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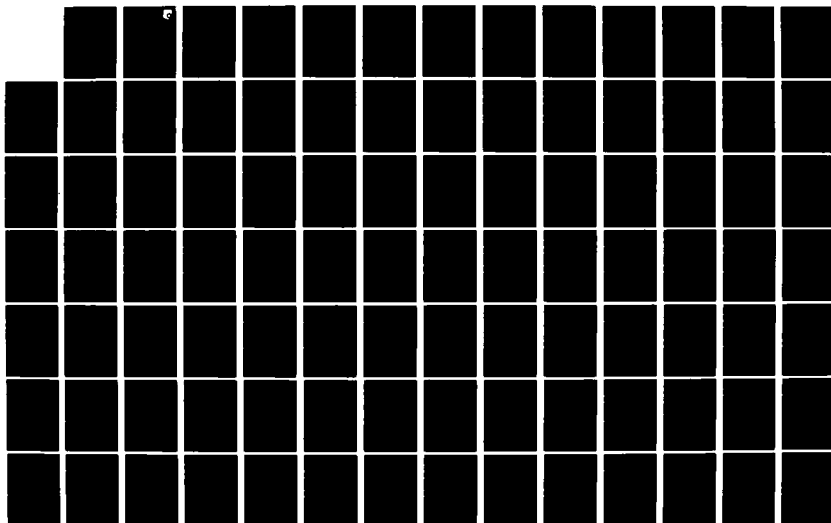
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MICHIGAN UNIV ANN ARBOR DEPT OF MECHANICAL ENGINEERING
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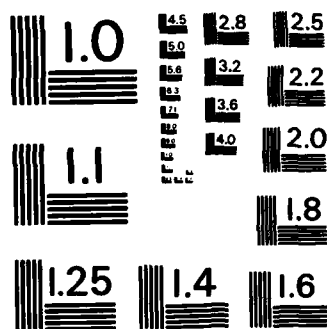
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STRENGTH OF MECHANICALLY FASTENED COMPOSITE JOINTS

Fu-kuo Chang
Richard A. Scott
George S. Springer

Department of Mechanical Engineering and Applied Mechanics
The University of Michigan
Ann Arbor, MI 48109

July, 1982

Final Report for Period June 1981-May 1982

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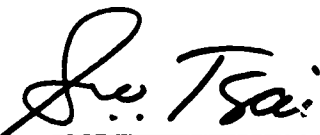
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This technical report has been reviewed and is approved for publication.



S. W. TSAI, Project Engineer & Chief
Mechanics and Surface Interactions Branch
Nonmetallic Materials Division

FOR THE COMMANDER



F. D. CHERRY, Chief
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REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER AFWAL-TR-82-4095	2. GOVT ACCESSION NO. AD-H121407	3. RECIPIENT'S CATALOG NUMBER
4. TITLE (and Subtitle) STRENGTH OF MECHANICALLY FASTENED COMPOSITE JOINTS		5. TYPE OF REPORT & PERIOD COVERED June, 1981-May, 1982
		6. PERFORMING ORG. REPORT NUMBER
7. AUTHOR(s) Fu-kuo Chang Richard A. Scott George S. Springer		8. CONTRACT OR GRANT NUMBER(s) F33615-81-C-5050
9. PERFORMING ORGANIZATION NAME AND ADDRESS Department of Mechanical Engineering and Applied Mechanics, The University of Michigan Ann Arbor, Michigan 48109		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS FY1457-81-02013
11. CONTROLLING OFFICE NAME AND ADDRESS Materials Laboratory (AFWAL/MLBM) Air Force Wright Aeronautical Laboratories Wright-Patterson, AFB, OH 45433		12. REPORT DATE July 1982
		13. NUMBER OF PAGES 97
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)		15. SECURITY CLASS. (of this report) Unclassified
		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report) Approved for public release, distribution unlimited		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18. SUPPLEMENTARY NOTES		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Composite Materials Failure Hypothesis Joints Bolted Joints		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) A method is presented for predicting the failure strength and failure mode of mechanically fastened joints made of fiber reinforced composite laminates. The method includes two steps. First, the stress distribution in the laminate is calculated by the use of a finite element method. Second, the failure load and the failure mode are predicted by means of a proposed failure hypothesis together with Yamada's failure criterion. A computer code was developed which can be used to calculate the maximum load and the mode of failure of joints involving laminates with different ply orientations,		

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different material properties, and different geometries. Results generated by the present method were compared to data and to existing analytical and numerical solutions. The results of the present method were found to agree well with those reported previously. Parametric studies were also performed to evaluate the effects of joint geometry and ply orientation on the failure strength and on the failure mode. -

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FOREWORD

This report was prepared by Fu-kuo Chang, Richard A. Scott, and George S. Springer, Department of Mechanical Engineering and Applied Mechanics, The University of Michigan for the Mechanics and Surface Interactions Branch (AFWAL/MLBM), Nonmetallic Materials Division, Materials Laboratory, Air Force Wright Aeronautical Laboratories, Wright-Patterson AFB, Ohio. The work was performed under Contract Number F 33615-81-C-5050, Project number FY1457-81-02013.

This report covers work accomplished during the period June, 1981-May 1982.

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LIST OF SYMBOLS

A	Total surface area of the laminate
A_L	Stress prescribed area
A_F	Stress free area
A_R	Displacement prescribed area (fixed boundary)
A_{Lg}	Surface area of an element g on which surface tractions is applied
B	Bearing stress
D	Diameter of the hole
E	Edge distance
E_{ijkl}	Elastic moduli
E_{mn}	Reduced laminate moduli
e	Failure indicator ($e < 1$ non-failure, $e \geq 1$ failure)
e_o	Maximum value of e on the characteristic curve
$F_{i\beta}$	Assembled load vector
H	Thickness of the laminate
h^p	Thickness of the p -th ply
$k_{i\beta k\alpha}^g$	Stiffness matrix of the g -th element
$\bar{K}_{i\beta k\alpha}$	Assembled stiffness matrix
L	Plate length
M	Number of elements
N	Number of plies in the laminate
N_α	Shape function
n_j	Unit vector normal to the surface
P	Applied Load
P_{max}	Failure (maximum) load
$\bar{O}_{ij}^p$	Transformed reduced stiffness matrix of the p -th ply

q_{ia}	Nodal displacement
r	Radial distance
r_c	Radial distance to the characteristic curve
R_{ot}	Characteristic length for tension
R_{oc}	Characteristic length for compression
S	Ply shear strength
S_c	Shear strength of cross-ply laminate
s	Total area of the two-dimensional laminate
s_g	Area of element g (2-dimensions)
SCF	Stress concentration factor
T_i	Surface traction components
u_i	Displacement
$\bar{u}_i$	Arbitrary displacement functions
V_o	Total volume of the laminate
v_g	Volume of element g
W	Width of the plate
X	Ply tensile strength
x	Coordinate along the fiber direction in each ply
x_1	Coordinate perpendicular to the loading direction in the laminate plane
x_2	Coordinate opposed to the loading direction and perpendicular to the x_1 -axis
x_3	Coordinate perpendicular to the x_1 and x_2 axes
y	Coordinate perpendicular to the fiber direction in each ply
Γ_L	Boundary curve of the hole on which the surface traction is applied
Γ_{Lg}	Boundary curve of the element g on which the surface traction is applied
$\Delta\theta$	Ply continuity

ϵ_{ij}	Strain components in the x_1 - x_2 coordinate system
η	Angle measured counterclockwise from the x_1 -axis
θ_f	Angle at which failure occurs
σ_{ij}	Stress components in the x_1 - x_2 coordinate system
$\sigma_x, \sigma_y, \sigma_{xy}$	Stress components in the x-y coordinate system
ϕ	Maximum angular range of the ply orientation

SECTION I

INTRODUCTION

Among the major advantages of laminated composite structures over conventional metal structures are their comparatively high strength to weight and stiffness to weight ratios. As a result, fiber reinforced composite materials have been gaining wide application in aircraft and spacecraft construction. These applications require joining composites either to composites or to metals. Most commonly, joints are formed using mechanical fasteners. Therefore, suitable methods must be found to determine the failure strengths of mechanically fastened joints. A knowledge of the failure strength would help in selecting the appropriate size joint in a given application.

Owing to the significance of the problem, several investigators have developed analytical procedures for calculating the strength of bolted joints in composite materials. Among the recent studies are those of Waszczak and Cruse [1], Agarwal [2], and Garbo and Ogonowski [3]. As will be discussed in Section VII, the previous methods provide conservative results and underestimate the failure strength, often by as much as fifty percent.

The major objective of this investigation was, therefore, to develop a method which a) predicts the failure strength and failure mode of mechanically fastened composite joints with better accuracy than the existing analytical methods and b) can be used readily in the design of mechanically fastened composite joints. In the present method first the stress distribution around the hole is calculated by the use of a finite element method. Second, the failure load and the failure mode are predicted by means of a proposed new failure

hypothesis together with Yamada's [4] failure criterion. On the basis of this analysis a computer code was developed which can be applied to joints involving laminates with different ply orientations, different material properties, and different configurations, including different hole sizes, hole positions, and joint thicknesses. Because of the accuracy of the method and the flexibility of the computer code, the code can be applied to the analysis and the design of mechanically fastened composite joints.

SECTION II

PROBLEM STATEMENT

Consider a plate (length L , width W , thickness H) made of N fiber reinforced unidirectional plies. The ply orientation is arbitrary, but must be symmetric with respect to the $x_3=0$ plane (symmetric laminate, Figure 1). Perfect bonding between each ply is assumed.

A hole of diameter D is located along the centerline of the plate ($x_1=0$) at a distance E from one end of the plate. A rigid pin (diameter D), supported outside the laminate, is inserted into the hole (Figure 1). A uniform tensile load P is applied to the plate, as shown in Figure 1. The load is parallel to the plate (in-plane loading) and is symmetric with respect to the centerline. Hence the load cannot create bending moments about either the x_1 , x_2 or x_3 axes. Moreover, for symmetric laminates, in plane and bending effects are uncoupled. It is desired to find

- 1) the stresses and strains in each ply,
- 2) the maximum (failure) load ($P_{\max}$) that can be applied before the joint fails, and
- 3) the mode of failure.

Point 2 refers to the fact that, according to experimental evidence, mechanically fastened joints under tensile loads generally fail in three basic modes referred to as tension mode, shearout mode, and bearing mode. The type of damage resulting from each of these modes is illustrated in Figure 2. The objective, listed in point 3 above, is to determine which of these modes will most be responsible for the failure.

The calculation proceeds in three steps. For a given geometry and

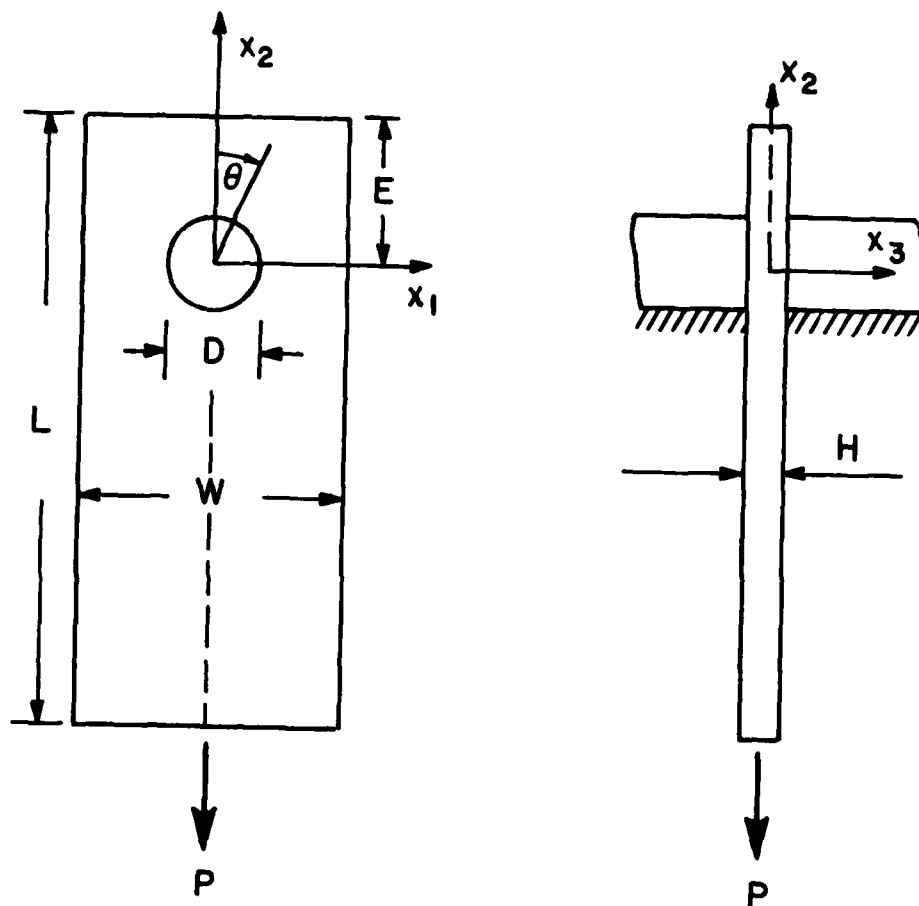


Figure 1. Geometry of the problem

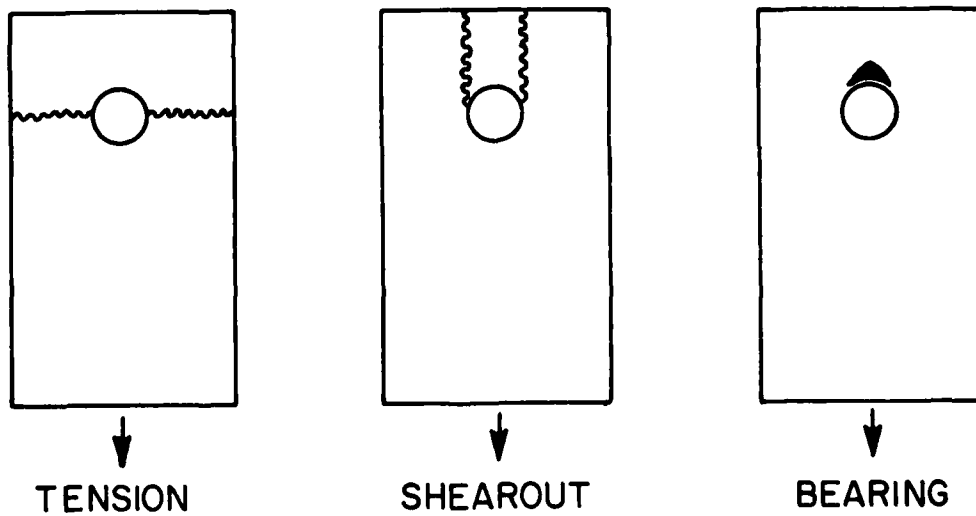


Figure 2. Illustration of the three basic failure modes

load

- 1) the stress distribution around the hole is calculated,
- 2) the maximum (failure) load is predicted , and
- 3) the mode of failure is determined.

The details of these steps are presented in Sections III-V.

SECTION III

STRESS ANALYSIS-GOVERNING EQUATIONS

The stresses in the laminate are calculated on the basis of anisotropic theory of elasticity and classical lamination plate theory. Accordingly, in the analysis planes are taken to remain planes, the strain across the thickness is taken to be constant $[\epsilon_{ij}=f(x_1, x_2)]$ and only plane stresses are considered ($\sigma_{13}=\sigma_{23}=\sigma_{33}=0$). Under these conditions, in the absence of body forces, the condition of force equilibrium can be expressed as [5]

$$\begin{aligned}\frac{\partial \sigma_{11}}{\partial x_1} + \frac{\partial \sigma_{12}}{\partial x_2} &= 0 \\ \frac{\partial \sigma_{21}}{\partial x_1} + \frac{\partial \sigma_{22}}{\partial x_2} &= 0\end{aligned}\tag{1}$$

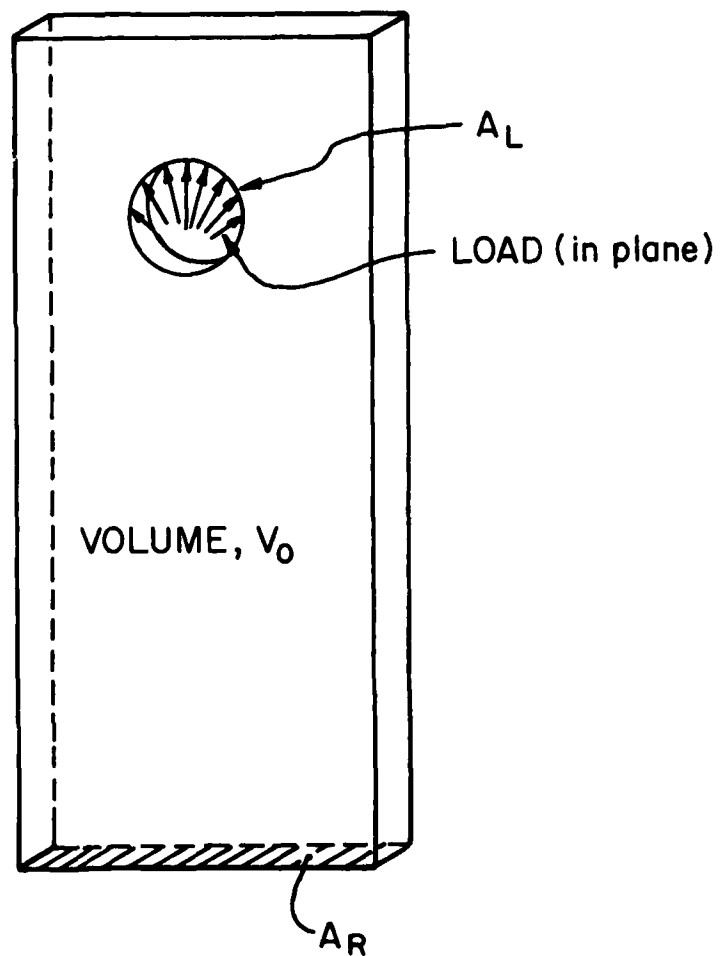
In index notation eq. (1) becomes

$$\sigma_{ij,j} = 0\tag{2}$$

σ_{ij} is the stress in the plane normal to the x_i axis and is in the x_j direction. The subscripts i and j may have the values 1 or 2. Consider now an elastic laminate of volume V_0 containing a loaded hole as shown in Figure 3. Stresses are applied over the surface area A_L . The surface area A_R is rigidly fixed (no displacement), while the surface area A_F is free of applied stress. The total surface area is

$$A = A_L + A_R + A_F\tag{3}$$

Let us denote by $\bar{u}_i$ any arbitrary displacement inside the body. $\bar{u}_i$ is a test function. The only requirement is that $\bar{u}_i$ be continuous, differentiable and be zero on A_R . By multiplying eq. (2) by $\bar{u}_i$ and by taking the volume integral of the



TOTAL SURFACE: A

LOADED SURFACE: A_L

FIXED SURFACE: A_R

STRESS FREE SURFACE: $A_F = A - A_L - A_R$

Figure 3. Configuration of an elastic laminate with a loaded hole

resulting expression we obtain

$$\iiint_{V_0} \sigma_{ij,j} \bar{u}_i dV = 0 \quad (4)$$

By employing the identity

$$\sigma_{ij,j} \bar{u}_i = (\sigma_{ij} \bar{u}_i)_{,j} - \sigma_{ij} \bar{u}_{i,j} \quad (5)$$

and by utilizing Gauss' theorem, eq. (4) may be written as

$$\iint_A \sigma_{ij} n_j \bar{u}_i dA - \iiint_{V_0} \sigma_{ij} \bar{u}_{i,j} dV = 0 \quad (6)$$

where n_j is the unit vector normal to the surface. On the free surface A_F the stresses are zero, while on the surface A_R the displacement is zero. These conditions give

$$\iint_{A_F} \sigma_{ij} n_j \bar{u}_i dA = 0 \quad (7)$$

$$\iint_{A_R} \sigma_{ij} n_j \bar{u}_i dA = 0 \quad (8)$$

The force per unit area (called surface traction) at each point of the surface area A_L is [5]

$$T_i = \sigma_{ij} n_j \quad (9)$$

Equations (6) - (9) yield

$$\iint_{A_L} T_i \bar{u}_i dA = \iiint_{V_0} \sigma_{ij} \bar{u}_{i,j} dV \quad (10)$$

The stress is related to the displacement through the stress-strain relationship, which for an elastic body is

$$\sigma_{ij} = E_{ijkl} \epsilon_{kl} \quad (11)$$

The subscripts k and l may take on the values of 1 or 2. The strains are related to the displacements u_j by the expression

$$\epsilon_{kl} = \frac{1}{2} \left(\frac{\partial u_k}{\partial x_l} + \frac{\partial u_l}{\partial x_k} \right) \quad (12)$$

By combining eqs. (10) - (12) we obtain

$$\iiint_{V_0} E_{ijkl} \bar{u}_{i,j} u_{k,l} dV = \iint_{A_L} T_i \bar{u}_i dA \quad (13)$$

E_{ijkl} are the moduli of elasticity of the laminate. Because of the laminate symmetry, the following simplification may be made

$$E_{ijkl} = E_{mn} \quad (14)$$

The subscripts i, j, k , and l are related to m and n as follows

$$\begin{aligned} i=j=1 &\rightarrow m=1 & k=l=1 &\rightarrow n=1 \\ i=j=2 &\rightarrow m=2 & k=l=2 &\rightarrow n=2 \\ i \neq j &\rightarrow m=3 & k \neq l &\rightarrow n=3 \end{aligned} \quad (15)$$

The reduced laminate modulus E_{mn} is given by

$$E_{mn} = \sum_{p=1}^N \frac{h^p}{H} \bar{Q}_{ij}^p \quad (16)$$

where h^p is the thickness of the p -th ply. $\bar{Q}_{ij}^p$ is the transformed reduced stiffness matrix for the p -th ply [6] (Appendix A).

SECTION IV

STRESS ANALYSIS-FINITE ELEMENT METHOD

In order to perform the calculations, the problem shown in Figure 1 was simulated by the geometry given in Figure 4. Because of symmetry the stresses were calculated only in one half of the body. Along the symmetry axis, displacement is allowed only in the x_2 direction. Along the lower edge of the plate, displacement is allowed only in the x_1 direction. The intersection of the symmetry axis and the lower edge is considered to be rigidly fixed.

The surface of the hole is subjected to a surface traction T_i . The parameter T_i is related to the applied load. The spatial distribution of T_i depends on the magnitude of the applied load, on the material properties, and on the geometry in a complex manner. It is extremely difficult, if not impossible, to determine the exact distribution of T_i inside the hole. To overcome this difficulty a cosine normal load distribution was assumed. With this approximation T_i becomes

$$T_i = - \frac{4P}{\pi D} n_i \cos \theta \quad (17)$$

The angle θ is in the x_1 - x_2 plane and is measured clockwise from the x_2 axis (Figure 1). For isotropic materials the cosine normal load distribution (eq. 17) was found to represent closely the actual load distribution [7]. Calculations performed by previous investigators also showed that for composite materials the stress distribution inside the body is insensitive to the assumed load distribution [1,3,8]. Therefore, eq. (17) should suffice for the purpose of the present analysis which is to determine the overall strength of the joint.

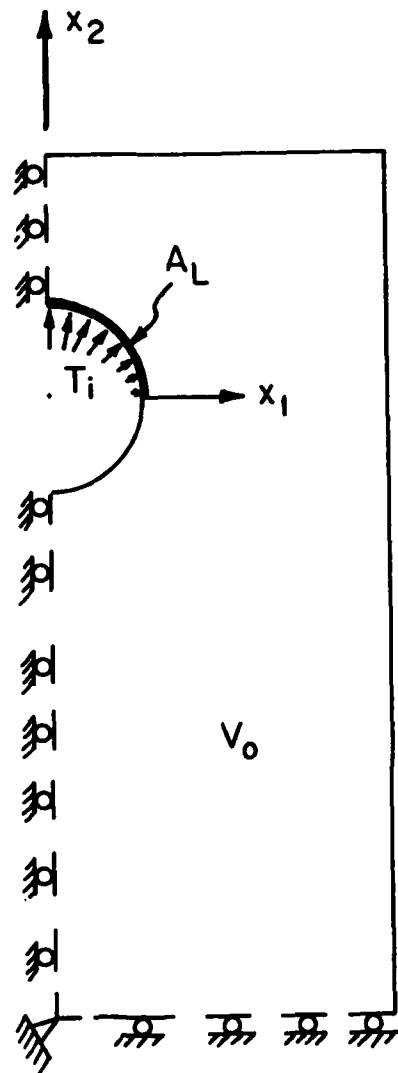


Figure 4. Configuration of a joint approximated in the finite element method

Equations (13) and (17) give

$$\iiint_{V_0} E_{ijkl} \bar{u}_{i,j} u_{k,l} dV = \iint_{A_L} - \frac{4P}{\pi D} n_i u_i \cos \theta dA \quad (18)$$

We recall that $\bar{u}_i$ are functions that can be selected arbitrarily. The unknowns in eq. (18) are the displacements $\bar{u}_i$. Once $\bar{u}_i$ are known the stress at every point can be calculated from eqs (11) and (12). The method of solution of eq. (18) is described below.

Solutions to eq.(18) were obtained by a finite element method (FEM). As a first step in the solution procedure the volume V_0 is subdivided into M subdomains of volume V_g

$$V_0 = \sum_{g=1}^M V_g \quad (19)$$

Equation (18) may now be written as

$$\sum_{g=1}^M \iiint_{V_g} E_{ijkl} \bar{u}_{i,j} u_{k,l} dV = \sum_{g=1}^M \iint_{A_{Lg}} - \frac{4P}{\pi D} n_i \bar{u}_i \cos \theta dA \quad (20)$$

A_{Lg} is the surface of an element on the inside of the hole where the surface traction is applied ($0 \leq \theta \leq \pi/2$). At any surface where load is not applied, A_{Lg} is zero. Advantage is taken now of the assumption that the strains ($\epsilon_{11}, \epsilon_{22}$ and ϵ_{12}) are independent of the thickness, i.e. the strains are independent of x_3 and depend only on x_1 and x_2 . Thus, the three dimensional grid consisting of M volume elements may be replaced by a two dimensional grid consisting of M surface elements of area s_g (Figure 5)

$$s = \sum_{g=1}^M s_g \quad (21)$$

Equation (20) thus becomes

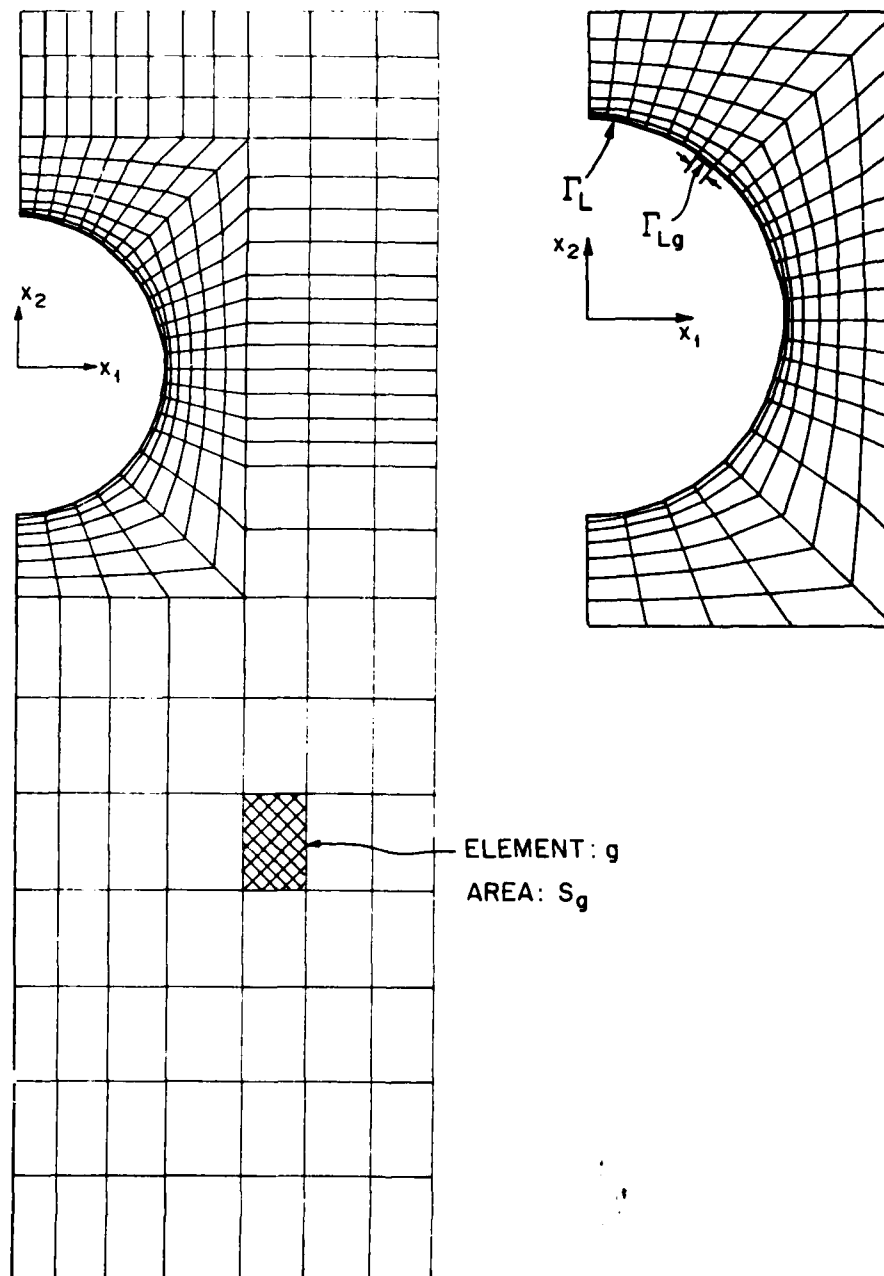


Figure 5. Grid used in the finite element method. Right hand figure is an enlarged view of the grid around the hole

$$\sum_{g=1}^M \iint_{S_g} E_{ijkl} \bar{u}_{i,j} u_{k,l} ds = \sum_{g=1}^M \int_{\Gamma_{Lg}} -\frac{4P}{\pi D} n_i \bar{u}_i \cos\theta d\Gamma \quad (22)$$

Γ_L is a line along the boundary of the hole where the surface traction is applied ($0 \leq \theta \leq \pi/2$, Figure 5). Each segment of this line (denoted by Γ_{Lg}) coincides with the boundary of an element g adjacent to the hole. For those elements which do not lie along this line Γ_{Lg} equals zero. Isoparametric 4-node elements were used in this investigation. The grid was generated using a grid generator [9]. The grid sizes were unequal. Smaller grids were used in the vicinity of the hole to obtain a better resolution of the stresses. Utilizing the symmetry about the x_2 axis, a grid (consisting of 306 elements) was placed on one half of the laminate, as illustrated in Figure 5.

The displacement in each element can be expressed in terms of the displacements of the four nodal points [10]

$$\begin{aligned} u_i &= N_\alpha q_{i\alpha} \\ \bar{u}_i &= N_\alpha \bar{q}_{i\alpha} \end{aligned} \quad (23)$$

The subscript α designates the nodal points ($\alpha=1,2,3$, or 4). N_α is the shape function described in detail in Appendix B. $q_{i\alpha}$ is the displacement at the nodal point α in the i direction.

We define a stiffness matrix for the g -th element as

$$K_{i\beta k\alpha}^g = \iint_{S_g} E_{ijkl} N_{\alpha,l} N_{\beta,j} ds \quad (24)$$

$K_{i\beta k\alpha}^g$ is an eight by eight matrix. The subscript β may take on the values 1,2,3, and 4. The nodal displacements $q_{k\alpha}$ and $\bar{q}_{i\beta}$ are independent of the surface and line integrations. Accordingly, eqs. (22) - (24) yield

$$\sum_{g=1}^M K_{i\beta k\alpha}^g q_{k\alpha} \bar{q}_{i\beta} = \sum_{g=1}^M \bar{q}_{i\beta} \int_{\Gamma_{Lg}} -\frac{4P}{\pi D} n_i N_{\beta} \cos\theta d\Gamma \quad (25)$$

The nodal displacements $\bar{q}_{i\beta}$ are arbitrary functions and hence eq. (25) can be written

$$\bar{K}_{i\beta k\alpha} q_{k\alpha} = \bar{F}_{i\beta} \quad (26)$$

where the global stiffness matrix, $\bar{K}_{i\beta k\alpha}$ and the load vector $\bar{F}_{i\beta}$ are given by

$$\bar{K}_{i\beta k\alpha} \equiv \sum_{g=1}^M K_{i\beta k\alpha}^g \quad (27)$$

$$\bar{F}_{i\beta} \equiv \sum_{g=1}^M \int_{\Gamma_{Lg}} -\frac{4P}{\pi D} n_i N_{\beta} \cos\theta d\Gamma \quad (28)$$

The elements of $\bar{K}_{i\beta k\alpha}$ and the components of the vector $\bar{F}_{i\beta}$ are known. Hence, $q_{k\alpha}$ can be obtained from eq. (26) using the Gaussian elimination method [12]. Once $q_{k\alpha}$ are known the displacements u_i are calculated from eq. (23). A computer code was developed for performing the calculations and for generating solutions (section VI).

SECTION V

PREDICTION OF FAILURE

In order to determine the load at which a joint fails and the mode of failure, the conditions for failure must be established. In this investigation the joint is taken to have failed when the combined stresses have exceeded a prescribed limit in any of the plies along an approximately chosen curve (denoted as the characteristic curve). The combined stress limit is evaluated using the failure criterion proposed by Yamada [4]. The coordinates of the characteristic curve are established by extending Whitney and Nuismer's failure hypothesis [13] (developed for open, unloaded holes) to loaded holes.

1) Failure Criterion

Numerous criteria for failure have been proposed in the past [14,15,16,17]. Although the concepts underlying the different failure criteria may be different, the results of the various criteria are generally quite similar. In this investigation Yamada's failure criterion was adopted [4]. This criterion is based on the assumption that just prior to failure of the laminate every ply has failed due to cracks along the fibers. The validity of this assumption is supported by tests performed during this investigation with 64 ply graphite-epoxy (AS/3501-6) laminates. Yamada's criterion states that failure occurs when the following condition is met in any one of the plies

$$\left(\frac{\sigma_x}{X}\right)^2 + \left(\frac{\sigma_{xy}}{S_c}\right)^2 = e^2 \begin{cases} e < 1 & \text{no failure} \\ e \geq 1 & \text{failure} \end{cases} \quad (29)$$

σ_x and σ_{xy} are the longitudinal and shear stresses in a ply, respectively (x and y being the coordinates parallel and normal to the fibers in the ply). X is the longitudinal tensile strength of the ply. S_c is the shear strength of a symmetric, cross ply laminate which has the same number of plies as the laminate under consideration. As indicated in eq. (29) failure occurs when e is equal to or greater than unity.

2) Failure Hypothesis-Characteristic Curve

The hypothesis is proposed here that failure occurs when in any one of the plies the combined stresses satisfy an appropriately chosen failure criterion at any point on a characteristic curve. The characteristic curve (Figure 6) is specified by the expression

$$r_c(\theta) = D/2 + R_{ot} + (R_{oc} - R_{ot}) \cos \theta \quad (30)$$

The angle θ , measured clockwise from the x_2 axis, may range in value from $-\pi/2$ to $\pi/2$. R_{ot} and R_{oc} are the characteristic lengths for tension and compression [13,18]. These parameters can be determined experimentally by measuring the tensile and compressive strengths of notched laminates. R_{ot} and R_{oc} depend only on the material. Therefore, the coordinates of the characteristic curve also depend only on the material, and are independent of the geometry and the stress distribution.

In this investigation the characteristic curve is used together with the Yamada failure criterion. Accordingly (see eq. 29), failure occurs when the parameter e is equal to or is greater than unity at any point on the characteristic curve

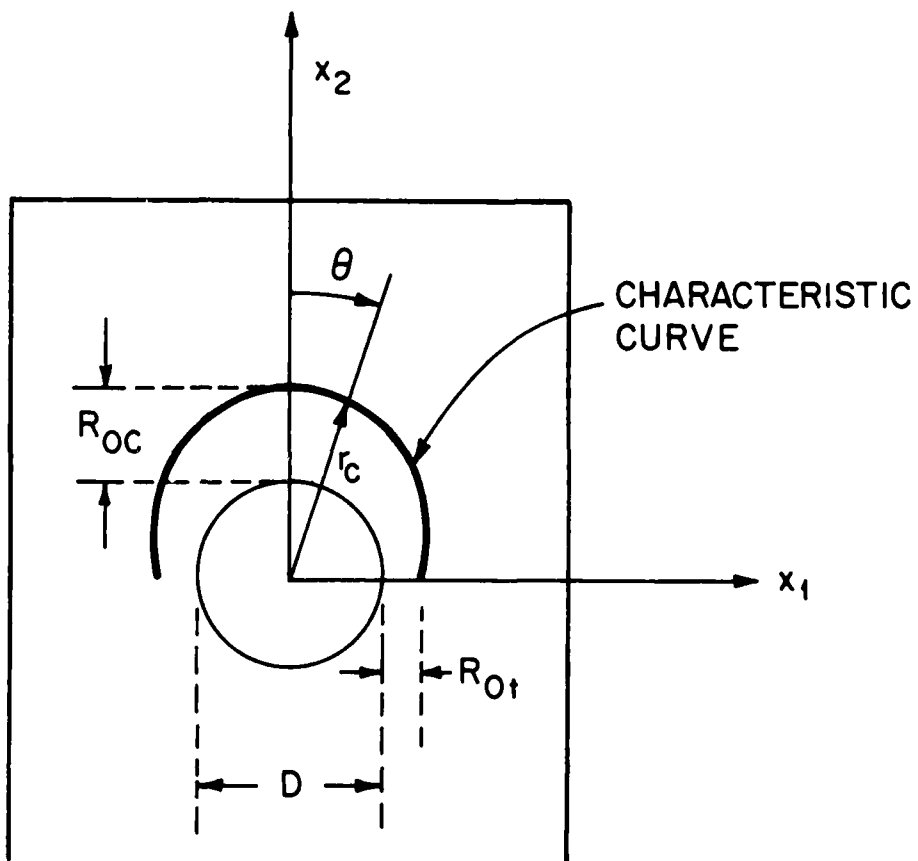


Figure 6. Description of the characteristic curve

$$\left. \begin{array}{ll} \text{No failure} & e < 1 \\ \text{Failure} & e \geq 1 \end{array} \right\} \text{ at } r = r_c \quad (31)$$

It is emphasized that the above failure hypothesis is used here in conjunction with the Yamada failure criterion (eq. 29). However, the hypothesis is general, and is not restricted to Yamada's criterion. The characteristic curve proposed here may be used with any other failure criterion.

3) Solution Procedure

Whether or not a joint fails under a given condition is determined as follows. For a given load

a) the stresses ($\sigma_1, \sigma_2, \sigma_{12}$) are calculated in each ply using eqs. (11), (14) and (16), and the FEM described in Sections II, and III,

b) the longitudinal and shear stresses (σ_x, σ_{xy}) are evaluated in each ply employing the transformation

$$\begin{aligned} \sigma_x &= \sigma_1 \cos^2 \eta + \sigma_2 \sin^2 \eta + 2\sigma_{12} \sin \eta \cos \eta \\ \sigma_{xy} &= -\sigma_1 \sin \eta \cos \eta + \sigma_2 \sin \eta \cos \eta + \sigma_{12} (\cos^2 \eta - \sin^2 \eta) \end{aligned} \quad (32)$$

where η is the angle measured counter clockwise from the x_1 -axis to the x -axis of each ply.

c) the parameter e is calculated (eq. 29) along the characteristic curve

d) if e equals or exceeds the value of unity ($e \geq 1$) in any ply along the characteristic curve, the joint is taken to have failed.

The procedure outlined above is used to predict whether or not failure occurs under a given load. Due to the assumption of a cosine normal load distribution around the hole (eq. 17), the calculated

stresses are linearly proportional to the applied load P . This fact together with Yamada's failure criterion (eq. 29) gives

$$P \sim e$$

This relationship is utilized to determine the maximum load ($P_{\max}$) which can be imposed on the joint. For a given load P , values of e are calculated on the characteristic curve as discussed above (points a-d). The highest value of e (e_0) is then determined, and the maximum load is calculated by the expression

$$P_{\max} = \frac{P}{e_0} \quad (34)$$

The calculation procedure described in the foregoing also provides the location (angle θ_f) at which e first reaches the value of unity ($e=1$) on the characteristic curve. (Figure 7). A knowledge of θ_f provides an estimate of the mode of failure. When θ_f is small ($\theta_f \approx 0^\circ$) failure is by the bearing mode. When $\theta_f \approx 45^\circ$ failure is due to shearout. When $\theta_f \approx 90^\circ$ failure is caused by tension. In summary

$$\begin{array}{ll} -15^\circ < \theta_f < 15^\circ & \text{bearing mode} \\ 30^\circ < \theta_f < 60^\circ & \text{shearout mode} \\ 75^\circ < \theta_f < 90^\circ & \text{tension mode} \end{array} \quad (35)$$

At intermediate values of θ_f failure may be caused by a combination of these modes.

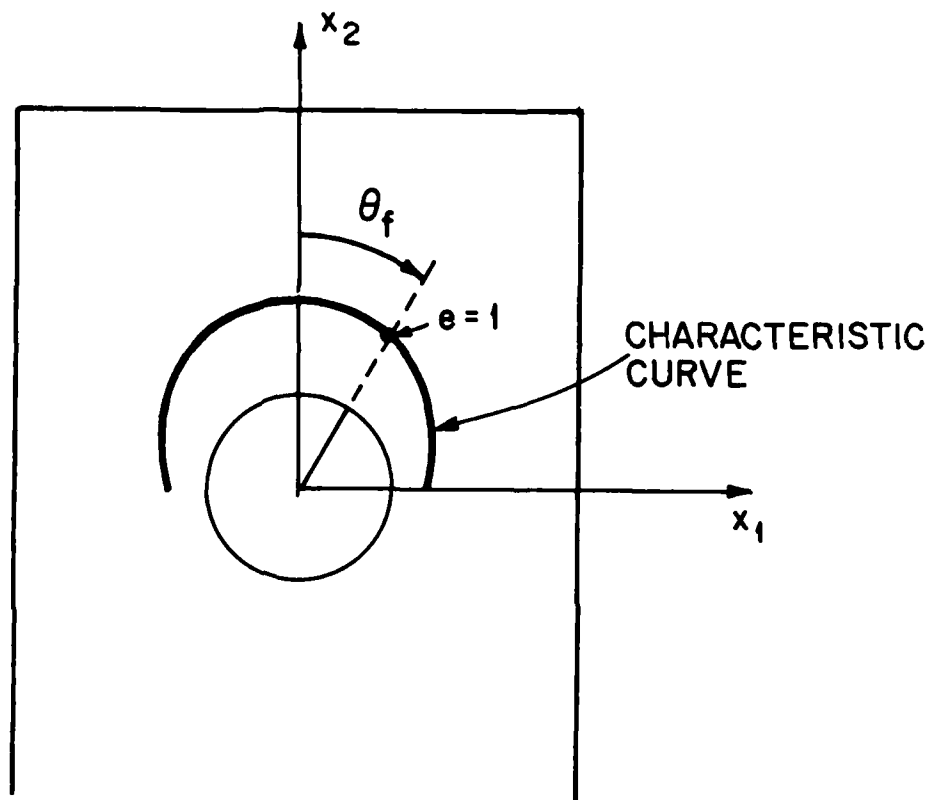


Figure 7. Location of failure ($e=1$) along the characteristic curve

SECTION VI

NUMERICAL SOLUTION

A computer code (designated as BOLT) was developed which is suitable for generating solutions to the problem formulated in Sections II-V. The required input parameters and the output provided by the code are summarized in Table 1.

A Fortran listing of the code and a sample input-output are included in Appendix C.

Table 1. Input parameters required by the computer code and the output provided by the code.

Input Parameters

- 1) Ply properties
 - a) Young's modulus, E_1
 - b) Shear modulus, G_{12}
 - c) Poisson's ratio, ν_{12}
- 2) Ply orientations
- 3) Geometry
 - a) hole diameter, D
 - b) thickness, H
 - c) width, W
 - d) length, L
 - e) edge distances, E
- 4) Characteristic lengths, R_{ot} and R_{oc}
- 5) Longitudinal tensile strength of each ply, X
- 6) Shear strength of cross ply laminate S_c

Output Parameters

- 1) Failure load
- 2) The failure mode
- 3) Stresses in the laminate

SECTION VII

RESULTS AND DISCUSSIONS

Results were generated in order to assess the validity and accuracy of the method and the computer code and to compare the results of the present method with the results of other existing methods of solutions. In addition, parametric studies were performed to evaluate the major characteristics of bolted joints.

1) Isotropic and Orthotropic Plates

Stress distributions were calculated in isotropic plates containing both unloaded (open) and loaded holes and in orthotropic plates containing unloaded holes. These problems were selected because analytical solutions are available for comparisons with the results of the present method.

An analytical solution for the stress distribution in an infinite ($W \rightarrow \infty$) isotropic plate containing an unloaded hole was given by Timoshenko [19]. The stress distribution in such a plate was also calculated by the present method. The parameters used in the numerical calculations are given in Figure 8. A large width ($W/D=14$) was used in the calculation to approximate an infinite plate. The results of the present method and the analytical solution of Timoshenko are compared in Figure 8. There is excellent agreement between the stresses calculated by the two methods.

The stresses in isotropic plates containing loaded holes were also calculated. Plates of infinite and finite widths were considered. Calculations were performed for the parameters given in Figure 9 and Table 2. From the calculated stresses, the stress concentration factor was determined. The stress concentration factor is defined as

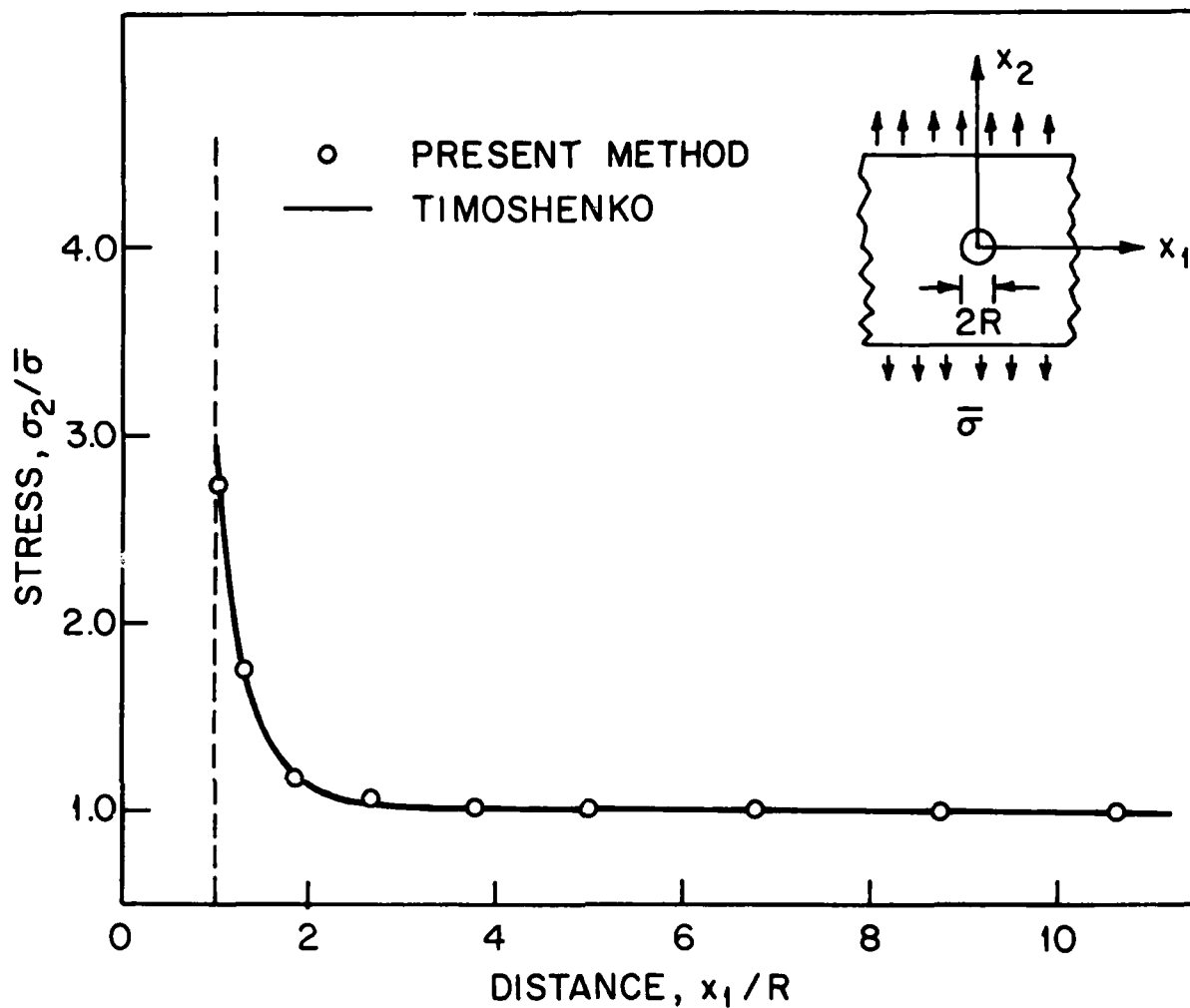


Figure 8. The stress σ_2 along the x_1 -axis in an isotropic infinite plate containing a circular hole. Comparison of the present results with the theoretical results given by Timoshenko [19]. Parameters used in the numerical calculations: $\bar{\sigma}=1.64$ MPa, $D=2R=7.62$ mm, $W/D=14$, $E/D=14$, $L/D=28$

$$SCF \equiv \frac{(\sigma_2)_{\max}}{B} \quad (36)$$

where $(\sigma_2)_{\max}$ is the maximum stress in the plate (perpendicular to the x_1 - x_3 plane, Figure 1) and B is the bearing stress

$$B \equiv \frac{P}{(H)(D)} \quad (37)$$

The stress concentration factors obtained by the present method were compared to those reported by previous investigators (Table 2). The maximum difference in the stress concentration factors given by the different methods is about 20 percent. The stress concentration factor given by the present method differs from the values given by previous investigators at most by 15 percent.

The stresses in an isotropic plate of finite width containing a loaded hole are shown in Figure 9. The stresses calculated by the present method are in excellent agreement with De Jong's approximate solution [8].

The stress distribution in an orthotropic plate of finite width containing an open (unloaded) hole was also calculated. The calculations were performed for a plate with the symmetric laminate lay up of $[0/90]_S$. An analytical solution for this problem was provided previously by Nuismer and Whitney [18], who modified Lekhnitskii's earlier solution [20] for an infinite plate. The results given in Figure 10 show excellent agreement between the stresses calculated by the present method and by the analytical solution.

The aforementioned comparisons indicate that the present method predicts the stress distribution around loaded and unloaded holes with high accuracy.

Table 2. Stress Concentration Factor (SCF) Around a Pin Loaded Hole Contained in an Isotropic Plate of Infinite Width. Comparison of Present Result with Those Obtained by Previous Investigators.

<u>Investigators</u>	<u>SCF</u>
Present Result*	0.985
Hong [22]	0.955
De Jong [8]	1.058
Eshwar et al [23]	0.922
Bickley [7]	0.81

*Present results was calculated for the case:

$$D = 7.62 \text{ mm}, W/D = 8, E/D = 4, L/D = 14.$$

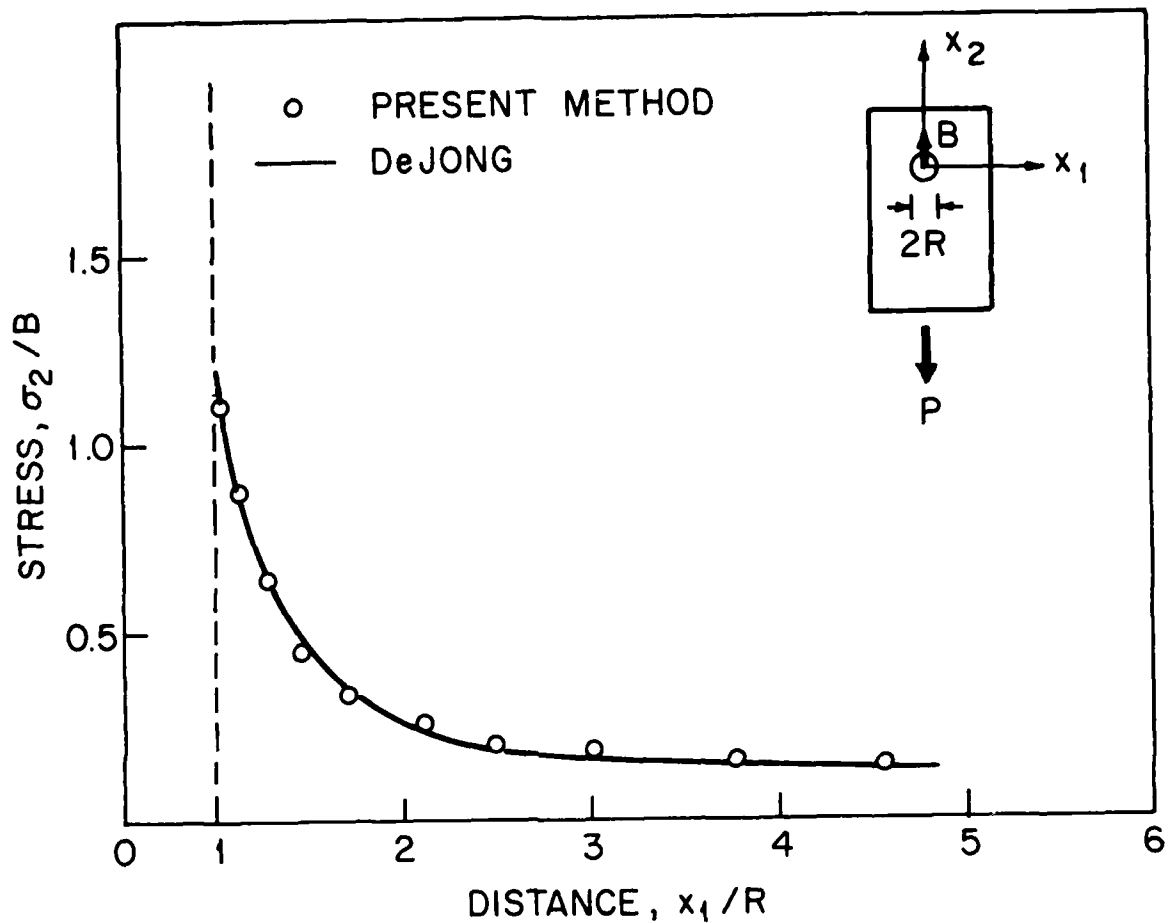


Figure 9. The stress σ_2 along the x_1 -axis in an isotropic plate of finite width containing a loaded hole. Comparison of the present results with the theoretical results given by De Jong [8]. Parameters used in the numerical calculations: $D=7.62$ mm, $W/D=5.0$, $E/D=4.0$, $L/D=14.0$

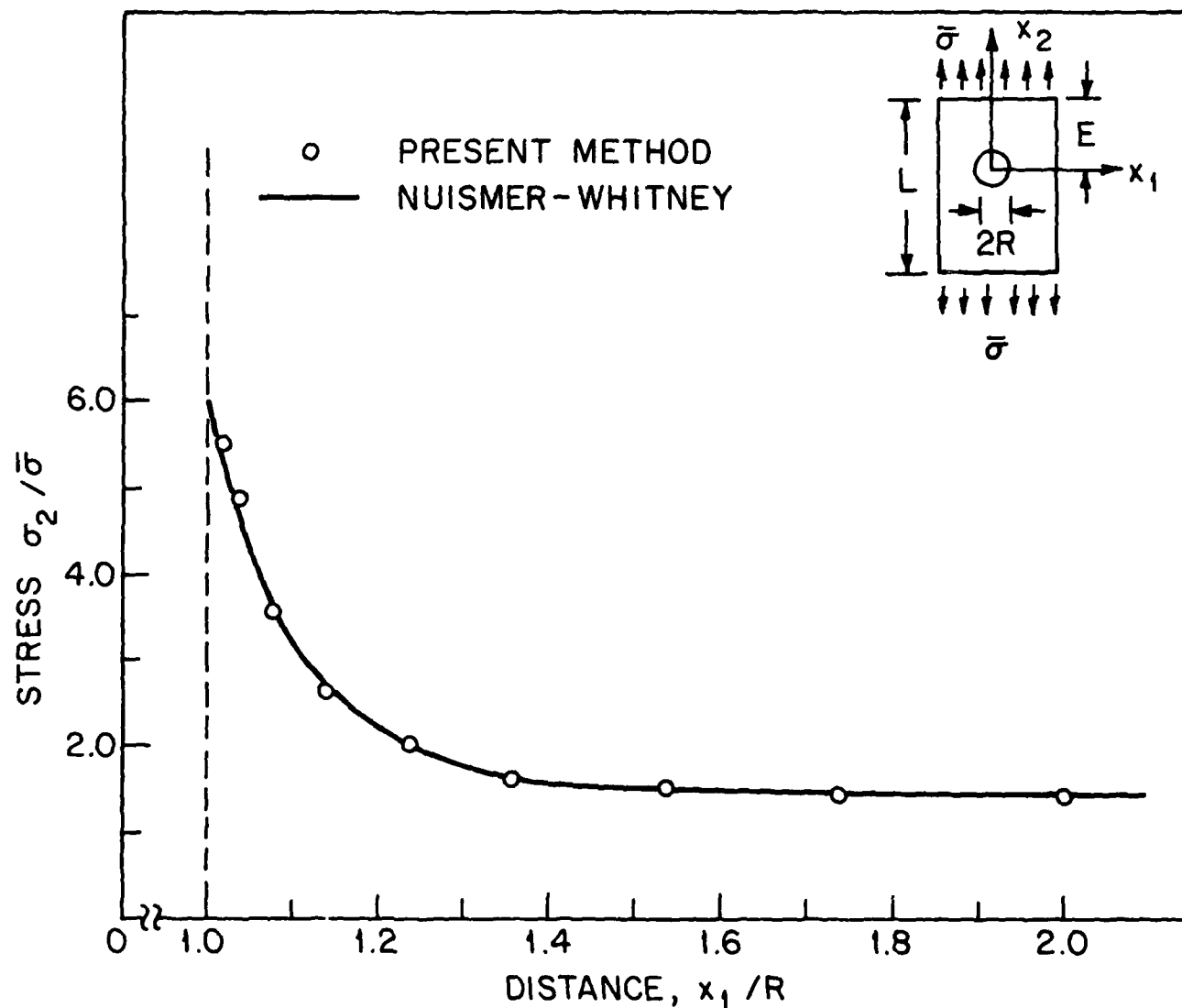


Figure 10. The stress σ_2 along the x_1 -axis in an orthotropic finite plate $[0/90]_s$ containing a circular hole. Comparison of the present results with the theoretical results obtained by Nuismer and Whitney [18]. Parameters used in the numerical calculations: Material: Graphite/Epoxy T300/5208, $E_1=149.8$ GPa, $E_2=11.2$ GPa, $G_{12}=5.39$ GPa, $\nu_{12}=0.29$, $\bar{\sigma}=2.3$ MPa, $D=24.5$ mm, $W/D=3.0$, $E/D=4.0$, $L/D=14.0$

2) Failure Strength and Failure Mode

Failure strengths of mechanically fastened composite joints calculated by the present method were compared to available data and to failure strengths predicted by other numerical methods.

Failure strengths of joints in graphite-epoxy laminates were measured by Van Siclen [21] and by Garbo and Oganowski [3]. The test conditions are summarized in Table 3. The failure strengths of the joints under these conditions were also calculated by the present method. The material properties used in the calculations are summarized in Table 4. Comparisons between the experimental and predicted failure strengths are given in Table 5. In these tables comparisons between the data and the failure strengths predicted by Agarwal [2] and by Garbo and Oganowski [3] are also included.

As can be seen, of the three analytical methods included in Table 5 the present one predicts the failure most accurately. In most cases, the failure load given by the present method agrees with the data within about 10%. On the other hand, the failure loads given by the Agarwal and by the Garbo and Oganowski methods are in error by as much as 50 percent.

One point is of interest here. When there is a high fraction of zero degree plies in the laminate (parallel to the body direction) the laminate fails in the shearout mode. In this case the calculated results are sensitive to the value of the laminate shear strength S_C . For example, when the S_C value obtained with cross ply laminates is used to calculate the failure load of $[0/\pm 45/90]_S$ laminates containing 70 percent of 0° plies the calculated and

Table 3. Summary of Test Conditions

Investigator	Material	Case No.	D(mm)	W/D	E/D	L/D	H(mm)	Lay up	Volume Fraction	
									80°	80° 845° 890°
Van Siclen [21]	T300/sp286	1	4.76	8.025	2.983	14.68	1.067	[0/±45/90] _s	25	50 25
		2	4.76	8.025	2.083	14.68	1.067	[0/±45/90] _s	25	50 25
		3	4.76	5.336	2.083	14.68	2.235	[0/±45/90] _s	25	50 25
		4	4.76	5.336	2.983	14.68	1.397	[0 ₂ /±45/90] _s	40	40 20
		5	4.76	5.336	2.983	14.68	1.067	[0/±45/90] _s	25	50 25
		6	4.76	8.025	4.013	14.68	1.067	[0/±45/90] _s	25	50 25
		7	4.76	5.336	2.983	14.68	1.118	[±45] _{2s}	0	100 0
		8	4.76	5.336	2.983	14.68	1.118	[0/90] _{2s}	50	0 50
		9	4.76	5.336	2.983	14.68	1.067	[0 ₂ /±45] _s	50	50 0
Garbo and Ogonowski [3]	AS/3501-6	10	6.35	6	3	14.68	5.283	[0/±45/90] _s	30	60 10
		11	6.35	6	3	14.68	5.283	[0/±45/90] _s	50	40 10
		12	6.35	6	3	14.68	5.283	[0/±45/90] _s	70	20 10

Table 4. Material Properties Used in the Calculations

<u>Ply Properties</u>	<u>T300/SP286</u>	<u>AS/3501-6</u>
Longitudinal Modulus E_1 Gpa(10 ksi)	130 (18.7)[21]	130(18.85)[3]
Transverse Modulus E_2 Gpa(10 ksi)	8.274(1.2)[21]	13.1(1.9)[3]
Shear Modulus G_{12} Gpa(10 ksi)	5.033(0.73)[21]	5.86(0.85)[3]
Poisson Ratio ν_{12}	0.30[21]	0.30[3]
Tensile Strength X Gpa(10 ksi)	1.23(0.178)[21]	1.58(0.23)[3]
Shear Strength S Gpa(10 ksi)	0.05(0.0073)[21]	0.12(0.017)[3]
<u>Laminate Properties</u>	<u>T300/SP286</u>	<u>AS/3501-6</u>
Cross-ply Laminate Shear Strength S_C Gpa(10 ksi)	0.125 (a)(0.018)	0.204 ^(b) (0.03) 0.12 ^(c) (0.017)
Characteristic Length R_{ot} mm(in)	1.092 (d)(0.043)	0.584 (0.023)[3]
Characteristic Length R_{oc} mm(in)	3.048 (e)(0.12)	1.727 ^(e) (0.068)

(a) Tests with Glass/Epoxy showed S_C to be about 2.5 times higher than the ply shear strength S [4]. The 2.5 multiplier was used to obtain S_C of Graphite/Epoxy T300/SP286 from the ply shear strength S given in [21].

(b) The value of S_C for AS/3501-6 was taken to be 1.7 times the ply shear strength S.

(c) This value was used for laminates containing more than 70% (by volume) of 0 degree plies.

(d) For T300/SP286 the value was chosen from [18] for T300/5208.

(e) The values of R_{oc} for T300/SP286 and AS/3501-6 were evaluated by the method given in [18] together with the data in [24,25].

Table 5. Comparisons Between the Experimental (P) and Calculated (P_c) Failure Loads. Case Numbers Correspond to Test Conditions Given in Table 3.

<u>Material</u>	<u>Case No.</u>	<u>Lay Up</u>	<u>Percent Difference $(1-P/P_c) \times 100$</u>	
			<u>Present Results</u>	<u>Agarwal(1980)</u>
T300/SP286	1	$[0/\pm 45/90]_s$	3.7	8.7
	2	$[0/\pm 45/90]_s$	0.01	0.01
	3	$[0/\pm 45/90]_{2s}$	1.81	7.78
	4	$[0_2/\pm 45/90]_s$	0.01	0.01
	5	$[0/\pm 45/90]_s$	8.66	1.16
	6	$[0/\pm 45/90]_s$	11.18	1.2
	7	$[\pm 45]_{2s}$	11.1	49.43
	8	$[0/90]_{2s}$	12.3	49.85
	9	$[0_2/\pm 45]_s$	7.64	20.0
			<u>Present Results</u>	<u>Garbo and Ogonowski(1981)</u>
AS/3501-6	10	$[0/\pm 45/90]_s$	0.5	45
	11	$[0/\pm 45/90]_s$	3.8	27
	12	$[0/\pm 45/90]_s$	15.0	14

measured values differ by 35 percent. On the other hand, if the shear stress of the individual ply is used in the calculations the difference between the calculated failure load and the data is only 15 percent. This indicates that the value of S_c must be selected carefully.

Failure loads calculated by Waszczak and Cruse [1] are compared to data in Table 6. Waszczak and Cruse's method also yields failure loads which are in error by as much as 50 percent. The present method was not applied to Waszczak and Cruse's data because the material properties needed for the calculation were unavailable.

It is interesting to note that the accuracies of all four methods (present, Agarwal, Garbo and Ogonowski, and Waszczak and Cruse) depend on the arrangements of the plies in the laminate. In general, the analytical predictions are most accurate for quasi-isotropic laminates, and are least accurate for angle ply and cross ply laminates. However, even for angle ply and cross ply laminates the present method yields results within about 10 percent accuracy, in contrast with the results of other existing methods of solutions, which may be in error by as much as 50 percent.

The failure modes predicted by the present method were also compared to failure modes observed experimentally. These comparisons, given in Table 7, show that the present method predicts well the mode of failure.

The aforementioned comparisons between the results of the present method and the data show that the method predicts with good accuracy both the load at which the joint fails and the mode of failure. The good agreements between the predictions of the model and the data

Table 6. Comparisons Between the Experimental Failure Loads and the Values Predicted by Waszczak and Cruse [1]

Material	Lay Up	Volume Fraction %			E/D	Percent Difference (1-[P/P _C)]x100
		45	0	90		
Graphite/Epoxy	[±45]	100	0	0	4	3
Boron/Epoxy	[(±45/0) _s /90] _s	72.3	18.2	9.1	4	2
	[±45 ₅ /90 ₆]	62.5	0	37.5	4	3
	[±45/(0 ₆ /90) _s] _s	13.3	80.0	6.7	6	6
	[0 ₆ /±45 ₅]	62.5	37.5	0	7.5	2.5
	[0 ₆ /±45]	62.5	37.5	0	7.35	4
						35

Table 7. Comparisons of Predicted Failure Modes with those Observed Experimentally
T-Tension Mode S-Shearout Mode B-Bearing Mode

Material	Case No.	Lay Up	Observed Failure Mode	Predicted Failure Mode	
				Present	Agarwal(1980)
T300/SP286	1	[0/±45/90] _s	S/R	S	S
	2	[0/±45/90] _s	S	S	S
	3	[0/±45/90] _{2s}	T/S	T	T
	4	[0 ₂ /±45/90] _s	S	S	S
	5	[0/±45/90] _s	T	T	T
	6	[0/ 45/90] _s	B	B	B
	8	[0/90] _{2s}	S	S	S
	9	[0 ₂ /±45] _s	B/S	S	S
AS/3501-6	10	[0/±45/90] _s	B/S	B	-
	11	[0/±45/90] _s	B/S	S	-
	12	[0/±45/90] _s	B/S	S	-
				Garbo & Ogonowski (1981)	
				Present	

(illustrated in Tables 5-7) create further confidence in the model.

3) Effects of Geometry and Ply Orientations

Parametric studies were performed to evaluate the effects of joint geometry and ply orientation on the failure strength and on the failure mode.

The effects of joint width on joint failure is illustrated in Figure 11. In this, and in subsequent figures, the failure load is normalized with respect to the ultimate tensile load of the laminate (without the hole) in the direction of the applied load. As is shown by the results in Figure 11, in general, the maximum load the joint can carry decreases as the hole size decreases, when the width to hole diameter ratio is greater than about 3. As the hole diameter approaches the width ($W/D \rightarrow 1$) the strength reduces to zero ($P \rightarrow 0$). Here failure loads were not calculated for W/D less than three because at such low W/D ratios the assumption of the cosine load distribution (eq. 17) is inaccurate [8].

The effect of edge distance E (Figure 1) on the failure load is shown in Figure 12. For the lay-ups $[0/\pm 45/90]_s$ and $[0/90]_{2s}$ increasing edge distance results in higher failure loads, as long as E/D is less than about 4. For higher edge ratios ($E/D > 4$) an increase in edge distance does not seem to influence significantly the failure load. For the lay-up $[0_2/\pm 45]_s$, the failure load does not vary significantly with E/D .

The effects of ply orientation on the failure load are given in Figure 13. This figure illustrates the effects of two parameters 1) the maximum ply angle ϕ in the laminate and 2) the change in

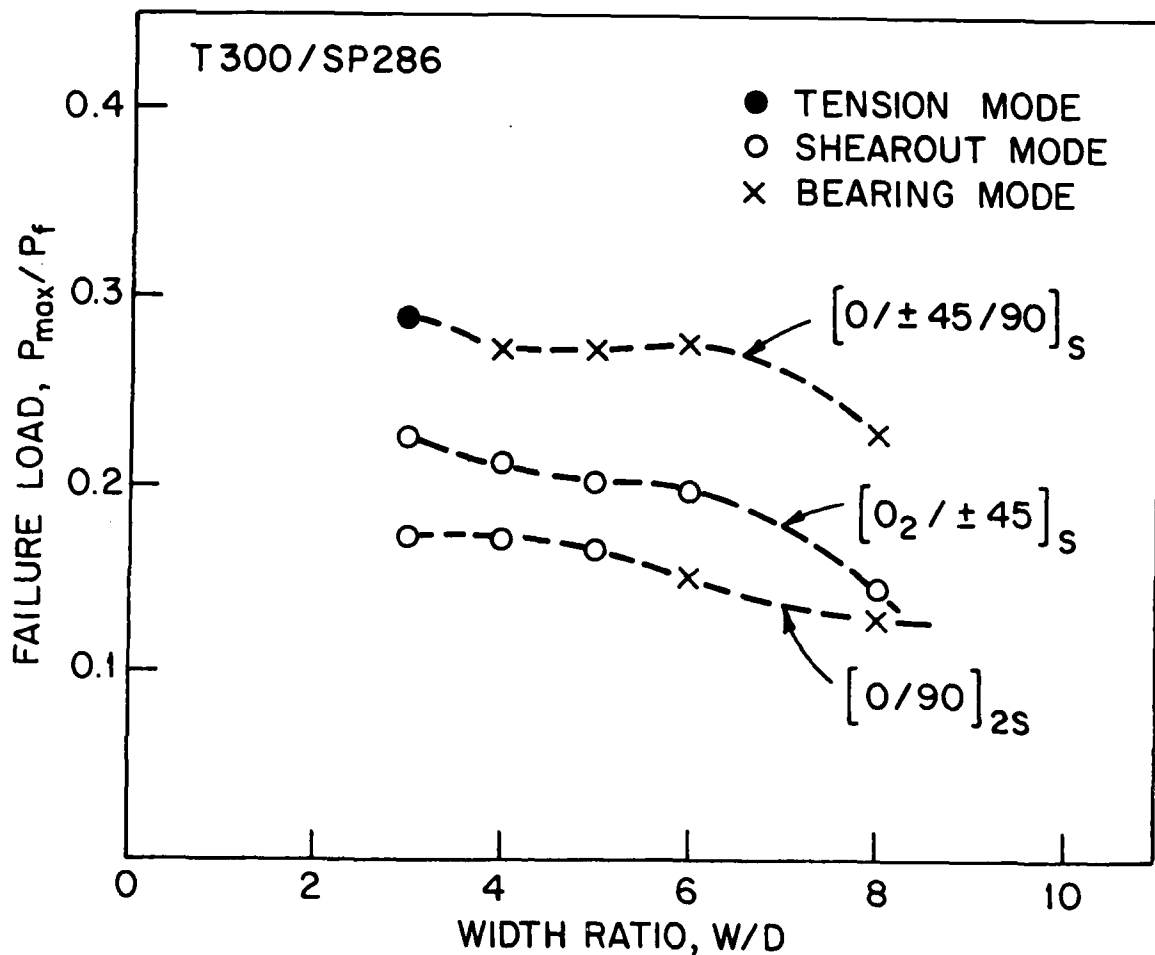


Figure 11. The effects of width ratio on the failure load of laminates with different ply orientations. P_f is the tensile failure load of laminates without holes. Parameters used in the numerical calculations: Material: Graphite/Epoxy T300/SP286, $W=38$ mm, $E=50.8$ mm, $L=203.2$ mm, $H=1.067$ mm for $[0/\pm 45/90]_s$ and $[0_2/\pm 45]_s$, and $H=1.18$ mm for $[0/90]_{2s}$

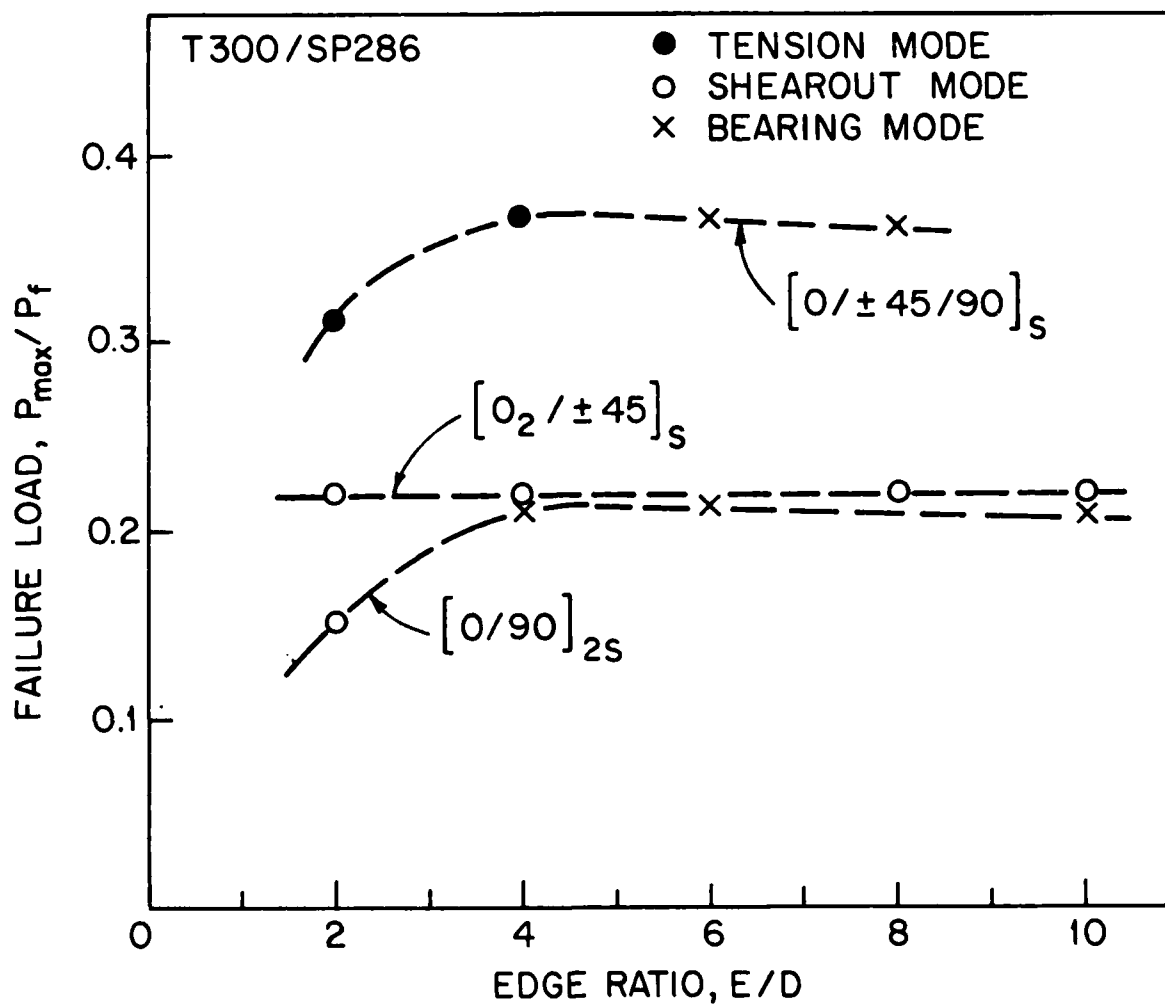


Figure 12. The effects of edge ratio on the failure load of laminates with different ply orientations.

Parameters used in the numerical calculations:

Material: Graphite/Epoxy T300/SP286, $D=5.08$ mm,
 $W/D=5$, $L/D=14$

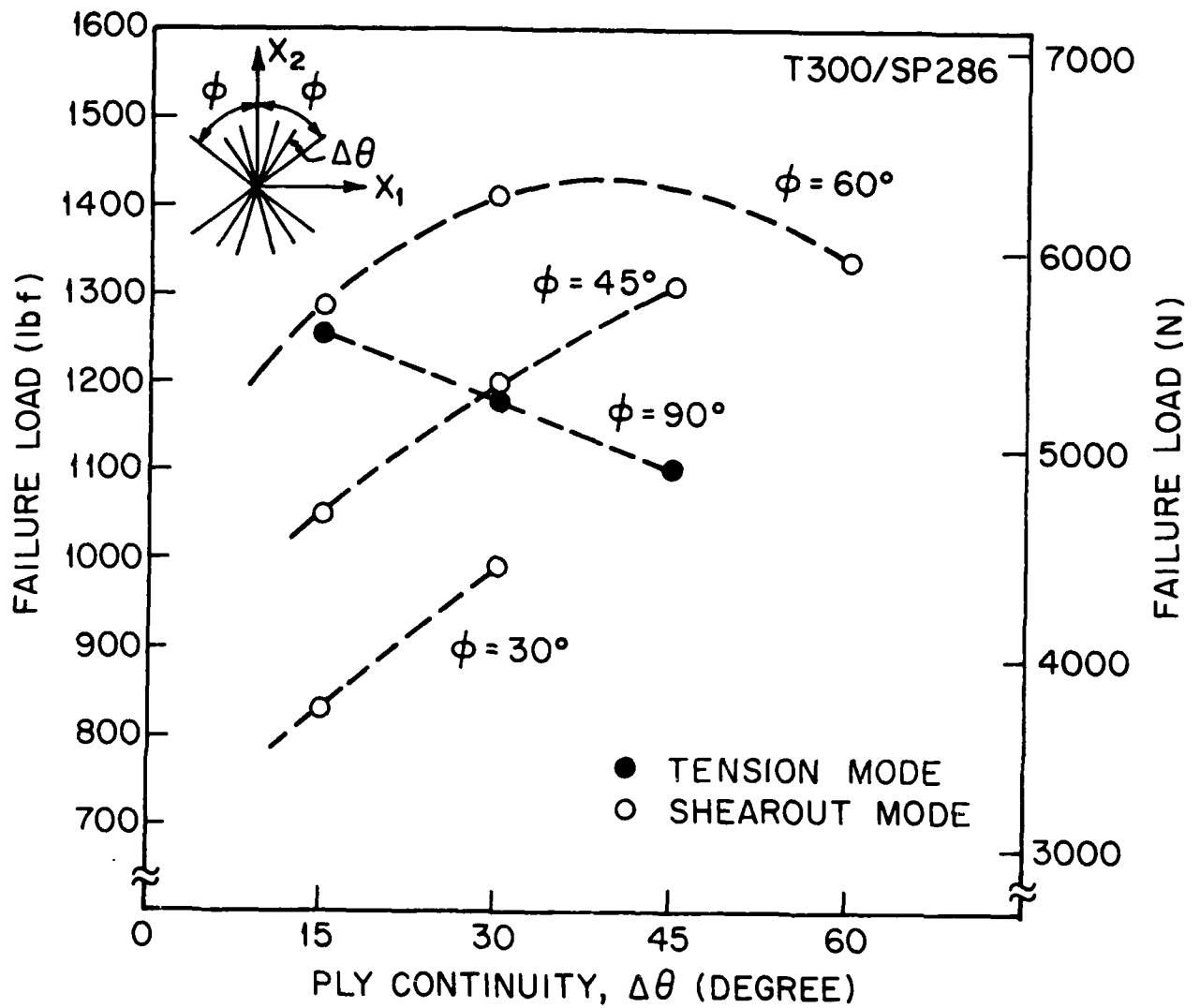


Figure 13. The effects of maximum ply angle ϕ and ply continuity $\Delta\theta$ on the failure load of mechanically fastened joints. Parameters used in the numerical calculations: Material: Graphite/Epoxy T300/SP286, $D=4.76$ mm, $W/D=5.336$, $E/D=2.983$, $L/D=14.68$, $H=1.397$ mm

orientation between two adjacent plies $\Delta\theta$. The latter parameter is referred to here as "ply continuity". The results in Figure 13 show that the failure load increases both with increasing ϕ and with increasing $\Delta\theta$, as long as failure is by shearout mode. On the other hand, the failure load decreases with increasing ϕ and with increasing $\Delta\theta$ when the failure is by tension mode. These results indicate that care must be exercised in designing bolted joints. If there are no other design constraints, the range of ply orientation ϕ and the ply continuity $\Delta\theta$ should be determined with the use of the computer code such that the joint can withstand the highest load.

SECTION VIII

CONCLUDING REMARKS

The model and computer code developed in this investigation can be used in the design of mechanically fastened joints involving fiber reinforced laminates. The computer code can be used to determine

- a) the optimum geometry of a joint for a given load,
- b) whether or not the joint will fail under a given load,
- c) the failure load,
- d) the mode of failure, and
- e) the ply in which failure first occurs.

The good accuracy of the method suggests that it might be worth it to extend the method to joints consisting of two or more fasteners.

The results of parametric studies performed with the present computer code show that the material properties, joint geometry, and ply orientation, all effect significantly the strength of mechanically fastened joints.

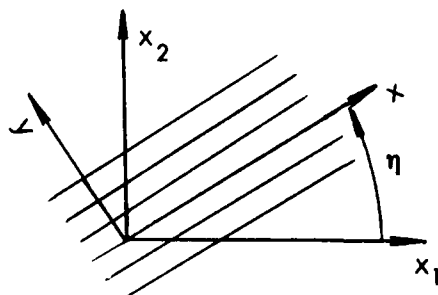
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Appendix A - The Transformed Reduced Stiffness Matrix Q_{ij}



The components of the matrix $\bar{Q}_{ij}^P$ appearing in Eq. (16) are

$$\begin{aligned}\bar{Q}_{11}^P &= Q_{11}^P \cos^4 \eta + 2(Q_{12}^P + 2Q_{66}^P) \sin^2 \eta \cos^2 \eta + Q_{22}^P \sin^4 \eta \\ \bar{Q}_{12}^P &= (Q_{11}^P + Q_{22}^P - 4Q_{66}^P) \sin^2 \eta \cos^2 \eta + Q_{12}^P (\sin^4 \eta + \cos^4 \eta) \\ \bar{Q}_{22}^P &= Q_{11}^P \sin^4 \eta + 2(Q_{12}^P + 2Q_{66}^P) \sin^2 \eta \cos^2 \eta + Q_{22}^P \cos^4 \eta \\ \bar{Q}_{13}^P &= (Q_{11}^P - Q_{12}^P - 2Q_{33}^P) \sin \eta \cos^3 \eta + (Q_{12}^P - Q_{22}^P + 2Q_{33}^P) \sin^3 \eta \cos \eta \\ \bar{Q}_{23}^P &= (Q_{11}^P - Q_{12}^P - 2Q_{33}^P) \sin^3 \eta \cos \eta + (Q_{12}^P - Q_{22}^P + 2Q_{33}^P) \sin \eta \cos^3 \eta \\ \bar{Q}_{33}^P &= (Q_{11}^P + Q_{22}^P - 2Q_{12}^P - 2Q_{33}^P) \sin^2 \eta \cos^2 \eta + Q_{33}^P (\sin^4 \eta + \cos^4 \eta)\end{aligned}$$

in which

$$Q_{11}^P = \frac{E_1^P}{1 - \nu_{12}^P \nu_{21}^P}$$

$$Q_{12}^P = \frac{\nu_{12}^P E_2^P}{1 - \nu_{12}^P \nu_{21}^P} = \frac{\nu_{21}^P E_1^P}{1 - \nu_{12}^P \nu_{21}^P}$$

$$Q_{22}^P = \frac{E_2^P}{1 - \nu_{12}^P \nu_{21}^P}$$

$$Q_{33}^P = G_{12}^P$$

The superscript p denotes the material properties of the p -th ply and the angle η is measured from the x_1 -axis to the x -axis. E_1^p , E_2^p and G_{12}^p are the longitudinal, transverse and shear moduli of the p -th ply, respectively. ν_{12}^p and ν_{21}^p are Poisson's ratios of p -th ply and satisfy the relation

$$\frac{\nu_{12}^p}{E_1^p} = \frac{\nu_{21}^p}{E_2^p}$$

Appendix B - Shape Function Used in the Finite Element Code.

In the isoparametric element, the geometry and the displacement of the element are described in terms of the shape function N_α by a transformation from a Master Element in the r - s coordinate system to the element in the x_1 - x_2 coordinate system (Figure 14)

$$X_i = N_\alpha(r,s) \bar{x}_{i\alpha} \quad i = 1,2$$

$$u_i = N_\alpha(r,s) q_{i\alpha} \quad \alpha = 1,2,3 \text{ or } 4$$

$$N_\alpha(r,s) = 1/4(1+rr_\alpha)(1+ss_\alpha) \quad -1 \leq r,s \leq 1$$

where $x_{i\alpha}$ is the coordinate of node α in the i -direction and $q_{i\alpha}$ is the displacement of node α in the i -direction, and r_α and s_α are the coordinates of node α referred to the Master Element

Note the property

$$N_\alpha(r_\beta, s_\beta) = \begin{cases} 1, & \text{if } \alpha=\beta \\ 0, & \text{if } \alpha \neq \beta \end{cases}$$

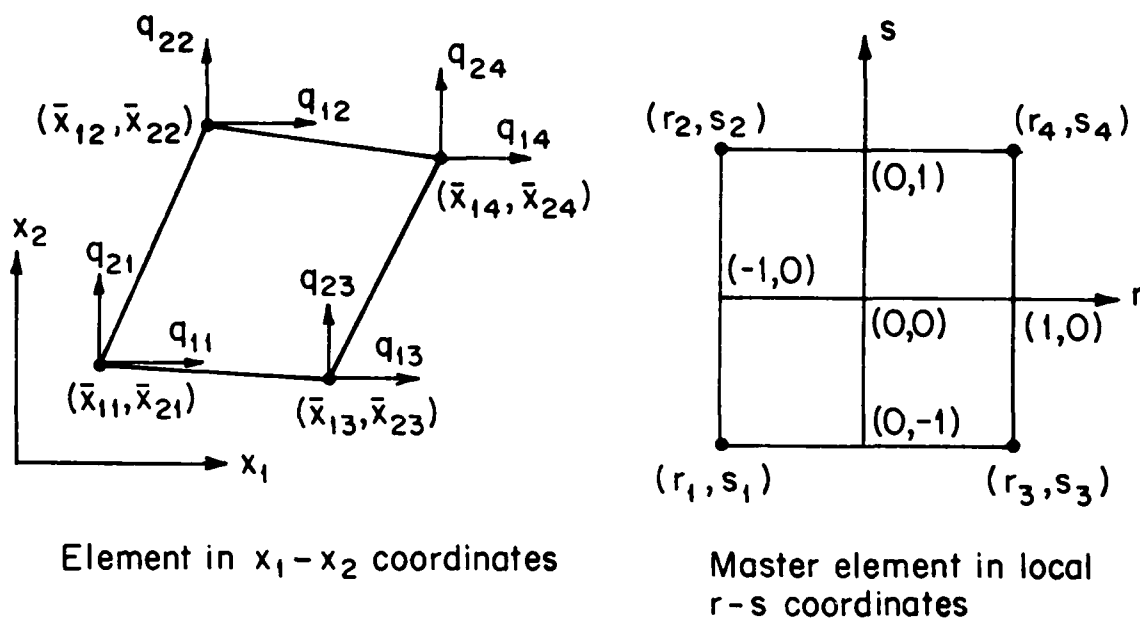


Figure 14. Geometry of an element used in the finite element calculations. Left: Element in the x_1 - x_2 coordinate system. Right: Element (master element) in the local $(r$ - $s)$ coordinate system. $x_{i\alpha}$ is the coordinate of node α in the i direction, $q_{i\alpha}$ is the displacement of node α in the i direction and (r_α, s_α) are the coordinates of node α in the r - s coordinate system, $i=1,2$, $\alpha=1,2,3$ or 4

Appendix C - Listing of the Computer Code "BOLT", and a Sample
of Input and Output.


```

C      DO 15 I=1,353
C      WRITE(6,400) I,(DIS(J,I),J=1,2)
C      400 FORMAT(/,5X,'POINT=',13,5X,'X-DIS',E15.8,5X,'Y-DIS',E15.8/)
C      15 CONTINUE
C      CALL STRESS(DIS)
C      CALL CFALL(KT,MT,FAL)
C      IF(FAL.GT.1.) GO TO 100
C      TN=DSQRT(1.0/FAL)
C      PF=TN*PF*HI*4.0
C      50 CALL OUTPUT(KT,MT,FAL)
C      GO TO 11
C      100 WRITE(6,111)
C      111 FORMAT(/,5X,'***** THE APPLIED LOAD IS TOO HIGH *****',/)
C      11 STOP
C      111 END

```

OPTIONS IN EFFECT ID,EBCDIC,SOURCE,NOLIST,NUDECK,LOAD,NOMAP

OPTIONS IN EFFECT NAME = MAIN , LINECNT = 57

STATISTICS SOURCE STATEMENTS = 32,PROGRAM SIZE = 798

STATISTICS NO DIAGNOSTICS GENERATED

0001 SUBROUTINE PGAUSS
0002 IMPLICIT REAL*8 (A-H,O-Z)
0003 COMMON /AC00R/ X(400),Y(400),RG(2),SG(2),WG(2)
0004 .SI(4),RI(4),P(20),Q(20)
0005 COMMON /ANOD/ NX,NP,NEQ,NBC(37),NFI(37),LINT,NBAND
0006 LINT=2
0007 RG(1)=0.577350269187
0008 RG(2)=-RG(1)
0009 SG(1)=RG(1)
0010 SG(2)=RG(2)
0011 WG(1)=1.
0012 WG(2)=1.
0013 RI(1)=-1.0
0014 SI(1)=-1.0
0015 RI(2)=-1.0
0016 SI(2)=1.0
0017 RI(3)=1.0
0018 SI(3)=-1.0
0019 RI(4)=1.0
0020 SI(4)=1.0
0021 RETURN
0022 END

OPTIONS IN EFFECT ID,EBDCIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
OPTIONS IN EFFECT NAME = PGAUSS , LINECNT = 57
STATISTICS SOURCE STATEMENTS = 21,PROGRAM SIZE = 424
STATISTICS NO DIAGNOSTICS GENERATED

0001	SUBROUTINE INPUT	
C	----- INPUT INSTRUCTION	94.000
C	1) INDICATOR	95.000
C	1.1) TEST : A INDICATOR TO SPECIFY THE PROBLEM	96.000
C	TEST=0 : OPEN HOLE (WITHOUT LOAD)	97.000
C	TEST=1 : LOADED HOLE (JOINT)	98.000
C	1.2) NRAN : A INDICATOR TO SPECIFY THE PLY ORIENTATION	99.000
C	NRAN=0 : MAXIMUM PLY ANGLE (ANG) AND PLY	100.000
C	CONTINUITY (DANG) AS INPUT PARAMETERS	101.000
C	NRAN=1 : INDIVIDUAL PLY ANGLE (ANT(I)) AS INPUT	102.000
C	PARAMETERS	103.000
C	2) MATERIAL PROPERTIES	104.000
C	2.1) E1 : PLY LONGITUDINAL TENSIL MODULOUS	105.000
C	2.2) V12 : POISSON'S RATIO FOR TRANSVERSE STRAIN IN	106.000
C	THE Y-DIRECTION WHEN STRESSED IN THE	107.000
C	X-DIRECTION	108.000
C	2.3) G12 : PLY SHEAR MODULUS	109.000
C	2.4) XX : PLY LONGITUDINAL TENSILE STRENGTH	110.000
C	2.5) SS : LAMINATE SHEAR STRENGTH OF CROSS-PLY	111.000
C	2.6) RT : CHARACTERISTIC LENGTH FOR TENSION	112.000
C	2.7) RC : CHARACTERISTIC LENGTH FOR COMPRESSION	113.000
C	3) PLY ORIENTATION	114.000
C	3.1) ANG : MAXIMUM PLY ANGLE (NRAN=0)	115.000
C	3.2) DANG : PLY CONTINUITY (NRAN=0)	116.000
C	3.3) ANT(I) : INDIVIDUAL PLY ANGLE (NRAN=1)	117.000
C	4) GEOMETRY	118.000
C	4.1) D : DIAMETER	119.000
C		120.000
C		121.000
C		122.000
C		123.000
C		124.000
C		125.000
C		126.000
C		127.000
C		128.000
C		129.000
C		130.000
C		131.000
C		132.000
C		133.000
C		134.000
C		135.000
C		136.000
C		137.000
C		138.000
C		139.000
C		140.000
C		141.000
C		142.000
C		143.000
C		144.000
C		145.000
C		146.000
C		147.000


```

0023 NX=353 200.000
0024 NELX=306 201.000
0025 NEQ=706 202.000
0026 NBAND=82 203.000
0027 E2=100 204.000
0028 PF=500 205.000
      C 206.000
      C 207.000
      C 208.000
0029 W=WD*D 209.000
0030 E=ED*D 210.000
0031 AL=ALD*D 211.000
0032 TP=4.0*PF*HI 212.000
      C 213.000
      C 214.000
      C 215.000
      C 216.000
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      C 250.000
      C 251.000
      C 252.000
      C 253.000
      C 254.000

```

16:14:21

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INPUT

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)

```

0069      40 NBC(27)=22
0070      NBC(28)=33
0071      NBC(29)=44
0072      NBC(30)=55
0073      NBC(31)=66
0074      NBC(32)=77
0075      NBC(33)=88
0076      NBC(34)=99
0077      NBC(35)=103
0078      NBC(36)=107
0079      NBC(37)=111
          C
          C
0080      50 NFIX(1)=11
0081      IF(ITEST.EQ.0) NFIX(1)=10
0082      NFIX(2)=10
0083      NFIX(3)=10
0084      NFIX(4)=10
0085      NFIX(5)=10
0086      NFIX(6)=10
0087      NFIX(7)=10
0088      NFIX(8)=10
0089      NFIX(9)=10
0090      NFIX(10)=10
0091      NFIX(11)=10
0092      NFIX(12)=10
0093      NFIX(13)=10
0094      NFIX(14)=10
0095      NFIX(15)=10
0096      NFIX(16)=10
          C
          C
          C
0097      IF(ITEST.EQ.1) NFIX(16)=11
0098      NFIX(17)=10
0099      NFIX(18)=10
0100      NFIX(19)=10
0101      NFIX(20)=10
0102      NFIX(21)=10
0103      NFIX(22)=10
0104      NFIX(23)=10
0105      NFIX(24)=10
0106      NFIX(25)=10
0107      NFIX(26)=11
          C
          C
          C
0108      IF(ITEST.EQ.1) NFIX(26)=10
0109      NFIX(27)=1
0110      NFIX(28)=1
0111      NFIX(29)=1
0112      NFIX(30)=1
0113      NFIX(31)=1
0114      NFIX(32)=1
0115      NFIX(33)=1
          C
          C
          C
0116      IF(ITEST.EQ.1) GO TO 55

```

INPUT

06-01-82

16:14:21

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)

```

0116 NFIX(34)=1
0117 NFIX(35)=1
0118 NFIX(36)=1
0119 NFIX(37)=1

C ***** INPUT NODAL UNIT FORCES *****
C
C
0120 55 FT=X(262)/(W/2.0)
0121 FT3=X(316)/(W/2.0)
0122 FT2=FT-FT3
0123 FT1=1-FT

C
0124 DO 100 I=1,706
0125 F(I)=0.0
0126 100 CONTINUE
0127 IF(ITEST.EQ. 1) GO TO 155

C
0128 F(586)=-FT1/6.0
0129 F(584)=-FT1/3.0
0130 F(582)=-FT1/3.0
0131 F(580)=-((FT1/6.0)-(FT2/4.0))
0132 F(616)=-FT2/2.0
0133 F(646)=-((FT2/4.0)-(FT3/4.0))
0134 F(676)=-FT3/2.0
0135 F(706)=-FT3/4.0
0136 IF(ITEST.EQ. 0) GO TO 150

C
0137 155 PI=3.141596535
0138 F(1)=2.0*(4.0/(D*PI))*0.0
0139 F(2)=0.06249988
0140 F(23)=0.012193140
0141 F(24)=0.12379906
0142 F(45)=0.023917705
0143 F(46)=0.12024245
0144 F(67)=0.034723127
0145 F(68)=0.11446683
0146 F(89)=0.044194158
0147 F(90)=0.10669416
0148 F(111)=0.051966834
0149 F(112)=0.097223130
0150 F(133)=0.057742454
0151 F(134)=0.086417711
0152 F(155)=0.061299064
0153 F(156)=0.074693147
0154 F(177)=0.062499988
0155 F(178)=0.062500007
0156 F(223)=0.061299072
0157 F(224)=0.050306867
0158 F(245)=0.057742469
0159 F(246)=0.038582300
0160 F(267)=0.051966855
0161 F(268)=0.02776877
0162 F(289)=0.044194186
0163 F(290)=0.018305843
0164 F(311)=0.034723159

```

0165 F(312)=0.010533165 365.000
 0166 F(333)=0.023917741 366.000
 0167 F(334)=0.0047575414 367.000
 0168 F(355)=0.012193179 368.000
 0169 F(356)=0.0012009269 369.000
 0170 F(377)=0.0020453255 370.000
 0171 F(378)=2.0*0.00 371.000

C 372.000
 C 373.000
 C 374.000
 C 375.000
 C 376.000
 C 377.000

0172 C 150 CONTINUE

0173 C RETURN
0174 C END

OPTIONS IN EFFECT ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
 OPTIONS IN EFFECT NAME = INPUT , LINECNT = 57
 STATISTICS SOURCE STATEMENTS = 174, PROGRAM SIZE = 2894
 STATISTICS NO DIAGNOSTICS GENERATED

```

0001 SUBROUTINE AMESH
0002 IMPLICIT REAL*8(A-H,O-Z)
0003 COMMON /ACONT/ ANG,DANG,NLY,ITEST,XX,SS,RT,RC,HI,ANT(9),THICK(9),N
1RAN
0004 COMMON /ADIM/ D,WD,ED,ALD,W,E,AL
0005 COMMON /ANOD/ NX,NP,NEQ,NBC(37),NFIX(37),LINT,NBAND
0006 COMMON /ALEM/ NODXY,NELX,IJK(4,400)
0007 COMMON /ACORR/ X(400),Y(400),RG(2),SG(2),WG(2)
1
0008 .SI(4),RI(4),P(20),Q(20)
0009 DIMENSION BX(33,10),BY(33,10)
0010 DIMENSION NES(30),NS(30),K12(30),K43(30),K4(30),MX(30),MY(30),KY(3
10),KSC(30)
0011 DIMENSION SH(8)
0012
0013 ***** READ INPUT DATA FOR THE GLOBAL MODEL*****
0014
0015 TH=0.09817477
0016 Z=W/3.6
0017 ZN=Z-(D/2.0)
0018
0019 BX(1,1)=0.0
0020 BY(1,1)=D/2.
0021 TH2=TH*2
0022 TH4=TH*4
0023 TH6=TH*6
0024 TH8=TH*8
0025 TH10=TH*10
0026 TH12=TH*12
0027 TH14=TH*14
0028 BX(1,5)=D/2.*DSIN(TH2)
0029 BY(1,5)=D/2.*DCOS(TH2)
0030 BX(1,2)=D/2.*DSIN(TH4)
0031 BY(1,2)=D/2.*DCOS(TH4)
0032 BX(1,6)=FTH(TH4)*DSIN(TH4)
0033 BY(1,6)=FTH(TH4)*DCOS(TH4)
0034 BX(1,3)=(D/2.+ZN)/DCOS(TH4)*DSIN(TH4)
0035 BY(1,3)=(D/2.+ZN)
0036 BX(1,7)=(D/2.+ZN)/DCOS(TH2)*DSIN(TH2)
0037 BY(1,7)=(D/2.+ZN)
0038 BX(1,4)=0.0
0039 BY(1,4)=(D/2.+ZN)
0040 BX(1,8)=0.0
0041 BY(1,8)=D/2.+ZN/2.0
0042
0043 BX(2,1)=0.0
0044 BY(2,1)=(D/2.)*ZN
0045 BX(2,2)=BX(1,3)
0046 BY(2,2)=BY(1,3)
0047 BX(2,3)=BX(2,2)
0048 BY(2,3)=E
0049 BX(2,4)=0.0

```

0044	C	BY(2.4)=E	431.000
	C		432.000
0045		BX(3.1)=BX(1.2)	433.000
0046		BY(3.1)=BY(1.2)	434.000
0047		BX(3.5)=D/2.*DSIN(TH6)	435.000
0048		BY(3.5)=D/2.*DCOS(TH6)	436.000
0049		BX(3.2)=D/2.*DSIN(TH8)	437.000
0050		BY(3.2)=D/2.*DCOS(TH8)	438.000
0051		BX(3.6)=FTH(TH8)*DSIN(TH8)	439.000
0052		BY(3.6)=FTH(TH8)*DCOS(TH8)	440.000
0053		BX(3.3)=((D/2.+ZN)/DCOS(TH8))*DSIN(TH8)	441.000
0054		BY(3.3)=(D/2.)+ZN	442.000
0055		BX(3.7)=(D/2.+ZN)/DCOS(TH6)*DSIN(TH6)	443.000
0056		BY(3.7)=(D/2.)+ZN	444.000
0057		BX(3.4)=BX(1.3)	445.000
0058		BY(3.4)=BY(1.3)	446.000
0059		BX(3.8)=BX(1.6)	447.000
0060		BY(3.8)=BY(1.6)	448.000
	C		449.000
	C		450.000
			451.000
0061		BX(4.1)=BX(1.3)	452.000
0062		BY(4.1)=BY(1.3)	453.000
0063		BX(4.2)=BX(3.3)	454.000
0064		BY(4.2)=BY(3.3)	455.000
0065		BX(4.3)=BX(4.2)	456.000
0066		BY(4.3)=E	457.000
0067		BX(4.4)=BX(2.3)	458.000
0068		BY(4.4)=BY(2.3)	459.000
	C		460.000
	C		461.000
0069		BX(5.1)=BX(3.3)	462.000
0070		BY(5.1)=BY(3.3)	463.000
0071		BX(5.2)=W/2.0	464.000
0072		BY(5.2)=BY(3.3)	465.000
0073		BX(5.3)=W/2.0	466.000
0074		BY(5.3)=BY(2.4)	467.000
0075		BX(5.4)=BX(4.3)	468.000
0076		BY(5.4)=BY(4.3)	469.000
	C		470.000
	C		471.000
0077		BX(6.1)=BX(3.2)	472.000
0078		BY(6.1)=BY(3.2)	473.000
0079		BX(6.5)=D/2.*DSIN(TH10)	474.000
0080		BY(6.5)=D/2.*DCOS(TH10)	475.000
0081		BX(6.2)=D/2.*DSIN(TH12)	476.000
0082		BY(6.2)=D/2.*DCOS(TH12)	477.000
0083		BX(6.6)=FTH(TH4)*DSIN(TH12)	478.000
0084		BY(6.6)=FTH(TH4)*DCOS(TH12)	479.000
0085		BX(6.3)=(D/2.)+ZN	480.000
0086		BY(6.3)=BX(6.3)/DCOS(TH4)*DSIN(TH4)	481.000
0087		BX(6.7)=(D/2.)+ZN	482.000
0088		BY(6.7)=BX(6.7)/DCOS(TH6)*DSIN(TH6)	483.000
0089		BX(6.4)=BX(3.3)	484.000
0090		BY(6.4)=BY(3.3)	485.000

16:14:34

06-01-82

AMESH

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)

```

0091      BX(6.8)=BX(3.6)
0092      BY(6.8)=BY(3.6)
C
C
0093      BX(7.1)=D/2.+ZN
0094      BY(7.1)=((BY(6.4)-BY(6.3))/4.)*3.+BY(6.3)
0095      BX(7.2)=BX(6.3)
0096      BY(7.2)=BY(6.3)
0097      BX(7.3)=W/2.0
0098      BY(7.3)=BY(6.3)
0099      BX(7.4)=W/2.0
0100      BY(7.4)=BY(7.1)
C
C
0101      BX(8.1)=BX(6.2)
0102      BY(8.1)=BY(6.2)
0103      BX(8.5)=D/2.*DSIN(TH14)
0104      BY(8.5)=D/2.*DCOS(TH14)
0105      BX(8.2)=D/2.
0106      BY(8.2)=0.0
0107      BX(8.6)=D/2.+ZN/2.0
0108      BY(8.6)=0.0
0109      BX(8.3)=(D/2.)+ZN
0110      BY(8.3)=0.0
0111      BX(8.7)=(D/2.)+ZN
0112      BY(8.7)=BX(8.7)/DCOS(TH2)*DSIN(TH2)
0113      BX(8.4)=BX(6.3)
0114      BY(8.4)=BY(6.3)
0115      BX(8.8)=BX(6.6)
0116      BY(8.8)=BY(6.6)
C
C
0117      BX(9.1)=BX(8.4)
0118      BY(9.1)=BY(8.4)
0119      BX(9.2)=BX(8.3)
0120      BY(9.2)=BY(8.3)
0121      BX(9.3)=W/2.0
0122      BY(9.3)=0.0
0123      BX(9.4)=W/2.0
0124      BY(9.4)=BY(8.4)
C
C
0125      BX(10.1)=D/2.
0126      BY(10.1)=0.0
0127      BX(10.5)=BX(8.5)
0128      BY(10.5)=-1*BY(8.5)
0129      BX(10.2)=BX(8.1)
0130      BY(10.2)=-BY(8.1)
0131      BX(10.6)=BX(8.8)
0132      BY(10.6)=-BY(8.8)
0133      BX(10.3)=BX(8.4)
0134      BY(10.3)=-BY(8.4)
0135      BX(10.7)=BX(8.7)
0136      BY(10.7)=-BY(8.7)
0137      BX(10.4)=BX(8.3)

```

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0138	BY(10,4)=-BY(8,3)	541.000
0139	BX(10,8)=BX(8,6)	542.000
0140	BY(10,8)=-BY(8,6)	543.000
	C	544.000
	C	545.000
0141	BX(11,1)=BX(9,2)	546.000
0142	BY(11,1)=-BY(9,2)	547.000
0143	BX(11,2)=BX(9,1)	548.000
0144	BY(11,2)=-BY(9,1)	549.000
0145	BX(11,3)=BX(9,4)	550.000
0146	BY(11,3)=-BY(9,4)	551.000
0147	BX(11,4)=BX(9,3)	552.000
0148	BY(11,4)=-BY(9,3)	553.000
	C	554.000
	C	555.000
0149	BX(12,1)=BX(6,2)	556.000
0150	BY(12,1)=-BY(6,2)	557.000
0151	BX(12,5)=BX(6,5)	558.000
0152	BY(12,5)=-BY(6,5)	559.000
0153	BX(12,2)=BX(6,1)	560.000
0154	BY(12,2)=-BY(6,1)	561.000
0155	BX(12,6)=BX(6,8)	562.000
0156	BY(12,6)=-BY(6,8)	563.000
0157	BX(12,3)=BX(6,4)	564.000
0158	BY(12,3)=-BY(6,4)	565.000
0159	BX(12,7)=BX(6,7)	566.000
0160	BY(12,7)=-BY(6,7)	567.000
0161	BX(12,4)=BX(6,3)	568.000
0162	BY(12,4)=-BY(6,3)	569.000
0163	BX(12,8)=BX(6,6)	570.000
0164	BY(12,8)=-BY(6,6)	571.000
	C	572.000
	C	573.000
0165	BX(13,1)=BX(7,2)	574.000
0166	BY(13,1)=-BY(7,2)	575.000
0167	BX(13,2)=BX(6,4)	576.000
0168	BY(13,2)=-BY(6,4)	577.000
0169	BX(13,3)=W/2.0	578.000
0170	BY(13,3)=- (D/2.+ZN)	579.000
0171	BX(13,4)=BX(7,3)	580.000
0172	BY(13,4)=-BY(7,3)	581.000
	C	582.000
	C	583.000
0173	BX(14,1)=BX(13,2)	584.000
0174	BY(14,1)=BY(13,2)	585.000
0175	BX(14,2)=BX(13,2)	586.000
0176	BY(14,2)=- (AL-E)	587.000
0177	BX(14,3)=W/2.0	588.000
0178	BY(14,3)=BY(14,2)	589.000
0179	BX(14,4)=W/2.0	590.000
0180	BY(14,4)=BY(13,3)	591.000
	C	592.000
	C	593.000
0181	BX(15,1)=BX(3,2)	594.000
0182	BY(15,1)=-BY(3,2)	595.000

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```

0183 BX(15.5)=BX(3.5)
0184 BY(15.5)=-BY(3.5)
0185 BX(15.2)=BX(3.1)
0186 BY(15.2)=-BY(3.1)
0187 BX(15.6)=BX(3.8)
0188 BY(15.6)=-BY(3.8)
0189 BX(15.3)=BX(3.4)
0190 BY(15.3)=-BY(3.4)
0191 BX(15.7)=BX(3.7)
0192 BY(15.7)=-BY(3.7)
0193 BX(15.4)=BX(3.3)
0194 BY(15.4)=-BY(3.3)
0195 BX(15.8)=BX(3.6)
0196 BY(15.8)=-BY(3.6)
C
C
0197 BX(16.1)=(BX(15.4)-BX(15.3))/2.+BX(15.3)
0198 BY(16.1)=- (D/2.+ZN)
0199 BX(16.2)=BX(15.3)
0200 BY(16.2)=- (D/2.+ZN)
0201 BX(16.3)=BX(15.3)
0202 BY(16.3)=- (AL-E)
0203 BX(16.4)=BX(16.1)
0204 BY(16.4)=- (AL-E)
C
C
0205 BX(17.1)=BX(1.2)
0206 BY(17.1)=-BY(1.2)
0207 BX(17.5)=BX(1.5)
0208 BY(17.5)=-BY(1.5)
0209 BX(17.2)=BX(1.1)
0210 BY(17.2)=-BY(1.1)
0211 BX(17.6)=BX(1.8)
0212 BY(17.6)=-BY(1.8)
0213 BX(17.3)=BX(1.4)
0214 BY(17.3)=-BY(1.4)
0215 BX(17.7)=BX(1.7)
0216 BY(17.7)=-BY(1.7)
0217 BX(17.4)=BX(1.3)
0218 BY(17.4)=-BY(1.3)
0219 BX(17.8)=BX(1.6)
0220 BY(17.8)=-BY(1.6)
C
C
0221 BX(18.1)=BX(17.4)
0222 BY(18.1)=BY(17.4)
0223 BX(18.2)=O.O
0224 BY(18.2)=BY(15.4)
0225 BX(18.3)=O.O
0226 BY(18.3)=- (AL-E)
0227 BX(18.4)=BX(17.4)
0228 BY(18.4)=- (AL-E)
C
0229 NBLOCK=18
0230 NES(1)=O

```

0231	NES(2)=28	651.000
0232	NES(3)=40	652.000
0233	NES(4)=68	653.000
0234	NES(5)=80	654.000
0235	NES(6)=89	655.000
0236	NES(7)=117	656.000
0237	NES(8)=126	657.000
0238	NES(9)=154	658.000
0239	NES(10)=166	659.000
0240	NES(11)=194	660.000
0241	NES(12)=206	661.000
0242	NES(13)=220	662.000
0243	NES(14)=226	663.000
0244	NES(15)=247	664.000
0245	NES(16)=261	665.000
0246	NES(17)=268	666.000
0247	NES(18)=282	667.000
		668.000
		669.000
		670.000
0248	NS(1)=0	671.000
0249	NS(2)=7	672.000
0250	NS(3)=44	673.000
0251	NS(4)=51	674.000
0252	NS(5)=95	675.000
0253	NS(6)=88	676.000
0254	NS(7)=118	677.000
0255	NS(8)=144	678.000
0256	NS(9)=151	679.000
0257	NS(10)=188	680.000
0258	NS(11)=195	681.000
0259	NS(12)=232	682.000
0260	NS(13)=239	683.000
0261	NS(14)=261	684.000
0262	NS(15)=254	685.000
0263	NS(16)=300	686.000
0264	NS(17)=308	687.000
0265	NS(18)=315	688.000
		689.000
		690.000
0266	D0 11 I=1, 18	691.000
0267	11 K12(I)=0	692.000
		693.000
0268	K12(2)=7	694.000
0269	K12(4)=7	695.000
0270	K12(7)=7	696.000
0271	K12(9)=7	697.000
0272	K12(11)=7	698.000
0273	K12(13)=7	699.000
0274	K12(16)=7	700.000
0275	K12(18)=7	701.000
		702.000
		703.000
0276	D0 12 I=1, 18	704.000
0277	12 K43(I)=0	705.000

```

0278      K43(1)=3      706.000
0279      K43(3)=3      707.000
0280      K43(6)=3      708.000
0281      K43(8)=3      709.000
0282      K43(10)=3     710.000
0283      K43(12)=3     711.000
0284      K43(15)=7     712.000
0285      K43(17)=7     713.000
          C             714.000
          C             715.000
0286      K2=0          716.000
          C             717.000
          C             718.000
0287      DO 13 I=1,18  719.000
0288      13 K4(I)=0    720.000
          C             721.000
0289      K4(6)=12      722.000
0290      K4(15)=24      723.000
          C             724.000
          C             725.000
0291      DO 14 I=1,18  726.000
0292      14 MX(I)=4     727.000
          C             728.000
0293      MX(5)=3       729.000
0294      MX(7)=3       730.000
0295      MX(12)=2       731.000
0296      MX(13)=2       732.000
0297      MX(14)=7       733.000
0298      MX(15)=2       734.000
0299      MX(16)=1       735.000
0300      MX(17)=2       736.000
0301      MX(18)=2       737.000
          C             738.000
          C             739.000
0302      DO 15 I=1,18  740.000
0303      15 MY(I)=7     741.000
          C             742.000
          C             743.000
0304      MY(2)=3       744.000
0305      MY(4)=3       745.000
0306      MY(5)=3       746.000
0307      MY(7)=3       747.000
0308      MY(9)=3       748.000
0309      MY(11)=3      749.000
0310      MY(13)=3      750.000
0311      MY(14)=3      751.000
          C             752.000
          C             753.000
0312      KX=1          754.000
          C             755.000
          C             756.000
0313      DO 16 I=1,18  757.000
0314      16 KY(I)=2     758.000
0315      KY(2)=1       759.000
0316      KY(4)=1       760.000

```



```

0360 Y(NIXY)=YI
0361 100 CONTINUE
      C
      C
      C
      C
      C
      ***** ELEMENT *****
      ***** 4-NODE ELEMENT *****
0362 DO 202 IX=1,MXX
0363 NSI=NS(I)+(IX-1)*(MYI+K12(I)+K43(I))
0364 DO 202 IY=1,MYI
0365 NN=NSI+IY
0366 NEL=NS(I)+(IX-1)*MY(I)+IY
0367 IJK(1,NEL)=NN+K4(I)
0368 IF(IX.EQ.1) IJK(1,NEL)=NN
0369 IJK(3,NEL)=NN+K4(I)+(MYI+K12(I)+K43(I))
0370 IJK(4,NEL)=IJK(3,NEL)+1
0371 IJK(2,NEL)=IJK(1,NEL)+1
0372 IF(IX.NE.MXX) GO TO 202
0373 IJK(2,NEL)=IJK(2,NEL)+K2
0374 IJK(3,NEL)=IJK(3,NEL)+K2
0375 202 CONTINUE
      C
0376 206 CONTINUE
      C
0377 300 CONTINUE
0378 IJK(1,297)=96
0379 IJK(2,297)=100
0380 IJK(3,297)=119
0381 IJK(4,297)=120
0382 IJK(1,298)=100
0383 IJK(2,298)=104
0384 IJK(3,298)=120
0385 IJK(4,298)=121
0386 IJK(1,299)=104
0387 IJK(2,299)=108
0388 IJK(3,299)=121
0389 IJK(4,299)=122
0390 IJK(1,300)=262
0391 IJK(2,300)=266
0392 IJK(3,300)=301
0393 IJK(4,300)=302
0394 IJK(1,301)=266
0395 IJK(2,301)=270
0396 IJK(3,301)=302
0397 IJK(4,301)=303
0398 IJK(1,302)=270
0399 IJK(2,302)=274
0400 IJK(3,302)=303
0401 IJK(4,302)=304
0402 IJK(1,303)=274
0403 IJK(2,303)=278
0404 IJK(3,303)=304
0405 IJK(4,303)=305
0406 IJK(1,304)=278
0407 IJK(2,304)=282
      815.000
      816.000
      817.000
      818.000
      819.000
      820.000
      821.000
      822.000
      823.000
      824.000
      825.000
      826.000
      827.000
      828.000
      829.000
      830.000
      831.000
      832.000
      833.000
      834.000
      835.000
      836.000
      837.000
      838.000
      839.000
      840.000
      841.000
      842.000
      843.000
      844.000
      845.000
      846.000
      847.000
      848.000
      849.000
      850.000
      851.000
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      856.000
      857.000
      858.000
      859.000
      860.000
      861.000
      862.000
      863.000
      864.000
      865.000
      866.000
      867.000
      868.000
      869.000

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```

0408      IJK(3,304)=305
0409      IJK(4,304)=306
0410      IJK(1,305)=282
0411      IJK(2,305)=286
0412      IJK(3,305)=306
0413      IJK(4,305)=307
0414      IJK(1,306)=286
0415      IJK(2,306)=290
0416      IJK(3,306)=307
0417      IJK(4,306)=308
           WRITE (6,900)
C          900 FORMAT(//5X,'<COORDINATE>',/)
C          WRITE(6,902) (I,X(I),Y(I),I=1,NX)
C          902 FORMAT(10X,15.2X,2F10.3,5X,15.2X,2F10.3)
C          WRITE(6,904)
C          904 FORMAT(//5X,'<ELEMENT>',/)
C          DO 906 NEL=1,NELX
C          WRITE(6,908) NEL, (IJK(I,NEL),I=1,NP)
C          908 FORMAT(10X,15.5X,9I5)
C          WRITE(6,909) (P(I),Q(I),LEB(I),I=1,18)
C          909 FORMAT(//5X,'P=' ,E15.6,5X,'Q=' ,E15.6,5X,'LEB=' ,I3/)
           RETURN
           END
0418
0419
*OPTIONS IN EFFECT*  ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
*OPTIONS IN EFFECT*  NAME = AMESH , LINECNT = 57
*STATISTICS*        SOURCE STATEMENTS = 419,PROGRAM SIZE = 13950
*STATISTICS*        NO DIAGNOSTICS GENERATED

```

```

870.000
871.000
872.000
873.000
874.000
875.000
876.000
877.000
878.000
879.000
880.000
881.000
882.000
883.000
884.000
885.000
886.000
887.000
888.000
889.000
890.000
891.000
892.000

```

```

0001      SUBROUTINE SEREND(SH,XL,YL,KKK)
          C
          C      IMPLICIT REAL*8(A-H,O-Z)
          C      DIMENSION SH(8)
          C
          C      K4=KKK/4
          C      GO TO (100,200),K4
          C      **** 4-NODE ELEMENT *****
          C      100 CONTINUE
          C      SH(1)=0.25*(1-XL)*(1.-YL)
          C      SH(2)=0.25*(1.+XL)*(1.-YL)
          C      SH(3)=0.25*(1.+XL)*(1.+YL)
          C      SH(4)=0.25*(1.-XL)*(1.+YL)
          C      RETURN
          C
          C      ****8-NODE ELEMENT ****
          C      200 CONTINUE
          C      SH(1)=-0.25*(XL+YL+1.)*(XL-1.)*(YL-1.)
          C      SH(2)=-0.25*(XL-YL-1.)*(XL+1.)*(YL-1.)
          C      SH(3)= 0.25*(XL+YL-1.)*(XL+1.)*(YL+1.)
          C      SH(4)= 0.25*(XL-YL+1.)*(XL-1.)*(YL+1.)
          C      SH(5)= 0.5*(XL*XL-1.)*(YL-1.)
          C      SH(6)=-0.5*(XL+1.)*(YL*YL-1.)
          C      SH(7)=-0.5*(XL*XL-1.)*(YL+1.)
          C      SH(8)= 0.5*(XL-1.)*(YL*YL-1.)
          C      RETURN
          C      END

```

OPTIONS IN EFFECT ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP

OPTIONS IN EFFECT NAME = SEREND , LINECNT = 57

STATISTICS SOURCE STATEMENTS = 22,PROGRAM SIZE = 868

STATISTICS NO DIAGNOSTICS GENERATED

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)

```

0001      FUNCTION FUN1(I,M,K)
          C
          C
0002      IMPLICIT REAL*8(A-H,O-Z)
0003      RI=I
0004      RM=M
0005      GO TO (100,102,104),K
0006      100 FUN1=-1.+2.*(RI-1.)/RM
0007      RETURN
0008      102 FUN1=-1.+2.*(RI-1.)*RI/(RM*(RM+1.))
0009      RETURN
0010      104 FUN1=-1.+2.*(RI-1.)*(2.*RM-RI+2.)/(RM*(RM+1.))
0011      RETURN
0012      END

```

```

*OPTIONS IN EFFECT* :D,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
*OPTIONS IN EFFECT* NAME = FUN1 , LINECNT = 57
*STATISTICS* SOURCE STATEMENTS = 12,PROGRAM SIZE = 634
*STATISTICS* NO DIAGNOSTICS GENERATED

```

935.000
936.000
937.000
938.000
939.000
940.000
941.000
942.000
943.000
944.000

FUNCTION FTH(TH)

C
C

IMPLICIT REAL*8(A-H,O-Z)
COMMON /ADIM/ D,WD,ED,ALD,W,E,AL
Z=W/3.6
ZN=Z-(D/2.0)
FTH=(((D/2.)+ZN)/DCOS(TH))-(D/2.)/2.+(D/2.)
RETURN
END

OPTIONS IN EFFECT ID,EBDCIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
OPTIONS IN EFFECT NAME = FTH , LINECNT = 57
STATISTICS SOURCE STATEMENTS = 8, PROGRAM SIZE = 442
STATISTICS NO DIAGNOSTICS GENERATED

```
0001 SUBROUTINE POINT(LEB)
0002 IMPLICIT REAL*8(A-H,O-Z)
0003 COMMON /ACONT/ ANG,DANG,NLY,ITEST,XX,SS,RT,RC,HI,ANT(9),THICK(9),N
      1RAN
0004 COMMON /ADIM/ D,WD,ED,ALD,W,E,AL
0005 COMMON /ALEM/ NDDXY,NELX,IJK(4,400)
0006 COMMON /ACDOR/ X(400),Y(400),RG(2),SG(2),WG(2)
      1
      .SI(4),RI(4),P(20),Q(20)
0007 DIMENSION LEB(20)
      C
      C
      RLC=RC*(D/2.O)
      RL=RT*(D/2.O)
      CA=O.09817477
      PI=3.141596535
      P(1)=(RL*(RC-RT)*DCOS(CA/6.O))*DSIN(CA/6.O)
      Q(1)=(RL*(RC-RT)*DCOS(CA/6.O))*DCOS(CA/6.O)
      P(2)=(RL*(RC-RT)*DCOS(CA/2.O))*DSIN(CA/2.O)
      Q(2)=(RL*(RC-RT)*DCOS(CA/2.O))*DCOS(CA/2.O)
      P(3)=(RL*(RC-RT)*DCOS(3*CA/2.O))*DSIN(3*CA/2.O)
      Q(3)=(RL*(RC-RT)*DCOS(3*CA/2.O))*DCOS(3*CA/2.O)
      P(4)=(RL*(RC-RT)*DCOS(5*CA/2.O))*DSIN(5*CA/2.O)
      Q(4)=(RL*(RC-RT)*DCOS(5*CA/2.O))*DCOS(5*CA/2.O)
      P(5)=(RL*(RC-RT)*DCOS(7*CA/2.O))*DSIN(7*CA/2.O)
      Q(5)=(RL*(RC-RT)*DCOS(7*CA/2.O))*DCOS(7*CA/2.O)
      P(6)=(RL*(RC-RT)*DCOS(9*CA/2.O))*DSIN(9*CA/2.O)
      Q(6)=(RL*(RC-RT)*DCOS(9*CA/2.O))*DCOS(9*CA/2.O)
      P(7)=(RL*(RC-RT)*DCOS(11*CA/2.O))*DSIN(11*CA/2.O)
      Q(7)=(RL*(RC-RT)*DCOS(11*CA/2.O))*DCOS(11*CA/2.O)
      P(8)=(RL*(RC-RT)*DCOS(13*CA/2.O))*DSIN(13*CA/2.O)
      Q(8)=(RL*(RC-RT)*DCOS(13*CA/2.O))*DCOS(13*CA/2.O)
      P(9)=(RL*(RC-RT)*DCOS(15*CA/2.O))*DSIN(15*CA/2.O)
      Q(9)=(RL*(RC-RT)*DCOS(15*CA/2.O))*DCOS(15*CA/2.O)
      P(10)=(RL*(RC-RT)*DCOS(17*CA/2.O))*DSIN(17*CA/2.O)
      Q(10)=(RL*(RC-RT)*DCOS(17*CA/2.O))*DCOS(17*CA/2.O)
      P(11)=(RL*(RC-RT)*DCOS(19*CA/2.O))*DSIN(19*CA/2.O)
      Q(11)=(RL*(RC-RT)*DCOS(19*CA/2.O))*DCOS(19*CA/2.O)
      P(12)=(RL*(RC-RT)*DCOS(21*CA/2.O))*DSIN(21*CA/2.O)
      Q(12)=(RL*(RC-RT)*DCOS(21*CA/2.O))*DCOS(21*CA/2.O)
      P(13)=(RL*(RC-RT)*DCOS(23*CA/2.O))*DSIN(23*CA/2.O)
      Q(13)=(RL*(RC-RT)*DCOS(23*CA/2.O))*DCOS(23*CA/2.O)
      P(14)=(RL*(RC-RT)*DCOS(25*CA/2.O))*DSIN(25*CA/2.O)
      Q(14)=(RL*(RC-RT)*DCOS(25*CA/2.O))*DCOS(25*CA/2.O)
      P(15)=(RL*(RC-RT)*DCOS(27*CA/2.O))*DSIN(27*CA/2.O)
      Q(15)=(RL*(RC-RT)*DCOS(27*CA/2.O))*DCOS(27*CA/2.O)
      P(16)=(RL*(RC-RT)*DCOS(29*CA/2.O))*DSIN(29*CA/2.O)
      Q(16)=(RL*(RC-RT)*DCOS(29*CA/2.O))*DCOS(29*CA/2.O)
      P(17)=(RL*(RC-RT)*DCOS(31*CA/2.O))*DSIN(31*CA/2.O)
      Q(17)=(RL*(RC-RT)*DCOS(31*CA/2.O))*DCOS(31*CA/2.O)
      P(18)=(RL*(RC-RT)*DSIN(CA/6.O))*DCOS(CA/6.O)
      Q(18)=(RL*(RC-RT)*DSIN(CA/6.O))*DSIN(CA/6.O)
      I=O
      5 I=I+1
      IF(I.GT.8) GO TO 110
      RLX=RLC-Y(I)
0048
0049
0050
0051
```

```

0052 IF(RLX .GT. 0.0) GO TO 5
0053 LEB(1)=I-1
0054 LEB(2)=I-1
0055 6 K=1
0056 LI=0
0057 50 K=K+1
0058 IF(K .GT. 16) GO TO 100
0059 KP=7
0060 IF(K .EQ. 5) KP=19
0061 IF(K .EQ. 9) KP=28
0062 IF(K .EQ. 13) KP=16

      C
0063 M=LEB(K)+KP+LI
0064 LI=0
0065 II=IJK(1,M)
0066 KK=IJK(3,M)
0067 K1=K+1
0068 40 IF(X(II) .EQ. X(KK)) TT=P(K1)-X(KK)
0069 IF(X(II) .EQ. X(KK)) GO TO 45
0070 TT=(-(P(K1)-X(II))*(Y(II)-Y(KK)))/(X(II)-X(KK))-Y(II)+Q(K1)
0071 45 IF(TT .GT. 0.0) GO TO 10
0072 LEB(K+1)=M-1
0073 II=IJK(1,M-1)
0074 KK=IJK(3,M-1)
0075 M=LEB(K+1)
0076 LI=LI+1
0077 GO TO 40
0078 10 LEB(K1)=M
0079 GO TO 50
0080 100 LEB(K+1)=M
0081 110 RETURN
0082 END

```

OPTIONS IN EFFECT ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP

OPTIONS IN EFFECT NAME = POINT , LINECNT = 57

STATISTICS SOURCE STATEMENTS = 82, PROGRAM SIZE = 5002

STATISTICS NO DIAGNOSTICS GENERATED

```

0001 SUBROUTINE MATRL
0002 IMPLICIT REAL*8(A-H,O-Z)
0003 COMMON /ACONT/ ANG,DANG,NLY, ITEST,XX,SS,RT,RC,HI,ANT(9),THICK(9),N
      1RAN
0004 COMMON /AMAT/ E1,E2,V12,G12,C(3,3),QA(40,3,3)
0005 COMMON /AFUN/ SHP(3,4),T(40,3,3),STRSS(40,18,3),STRA(18,3),ST(11,3
      1)
      C
      C
0006 IF(NRAN.EQ.O) NLY=2.O*ANG/DANG+1
0007 V21=E2*V12/E1
0008 DIV=1-V12*V21
0009 Q22=E2/DIV
0010 Q11=E1/DIV
0011 Q12=V12*E2/DIV
0012 Q66=G12
      C
      C
0013 *** COMPUTE INVARIANT PROPERTIES
0014 U1=(3.*Q11+3.*Q22+2.*Q12+4.*Q66)/8.
0015 U2=(Q11-Q22)/2.
0016 U3=(Q11+Q22-2.*Q12-4.*Q66)/8.O
0017 U4=(Q11+Q22+6.*Q12-4.*Q66)/8.O
      U5=(Q11+Q22-2.*Q12+4.*Q66)/8.O
      C
      C
0018 DO 100 I=1,NLY
0019 L=I-1
0020 THTA=90.O-(ANG-DANG*L)
0021 IF(NRAN.EQ.1) THTA=90.O-ANT(I)
0022 IF(THTA.EQ.180.O) THTA=0.O
0023 DEG=THTA*.1415926535/180.O
0024 QA(I,1)=U1+U2*DCOS(2.*DEG)+U3*DCOS(4.*DEG)
0025 QA(I,2)=U4-U3*DCOS(4.*DEG)
0026 QA(I,2)=U1-U2*DCOS(2.*DEG)+U3*DCOS(4.*DEG)
0027 QA(I,3)=+0.5*U2*DSIN(2.*DEG)+U3*DSIN(4.*DEG)
0028 QA(I,2,3)=+0.5*U2*DSIN(2.*DEG)-U3*DSIN(4.*DEG)
0029 QA(I,3)=U5-U3*DCOS(4.*DEG)
0030 QA(I,2,1)=QA(I,1,2)
0031 QA(I,3,1)=QA(I,1,3)
0032 QA(I,3,2)=QA(I,2,3)
      C
      C
0033 *** COMPUTE ROTATION TRANSFORMATION PER PLY *****
0034 T(I,1,1)=DCOS(DEG)**2
0035 T(I,1,2)=DSIN(DEG)**2
0036 T(I,1,3)=2*DSIN(DEG)*DCOS(DEG)
0037 T(I,2,1)=T(I,1,2)
0038 T(I,2,2)=T(I,1,1)
0039 T(I,2,3)=-T(I,1,3)
0040 T(I,3,1)=T(I,2,3)/2.O
0041 T(I,3,2)=T(I,1,3)/2.O
      T(I,3,3)=T(I,1,1)-T(I,1,2)
      C
      C

```

```

0042      100 CONTINUE
C
C      *** COMPUTE LAMINATE PROPERTIES *****
C
      DO 200 M=1,3
      DO 200 N=1,3
      C(M,N)=0.0
      DO 150 I=1,NLY
      AZ=THICK(I)
      IF(NRAN.EQ.0) AZ=1.0
      150 C(M,N)=C(M,N)+QA(I,M,N)*AZ
      IF(NRAN.EQ.0) GO TO 10
      C(M,N)=C(M,N)/HI
      GO TO 200
      10 C(M,N)=C(M,N)/NLY
      200 CONTINUE
      RETURN
      END

```

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*OPTIONS IN EFFECT* ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
*OPTIONS IN EFFECT* NAME = MATRL , LINECNT = 57
*STATISTICS* SOURCE STATEMENTS = 56,PROGRAM SIZE = 2044
*STATISTICS* NO DIAGNOSTICS GENERATED

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0001 SUBROUTINE STIFF(MM,A)
0002 IMPLICIT REAL*8(A-H,O-Z)
C
0003 COMMON /AMAT/ E1,E2,V12,G12,C(3,3),QA(40,3,3)
0004 COMMON /ANOD/ NX,NP,NEQ,NBC(37),NFIX(37),LINT,NBAND
0005 COMMON /ACORR/ X(400),Y(400),RG(2),SG(2),WG(2)
0006 COMMON /AFUN/ SHP(3,4),T(40,3,3),STRESS(40,18,3),STRA(18,3),ST(11,3)
C
0007 DIMENSION A(8,8)
0008 DO 10 I=1,8
0009 DO 10 J=1,8
0010 A(I,J)=0.0
0011 10 CONTINUE
C
0012 DO 100 N=1,LINT
0013 R=RG(N)
0014 DO 100 M=1,LINT
0015 S=SG(M)
0016 CALL SHAPEF(MM,XSJ,R,S)
0017 DV=XSJ*WG(N)*WG(M)
C
C *** FOR EACH J NODE COMPUTE CB=C*B ***
C
0018 DO 100 J=1,NP
0019 CB11=C(1,1)*SHP(1,J)*DV+C(1,3)*SHP(2,J)*DV
0020 CB12=C(1,2)*SHP(2,J)*DV+C(1,3)*SHP(1,J)*DV
0021 CB21=C(1,2)*SHP(1,J)*DV+C(2,3)*SHP(2,J)*DV
0022 CB22=C(2,2)*SHP(2,J)*DV+C(2,3)*SHP(1,J)*DV
0023 CB31=C(1,3)*SHP(1,J)*DV+C(3,3)*SHP(2,J)*DV
0024 CB32=C(2,3)*SHP(2,J)*DV+C(3,3)*SHP(1,J)*DV
C
C *** FOR EACH I NODE COMPUTE S=BT*CB ***
C
0025 DO 100 I=1,J
0026 I1=2*I-1
0027 J1=2*J-1
0028 I2=2*I
0029 J2=2*J
C
0030 A(I1,J1)=A(I1,J1)+SHP(1,I)*CB11+SHP(2,I)*CB31
0031 A(I1,J2)=A(I1,J2)+SHP(1,I)*CB12+SHP(2,I)*CB32
0032 A(I2,J1)=A(I2,J1)+SHP(2,I)*CB21+SHP(1,I)*CB31
0033 A(I2,J2)=A(I2,J2)+SHP(2,I)*CB22+SHP(1,I)*CB32
0034 100 CONTINUE
C
C *** COMPUTE LOWER TRIANGULAR PART BY SYMMETRY ***
C
NL=NP*2
0035 DO 200 I=1,NL
0036 DO 200 J=1,I
0037 200 A(I,J)=A(J,I)
C
0038 RETURN
0039

```

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STIFF

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)

1157.000

0040 END

OPTIONS IN EFFECT ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP

OPTIONS IN EFFECT NAME = STIFF . LINECNT = 57

STATISTICS SOURCE STATEMENTS = 40, PROGRAM SIZE = 1516

STATISTICS NO DIAGNOSTICS GENERATED

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0001      SUBROUTINE CFAIL (KT,MT,FAL)
0002      IMPLICIT REAL*8(A-H,O-Z)
0003      COMMON /ACONT/ ANG,DANG,NLY,ITEST,XX,SS,RT,RC,HI,ANT(9),THICK(9),N
1RAN
0004      COMMON /AFUN/ SHP(3,4),T(40,3,3),STRSS(40,18,3),STRA(18,3),ST(11,3
1)
C
C
C *** FAILURE CRITERIAN *****
C
C      FAL=0.0
C      DO 100 K=1,NLY
C      DO 100 M=1,18
C
C      FL=(STRSS(K,M,1)/XX)**2+(STRSS(K,M,3)/SS)**2
C      IF (FL .LT. FAL) GO TO 100
C      FAL=FL
C      KT=K
C      MT=M
C      IF (FAL .GT. 1.0) GO TO 200
C      100 CONTINUE
C      200 RETURN
0016      END

```

```

*OPTIONS IN EFFECT* ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
*OPTIONS IN EFFECT* NAME = CFAIL . LINECNT = 57
*STATISTICS* SOURCE STATEMENTS = 16,PROGRAM SIZE = 584
*STATISTICS* NO DIAGNOSTICS GENERATED

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0001 SUBROUTINE STRESS(DIS)
0002 IMPLICIT REAL*8(A-H,O-Z)
0003 C
0004 COMMON /ACONT/ ANG,DANG,NLY,ITEST,XX,SS,RT,RC,HI,ANT(9),THICK(9),N
0005 1RAN
0006 COMMON /AMAT/ E1,E2,V12,G12,C(3,3),QA(40,3,3)
0007 COMMON /ALEM/ NODXY,NELX,IJK(4,400)
0008 COMMON /ACORR/ X(400),Y(400),RG(2),SG(2),WG(2)
0009 1
0010 .SI(4),RI(4),P(20),Q(20)
0011 COMMON /AFUN/ SHP(3,4),T(40,3,3),STRSS(40,18,3),STRA(18,3),ST(11,3
0012 1)
0013 COMMON /ALOA/ XLC(20),YLC(20)
0014 C
0015 C
0016 C
0017 DIMENSION HP(3,4),LEB(20),XI(4),YI(4),DIS(2,400),STRAS(11,3)
0018 DO 10 I=1,18
0019 DO 10 J=1,3
0020 STRA(I,J)=0.0
0021 10 CONTINUE
0022 C
0023 DO 13 I=1,20
0024 XLC(I)=0.0
0025 YLC(I)=0.0
0026 13
0027 C
0028 CALL POINT(LEB)
0029 C
0030 DO 200 M=1,18
0031 II=LEB(M)
0032 I1=IJK(1,II)
0033 I2=IJK(2,II)
0034 I3=IJK(3,II)
0035 I4=IJK(4,II)
0036 XI(1)=X(I1)
0037 XI(2)=X(I2)
0038 XI(3)=X(I3)
0039 XI(4)=X(I4)
0040 C
0041 YI(1)=Y(I1)
0042 YI(2)=Y(I2)
0043 YI(3)=Y(I3)
0044 YI(4)=Y(I4)
0045 C
0046 AP=P(M)
0047 AQ=Q(M)
0048 C
0049 CALL NEWTON (II,AP,AQ,R,S)
0050 C
0051 CALL SHAPEF(II,XSJ,R,S)
0052 C
0053 DO 100 N=1,4
0054 NN=IJK(N,II)
0055 STRA(M,1)=STRA(M,1)+SHP(1,N)*DIS(1,NN)
0056 STRA(M,2)=STRA(M,2)+SHP(2,N)*DIS(2,NN)
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1000 C

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0081	STRAS(MT,2)=STRAS(MT,2)+SHP(2,NT)*DIS(2,NN)	1288.000
0082	STRAS(MT,3)=STRAS(MT,3)+SHP(1,NT)*DIS(2,NN)+SHP(2,NT)*DIS(1,NN)	1289.000
0083	C 500 CONTINUE	1290.000
0084	C	1291.000
0085	DO 520 J=1,3	1292.000
0086	ST(MT,J)=O.O	1293.000
0087	DO 550 J=1,3	1294.000
0088	ST(MT,1)=ST(MT,1)+C(1,J)*STRAS(MT,J)	1295.000
0089	ST(MT,2)=ST(MT,2)+C(2,J)*STRAS(MT,J)	1296.000
0090	ST(MT,3)=ST(MT,3)+C(3,J)*STRAS(MT,J)	1297.000
0091	C 550 CONTINUE	1298.000
0092	C	1299.000
0093	DO 400 CONTINUE	1300.000
0094	C	1301.000
0095	400 CONTINUE	1302.000
0096	C	1303.000
0097	C	1304.000
0098	MX=164	1305.000
0099	MY=166	1306.000
0100	MT=7	1307.000
0101	DO 401 IN=MX,MY	1308.000
0102	MT=MT+1	1309.000
0103	II1=IJK(1,IN)	1310.000
0104	II2=IJK(2,IN)	1311.000
0105	II3=IJK(3,IN)	1312.000
0106	II4=IJK(4,IN)	1313.000
0107	C	1314.000
0108	XI(1)=X(II1)	1315.000
0109	XI(2)=X(II2)	1316.000
0110	XI(3)=X(II3)	1317.000
0111	XI(4)=X(II4)	1318.000
0112	C	1319.000
0113	YI(1)=Y(II1)	1320.000
0114	YI(2)=Y(II2)	1321.000
0115	YI(3)=Y(II3)	1322.000
0116	YI(4)=Y(II4)	1323.000
0117	C	1324.000
0118	AP=RG(1)	1325.000
0119	AQ=SG(2)	1326.000
0120	CALL SHAPEF(IN,XS,AP,AQ)	1327.000
0121	C	1328.000
0122	DO 501 NT=1,4	1329.000
0123	NN=IJK(NT,IN)	1330.000
0124	C	1331.000
0125	XLC(MT)=XLC(MT)+SHP(3,NT)*X(NN)	1332.000
0126	YLC(MT)=YLC(MT)+SHP(3,NT)*Y(NN)	1333.000
0127	C	1334.000
0128	C	1335.000
0129	STRAS(MT,1)=STRAS(MT,1)+SHP(1,NT)*DIS(1,NN)	1336.000
0130	STRAS(MT,2)=STRAS(MT,2)+SHP(2,NT)*DIS(2,NN)	1337.000
0131	STRAS(MT,3)=STRAS(MT,3)+SHP(1,NT)*DIS(2,NN)+SHP(2,NT)*DIS(1,NN)	1338.000
0132	C	1339.000
0133	501 CONTINUE	1340.000
0134	C	1341.000
0135	C	1342.000

```

O120 C
O121 DO 521 J=1,3
O122 521 ST(MT,J)=0.0
O123 DO 551 J=1,3
O124 ST(MT,1)=ST(MT,1)+C(1,J)*STRAS(MT,J)
O125 ST(MT,2)=ST(MT,2)+C(2,J)*STRAS(MT,J)
O126 ST(MT,3)=ST(MT,3)+C(3,J)*STRAS(MT,J)
O127 C
O128 551 CONTINUE
O129 C
O130 C
O131 401 CONTINUE
O132 CC
O133 DO 666 I=1,10
O134 C WRITE(6,677) I,(STRAS(I,J),J=1,3)
O135 C 677 FORMAT(/,5X,'POINT=',15.5X,'STRA11=',E15.8,5X,'STRA22=',E15.8,
O136 C 15X,'STRA12=',E15.8,/)
O137 C 666 CONTINUE
O138 C
O139 300 RETURN
O140 END
O141 *OPTIONS IN EFFECT* ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
O142 *OPTIONS IN EFFECT* NAME = STRESS , LINECNT = 57
O143 *STATISTICS* SOURCE STATEMENTS = 129,PROGRAM SIZE = 4434
O144 *STATISTICS* NO DIAGNOSTICS GENERATED

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1343.000
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[illegible]

AD-A121 407

STRENGTH OF MECHANICALLY FASTENED COMPOSITE JOINTS(0)
MICHIGAN UNIV ANN ARBOR DEPT OF MECHANICAL ENGINEERING
AND APPLIED MECHANICS F CHANG ET AL. JUL 82
AFWAL-TR-82-4095 F33615-81-C-5050

272

UNCLASSIFIED

F/G 13/5

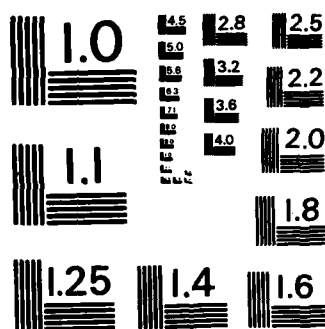
NL

END

FORMED

1

OTIC



MICROCOPY RESOLUTION TEST CHART
NATIONAL BUREAU OF STANDARDS-1963-A

C

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0037      901 FORMAT (//,20X,'MATERIAL PROPERTIES OF SINGLE PLY')
0038      311 FORMAT (/,20X,'E1=',F15.3,X,'V12=',F10.3,
0039      15X,'G12=',F15.3/)
0039      213 FORMAT (/,20X,'** THE MAXIMUM ORIENTATION ANGLE=',
0040      11X,F6.2,X,'** THE INCREMENTAL ORIENTATION ANGLE=',
0041      21X,F6.2/)
0040      400 FORMAT (//,20X,'DIAMETER=',F10.4,X,'W/D=',F10.4,/,20X,'E/D=',
0041      F10.4,X,'L/D=',F10.4,/,20X,'THICKNESS=',F10.5//)
0041      902 FORMAT (//,20X,'PLY ORIENTATION ',10X,'PLY THICKNESS')
0042      903 FORMAT (/,20X,'PLY',13,'=',F7.3,11X,F8.5,X)
0043      904 FORMAT (//,20X,'GEOMETRY')
0044      905 FORMAT (-----,///)

```

C

```

0045      WRITE(6,101) PF
0046      101 FORMAT(/,10X,'***** THE MAXIMUM LOAD=',F10.3,2X,' *****',1
0047      1X)
0047      IF(MT.LE. 5) WRITE(6,91)
0048      IF(MT.LE. 11 .AND. MT.GE. 8) WRITE(6,92)
0049      IF(MT.GE. 14 .AND. MT.LE. 18) WRITE(6,93)
0050      IF(MT.LE. 7 .AND. MT.GE. 6) WRITE(6,94)
0051      IF(MT.GE. 12 .AND. MT.LE. 13) WRITE(6,95)
0052      WRITE(6,102)
0053      102 FORMAT(/,3X,'FAILURE LAYER',3X,'FAILURE POINT',
0054      13X,'FAILURE POSITION',3X,'STRESS11',3X,'STRESS22',3X,'STRESS12',/)
0054      WRITE(6,103) KT,MT,P(MT),Q(MT),XS1,XS2,XS3
0055      103 FORMAT (/,7X,13,12X,13,7X,(' ',1X,F5.3,2X,F5.3,') ',3X,E10.4,
0056      13X,E10.4,3X,E10.4,///)

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C

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0056      WRITE(6,96)
0057      WRITE(6,97) TP
0058      WRITE(6,98) FAL
0059      WRITE(6,99)

```

C

```

0060      91 FORMAT(/,10X,'*****', ' THE FAILURE MODE = BEARING MODE',/)
0061      92 FORMAT(/,10X,'*****', ' THE FAILURE MODE = SHEAROUT MODE',/)
0062      93 FORMAT(/,10X,'*****', ' THE FAILURE MODE = TENSION MODE',/)
0063      94 FORMAT(/,10X,'*****', ' THE FAILURE MODE = BEARING AND SHEAROUT
0064      1MODE',/)
0064      95 FORMAT(/,10X,'*****', ' THE FAILURE MODE = TENSION AND SHEAROUT
0065      1MODE',/)
0065      96 FORMAT(-----,/)
0066      97 FORMAT(/,10X,'*****', ' THE INITIAL LOAD=',F15.5,/)
0067      98 FORMAT(/,10X,'*****', ' THE FAILURE INDICATOR =',F7.4,/)
0068      99 FORMAT(//, '***** THE STRESS DISTRIBUTIONS DUE TO
0069      1THE INITIAL LOAD ON THE CHARACTERISTIC CURVE FOR EACH PLY *',/)
0069      DO 200 K=1,NLY
0070      WRITE(6,104)
0071      104 FORMAT(/,3X,'LAYER',7X,'NO.',5X,'X1',8X,'X2',8X,'TXX',10X,'TYX',10
0072      1X,'TXV',/)
0072      DO 150 N=1,18
0073      WRITE(6,105) K,N,P(N),Q(N),STRSS(K,N,1),STRSS(K,N,2),STRSS(K,N,3)

```

```

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)      06-01-82      16:15:13
0074      105 FORMAT(/,5X,I3,5X,I3,5X,F6.3,3X,F6.3,6X,E10.4,5X,E10.4,5X,E10.4,/)
0075      150 CONTINUE
C
0076      200 CONTINUE
0077      WRITE(6,17)
0078      17 FORMAT (///,'***** THE STRESS DISTRIBUTIONS
      1ACROSS THE LIGAMENT OF THE PLATE *****',/)
C
0079      300 DO 301 I=1,10
0080      WRITE(6,305) I,XLC(I),YLC(I),(ST(I,J),J=1,3)
0081      305 FORMAT(/,5X,'POINT=',I5,5X,'X1=',F7.4,5X,'X2=',F7.4,5X,'T11=',
      1E15.8,5X,'T22=',E15.8,5X,'T12=',E15.8,/)
0082      301 CONTINUE
C
0083      GO TO 500
0084      500 RETURN
0085      END
*OPTIONS IN EFFECT* ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
*OPTIONS IN EFFECT* NAME = OUTPUT , LINECNT = 57
*STATISTICS* SOURCE STATEMENTS = 85,PROGRAM SIZE = 4414
*STATISTICS* NO DIAGNOSTICS GENERATED

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0001 SUBROUTINE FORMK
0002 IMPLICIT REAL*8 (A-H,O-Z)
0003 COMMON /ANGD/ NX,NP,NEQ,NBC(37),NFX(37),LINT,NBAND
0004 COMMON /ALEN/ NODXY,NELX,IJK(4,400)
0005 COMMON /AMAX/ SK(706,90),R1(706)
C
C
C
0006 DIMENSION A(8,8)
C
C
0007 DO 300 N=1,NEQ
0008 DO 300 M=1,NBAND
0009 300 SK(N,M)=0.0
C
C
0010 DO 400 N=1,NELX
0011 CALL STIFF(N,A)
C
0012 DO 350 JJ=1,NP
0013 NROWB=(IJK(JJ,N)-1)*2
0014 DO 350 J=1,2
0015 NROWB=NROWB+1
0016 I=(JJ-1)*2+J
0017 DO 330 KK=1,NP
0018 NCOLB=(IJK(KK,N)-1)*2
0019 DO 320 K=1,2
0020 L=(KK-1)*2+K
0021 NCOL=NCOLB+K+1-NROWB
C
0022 IF(NCOL) 320,320,310
C
0023 310 SK(NROWB,...)=SK(NROWB,NCOL)*A(I,L)
C
0024 320 CONTIN
0025 330 CONTIN
0026 350 CONTIN
0027 400 CONTIN
C
0028 RETURN
0029 END
*OPTIONS IN EFFECT* ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
*OPTIONS IN EFFECT* NAME = FORMK , LINECNT = 57
*STATISTICS* SOURCE STATEMENTS = 29,PROGRAM SIZE = 1428
*STATISTICS* NO DIAGNOSTICS GENERATED

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```

0001 SUBROUTINE SOLVE
0002 IMPLICIT REAL*8 (A-H,O-Z)
0003 COMMON / AFORE/ PF,DP,F(706)
0004 COMMON /ANOD/ NX,NP,NEQ,NBC(37),NFIX(37),LINT,NBAND
0005 COMMON /ALEM/ NODXY,NELX,IJK(4,400)
0006 COMMON /AMAX/ SK(706,90),R1(706)
0007 DO 100 I=1,NEQ
0008 R1(I)=F(I)
0009 100 CONTINUE
0010 DO 300 N=1,NEQ
0011 I=N
0012 DO 290 L=2,NBAND
0013 I=I+1
0014 IF (I-NEQ) 230,230,290
0015 230 IF (SK(N,L)) 240,290,240
0016 240 C1=SK(N,L)/SK(N,1)
0017 J=0
0018 DO 270 K=L,NBAND
0019 J=J+1
0020 IF (SK(N,K)) 260,270,260
0021 260 SK(I,J)=SK(I,J)-C1*SK(N,K)
0022 270 CONTINUE
0023 280 SK(N,L)=C1
0024 R1(I)=R1(I)-C1*R1(N)
0025 290 CONTINUE
0026 300 R1(N)=R1(N)/SK(N,1)
0027
0028 N=NEQ
0029 350 N=N-1
0030 IF (N) 500,500,360
0031 360 L=N
0032 DO 400 K=2,NBAND
0033 L=L+1
0034 IF (SK(N,K)) 370,400,370
0035 370 R1(N)=R1(N)-SK(N,K)*R1(L)
0036 400 CONTINUE
GO TO 350

```

16:15:19

06-01-82

SOLVE

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)

1622.000
1623.000

0037 500 RETURN

0038 END

OPTIONS IN EFFECT ID,EBDCIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP

OPTIONS IN EFFECT NAME = SOLVE , LINECNT = 57

STATISTICS SOURCE STATEMENTS = 38, PROGRAM SIZE = 1068

STATISTICS NO DIAGNOSTICS GENERATED

```

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)          SHAPEF          06-01-82          16:15:20
0001 SUBROUTINE SHAPEF(MM,XSJ,R,S)          1624.000
0002 IMPLICIT REAL*8 (A-H,O-Z)          1625.000
0003 COMMON /ANOD/ NX,NP,NEQ,NBC(37),NFIX(37),LINT,NBAND          1626.000
0004 COMMON /ALEM/ NODXY,NELX,IJK(4,400)          1627.000
0005 COMMON /ACCOOR/ X(400),Y(400),RG(2),SG(2),WG(2)          1628.000
0006 COMMON /AFUN/ SHP(3,4),T(40,3,3),STRESS(40,18,3),ST(11,3          1629.000
1)          1630.000
C          1631.000
C          1632.000
0007 DIMENSION XS(2,2)          1633.000
C          1634.000
C          1635.000
C ***** COMPUTE SHAPE FUNCTION AND DEVATIVE IN LOCAL COORD. *****          1636.000
C          1637.000
C          1638.000
0008 DO 100 I=1,NP          1639.000
C          1640.000
0009 SHP(3,I)=0.25*(1.0+R*RI(I))*(1.0+S*SI(I))          1641.000
0010 SHP(1,I)=0.25*RI(I)*(1.0+S*SI(I))          1642.000
0011 SHP(2,I)=0.25*SI(I)*(1.0+R*RI(I))          1643.000
C          1644.000
0012 100 CONTINUE          1645.000
C          1646.000
C          1647.000
C ***** COMPUTE JACOBIAN MATRIX XS: DX/DR *****          1648.000
C          1649.000
0013 DO 200 I=1,2          1650.000
0014 DO 200 J=1,2          1651.000
0015 200 XS(I,J)=0.0          1652.000
C          1653.000
C          1654.000
0016 DO 210 J=1,2          1655.000
0017 DO 210 K=1,NP          1656.000
0018 NN=IJK(K,MM)          1657.000
0019 XS(1,J)=XS(1,J)+X(NN)*SHP(J,K)          1658.000
0020 XS(2,J)=XS(2,J)+Y(NN)*SHP(J,K)          1659.000
C          1660.000
0021 210 CONTINUE          1661.000
C          1662.000
C          1663.000
0022 *** COMPUTE DEETERMINENT OF JACOBIAN MATRIX *****          1664.000
C          1665.000
C          1666.000
C          1667.000
C          1668.000
C          1669.000
C          1670.000
0023 DO 300 I=1,4          1671.000
C          1672.000
0024 TEMP=(XS(2,2)*SHP(1,1)-XS(2,1)*SHP(2,1))/XSJ          1673.000
0025 SHP(2,1)=(-XS(1,2)*SHP(1,1)+XS(1,1)*SHP(2,1))/XSJ          1674.000
C          1675.000
0026 300 SHP(1,1)=TEMP          1676.000
C          1677.000
0027 RETURN
0028 END

```

MICHIGAN TERMINAL SYSTEM FORTRAN G(21.8)

SHAPEF

06-01-82

16:15:20

PAGE E002

OPTIONS IN EFFECT ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
OPTIONS IN EFFECT NAME = SHAPEF , LINECNT = 57
STATISTICS SOURCE STATEMENTS = 28, PROGRAM SIZE = 1028
STATISTICS NO DIAGNOSTICS GENERATED

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0001 SUBROUTINE NEWTON(IN,AP,AQ,R,S)
0002 IMPLICIT REAL*8 (A-H,O-Z)
0003 COMMON /ACORR/ X(400),Y(400),RG(2),SG(2),WG(2)
0004 1 SI(4),RI(4),P(20),Q(20)
0005 COMMON /ANOD/ NX,NP,NEQ,NBC(37),NFI(37),LINT,NBAND
0006 COMMON /ALEM/ NODXY,NELX,IJK(4,400)
0007 DIMENSION RP(2)
0008 F(R,S)=A3*R*S+A2*S+A1*R+AO-AP
0009 G(R,S)=B3*R*S+B2*S+B1*R+BO-AQ
0010
0011 FR(R,S)=A3*S+A1
0012 FS(R,S)=A3*R+A2
0013
0014 GR(R,S)=B3*S+B1
0015 GS(R,S)=B3*R+B2
0016
0017 AO=O.O
0018 A1=O.O
0019 A2=O.O
0020 A3=O.O
0021
0022 BO=O.O
0023 B1=O.O
0024 B2=O.O
0025 B3=O.O
0026
0027 DO 100 I=1,4
0028 N=IJK(I,IN)
0029
0030 AO=AO+O.25*X(N)
0031 A1=A1+O.25*X(N)*RI(I)
0032 A2=A2+O.25*X(N)*SI(I)
0033 A3=A3+O.25*X(N)*RI(I)*SI(I)
0034
0035 BO=BO+O.25*Y(N)
0036 B1=B1+O.25*Y(N)*RI(I)
0037 B2=B2+O.25*Y(N)*SI(I)
0038 B3=B3+O.25*Y(N)*RI(I)*SI(I)
0039
0040 100 CONTINUE
0041
0042 R=O.O
0043 S=O.O
0044
0045 AA=F(R,S)
0046 BB=G(R,S)
0047
0048 WRITE(6,3) R,S,AA,BB
0049
0050 DO 10 I=1,20
0051 XU=FR(R,S)*GS(R,S)-FS(R,S)*GR(R,S)
0052 IF(XJ.EQ.O.O) WRITE(6,6)
0053 IF(XJ.EQ.O.O) GO TO 150

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0040      C      DELTAX=(-F(R,S)*GS(R,S)+G(R,S)*FS(R,S))/XJ
0041      C      DELTAY=(-G(R,S)*FR(R,S)+F(R,S)*GR(R,S))/XJ
0042      C      R=R+DELTAX
0043      C      S=S+DELTAY
0044      C      AA=F(R,S)
0045      C      BB=G(R,S)
0046      C      WRITE(6,4) I,R,S,AA,BB
0047      C      IF(DABS(DELTAX) .LT. 1.E-7 .AND. DABS(DELTAY) .LT. 1.E-7)
0048      C      1GO TO 150
0049      C      10 CONTINUE
0050      C      WRITE(6,5)
0051      C      3 FORMAT(/,10X,'R',15X,'S',15X,'F(R,S)',15X,'G(R,S)',/,
0052      C      15X,F10.7,5X,F10.7,5X,F12.7,5X,F12.7/)
0053      C      4 FORMAT(/,1X,'I',13,F10.7,5X,F10.7,5X,F12.7,5X,F12.7/)
0054      C      5 FORMAT(/,,'FAIL TO CONVERGE IN 20 ITERATION ',/)
0055      C      6 FORMAT(/,,'**** JACOBIAN=O.O ****',/)
0056      C      150 RETURN
0057      C      END
0058      *OPTIONS IN EFFECT* ID,EBCDIC,SOURCE,NOLIST,NODECK,LOAD,NOMAP
0059      *OPTIONS IN EFFECT* NAME = NEWTON , LINECNT = 57
0060      *STATISTICS* SOURCE STATEMENTS = 52, PROGRAM SIZE = 2348
0061      *STATISTICS* NO DIAGNOSTICS GENERATED

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NO STATEMENTS FLAGGED IN THE ABOVE COMPILATIONS.

TEST SAMPLE

CONSIDER A SINGLE FASTENER COMPOSITE JOINT MADE OF
T300/SP286 GRAPHITE EPOXY LAMINATE. THE MATERIAL PRO-
PERTIES ARE GIVEN AS E1=18.70 E06 PSI, G12=0.719 E06
PSI, V12=0.30, X(PLY TENSILE STRENGTH)=0.15 E 06 PSI,
SS(LAMINATE SHEAR STRENGTH--CROSS PLY)=0.018 E06 PSI.
QUASI-ISOTROPIC LAMINATE IS USED IN THIS CALCULATIONS.

THE GEOMETRY OF THE JOINT ARE DESCRIBED AS BELOW:

D(DIAMETER)=0.25 IN
W/D(WIDTH RATIO)=6.0
E/D(EDGE RATIO)=3.0
L/D(LENGTH RATIO)=14.0
H(THICKNESS)=0.021 IN

THE CHARACTERISTIC LENGTH FOR TENSION IS EQUAL TO
0.043 IN AND FOR COMPRESSION IS 0.120 IN.

THE FORMAT OF INPUT DATA IS GIVEN IN THE FOLLOWING:

(SEE INPUT INSTRUCTIONS IN PROGRAM)

1	0.0	1.0	719000.0	14.0
1	18700000.0	0.30	3.000	0.1200
	0.2500	6.000	0.043	
	150000.	18000.0		
	0.021			
4	0.0	0.00525	45.0	0.00525
	-45.0	0.00525	90.0	0.00525

Output

***** THE STRENGTH PREDICTION OF FASTENED COMPOSITE JOINTS *****

MATERIAL PROPERTIES OF SINGLE PLY
 E1= 1870000.000 V12= 0.300 G12= 719000.000
 MATERIAL PROPERTIES
 X(PLY T-STRENGTH)= 150000.000 SC(LAMINATE S-STRENGTH)= 18000.000
 RT(CHAR. - TEN.)= 0.0430 RC(CHAR. - COMP.)= 0.1200
 PLY ORIENTATION PLY THICKNESS
 PLY 1= 0.0 0.00525
 PLY 2= 45.000 0.00525
 PLY 3= -45.000 0.00525
 PLY 4= 90.000 0.00525
 GEOMETRY
 DIAMETER= 0.2500 W/D= 6.0000
 E/D= 3.0000 L/D= 14.0000
 THICKNESS= 0.04200

***** THE MAXIMUM LOAD= 1161.505 *****
 ***** THE FAILURE MODE = BEARING AND SHEAROUT MODE *****
 FAILURE LAYER FAILURE POINT FAILURE POSITION STRESS11 STRESS22 STRESS12
 2 6 (0.102 0.215) - .1448E+06 0.1820E+00 - .4687E+04

***** THE INITIAL LOAD= 42.00000 *****
 ***** THE FAILURE INDICATOR = 0.0013 *****
 ***** THE STRESS DISTRIBUTIONS DUE TO THE INITIAL LOAD ON THE CHARACTERISTIC CURVE FOR EACH PLY *****

LAYER	NO.	X1	X2	TXX	TTY	TX	TY
1	1	0.004	0.245	- .4941E+04	0.8567E-02	0.3624E+02	
1	2	0.012	0.245	- .4906E+04	0.8634E-02	0.4183E+02	
1	3	0.036	0.242	- .4525E+04	0.7968E-02	0.1229E+03	
1	4	0.059	0.235	- .3780E+04	0.6694E-02	0.1953E+03	
1	5	0.081	0.226	- .2723E+04	0.4975E-02	0.2530E+03	
1	6	0.102	0.215	- .3422E+04	- .2146E-03	0.3091E+03	
1	7	0.120	0.201	- .1855E+04	- .2014E-02	0.3317E+03	
1	8	0.137	0.185	- .1380E+03	- .3547E-02	0.3319E+03	
1	9	0.151	0.167	0.1630E+04	- .4515E-02	0.3095E+03	
1	10	0.163	0.148	0.2002E+04	- .6198E-02	0.2530E+03	
1	11	0.172	0.127	0.2841E+04	- .7092E-02	0.2235E+03	
1	12	0.178	0.107	0.3552E+04	- .6921E-02	0.1816E+03	
1	13	0.182	0.086	0.4095E+04	- .5661E-02	0.1340E+03	
1	14	0.183	0.065	0.4461E+04	- .3454E-02	0.8795E+02	
1	15	0.181	0.045	0.4572E+04	- .3443E-02	0.1065E+03	
1	16	0.177	0.026	0.4909E+04	0.2985E-03	0.6738E+02	
1	17	0.172	0.008	0.5220E+04	0.3879E-02	0.4397E+02	
1	18	0.169	0.003	0.5407E+04	0.5019E-02	0.2622E+02	
LAYER	NO.	X1	X2	TXX	TTY	TX	TY
2	1	0.004	0.245	- .1400E+04	- .4691E-02	- .3086E+03	
2	2	0.012	0.245	- .1454E+04	- .4289E-02	- .3073E+03	
2	3	0.036	0.242	- .2437E+04	0.1522E-03	- .2835E+03	
2	4	0.059	0.235	- .3237E+04	0.4662E-02	- .2371E+03	
2	5	0.081	0.226	- .3777E+04	0.8923E-02	- .1719E+03	
2	6	0.102	0.215	- .5237E+04	0.6581E-02	- .1695E+03	
2	7	0.120	0.201	- .5151E+04	0.1032E-01	- .7824E+02	
2	8	0.137	0.185	- .4696E+04	0.1352E-01	0.1860E+02	

2	0.151	0.167	- .3876E+04	0.1610E-01	0.1139E+03
2	0.163	0.148	- .3168E+04	0.1316E-01	0.1447E+03
2	0.172	0.127	- .2575E+04	0.1318E-01	0.1930E+03
2	0.178	0.107	- .1765E+04	0.1298E-01	0.2273E+03
2	0.182	0.086	- .8389E+03	0.1281E-01	0.2454E+03
2	0.183	0.065	0.9469E+02	0.1289E-01	0.2478E+03
2	0.181	0.045	- .1070E+03	0.1407E-01	0.2533E+03
2	0.177	0.026	0.8699E+03	0.1542E-01	0.2432E+03
2	0.172	0.008	0.1618E+04	0.1736E-01	0.2330E+03
2	0.169	0.003	0.2021E+04	0.1770E-01	0.2342E+03
NO.	X1	X2	TX	TY	TX
1	0.004	0.245	- .4572E+03	- .8219E-02	0.3086E+03
2	0.012	0.245	- .3658E+03	- .8361E-02	0.3073E+03
3	0.036	0.242	0.7596E+03	- .1181E-01	0.2835E+03
3	0.059	0.235	0.1843E+04	- .1436E-01	0.2371E+03
3	0.081	0.226	0.2802E+04	- .1570E-01	0.1719E+03
3	0.102	0.215	0.2802E+04	- .2351E-01	0.1695E+03
3	0.120	0.201	0.3476E+04	- .2197E-01	0.7824E+02
3	0.137	0.185	0.3936E+04	- .1880E-01	- .1860E+02
3	0.151	0.167	0.4173E+04	- .1403E-01	- .1139E+03
3	0.163	0.148	0.3411E+04	- .1147E-01	- .1447E+03
3	0.172	0.127	0.3237E+04	- .8575E-02	- .1930E+03
3	0.178	0.107	0.2957E+04	- .4693E-02	- .2273E+03
3	0.182	0.086	0.2647E+04	- .2402E-03	- .2454E+03
3	0.183	0.065	0.2382E+04	0.4328E-02	- .2478E+03
3	0.181	0.045	0.2663E+04	0.3700E-02	- .2533E+03
3	0.177	0.026	0.2622E+04	0.8859E-02	- .2432E+03
3	0.172	0.008	0.2761E+04	0.1308E-01	- .2330E+03
3	0.169	0.003	0.2703E+04	0.1514E-01	- .2342E+03
NO.	X1	X2	TX	TY	TX
1	0.004	0.245	0.3084E+04	- .2148E-01	- .3624E+02
2	0.012	0.245	0.3086E+04	- .2128E-01	- .4183E+02
3	0.036	0.242	0.2848E+04	- .1963E-01	- .1229E+03
3	0.059	0.235	0.2386E+04	- .1639E-01	- .1953E+03
3	0.081	0.226	0.1747E+04	- .1176E-01	- .2530E+03
3	0.102	0.215	0.9863E+03	- .1671E-01	- .3091E+03
3	0.120	0.201	0.1799E+03	- .9631E-02	- .3317E+03
3	0.137	0.185	- .6218E+03	- .1735E-02	- .3319E+03
3	0.151	0.167	- .1333E+04	0.6576E-02	- .3095E+03
3	0.163	0.148	- .1760E+04	0.7885E-02	- .2530E+03
3	0.172	0.127	- .2179E+04	0.1170E-01	- .2235E+03
3	0.178	0.107	- .2360E+04	0.1521E-01	- .1816E+03
3	0.182	0.086	- .2287E+04	0.1823E-01	- .1340E+03
3	0.183	0.065	- .1984E+04	0.2067E-01	- .8795E+02
3	0.181	0.045	- .2015E+04	0.2121E-01	- .1065E+03
3	0.177	0.026	- .1417E+04	0.2398E-01	- .6738E+02
3	0.172	0.008	- .8404E+03	0.2656E-01	- .4397E+02
3	0.169	0.003	- .6836E+03	0.2782E-01	- .2622E+02
NO.	X1	X2	TX	TY	TX
1	0.1271	X2= 0.0026	T11= 0.1046191E+03	T22= 0.43780118E+04	T12= 0.10341868E+02
2	0.1397	X2= 0.0029	T11= 0.29470390E+03	T22= 0.33064404E+04	T12= -0.26063698E+02
3	0.1627	X2= 0.0034	T11= 0.34292025E+03	T22= 0.22448220E+04	T12= -0.79359774E+02
4	0.1962	X2= 0.0041	T11= 0.29215934E+03	T22= 0.15173410E+04	T12= -0.11360285E+03
5	0.2401	X2= 0.0050	T11= 0.21404444E+03	T22= 0.10787318E+04	T12= -0.12442130E+03
6	0.2944	X2= 0.0061	T11= 0.14530340E+03	T22= 0.81755422E+03	T12= -0.11835797E+03
7	0.3591	X2= 0.0075	T11= 0.93844418E+02	T22= 0.65169154E+03	T12= -0.10077351E+03
8	0.4401	X2= 0.0091	T11= 0.55440756E+02	T22= 0.53468656E+03	T12= -0.71020255E+02
9	0.5513	X2= 0.0091	T11= 0.25849230E+02	T22= 0.42057408E+03	T12= -0.36908192E+02