Lower Reservoir with 90° bend around a water supply intake the other leg stretches across Smith Brook. The upstream slope of the embankment is protected with stone. The spillway is a concrete overflow section discharging onto a sloped concrete apron and riprapped channel. There is a 12" drain located through the highest portion of the embankment across Smith Brook. It is a cast iron pipe with a concrete gate tower at the dam crest just west of the spillway. The embankment is approximately 30 feet high at its highest point across Smith Brook.

b. Location

The dam is located on Smith Brook, a tributary of Cobleskill Creek and Mohawk River approximately 2.0 miles southeast of Cobleskill, New York.

c. Size

The dam is 30 feet high and impounds 239. acre feet at normal pool elevation. Therefore, it is classified as "small" in size (less than 40 feet in height.)

d. Hazard Classification

The dam is classified as high hazard due to its location above several homes in Mineral Springs, NY. The reservoir is also a major part of the Cobleskill, NY water supply system.

e. Ownership

The dam is owned and operated by the Village of Cobleskill, New York, 12093. Mr. John Barber, Water Supervisor, Village of Cobleskill, Cobleskill, New York, was the contact with the Village, he can be reached at (518) 234-2195.

f. Purpose of the Dam

The dam provides storage for the Cobleskill water supply.

g. Design and Construction History

The dam was constructed in 1967 according to the design of and under the direction of James S. Van Deusen, P.E. Cobleskill, New York. The original application for permit was filed in 1963 but funding held up start of construction 4 years.

h. Normal Operating Procedures

Water releases from the Cobleskill Upper Reservoir are normally through the intake and into the water supply either directly or through the lower reservoir. Any excess flow goes through the uncontrolled spillway.

1.3 PERTINENT DATA

a. Drainage Area (sq. mi.) 1.26

b. Height of Dam (ft.) 30.0

c. Discharge @ Dam Site (cfs.)

Spillway at Top of Dam. 1195.

Reservoir Drain (Normal) 20.

12" Cast Iron Overflow to Lower Reservoir (Normal) 12.

d. Elevations (ft. I.S. G.S.)

Top of Dam 1184.5

Spillway Crest 1180.6

Reservoir Drain 1150.6

e. Reservoir (acres)

Surface area of Top of Dam

Surface Area of Spillway Crest.

f. Storage (Acre feet)

Top of Dam 353.

Spillway Crest 239.

g. Dam

Type: Homogeneous earth embankment with core trench. Upstream slope protected with rockfill.

Length (ft): 2200'

Upstream Slope: 14: 1

Downstream Slope: 1: 3

Crest Width (ft): 15.

h. Spillway

Type: Concrete overflow section. Concrete apron and riprap discharge channel.

Weir length (ft): 41.0

i. Reservoir Drain

Type: 12" cast iron pipe, with gate tower at crest of dam.

Maximum Capacity (cfs). 20.00

SECTION 2: ENGINEERING DATA

2.1 GEOLOGY

The Cobleskill Upper Dam is located in the glaciated portion of the Appalachian Uplands (northern extreme of the Appalachian Plateau) physiographic province of New York State. These uplands were formed by the dissection of the uplifted but flat lying sandstones, siltstones and shales of the Lower and Middle Devonian Period (395 to 365 million years ago). The plateau surface is represented by flat-topped divides with drainage generally southward. Drainage in the vicinity of the dam is northward toward the Mohawk River.

Glacial cover is generally thin, the deposits of which have resulted from glaciations during the Wisconsin glaciation, approximately 11,000 years ago.

The "Preliminary Brittle Structures Map of New York" prepared by Yngvar W. Isohson and William G. McKendree (dated 1977) indicates the presence of a subsurface fault, showing relative movement as inferred from drill hole data, and a topographic linear feature observed on one or more of the following: topographic map, Landsat (ERTS), Skylab, or U-2 photographic product, within the drainage area of the reservoir.

2.2 SUBSURFACE INVESTIGATION

Two borings were located for the design of the dam. These explorations indicate that the soils encountered are varying mixtures of clay, silt, sand, and gravel with boulders, of glacial till origin. No water table is indicated.

2.3 DAM AND APPURTENANT STRUCTURES

The dam was erected in 1967 under the supervision of James S. Van Deusen, P.E. Cobleskill, NY.

2.4 CONSTRUCTION RECORDS

No construction records are available.

2.5 OPERATION RECORDS

All operation records are maintained at the treatment plant below the Lower Dam.

2.6 EVALUATION

The data presented in this report has been compiled from information obtained from Mr. John Barber, Water Supervisor, Village of Cobleskill, NY, and the New York State DEC files. This information appears adequate and reliable for Phase I Inspection purposes.

SECTION 3: VISUAL INSPECTION

3.1 FINDINGS

a. General

Visual inspection of the Cobleskill Upper Reservoir Dam and watershed was conducted on October 30, 1980. The weather was cloudy and the temperatures ranged in the thirties. The reservoir level at the time of inspection was approximately 5. feet below the spillway crest.

b. Embankment

The embankment shows no signs of distress and appears to be in good condition. All slopes seem to be stable with no erosion or depressions. However, the embankment is in need of maintenance, there is tree and brush growth on both slopes. The upstream riprap is generally in good shape but the trees and brush will begin to displace it, if left unattended to. The crest is in good condition. No evidence of seepage was found on the embankment or beyond the downstream toe. Low reservoir elevation at the time of inspection may have reduced the possibility of finding seepage, however, there were no other factors pointing to seepage problems. The discharge channel of the reservoir drain is in need of small tree and brush removal.

c. Spillway

The concrete spillway is in good condition but also in need of some minor maintenance. All joints in walls and floor slabs must be cleaned out and recaulked.

d. Downstream Channel

The riprapped channel is in need of further investigation, several voids were found under the grouted stone in the lower end of the channel. See photo # 6. There is also some brush and tree growth along the channel which must be cleared.

e. Reservoirs

No sediment or problems were reported within the reservoir area.

f. Appurtenant Structures

Both the reservoir drain and the lines to the water system are all reported to be operational. The concrete intake wells appeared to be in very good condition.

3.2 EVALUATION OF OBSERVATIONS

The problem areas observed during the inspection and the recommended remedial measures are as follows:

1. Joints in the concrete spillway are full of debris and vegetation is growing in them. They must be cleaned out and recaulked.
2. There are voids under the rip rapped spillway channel. This problem must be investigated further to determine the extent of undermining and backfilled or regrouted.
3. Both the embankment and channel have tree and brush growth which must be removed. Provide a program of periodic cutting and mowing of the embankment and areas adjacent to the spillway.
4. Provide a program of periodic inspection and maintenance of the dam and appurtenances including yearly operation and lubrication of all gates and valves. Document this information for future reference.
5. Develop an emergency action plan for notification of downstream residents and the proper governmental authorities.

SECTION 4: OPERATION AND MAINTENANCE PROCEDURES

4.1 PROCEDURES

The normal water surface elevation is approximated by the crest of the spillway. Discharges can be to the treatment plant, lower reservoir, low level drain or over the spillway.

4.2 MAINTENANCE OF THE DAM

Maintenance of the dam is provided by the owner, the Village of Cobleskill, NY. Maintenance is not considered satisfactory as evidenced by the tree and brush growth on the embankment and deterioration of the spillway joints.

4.3 WARNING SYSTEM

There is no warning system in effect or in preparation.

4.4 EVALUATION

The dam and appurtenances are in need of maintenance. The areas sited are noted in "Section 3: Visual Inspection".

SECTION 5: HYDRAULIC/HYDROGOGIC

5.1 DRAINAGE AREA CHARACTERISTICS

The Cobleskill Upper Dam is located on Smith Brook, adjacent to the Lower Reservoir. Smith Brook has a drainage area of 1.26 square miles at the site and is a tributary of Cobleskill Creek and Mohawk River. The watershed is primarily wooded with some pasture in the lower, flatter portions. The cover is glacial till and generally thin.

5.2 ANALYSIS CRITERIA

The analysis of the spillway capacity of the dam and storage of the reservoir was performed using the Corps of Engineers HEC-1 computer model. The unit hydrograph was defined by the Snyder Synthetic Unit Hydrograph method, and the Modified Puls routing procedure was incorporated. The Probable Maximum Precipitation (PMP) was 19.5 inches (24 hrs., 200 sq. miles) from Hydrometeorological Report #33 in accordance with the recommended guidelines of the Corps of Engineers. Several floods (%s of the Probable Maximum Flood (PMF) were selected for analysis. The full (PMF) inflow of 2273. cfs was routed through the reservoir and found to produce an outflow of 2271 cfs.

5.3 SPILLWAY CAPACITY

The spillway is a 42.5 feet wide uncontrolled concrete section with a 1.5 feet thick pier leaving 41 feet of weir length. The crest elevation is 1180.55 U.S.G.S. with a maximum flow height of 4. feet before overtopping occurs. The maximum flow is 1195. cfs before overtopping.

5.4 RESERVOIR CAPACITY

The reservoir at the crest of the spillway, and at the top of the dam are 239. acre feet and 353 acre feet respectively. Surcharge, storage between spillway and top of dam is equivalent to 1.70 inches of runoff from the watershed area.

5.5 FLOODS OF RECORD

There are no gaging stations located on or near the dam site nor are there any accounts of high flows or levels.

5.6 OVERTOPPING POTENTIAL

The maximum capacity of the spillway before overtopping occurs is 1195 cfs, which is 57% of the routed PMF inflow of 2273 cfs. The dam is overtopped by 0.28 feet during the full PMF event.

5.7 EVALUATION

The spillway of the Cobleskill Upper Reservoir will handle 57% of the PMF. Based on the Corps of Engineers screening criteria it is considered inadequate.

SECTION 6: STRUCTURAL STABILITY

6.1 EVALUATION OF STRUCTURAL STABILITY

a. Visual Observations

No signs of major distress were observed in connection with the earth embankment. The gate house and spillway both appeared to be in generally good condition.

b. Design and Construction Data

No information could be located concerning the structural stability of the embankment portion of the dam.

c. Post Construction Changes

No post construction changes have been initiated.

SECTION 7: ASSESSMENT/RECOMMENDATIONS

7.1 ASSESSMENT

a. Safety

The Phase I Inspection of the Cobleskill Upper Reservoir Dam revealed that the spillway is "Inadequate" based upon the Corps of Engineers screening criteria. The dam will be overtopped by all storms in excess of 57% of the PMF. The embankment appears stable with few minor maintenance problems.

b. Adequacy of Information

The information reviewed is considered adequate for Phase I Inspection purposes.

c. Need for Additional Investigation

The only additional investigation required is that of the voids under the spillway channel. Once the degree of undermining is determined the best solution can be applied.

d. Urgency

The areas listed below requiring remedial action should be initiated within 3 months and completed within 1 year of notification to the owner.

7.2 RECOMMENDATIONS

1. Clean out and recaulk the joints in the spillway.
2. The voids under the riprapped spillway channel must be investigated and repaired.
3. Remove the tree and brush growth from the embankment and around the spillway.
4. Provide a program of periodic inspection and maintenance of the dam and appurtenances. Document this information for future reference.
5. An emergency action plan must be developed.

APPENDIX A

PHOTOGRAPHS



Photo # 2.
View from upper end of reservoir.
Note: Spillway on left and water supply intake in corner
of reservoir, also tree growth.



Photo # 3.
Relative location of Cobleskill Upper and Lower Reservoirs.



Photo # 4.
Spillway Crest.



Photo #5
Spillway apron and downstream channel.



Photo # 6.
Opening to larger void under spillway channel.



Photo # 7.
Reservoir drain control, Note: trees and brush.



Photo # 8.
Reservoir drain outlet. Note: trees and brush.

APPENDIX B

VISUAL INSPECTION CHECKLIST

VISUAL INSPECTION CHECKLIST1) Basic Data

a. General

Name of Dam Cobleskill Upper Reservoir DAM
Fed. I.D. # NY 656 DEC Dam No. 174 A - 3138 B
River Basin MOHAWIC RIVER BASIN
Location: Town Cobleskill County SCHENECTADY
Stream Name Smith Brook
Tributary of Cobleskill Creek
Latitude (N) 42°40.0' Longitude (W) 74°07.0'
Type of Dam homogeneous earthfill
Hazard Category high (C)
Date(s) of Inspection Oct. 30, 1980
Weather Conditions cloudy - 30's
Reservoir Level at Time of Inspection 5 feet below spillcrest

b. Inspection Personnel [McGowan] 1 tech

c. Persons Contacted (Including Address & Phone No.)

John D. Parker
Water Supervisor
Village of Cobleskill
NY 12093 (518) 234-2195

d. History:

Date Constructed 1967 Date(s) ReconstructedDesigner James S. Vign. DesignConstructed By —Owner Village of Cobleskill

2) Embankment

a. Characteristics

- (1) Embankment Material fill
- (2) Cutoff Type excavated earth trench
- (3) Impervious Core none
- (4) Internal Drainage System none
- (5) Miscellaneous —

b. Crest

- (1) Vertical Alignment good
- (2) Horizontal Alignment good
- (3) Surface Cracks none
- (4) Miscellaneous —

c. Upstream Slope

- (1) Slope (Estimate) (V:H) 1.5
- (2) Undesirable Growth or Debris, Animal Burrows tree and brush growth
- (3) Sloughing, Subsidence or Depressions none

(4) Slope Protection gravel - filter on sand
rock armor

(5) Surface Cracks or Movement at Toe None

d. Downstream Slope

(1) Slope (Estimate - V:H) 1:3

(2) Undesirable Growth or Debris, Animal Burrows tree and brush
growth

(3) Sloughing, Subsidence or Depressions None

(4) Surface Cracks or Movement at Toe None

(5) Seepage None

(6) External Drainage System (Ditches, Trenches; Blanket) _____

(7) Condition Around Outlet Structure good - works earlier
spillway channel

(8) Seepage Beyond Toe None

e. Abutments - Embankment Contact

good

93-15-3(9/80)

(1) Erosion at Contact None(2) Seepage Along Contact None3) Drainage Systema. Description of System Noneb. Condition of System ✓c. Discharge from Drainage System —4) Instrumentation (Monumentation/Surveys, Observation Wells, Weirs, Piezometers, Etc.)None

93-15-3(9/80)

5) Reservoir

- a. Slopes stable - no problem
- b. Sedimentation little - no problem
- c. Unusual Conditions Which Affect Dam None

6) Area Downstream of Dam

- a. Downstream Hazard (No. of Homes, Highways, etc.) Some
Homes in Mineral Springs
- b. Seepage, Unusual Growth no seepage or unusual
growth
- c. Evidence of Movement Beyond Toe of Dam None
- d. Condition of Downstream Channel trash & trees must be cleared

7) Spillway(s) (Including Discharge Conveyance Channel)

- good condition generally - just need cleaning
& repaving
- a. General

- b. Condition of Service Spillway good

c. Condition of Auxiliary Spillway noned. Condition of Discharge Conveyance Channel 100% eachgrated riprap - must be investigated
generally good8) Reservoir Drain/OutletType: Pipe ☒ Conduit _____ Other _____Material: Concrete _____ Metal P.I. Other _____Size: 12" Length _____

Invert Elevations: Entrance _____ Exit _____

Physical Condition (Describe): operable Unobservable _____Material: P.I.

Joints: _____ Alignment _____

Structural Integrity: assumed goodHydraulic Capability: 20 cfsMeans of Control: Gate _____ Valve ☒ Uncontrolled _____Operation: Operable ☒ Inoperable _____ Other _____Present Condition (Describe): good

9) Structural

- a. Concrete Surfaces yes
- b. Structural Cracking minor surface
- c. Movement - Horizontal & Vertical Alignment (Settlement) none
- d. Junctions with Abutments or Embankments yes
- e. Drains - Foundation, Joint, Face none
- f. Water Passages, Conduits, Sluices —
- g. Seepage or Leakage none?

- h. Joints - Construction, etc. Good condition
- i. Foundation apparently good
- j. Abutments good
- k. Control Gates good
- l. Approach & Outlet Channels good
- m. Energy Dissipators (Plunge Pool, etc.) —
- n. Intake Structures good condition
- o. Stability in good condition
- p. Miscellaneous —

10) Appurtenant Structures (Power House, Lock, Gatehouse, Other)

a. Description and Condition

gate tower & drain tower
good condition11) Operation Procedures (Lake Level Regulation):water supply - checked daily

APPENDIX C

HYDROLOGIC / HYDRAULIC

ENGINEERING DATA AND COMPUTATIONS

CHECK LIST FOR DAMS
HYDROLOGIC AND HYDRAULIC
ENGINEERING DATA

1

AREA-CAPACITY DATA:

	<u>Elevation</u> (ft.)	<u>Surface Area</u> (acres)	<u>Storage Capacity</u> (acre-ft.)
1) Top of Dam	<u>1184.55</u>	<u>32</u>	<u>353</u>
2) Design High Water (Max. Design Pool)	<u>-</u>	<u>-</u>	<u>-</u>
3) Auxiliary Spillway Crest	<u>-</u>	<u>-</u>	<u>-</u>
4) Pool Level with Flashboards	<u>-</u>	<u>-</u>	<u>-</u>
5) Service Spillway Crest	<u>1186.55</u>	<u>25</u>	<u>237</u>

DISCHARGES

	<u>Volume</u> (cfs)
1) Average Daily	<u>1.</u>
2) Spillway @ Maximum High Water	<u>1125</u>
3) Spillway @ Design High Water	<u>-</u>
4) Spillway @ Auxiliary Spillway Crest Elevation	<u>-</u>
5) Low Level Outlet	<u>20</u>
6) Total (of all facilities) @ Maximum High Water	<u>1027</u>
7) Maximum Known Flood	<u>-</u>
8) At Time of Inspection	<u>0.</u>

CREST:

ELEVATION: 1124.55Type: Concrete with gabion crestWidth: 15' Length: 2300'Spillover ~Location ~

SPILLWAY:

SERVICE

AUXILIARY

1180.55 Elevation —uncontrolled overflow section Type —43.5' Width —✓ Type of Control— Uncontrolled —

Controlled:

— Type —
(Flashboards; gate)— Number —— Size/Length —Invert Material —Anticipated Length
of operating service —30' Chute Length —1:3 slope Height Between Spillway Crest
& Approach Channel Invert —
(Weir Flow)

HYDROMETEROLOGICAL GAGES:

Type : 1000Location: -

Records:

Date - -Max. Reading - -

FLOOD WATER CONTROL SYSTEM:

Warning System: 1000

Method of Controlled Releases (mechanisms):

12" C.I. reservoir down to stream12" C.I. pipe to lower reservoir

DRAINAGE AREA: 1.26 square miles

DRAINAGE BASIN RUNOFF CHARACTERISTICS:

Land Use - Type: mostly wooded 3% cropland

Terrain - Relief: rolling top with some hills

Surface - Soil: thin fill overlying rock

Runoff Potential (existing or planned extensive alterations to existing (surface or subsurface conditions)

no alterations made or to be made to basin

Potential Sedimentation problem areas (natural or man-made; present or future)

none existing at time of inspection

Potential Backwater problem areas for levels at maximum storage capacity including surcharge storage:

secondary run, possibly

Dikes - Floodwalls (overflow & non-overflow) - Low reaches along the Reservoir perimeter:

Location: None.

Elevation: _____

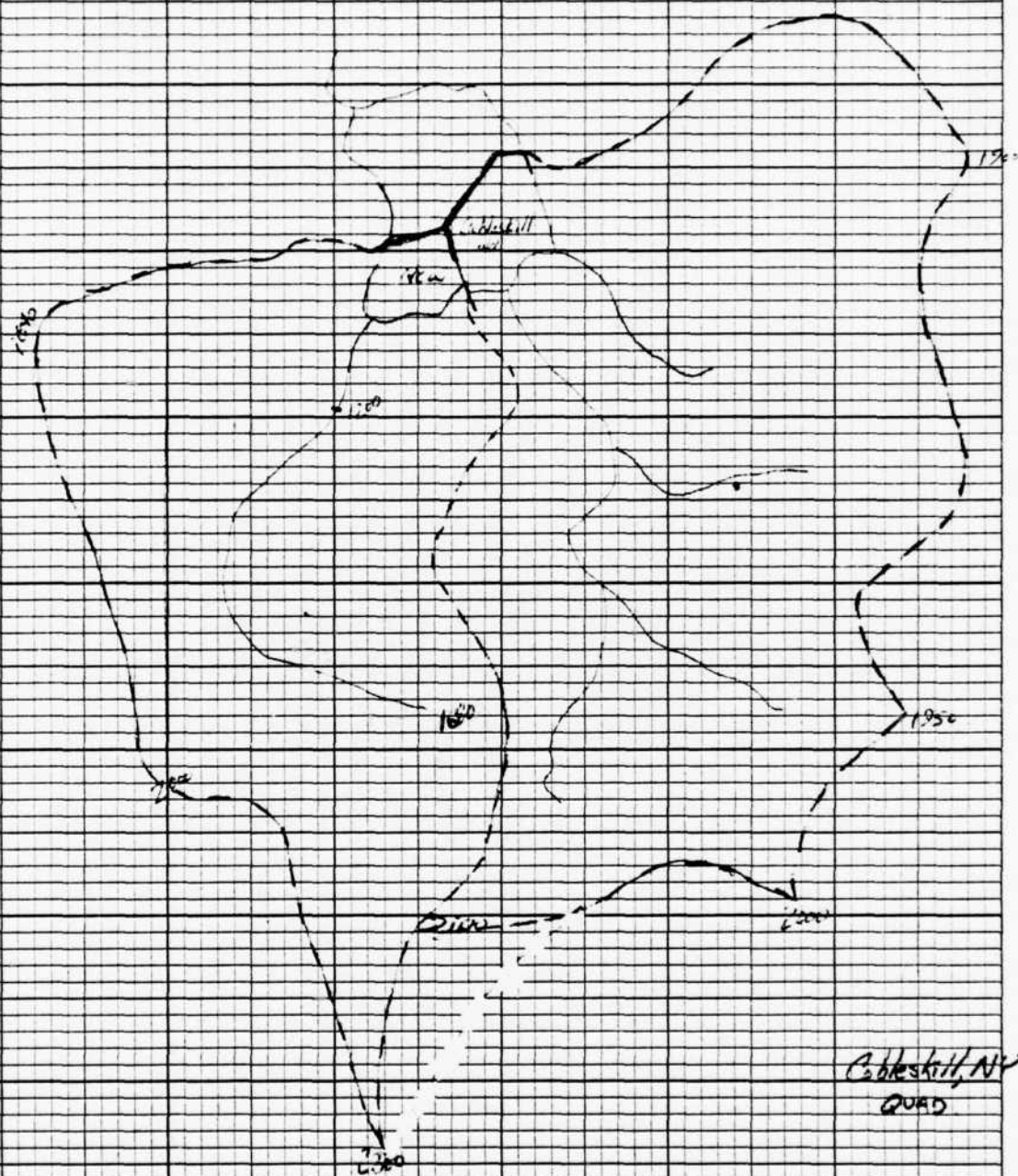
Reservoir:

Length @ Maximum Pool 1200 ft. (Miles)

Length of Shoreline (@ Spillway Crest) 2500 ft. (Miles)

46 0782

10 X 10 TO THE INCH • 7 X 10 INCHES
KEUFFEL & ESSER CO. MADE IN U.S.A.



Cobleskill old res. 1170.00 }
New res. 1180.55 } U.S.G.S.

New basin $\frac{6 \frac{1}{2} (24000)}{12 (5.00)} = 2.0 \text{ mi} = L$

$2 \frac{1}{2} (1.1) = 1.10 = L_{eq}$

42°39' N

$t_p = C_p (L \times t_r)^{0.2} = 2 (2.27 \times 11)^{0.2} = 2.63$
 $t_r = 4.8$

$C_p = 0.625$ $T_p = t_p + 0.5 (t_r) = 2.87 \text{ hr.}$

Cobblekill (New Reservoir) No data UNANNO TRIB OR Cobblekill Creek.

Planimetered AREA: 8.80 sq. in. $\left(\frac{24000^2}{144(43560)} \right) = 808.1 \text{ ACRES}$
 { SMITH BROOK NEW }
 { DOW BROOK old }

Reservoir Capacity / SPILLWAY CAPACITY

41' long 4.0' d C=3.5
 crest el 1180.55
 Top of Dam 1184.55
 $L_{DAM} = 966 + 1200 = 2166'$ C=3.0

STORAGE

CAPACITY

EL. as per plan	DSGS EL	AREA	AREA	Δ Vol. (K-Ft.)	Σ Vol.
1126	1156.55	0	0	0	0
1130	1160.55	.22	.11	.44	.44
1136	1166.55	4.2	2.21	13.3	13.7
1140	1170.55	6.6	5.40	21.6	35.3
1146	1176.55				122.
1150	1180.55	25.	14.4	144.0	267.

SPILLWAY CAP.

C = 41' 2% pier reduction.

EL.	H.	C	Q
1180.55	0.0	—	—
1181.00	0.45	3.2	39
1181.55	1.00	3.4	136
1182.00	1.45	3.5	244
1182.55	2.00	3.6	407
1183.55	3.00	3.8	790
1184.55	4.00	3.8	1216
1185.55	5.00	3.8	1700
1187.55	7.00	3.8	

TOP OF DAM $C = 3$. $L = 2166'$

$$A = 1.26 \text{ mi.}^2 \quad L = 8000' \quad \Delta H = 1840 - 1160 = 680'$$

$$T_c = \left(\frac{11.9 \cdot L^3}{H} \right)^{0.385} = \left(\frac{11.9 (1.515)^3}{680} \right)^{0.385} = .34 \text{ hr} = 20 \text{ MIN.}$$

Q = 1.0 "

$$T_p = \frac{P}{2} + 0.6 T_c = .125 + .6(.34) = 0.33 \text{ hr} = 20 \text{ min.}$$

$$q_p = \frac{484 \text{ AQ}}{T_p} = \frac{484(1.26) 1.}{.33} = 1898. \text{ cfs}$$

$$T_p = .704 \text{ hr} = 42. \text{ MIN}$$

9. = 871 cfs.

PREVIEW OF SEQUENCE OF STREAM NETWORK CALCULATIONS

1

RUNOFF HYDROGRAPH AT

ROUTE HYDROGRAPH TO

END OF NETWORK

 NEW YORK STATE
 DEPT OF ENVIRONMENTAL CONSERVATION
 FLECC PROTECTION BUREAU

 FLECC PROTECTION BUREAU (FPC-1)
 LAST MODIFICATION JULY 1975
 LAST MODIFICATION 26 FEB 79
 PREPARED FOR NEWYORK STATE

PC DATE 02/04/81

CABLESKILL NEW RESERVOIR DAM
 SMITH BROOK
 3FEB1981

JOB SPECIFICATION
 AS NHR NMN IDAY IMR IMIN METRC IPLT IPRT INSTAN
 200 C 15 C C C C C C C
 JSEPR NWT LRPT TRACE
 5 C O C

MULTI-PLAN ANALYSES TO BE PERFORMED
 PLAN= 1 PARTIO= 6 LRATIO= 1
 RTICS= 0.20 0.40 0.50 0.60 0.80 1.00

***** ***** *****

SUB-AREA RUNOFF COMPUTATION

INFLOW FROM SUB-BASIN
 ISTAQ ICOMP IECCN ITAPE JPLT JPRT INAME ISTAGE IAUO
 1 0 0 0 2 0 1 0 0

HYDROGRAPH DATA
 INVDC ICHG TAREA SNAP TRSCA TRSPC PATIC ISNEW ISAME LOCAL
 1 1 1.25 0. 1.25 0. 0. 0 0 C

PRECIP DATA
 SPFE PMS R6 R12 R24 R48 R72 R96
 0. 19.50 111.00 123.00 133.00 142.00 C. C.

TRSPC COMPUTED BY THE PROGRAM IS 0.500

LUSS DATA
 LRPT STRKR DLTVR RTIDL ERAIN STRKS RTICK STARTL CNSTL ALSMX RTIMP
 0 C. 1.00 0. 0. 1.00 1.00 0.10 0. C.

UNIT HYDROGRAPH DATA
 TP= 2.57 CP=0.63 NTA= C

RECESSION DATA
 STRTQ= -2.00 GRCSL= 2.00 RTICR= 1.00
 APPROXIMATE CLAPK COEFFICIENTS FROM GIVEN SNYDER CP AND TP ARE TC=12.79 AND R=10.46 INTERVALS

UNIT HYDROGRAPH 63 END-OF-PERIOD ORDINATES, LAG= 2.65 FCLPS, CP= 0.63 VOL= 1.00
 5. 17. 35. 55. 73. 101. 125. 147. 163. 175.
 182. 184. 177. 165. 150. 136. 124. 112. 102. 92.
 84. 77. 70. 63. 53. 43. 35. 25. 14.
 32. 25. 27. 24. 20. 18. 17. 15. 6.
 12. 11. 10. 9. 8. 7. 6. 5. 2.
 5. 4. 4. 3. 3. 3. 2. 2.

END OF RECORD

[illegible]

[illegible]

06060-2
0615-1
0616-1
0617-1
0618-1
0619-1
0620-1
0621-1
0622-1

[illegible]

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL	VOLUME
CFS	1819.	1367.	460.	233.	4653.	4653.
CMS	51.	39.	14.	7.	1319.	1319.
1-C-FES		10.09	14.17	14.22	14.12	14.12
PM		256.32	359.85	363.22	363.22	363.22
1-C-FT		676.	952.	962.	962.	962.
TFOLS CO M		836.	1174.	1197.	1197.	1197.

	PEAK	6-HCUP	24-HCUP	72-HCUP	TOTAL	VOLUME
CFS	2273.	1709.	600.	291.	58204.	
CMS	64.	48.	17.	8.	1645.	
1-C-ES		12.61	17.71	17.50	17.90	
NM		320.39	449.82	454.78	454.78	
LC-FT		847.	1189.	1203.	1203.	
TOTALS		1045.	1467.	1482.	1482.	

SUMMARY OF CAM SAFETY ANALYSIS

ELEVATION	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	1160.55	1180.14	1184.50

238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.	238.
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[illegible]

PEAK CUTOFF IS 836. AT TIME 43.25 HOURS

PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL	VOLUME
CFS	835.	225.	105.	2170.	616.
CMS	24.	6.	3.	6.	6.69
1-CMS	4.81	6.65	6.65	170.02	450.
MY	122.13	168.58	170.02	450.	555.
AC-FT	323.	447.	450.	555.	
TD-DCS CC M	358.	551.	555.		

```

***** TOP OF DAM, BOTTOM OF BREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
***** BOTTOM OF RESERVOIR ASSUMED TO BE AT 1156.50
***** STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 1181.55

```

STATION: 1, PLAN 1, RATIC 3

[illegible]

WARNING *** TOP OF DAM, BOTTOM OF BREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
 BOTTOM OF RESERVOIR ASSUMED TO BE AT 1155.50
 STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 1161.55

[illegible]

PEAK OUTPUT IS 1342. AT TIME 42.75 HOURS

[illegible][illegible]

PEAK OUTPUT IS 1815, AT TIME 42.50 HOURS

	PEAK	6-HOUR	24-HOUR	72-HOUR	TOTAL	VOLUME
DES	1815.	1325.	459.	222.		44325.
DES	51.	35.	13.	6.		1257.
DES		9.60	13.35	13.65		13.45
DES		249.01	344.18	346.50		346.30
DES		656.	910.	917.		917.
DES	812.		1123.	1131.		1131.

WARNING *** TOP OF DAM, BOTTOM OF BREACH, OR LOW-LEVEL OUTLET IS NOT WITHIN RANGE OF GIVEN ELEVATIONS IN STORAGE-ELEVATION DATA
 BOTTOM OF RESERVOIR ASSUMED TO BE AT 1156.50
 STORAGE-ELEVATION DATA WILL BE EXTRAPOLATED ABOVE ELEVATION 1161.55

STATION 1, PLAN 1, RATIC 6
END-OF-PERIOD HYDROGRAPH CIRCINATES

[illegible]

10-27
T-OLUS C-14

555:
1021:

11-4:
1416:

11-4:
1421:

11-4:
1421:

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FORMULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLCKS IN CUBIC FEET PER SECCND (CURIC METERS PER SECCND)
 AREA IN SQUARE MILES (SQUARE KILCMETERS)

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO FLCKS					
				RATIO 1	RATIO 2	RATIO 3	RATIO 4	RATIO 5	RATIO 6
				0.20	0.40	0.50	0.60	0.80	1.00
HYDROGRAPH AT	1	1.26	1	435.	909.	1137.	1364.	1719.	2272.
	(	5881.00)	(	12.67)(	25.75)(	32.19)(	38.62)(	51.50)(	64.37)(
ROUTED TO	1	1.26	1	397.	836.	1052.	1242.	1615.	2271.
	(	5881.00)	(	11.23)(	23.67)(	29.60)(	36.00)(	51.40)(	64.30)(

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1

RATIO CF PWF	ELEVATION STORAGE OUTFLOW	INITIAL VALUE 1180.55 238. 0.	SPILLWAY CREST 1180.60 239. C.	TCP OF DAM 1184.50 353. 1195.	DURATION OVER TCP HOURS	MAXIMUM OUTFLOW CFS	MAXIMUM STORAGE AC-FT	MAXIMUM DEPTH OVER DAM	MAXIMUM RESERVOIR W.S. ELEV	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
C.20	1182.51		235.		C.	397.	295.	C.	1182.51	43.50	C.
C.40	1183.66		328.		C.	836.	328.	C.	1183.66	43.25	C.
C.50	1184.17		343.		C.	1052.	343.	C.	1184.17	43.25	C.
C.60	1184.57	0.07	355.	1.25	1.25	1342.	355.	1.25	1184.57	42.75	C.
C.80	1184.69	0.19	358.	3.25	3.25	1815.	358.	3.25	1184.69	42.50	C.
1.00	1184.78	0.28	361.	4.50	4.50	2271.	361.	4.50	1184.78	42.50	C.

APPENDIX D

REFERENCES

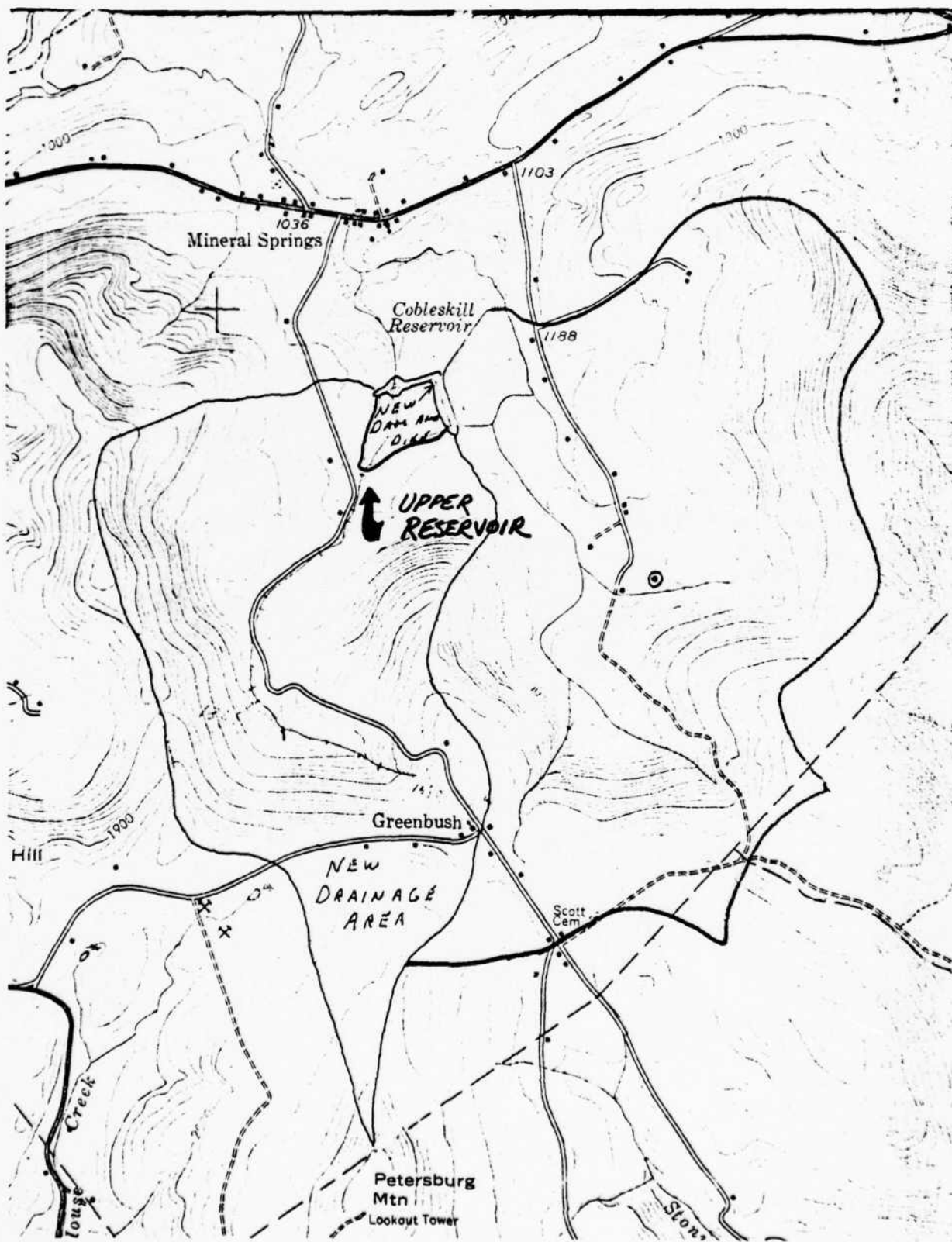
APPENDIX D

REFERENCES

- 1) U.S. Department of Commerce, Technical Paper No. 40, Rainfall Frequency Atlas of the United States, May 1961.
- 2) U.S. Department of Commerce, Hydrometeorological Report No. 33, Seasonal Variation of the Probable Maximum Precipitation East of the 105th Meridian for Areas from 10 to 1,000 Square Miles and Durations of 6, 12, 24, and 48 Hours; April 1956.
- 3) Soil Conservation Service, National Engineering Handbook, Section 4, Hydrology, August 1972 (U.S. Department of Agriculture).
- 4) H.W. King and E.F. Brater, Handbook of Hydraulics, 5th edition, McGraw-Hill, 1963.
- 5) T.W. Lambe and R.V. Whitman, Soil Mechanics, John Wiley and Sons, 1965.
- 6) W.D. Thornbury, Principles of Geomorphology, John Wiley and Sons, 1969.
- 7) University of the State of New York, Geology of New York, Education Leaflet 20, Reprinted 1973.
- 8) Cornell University Agriculture Experiment Station (compiled by M.G. Cline and R.L. Marshall); General Soil Map of New York State and Soils of New York Landscapes, Information Bulletin 119, 1977.

APPENDIX E

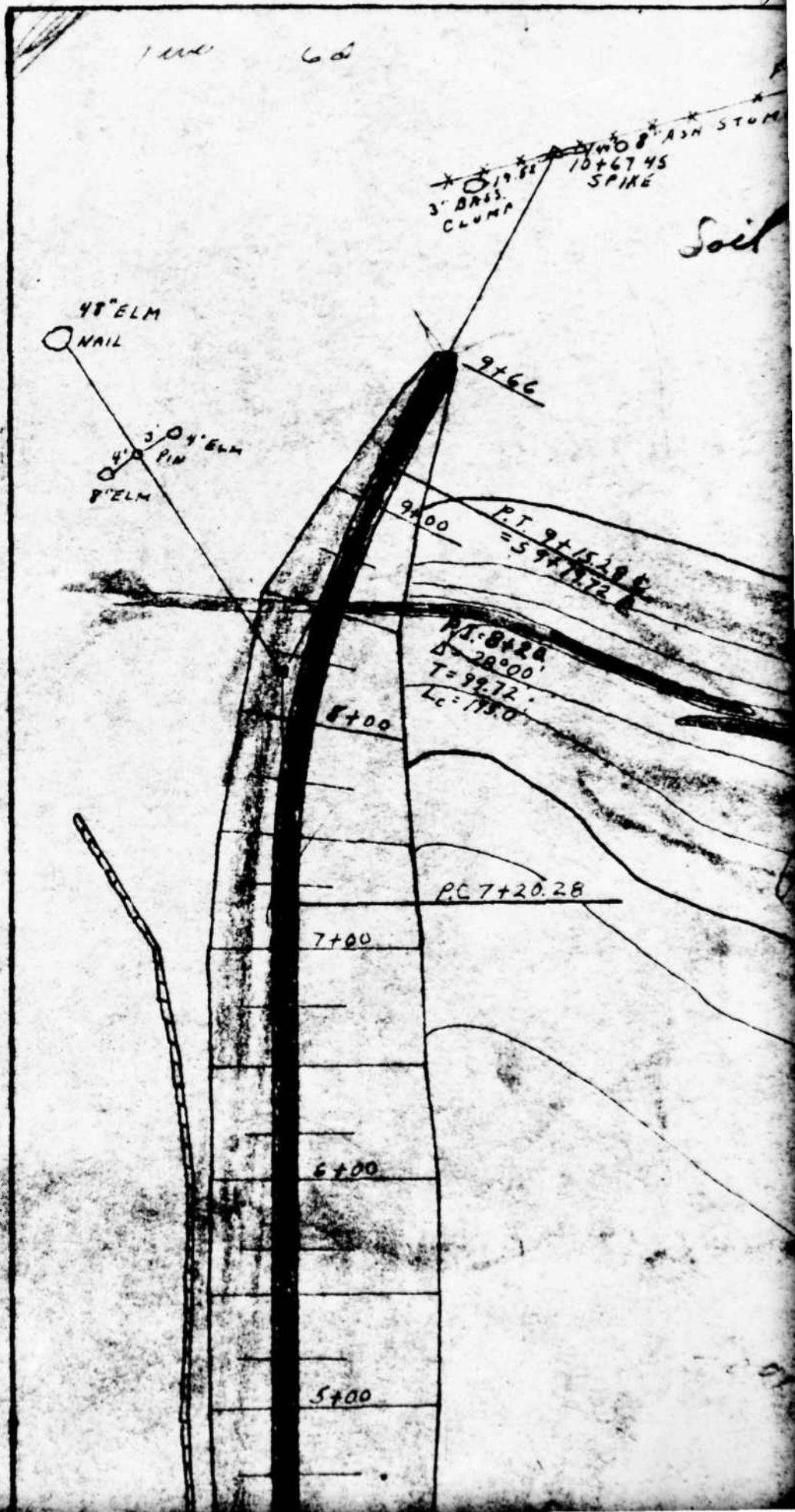
DRAWINGS



TOPOGRAPHIC MAP

1

64



12

FENCE AND 12
ASH STUMP
6745
PINE

65,000 compacted only

Soil slaty

110
105
100

150
150
150

Burn

7400
16400
15400
TRANS. SECTION
7400

119

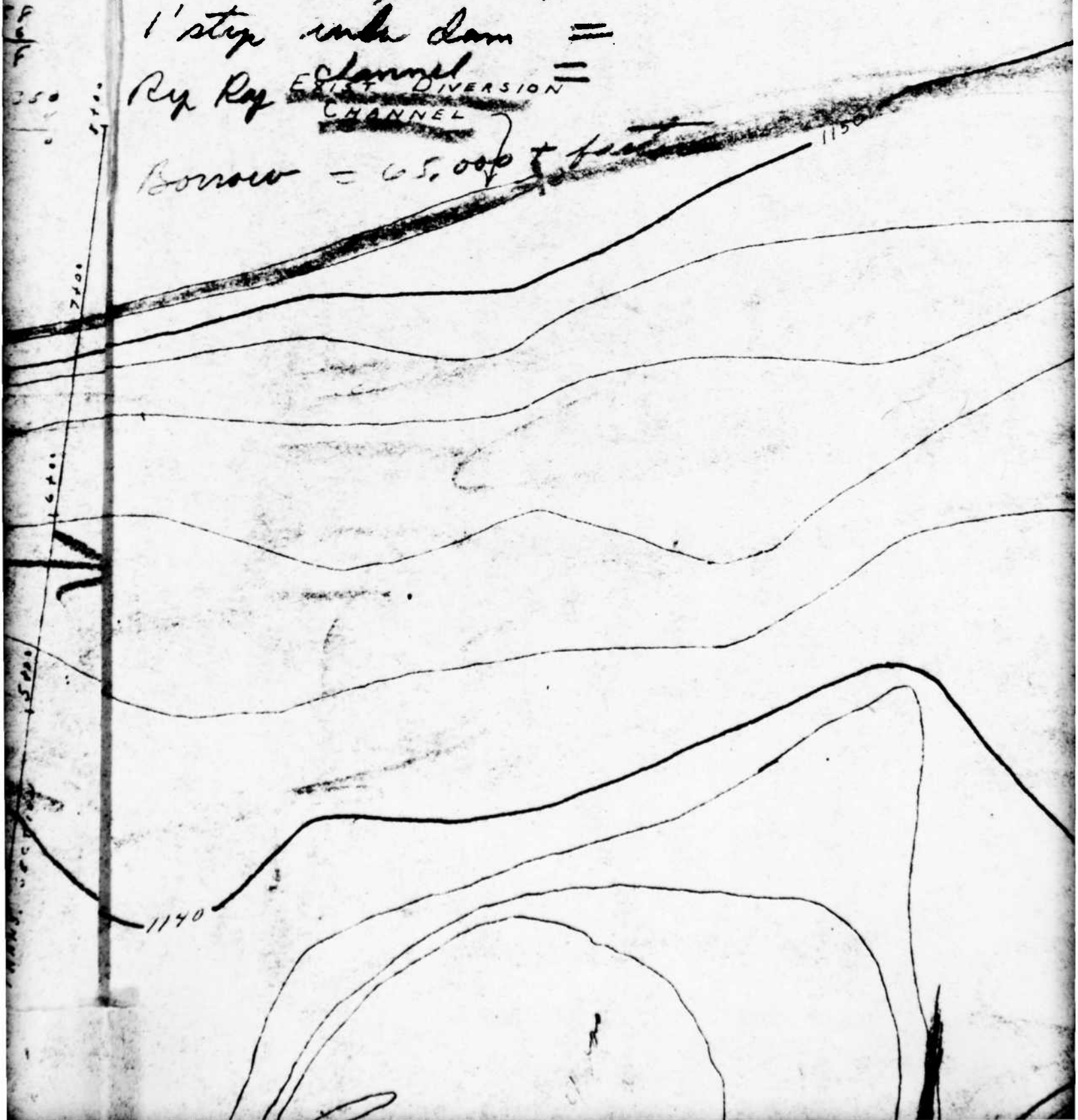
step enters area 6" = 20,000 ±

Cutoff trenchy benching = 5,000 ±

1' step under dam =

Ry Ry ~~channel~~ ~~EXIST DIVERSION~~ ~~CHANNEL~~ =

Borrow = 65,000 ± feet



4
Test hole here

Temp. 21.0

NOTE: IT WILL BE THE
CONTRACTOR'S OBLIGATION
TO DIVERT ALL WATER
FROM THE SITE OF THE
PROPOSED DAM DURING
CONSTRUCTION BY
THE USE OF THE
EXISTING AND OTHER
DIVERSION CHANNELS.

Inv.
1150.0

ROAD

1150

15

IT WILL BE THE
 FACTOR'S OBLIGATION
 TO DIVERT ALL WATER
 FROM THE SITE OF THE
 PROPOSED DAM DURING
 CONSTRUCTION BY
 USE OF THE
 EXISTING AND OTHER
 DIVERSION CHANNELS.



Ex
T
Es

P.C. : 4407.6

INTAKE

HOO DYKE

14.00	DAM
-------	-----

too damn?

Diagram showing a cross-section of a headwall and stone fill. The headwall is labeled "HEAD-WALL" and the stone fill is labeled "STONE FILL". The structure is shown over a "12\" C.I. PIPE". An arrow points to the "OVERFLOW" area.

12" C.I. PIPE

119

2+00

3+00

4+00

5+00

6+00

7+00

EXCAVATE STORAGE AREA
TO ELEV. 1140±
ESTIMATED VOLUME
87,000,000 GAL.

20.00

12+00

17+00

1140

STONE
FILLING

P.C. 1+77.6

P.T. 3+11.6
= 3+11.4

P.I. 2+50

A = 23°30'

T = 62.40'

Lc = 123.20'

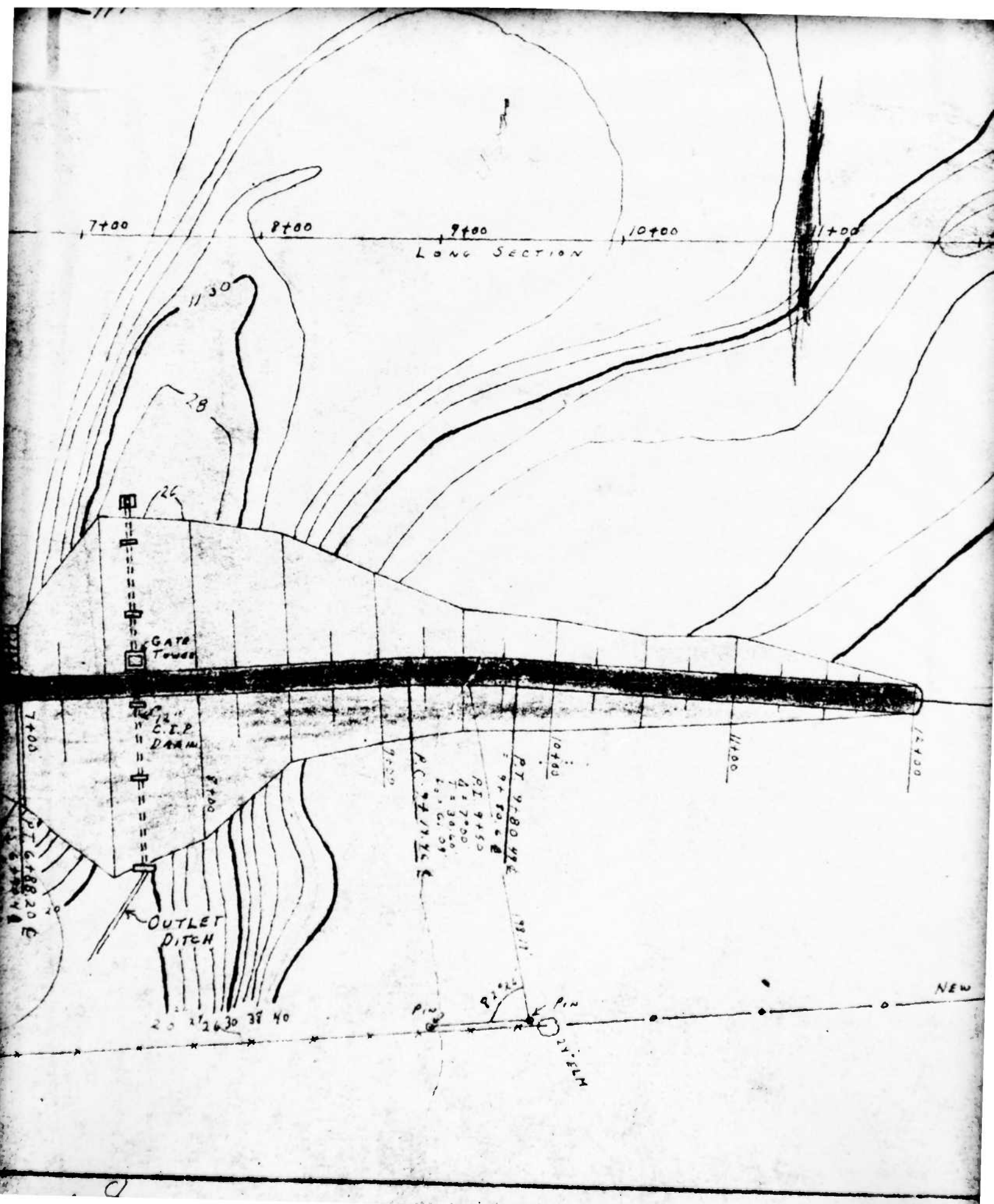
P.I. 6+10
A = 20°30'
T = 90.7'

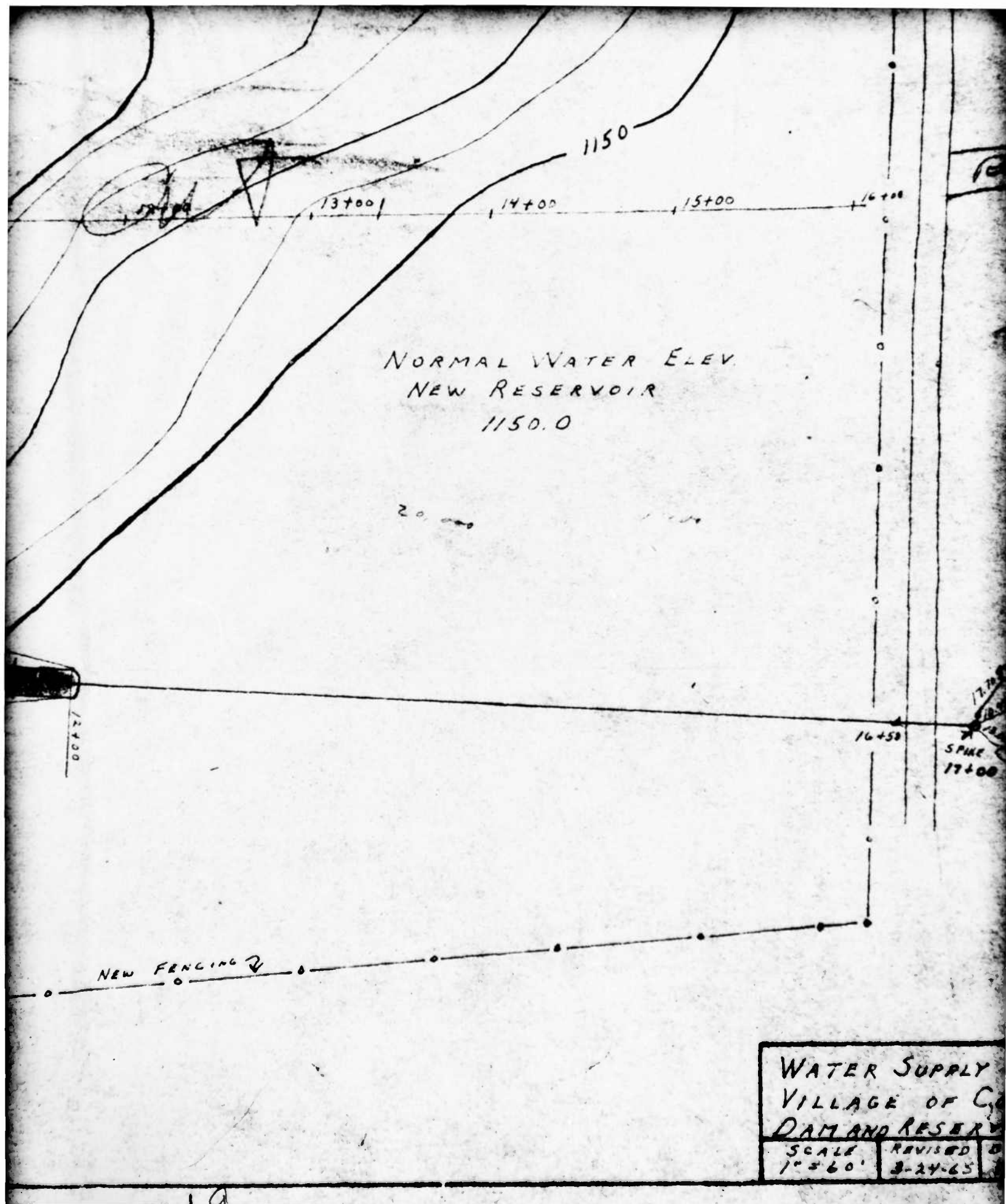
P.C. 5+09.6

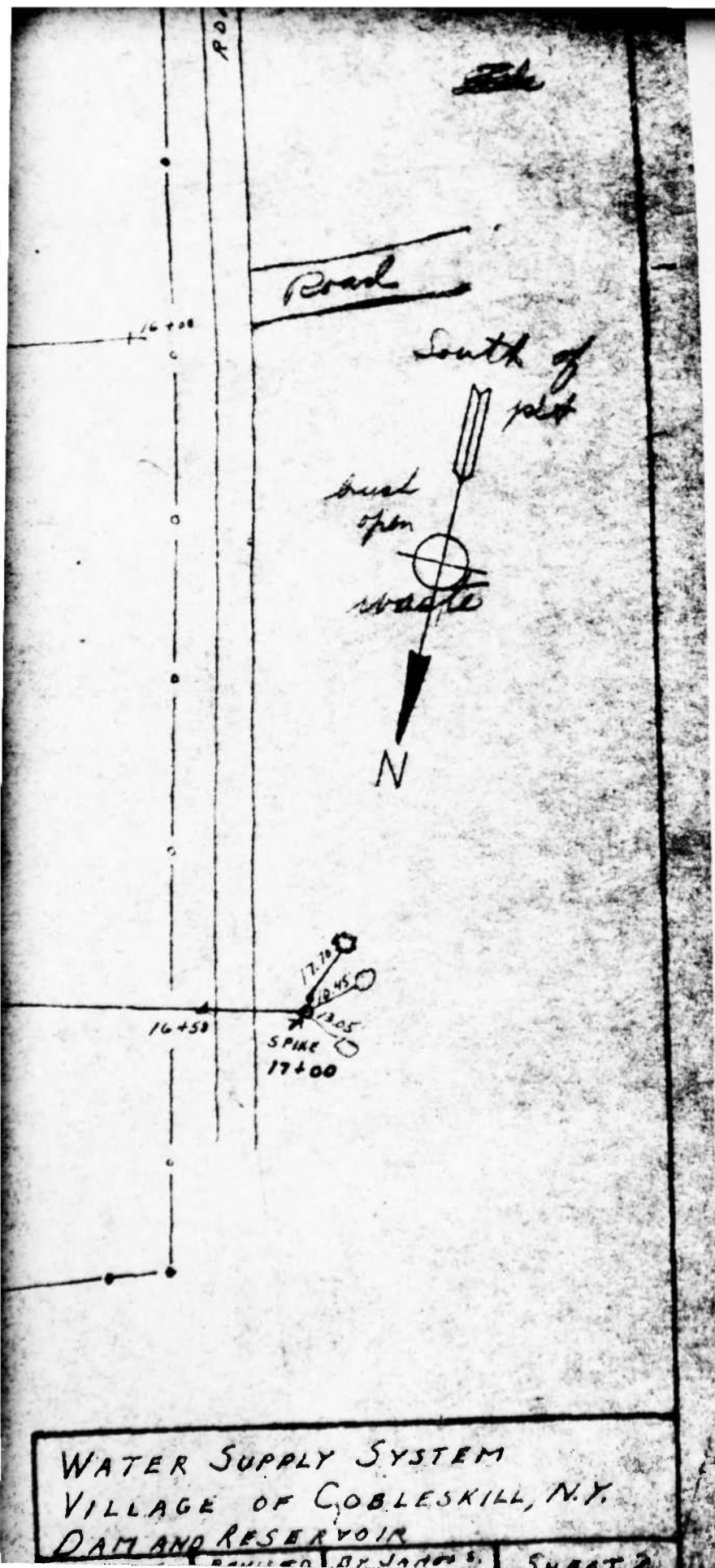
7+00

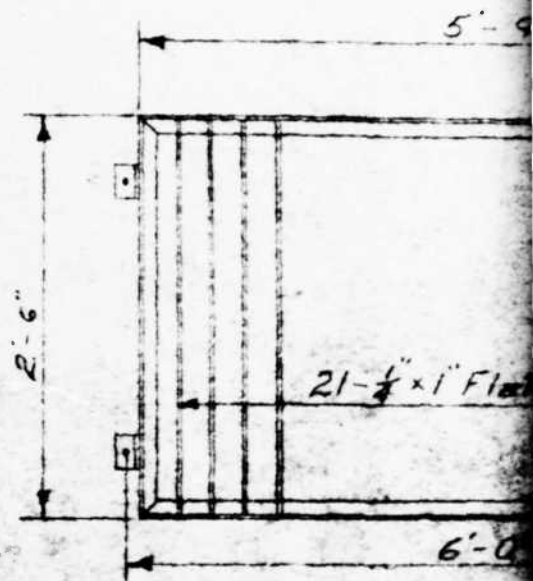
P.T. 6+88.20 E
= 6+88.4

RIP
RAI







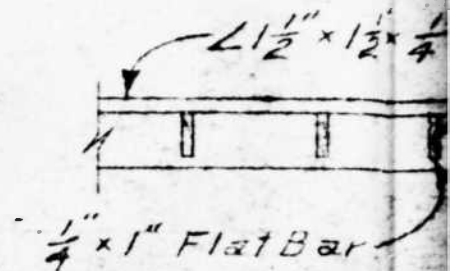
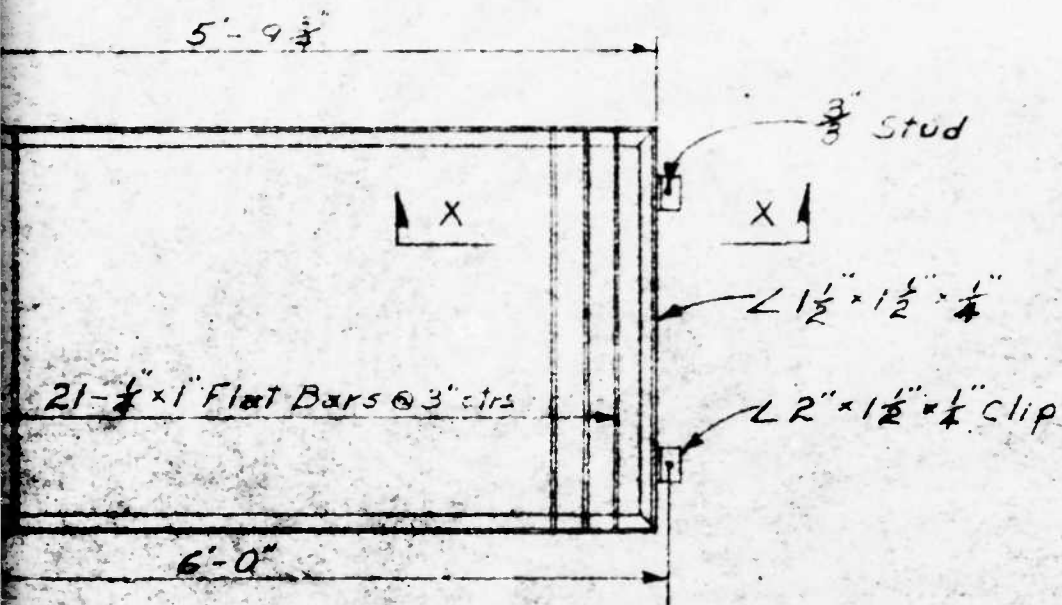


TRASH GR
Scale 1/2" = 1'



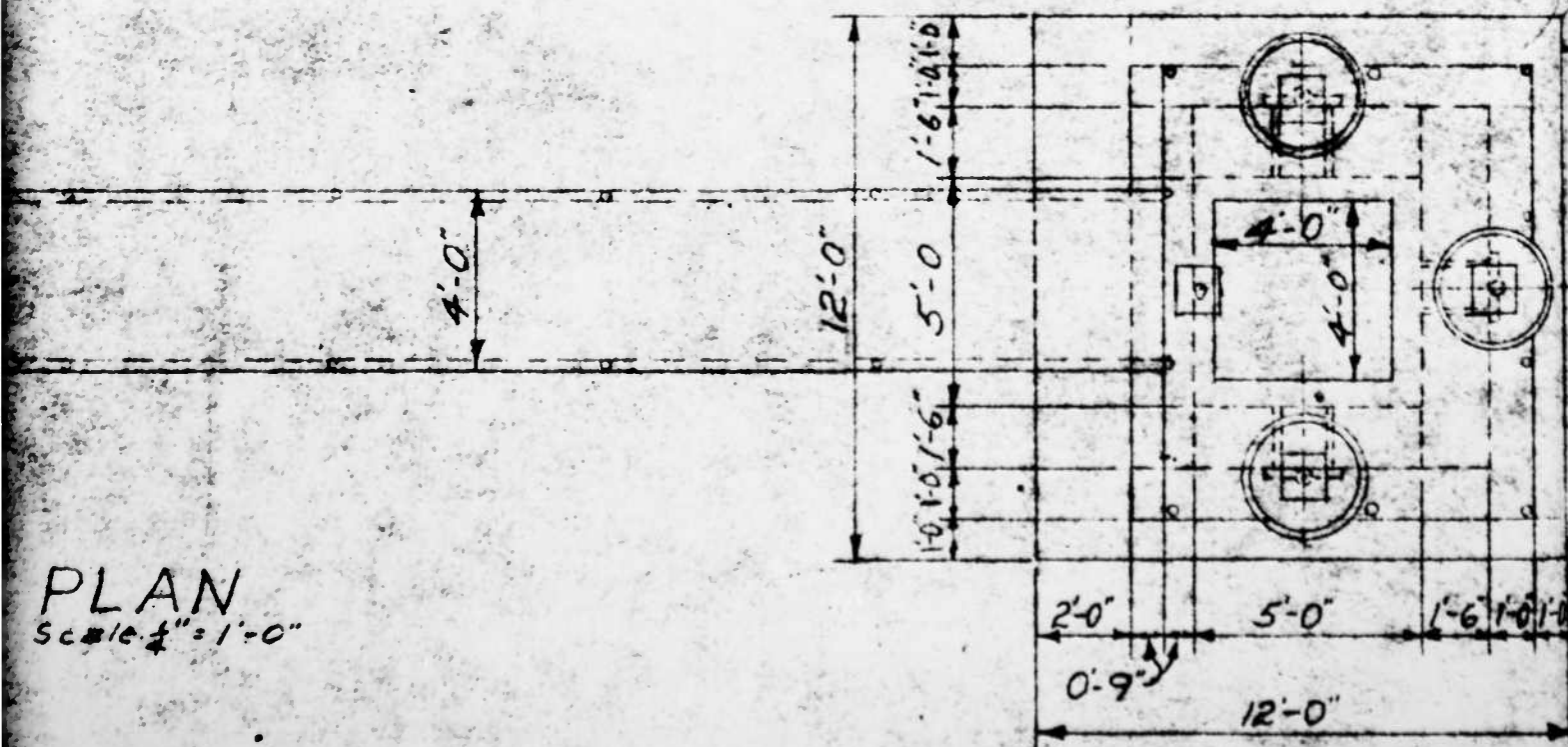
PLAN
Scale 1/4" = 1'

2

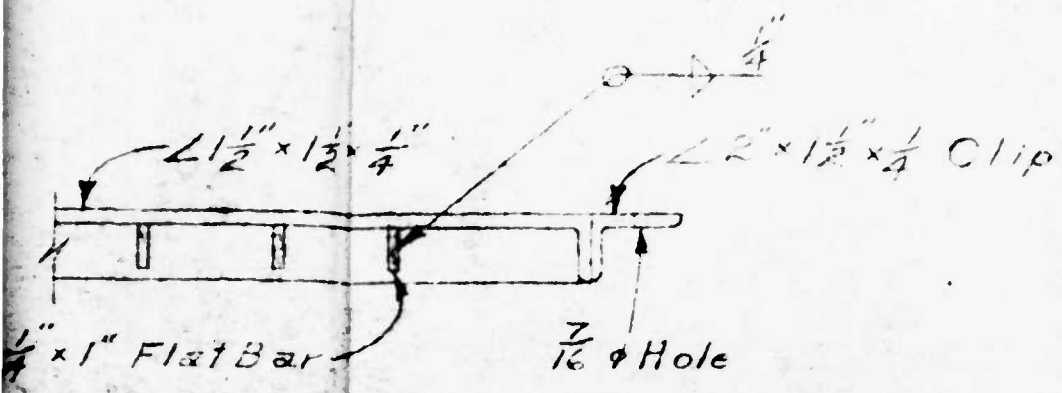


SECTION
Scale

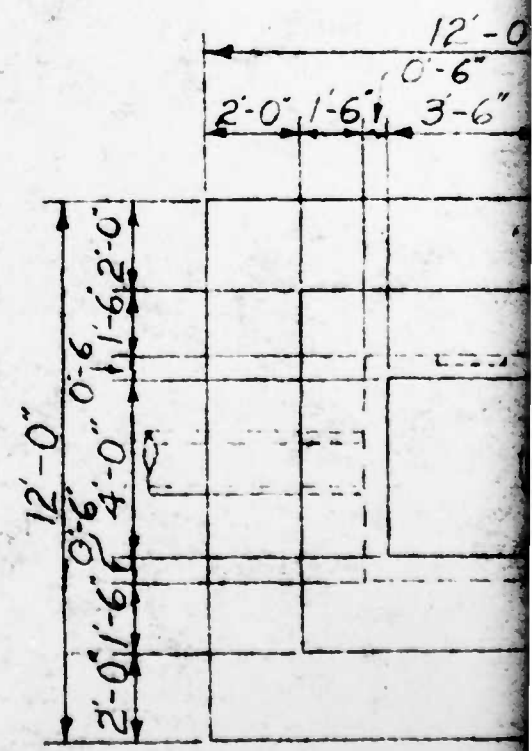
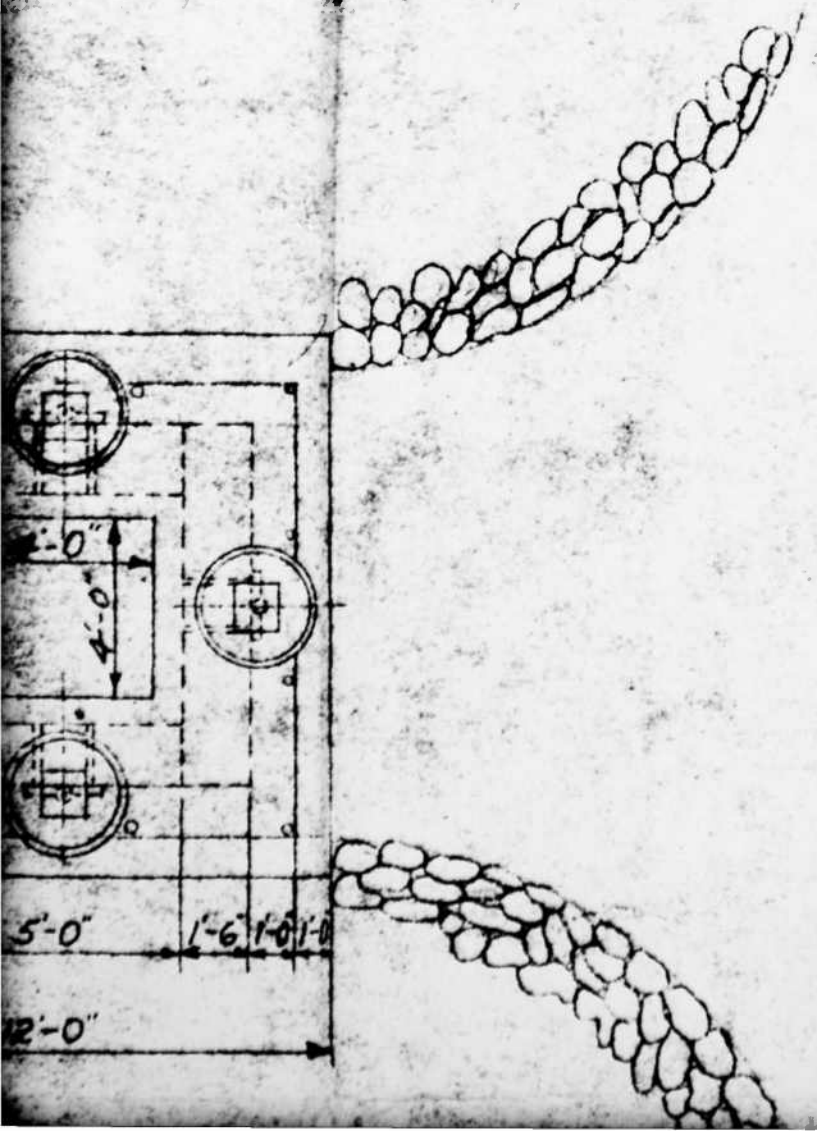
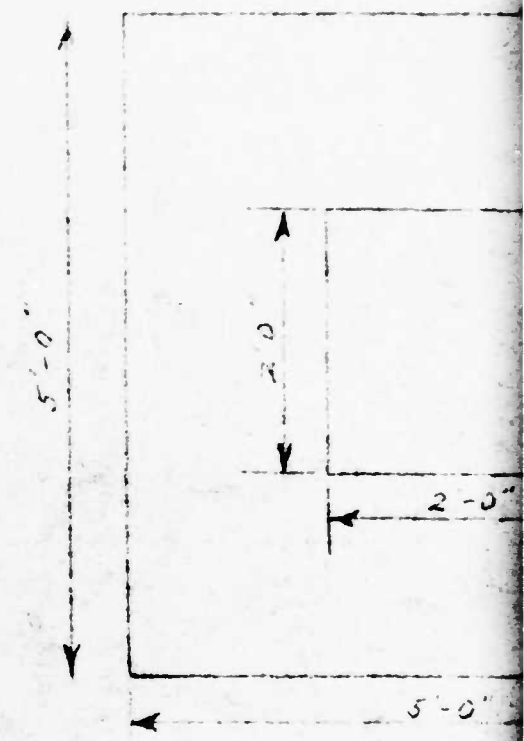
RASH GRILL
Scale 1/2" = 1'-0"



3



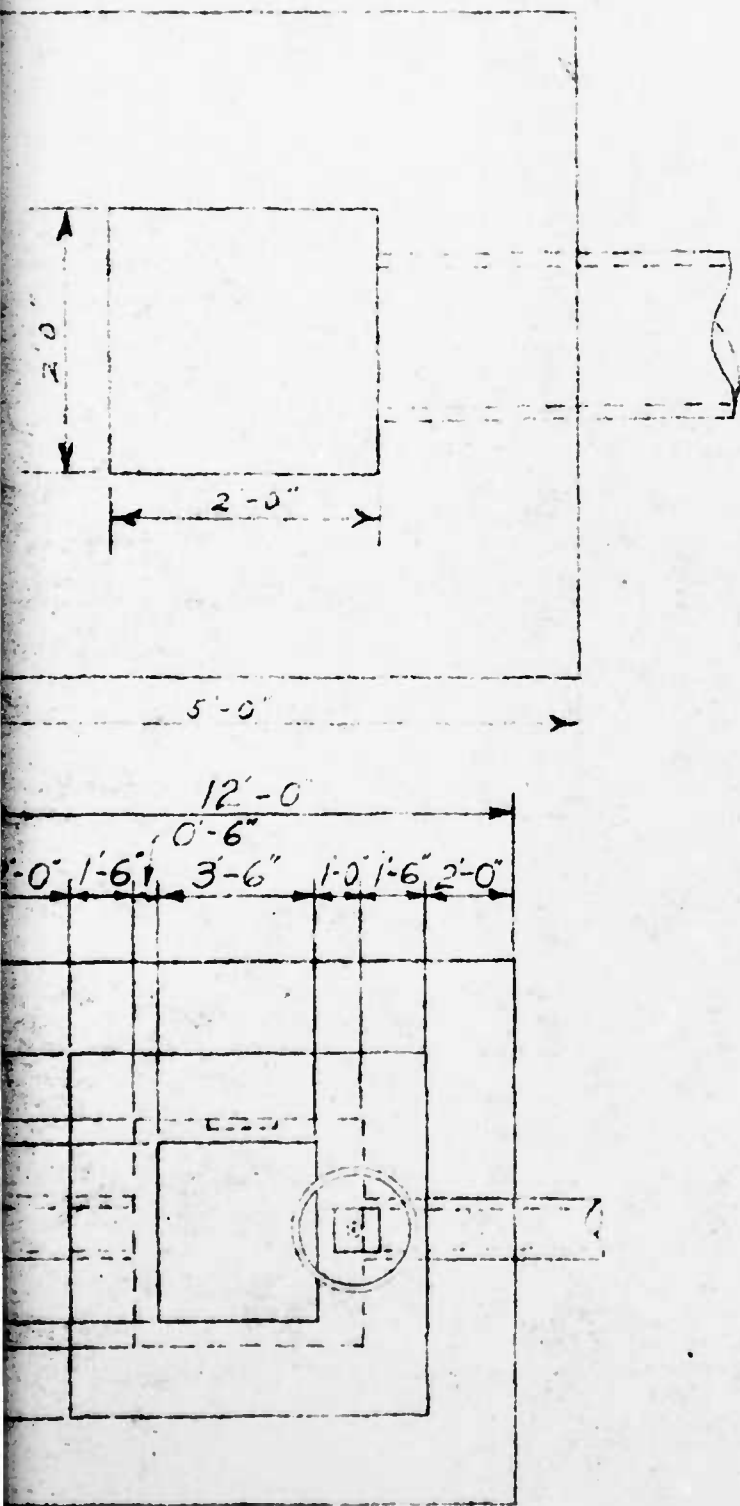
SECTION X-X
Scale 3" = 1'-0"



PLA
Scale $\frac{1}{4}$ "

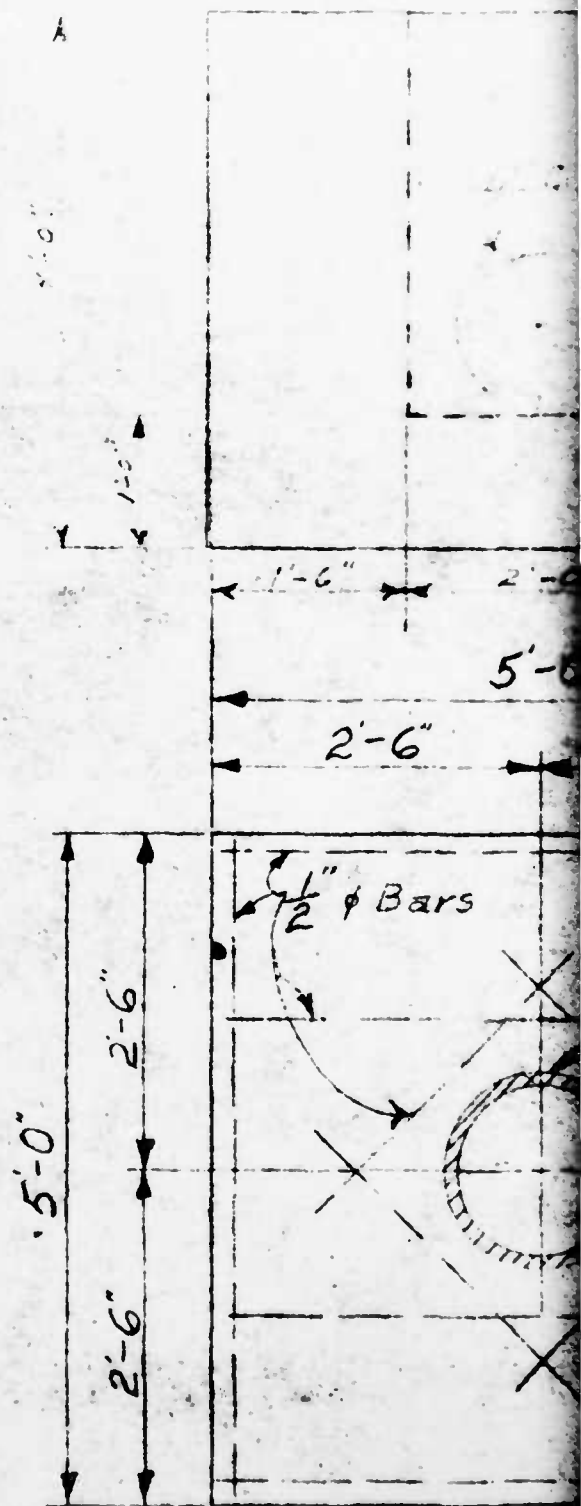
1A

PLAN C.B.
Scale 1/4" = 1'-0"



PLAN
Scale $\frac{1}{4}" = 1'-0"$

ELEVATION
Scale 1/4" = 1'-0"

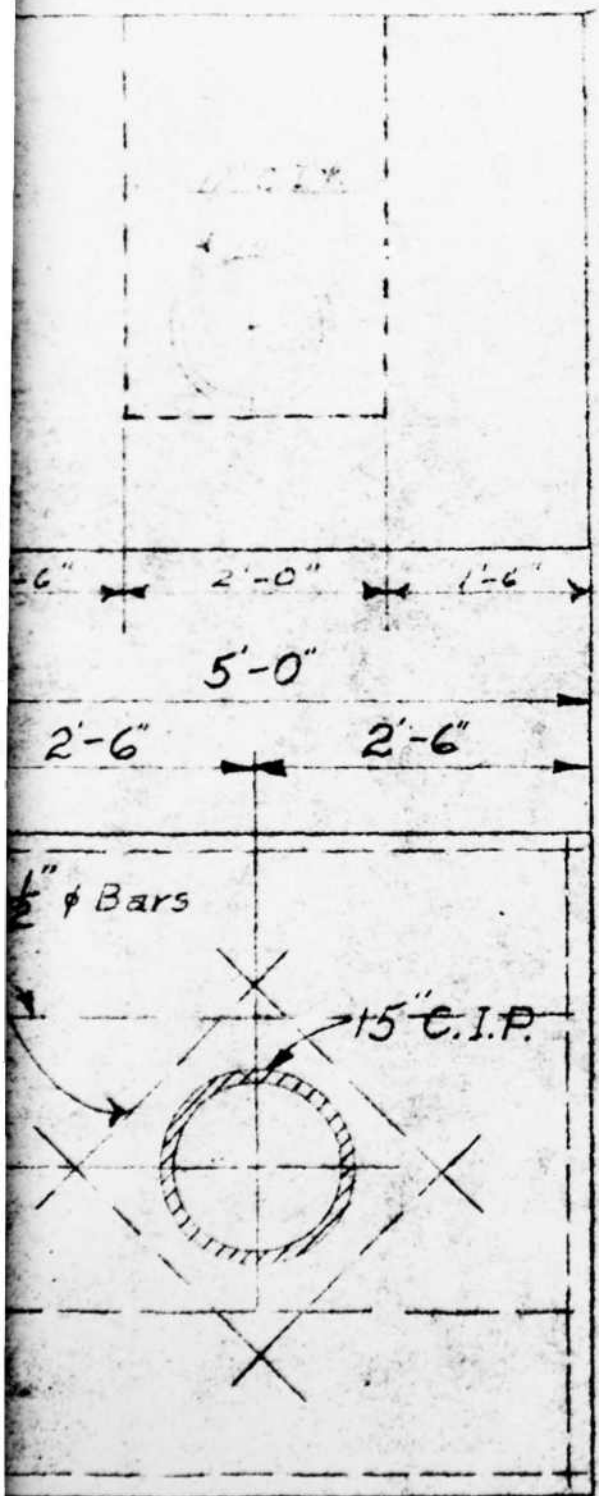


SECTION
Scale $\frac{1}{4}" = 1'-0"$

15

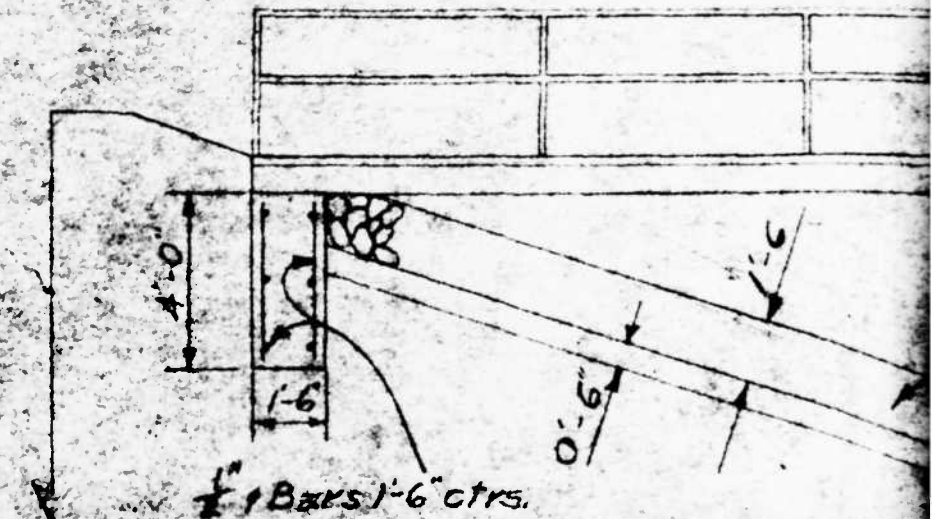
ELEVATION, C.E.
Scale 2" = 1'-0"

Sheet 10
Twin to
South



SECTION A-A
Scale 3/4" = 1'-0"

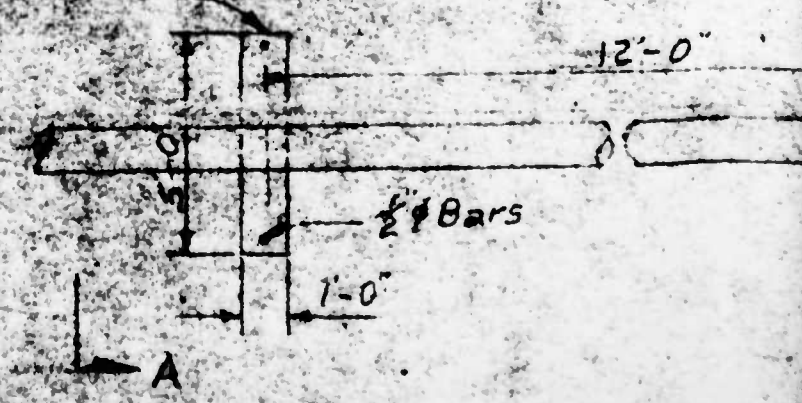
INTAKE STRUCT



OVERFLOW TO
EXISTING RESERVOIR

POROUS FILL

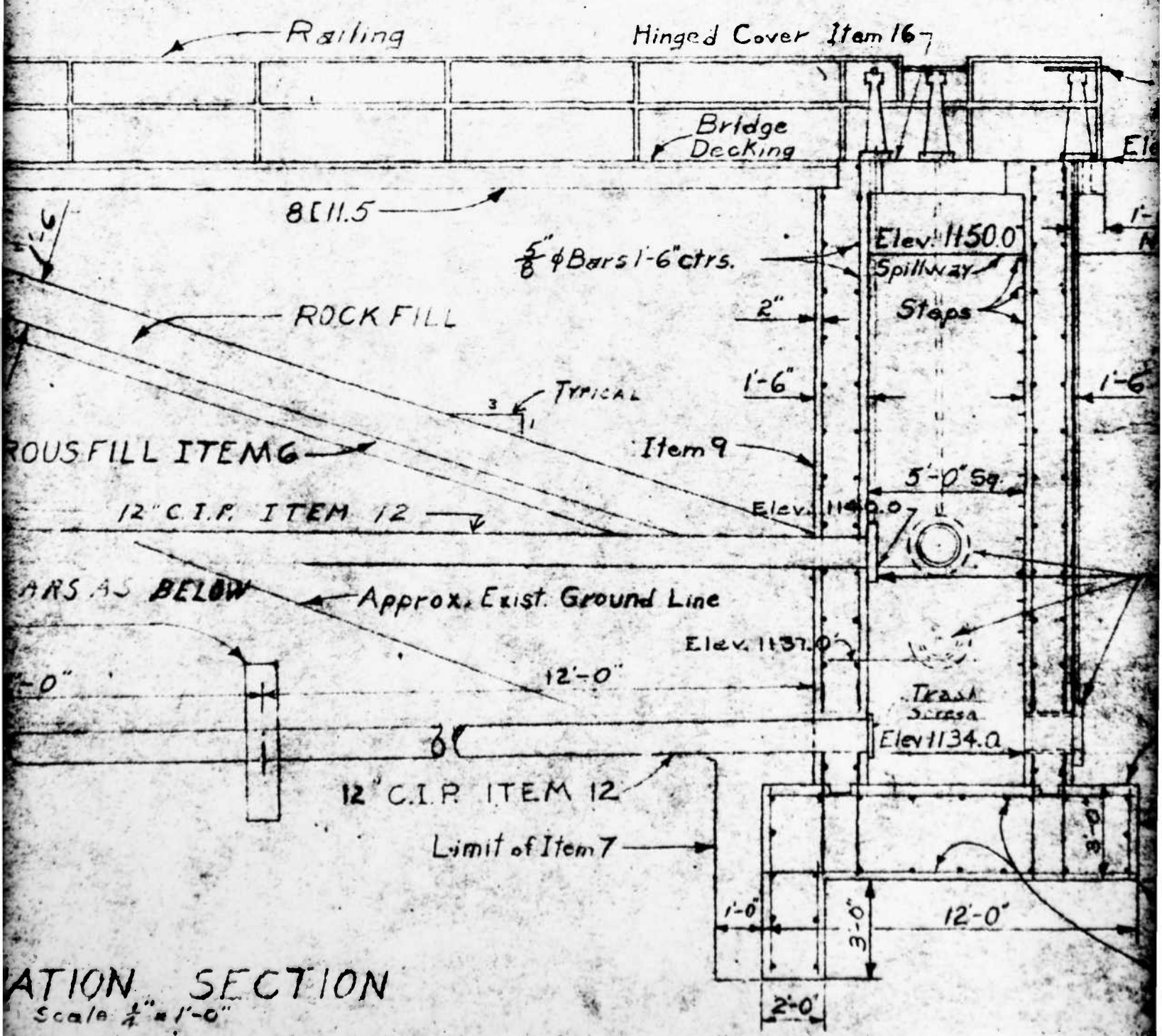
PROVIDE COLLARS AS
Anti Seepage Collar



ELEVATION
Scale $\frac{1}{4}$ "

6

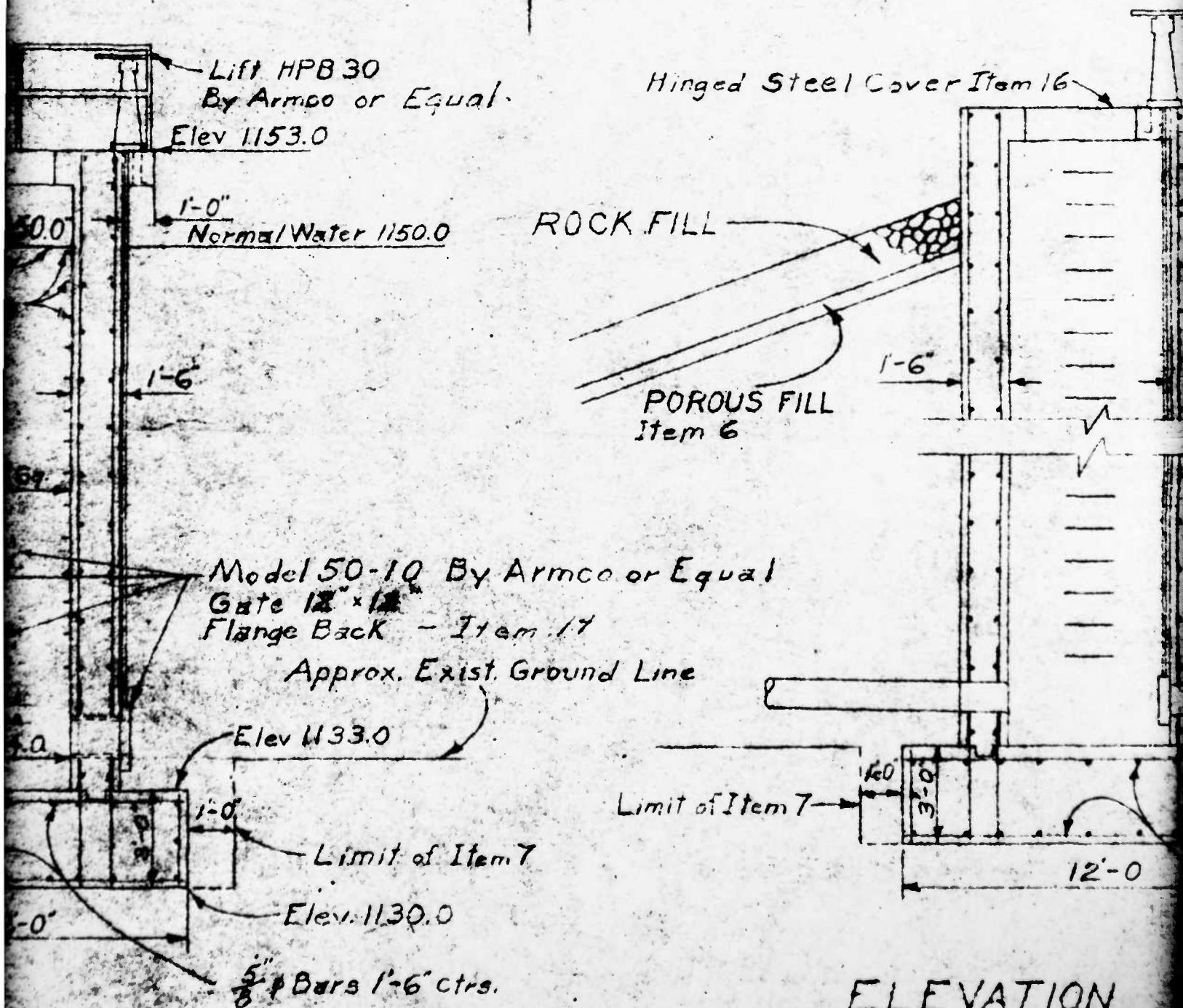
STRUCTURE Sta. 0+60 DYKE 60' RT.



SECTION
Scale $\frac{1}{4}" = 1'-0"$

Scale $\frac{1}{4}" = 1'-0"$

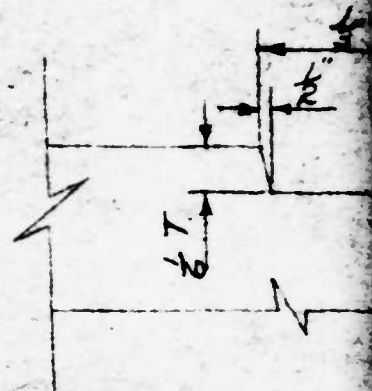
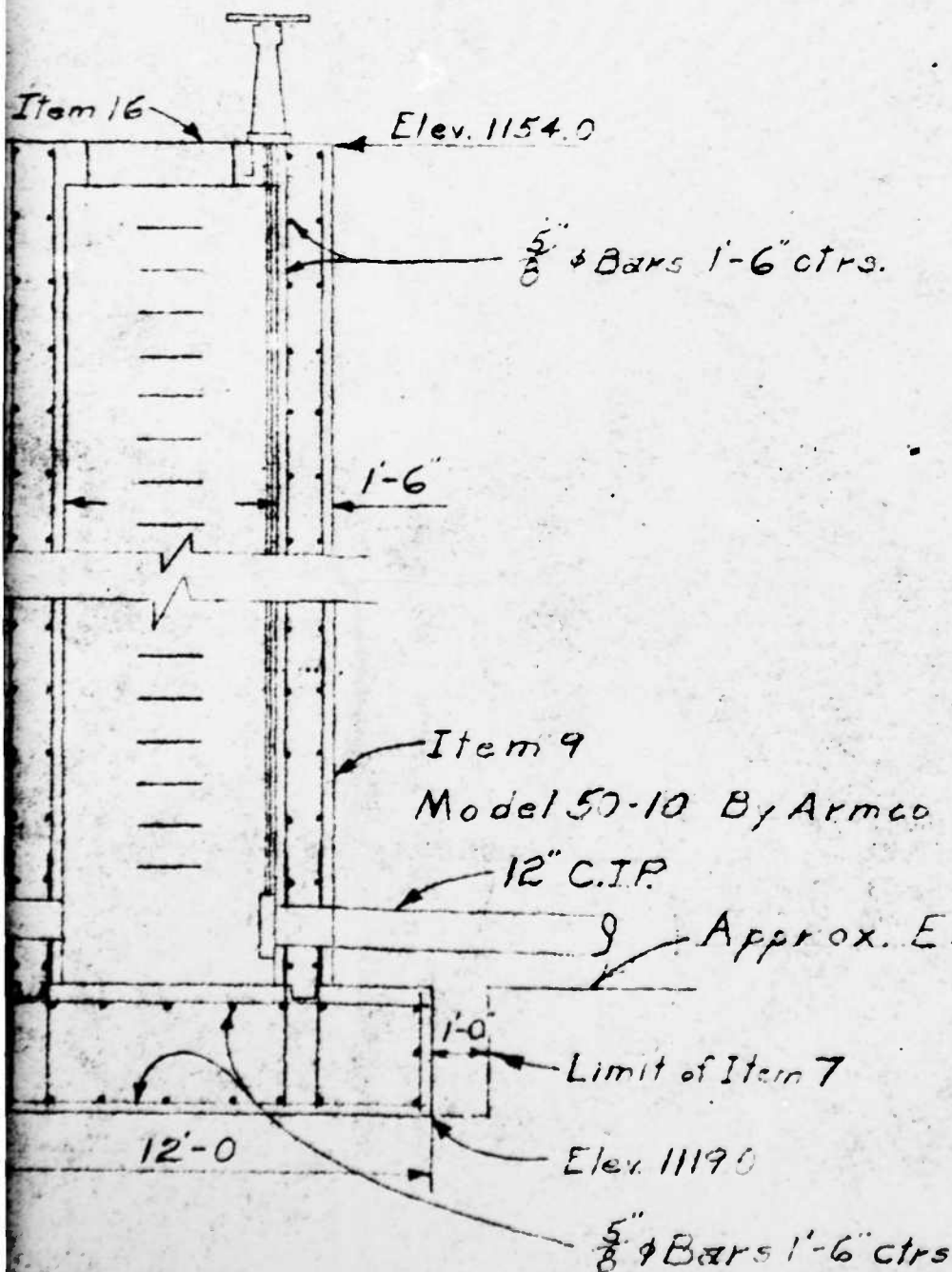
TOWER FOR DRAIN



ELEVATION

Scale $\frac{1}{4}" = 1'-0"$

OR DRAIN Sta. 7+65 DAM &



KEYWAY
Scale

Notes:

1. Location of Int. may have to be due to foundation
2. Cat Walk, Well and Trash Gr. will be paid for

Item 9

Model 50-10 By Armco or Equal Gate 12"x12" F

12" C.I.P.

9 Approx. Exist. Ground

1'-0"

Limit of Item 7

12'-0"

Elev. 1119.0

5/8\"/>

ATION SECTION

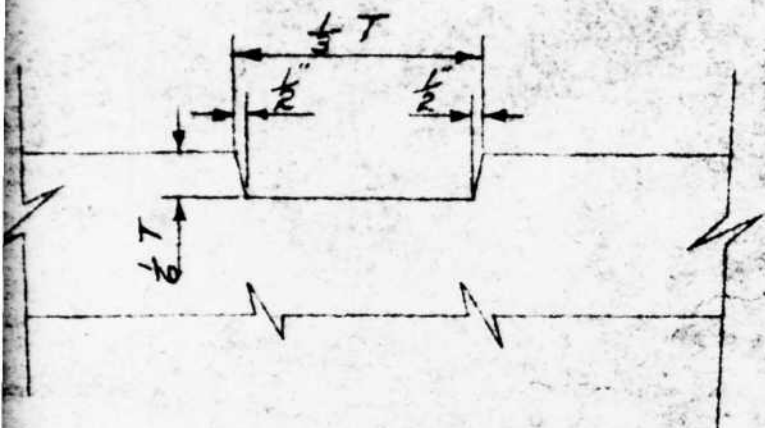
Scale 1/4" = 1'-0"

WATER
VILLAGE
INTAKE

Scale A

2-3-63 B

Scale $\frac{1}{4}" = 1'-0"$



KEYWAY DETAIL

Scale 3" = 1'-0"

es:

Location of Intake Structure and Tower may have to be moved to other location due to foundation conditions. Cat Walk, Well Covers, Hand Rails, and Trash Grill for Intake Structure will be paid for as Misc. Metals. Item 16

Gate 12" x 12" Flat Back

W9-0

11 22 3

WATER SUPPLY SYSTEM
VILLAGE of COBLESKILL, N.Y.
INTAKE and TOWER DETAILS

Scale As Shown

SHEET 10

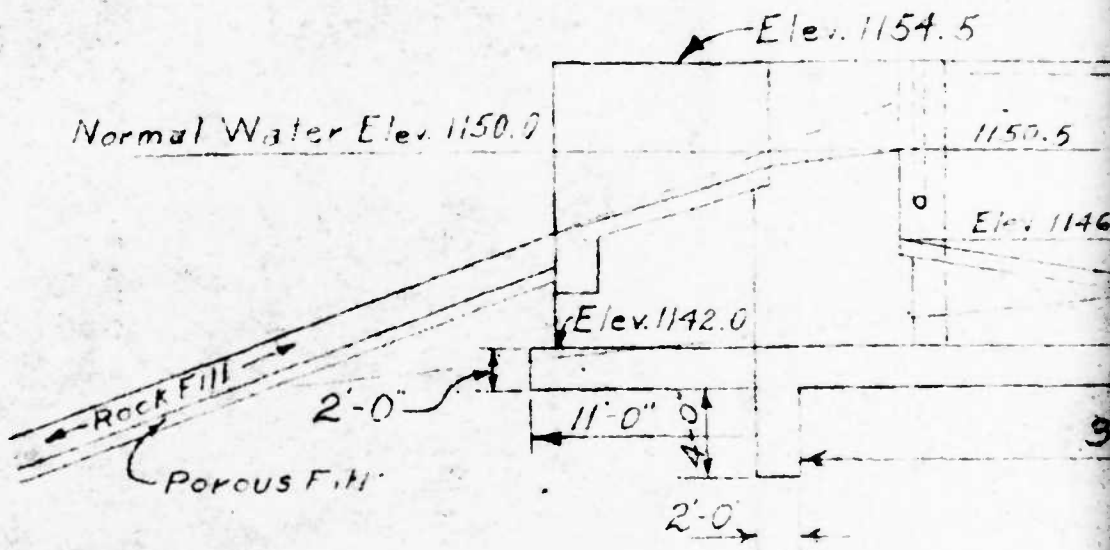
2-3-63

By JL

Checked J. V. V.

2-12-63

11/24/01

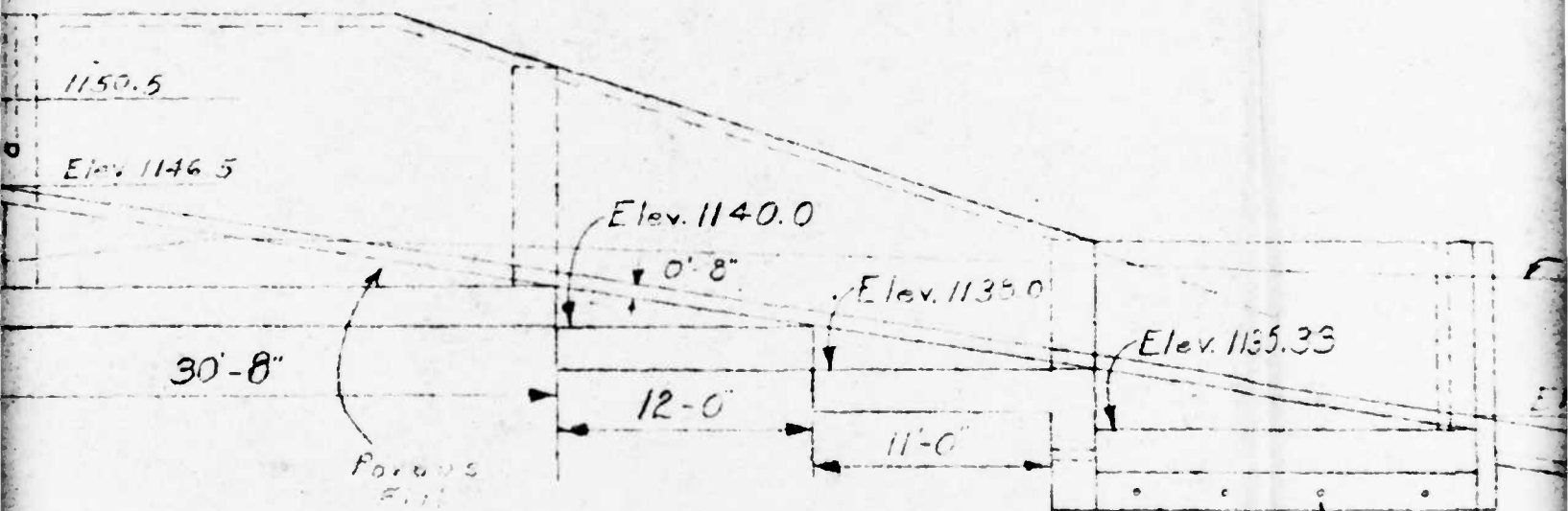


ELEVATION
Scale $\frac{1}{8}" = 1'-0"$

Porous Fill
 $\frac{1}{2}"$ Dowels - 3'-0" long at 1'-0" ctr.
Lug...

12

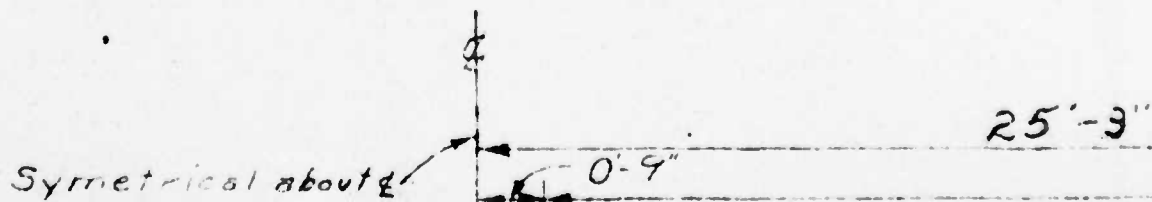
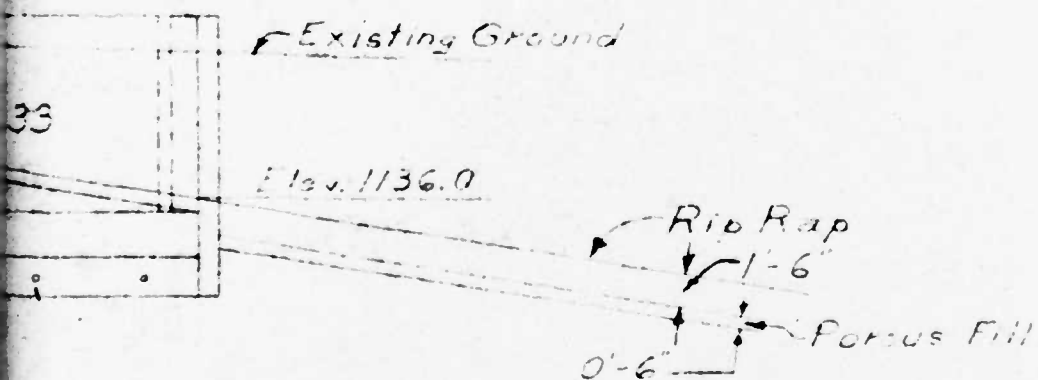
4.5



SECTION
1'-0"



13



14



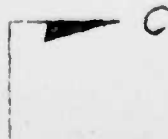
SECTION A-A scale $\frac{1}{2}" = 1'-0"$

25'-3"

20'-6"

1'-0"

5 # Bars 1'-0"



0'-3"



AD-A105 934

NEW YORK STATE DEPT OF ENVIRONMENTAL CONSERVATION ALBANY F/G 13/13
NATIONAL DAM SAFETY PROGRAM. COBLESKILL UPPER RESERVOIR DAM (IN--ETC(U)
JUL 81 G KOCH DACW51-79-C-0001

UNCLASSIFIED

NL

2 OF 2

AD A
10 5934

END

DATE
FILMED

11-81

DTIC

A
59

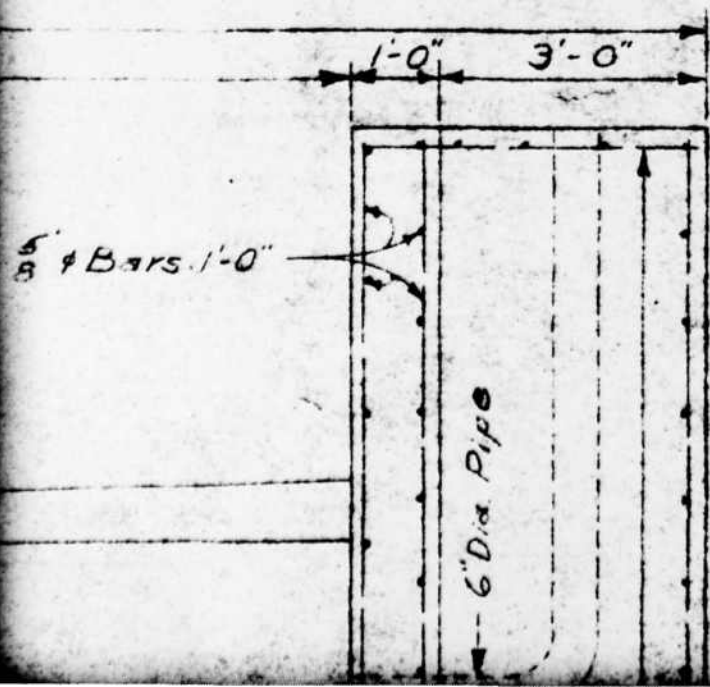
15
Sheet 11

Bars 1'-0" c/s

Bars 1'-6"
Approx. Existing Ground

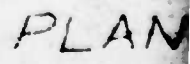
Reinforcement Line
Item 1

Scale $\frac{1}{2}'' = 1'-0''$

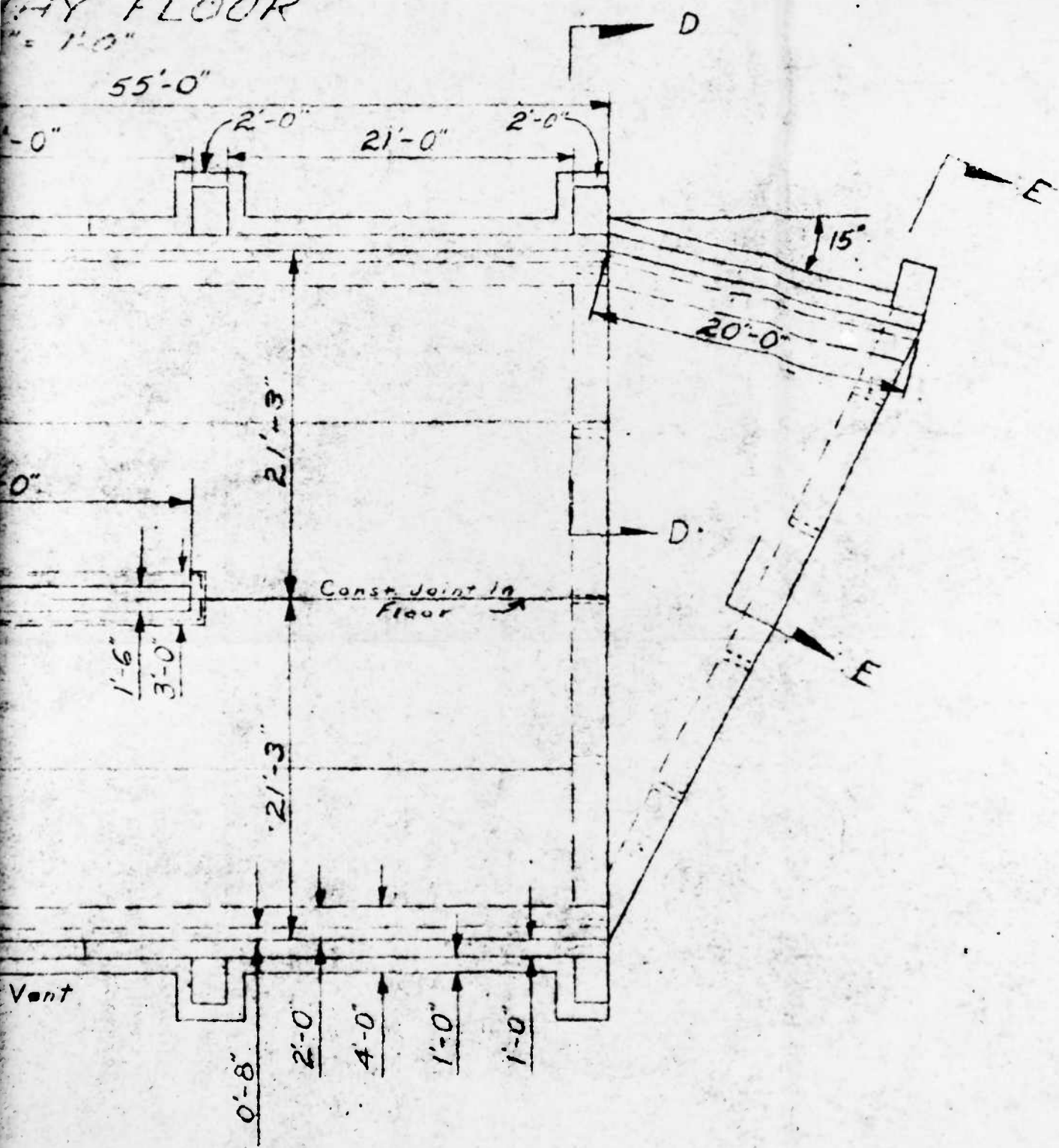


19

55'-0



CONST JOINT
 1st FLOOR
 1" = 1'-0"



PLAN Scale $\frac{1}{8}" = 1'-0"$

5 #4 bars @ 1'-6" cts
5 #4 bars @ 1'-6" cts
0'-8"

5 #4 bars @ 1'-6" cts

SECTION B-B
Scale $\frac{1}{2}" = 1'-0"$

$\frac{5}{8}$ " Bars 1'-6" ctrs.

1'-8"

6" Dia. Pipe

$\frac{5}{8}$ " Bars 1'-6" ctrs.

$\frac{5}{8}$ " Bars 1'-6" ctrs.

-B

C

WATER SUPPLY SYSTEM
VILLAGE of COBLESKILL, N.Y.
DETAIL of SPILLWAY

Scale As Shown

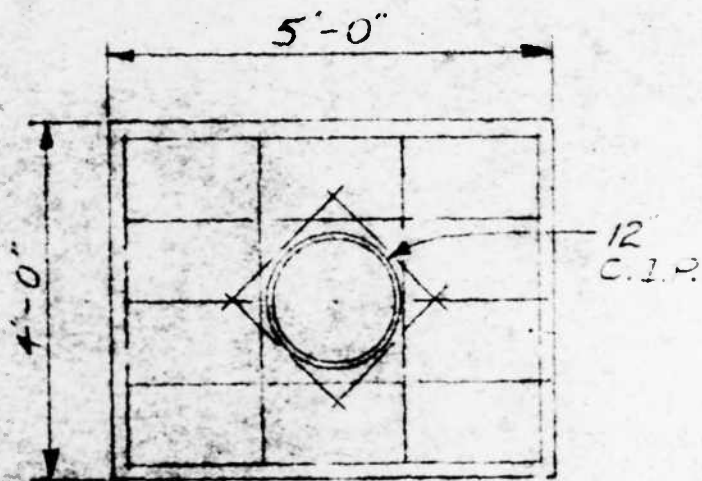
SHEET 11

2-10-63

BY JL

Checked J.S.V.

2-11-63



HEADWALL DETAIL
Scale $\frac{1}{2}" = 1'-0"$

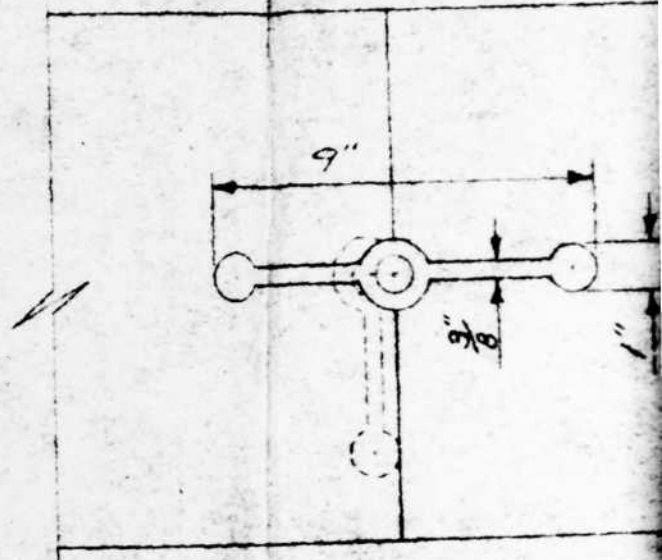
6" Dia. Pipe

6'-0"

12

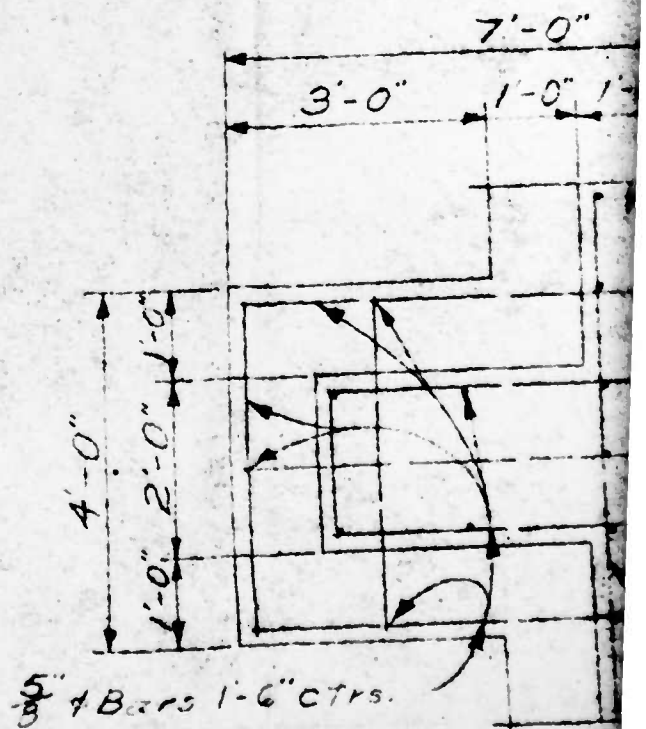


I.P.

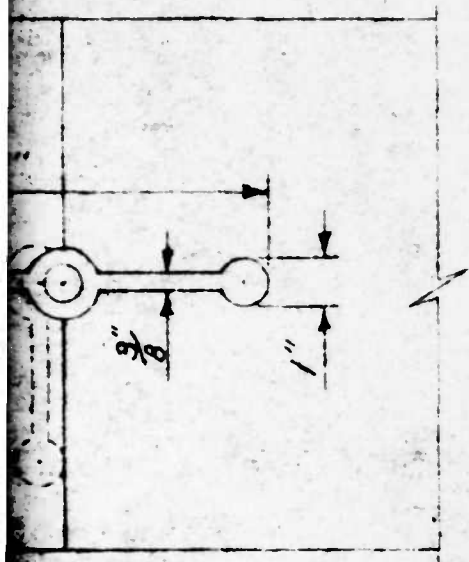


LL DETAIL
1/2" = 1'-0"

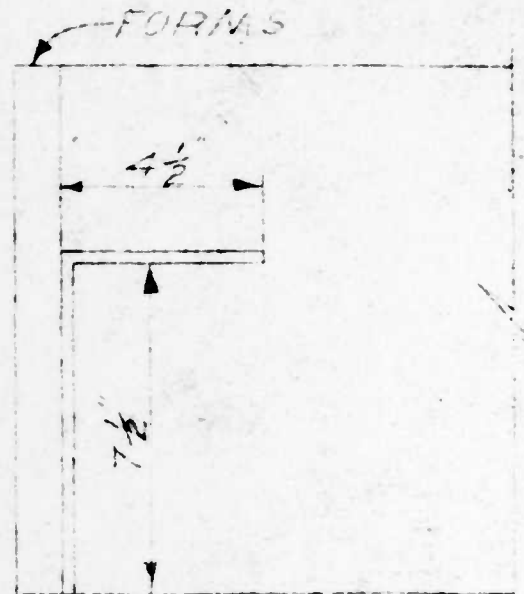
RUBBER WATER
Scale 3" = 1'-0"



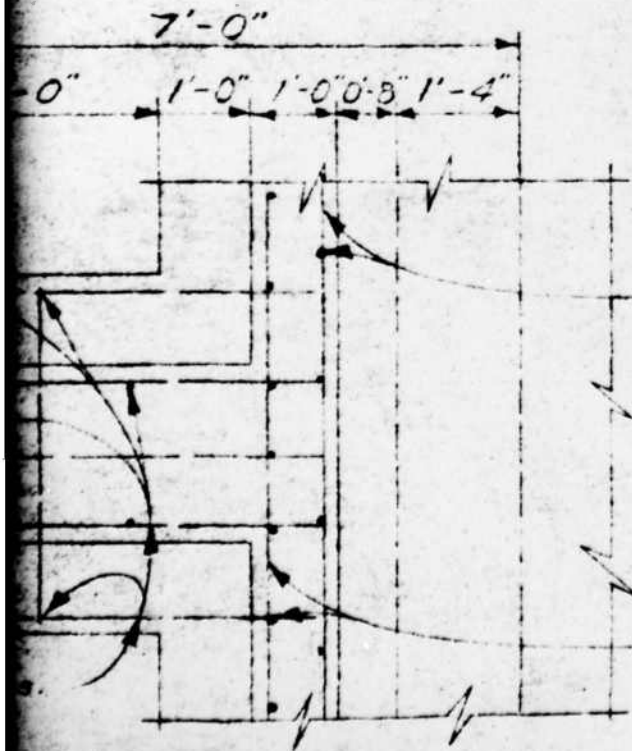
5/8" 4 Bars 1'-6" ctrs.



R WATER STOP
size 3" x 1'-0"



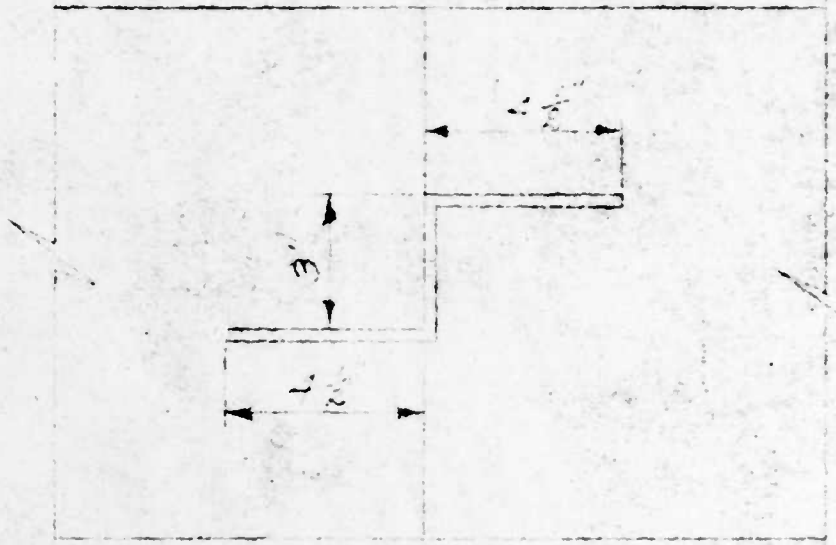
MET



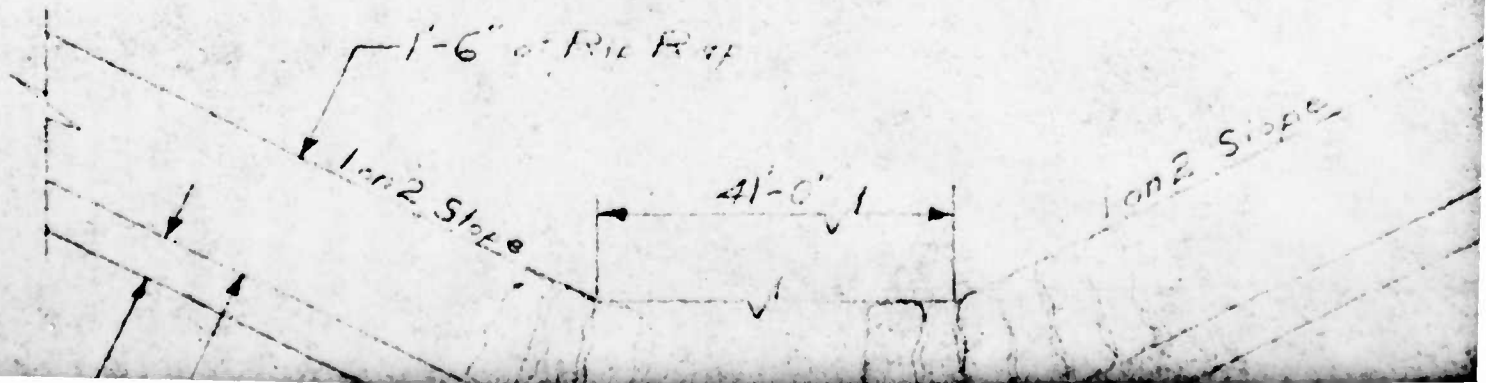
$\frac{5}{8}$ " Bars 1'-0" ctrs.

$\frac{5}{8}$ " Bars 1'-0" ctrs.

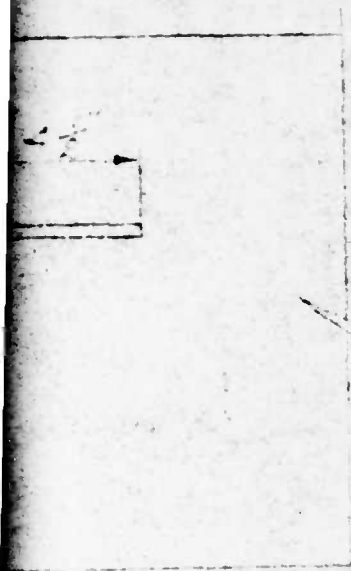
14



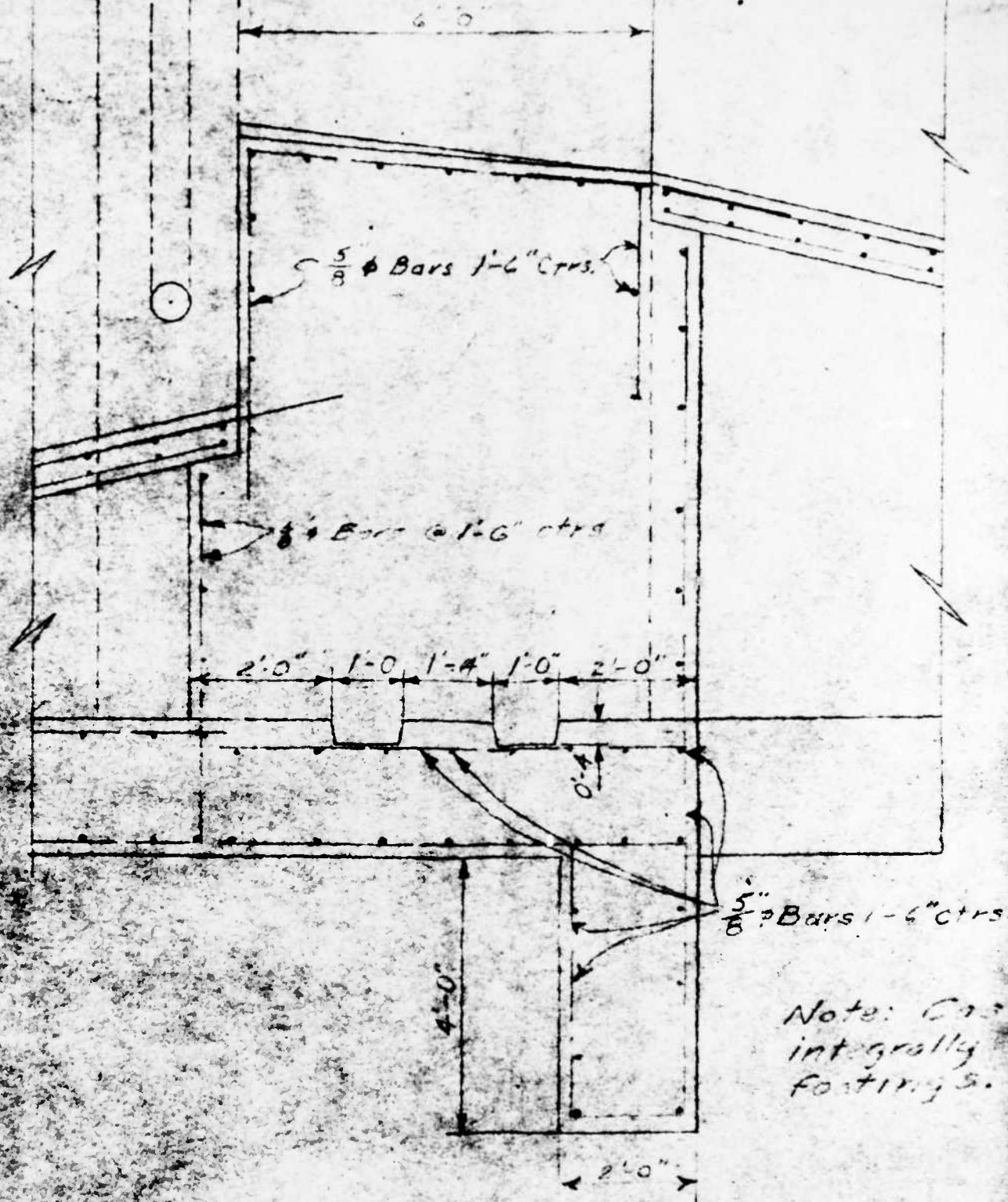
METAL WATER STOP
Scale 3"=1'-0"



Sheet 12



LONG SIDE

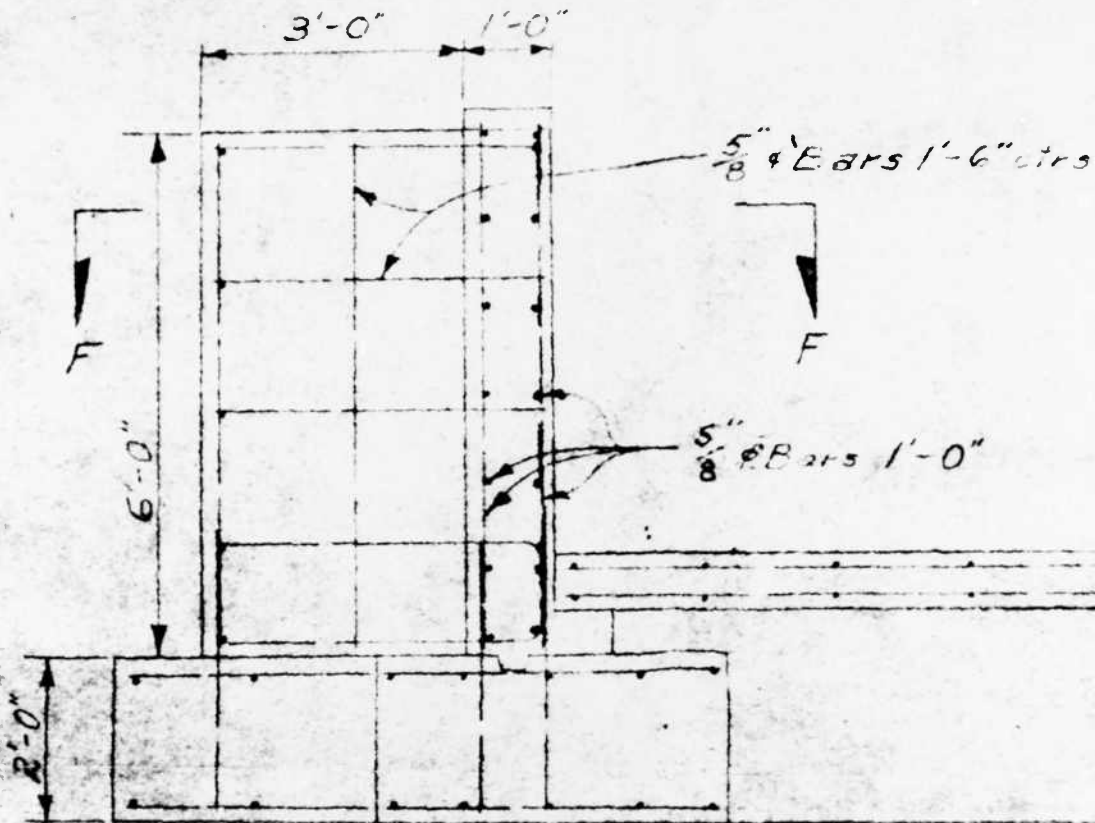


SECTION C-C
Scale $\frac{1}{2}$ " = 1'-0"

60

$\frac{5}{8}$ " Bars 1'-0" ctrs.

SECTION F-F
Scale $\frac{1}{2}$ " = 1'-0"



6" ctrs

Cast spillway foot.
ally with wall
ing 5.

SECTION D-D
Scale $\frac{1}{2}$ " = 1'-0"

$\frac{3}{8}$ " Bars 1'-0" ctrs.

0'-6" of Porous Fill

TYPICAL S.

$\frac{1}{2}$ " Bars 1'-6"

1'-6" of
Rip Rap

CAL SECTION of DOWNSTREAM CHANNEL

0'-6" of Porous Fill

WATER SUPPLY SYSTEM
VILLAGE OF GOBLESKILL, N.Y.
DETAIL OF SPILLWAY

SECTION N

Scale 1" = 10'

2-12-03 2-13-03

DATE
ILME