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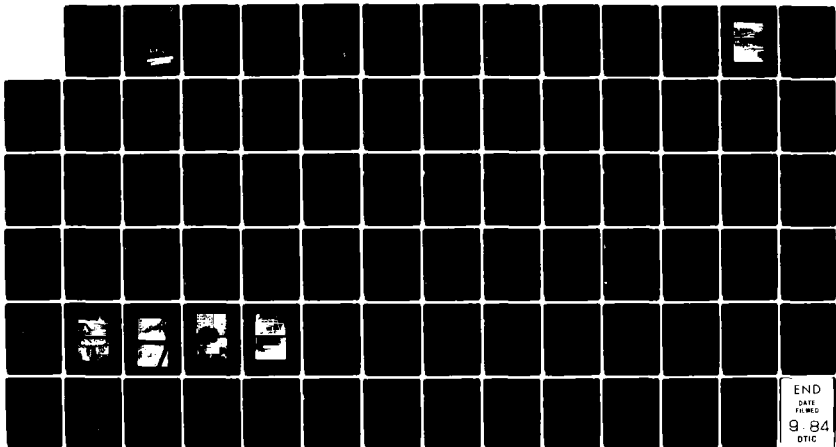
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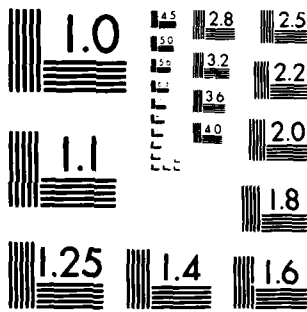
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CONNECTICUT RIVER BASIN  
PLYMOUTH, CONNECTICUT

OLD MARSH POND DAM

CT 00285

PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION PROGRAM

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20. ABSTRACT (Continue on reverse side if necessary and identify by block number) Old Marsh Pond Dam is a zoned earth embankment about 1,100 ft. long with a concrete core wall, is 33 ft. wide at the crest, and has a maximum height of about 32 ft. Both the dam and dike appear to be in good condition. The full PMF test flood would not overtop the dam (provided that the flashboards were not in place), but would overtop the dike by 1 ft. Based on maximum storage capacity, the dam is classified as intermediate in size. It has been classified as having a high hazard potential.		

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DEPARTMENT OF THE ARMY  
NEW ENGLAND DIVISION, CORPS OF ENGINEERS  
424 TRAPELO ROAD  
WALTHAM, MASSACHUSETTS 02154

REPLY TO  
ATTENTION OF:

NEDED

OCT 2 1979

Honorable Ella T. Grasso  
Governor of the State of Connecticut  
State Capitol  
Hartford, Connecticut 06115

Dear Governor Grasso:

I am forwarding to you a copy of the Old Marsh Pond Dam Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Department of Environmental Protection, the cooperating agency for the State of Connecticut. In addition, a copy of the report has also been furnished the owner, City of Bristol, Connecticut 06010.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Department of Environmental Protection for your cooperation in carrying out this program.

Sincerely,

MAX B. SCHEIDER  
Colonel, Corps of Engineers  
Division Engineer

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As stated

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OLD MARSH POND

CT 00285

CONNECTICUT RIVER BASIN  
PLYMOUTH, CONNECTICUT

PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION PROGRAM

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NATIONAL DAM INSPECTION PROGRAM  
PHASE I INSPECTION REPORT

Identification No. CT 00285  
Name of Dam: Old Marsh Pond Dam  
Town: Plymouth  
County and State: Litchfield County, Connecticut  
Stream: Marsh Brook  
Date of Inspection: 9 November 1978

BRIEF ASSESSMENT

Old Marsh Pond Dam is a zoned earth embankment about 1,100 ft. long with a concrete core wall, is 33 ft. wide at the crest, and has a maximum height of about 32 ft. Marsh Road traverses the crest of the dam. The dam is operated as a water supply facility for the City of Bristol. The closure dike some 1,000 ft. to the right of the dam is a homogeneous embankment about 526 ft. long, 14 ft. high and 24 ft. wide at the crest.

The main dam has a spillway consisting of a 60 ft. long ungated overflow ogee crest 6.3 ft. below the top of the dam. Fourteen (14) in. high flashboards are fixed on the crest from spring to fall. A 16 in. dia. outlet pipe leads downstream about one mile to Bristol Reservoir No. 1.

Maximum storage capacity of the reservoir to top of dam is 3,094 acre-ft. and the drainage area is 2.34 square miles. The reservoir is about 7,300 ft. long and has a surface at normal storage (without flashboards) of 183 acres. Based on maximum storage capacity, the dam is classified as intermediate in size. Because of the threat to life and property which would result if the dam or dike were breached, it has been classified as having a high hazard potential.

Both the dam and dike appear to be in good condition. The full PMF test flood would not overtop the dam (provided that the flashboards were not in place), but would overtop the dike by 1 ft. The facility could handle 75% of the test flood without overtopping the dike. While the spillway crest is adequate to pass the test flood outflow, the spillway chute and stilling basin walls would all be overtopped. At a 0.5 PMF outflow, the stilling basin walls would be overtopped and the left chute wall would be on the verge of overtopping.

There is considerable seepage in the vicinity of the center of the dam, which has produced a swampy area below the downstream slope. The stilling basin side walls have deflected inwards relative to the spillway chute walls. The outlet is too small to permit rapid drawdown of the reservoir. There is some minor displacement of riprap, some local brush growth and a few animal burrows on the dam.

Within two years after receipt of this Phase I Inspection Report, the owner, the City of Bristol, should retain the services of a registered professional engineer to make further investigations, and should implement the results. These studies should cover:

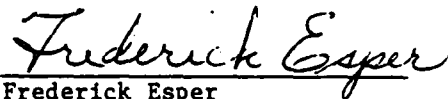
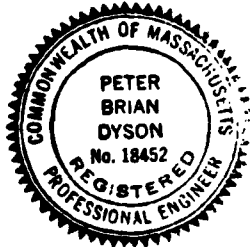
- (1) whether the dike should be raised;
- (2) whether spillway channel modifications are required;
- (3) elimination of use of flashboards, or modifications to facilitate their quick removal;
- (4) source of leakage producing a swamp below the dam;
- (5) cause of relative deflection of spillway chute walls;
- and (6) whether a new outlet for rapid drawdown of the reservoir is required.

The owner should also implement the following measures:

- (1) monitor the seepage monthly at the center of the dam;
- (2) monitor the marshy area right of the maintenance building access ramp monthly;
- (3) restore riprap in the vicinity of the spillway walls;
- (4) control growth on the right downstream slope;
- (5) fill animal burrows and eliminate animal infestation;
- (6) repair the top of the concrete wall which extends from the right stilling basin wall;
- (7) remove plywood forms from the underside of the bridge deck, make necessary repairs, paint all bridge steelwork;
- (8) institute procedures for a biennial periodic technical inspection; and
- (9) develop a formal surveillance and warning plan.



Peter B. Dyson  
Project Manager



Frederick Esper  
Vice President



This Phase I Inspection Report on Old Marsh Pond Dam has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

*Joseph W. Finegan*  
JOSEPH W. FINEGAN, JR., MEMBER  
Water Control Branch  
Engineering Division

*Carney M. Terzian*  
CARNEY M. TERZIAN, MEMBER  
Design Branch  
Engineering Division

*Joseph A. McElroy*  
JOSEPH A. MCELROY, CHAIRMAN  
Chief, NED Materials Testing Lab.  
Foundations & Materials Branch  
Engineering Division

APPROVAL RECOMMENDED:

*Joe B. Fryar*  
JOE B. FRYAR  
Chief, Engineering Division

## PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation, and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test Flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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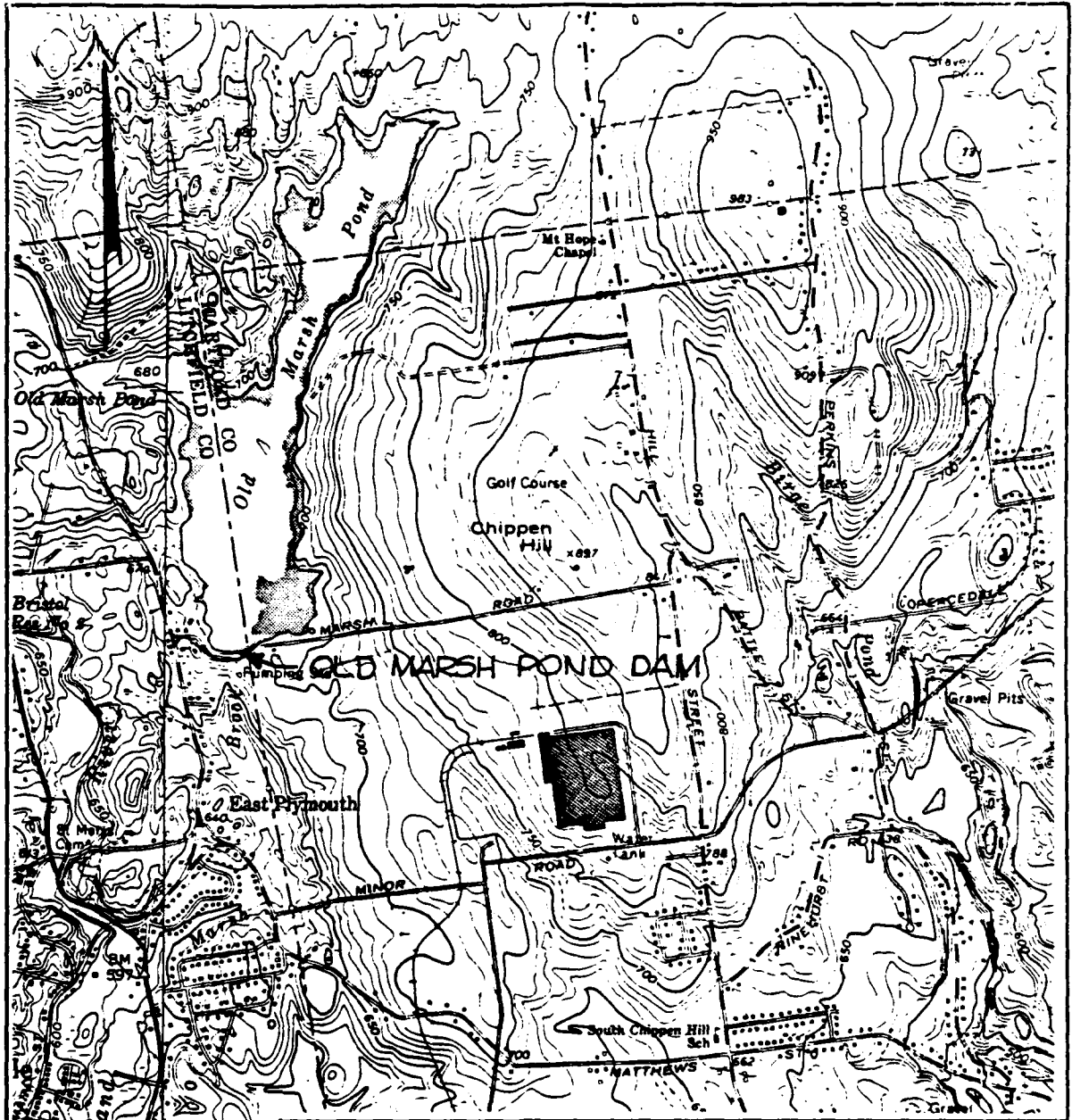
OLD MARSH POND DAM



Overview of dam from right abutment of dam.



Overview of closure dike from left abutment of dike.



LOUIS BERGER & ASSOC., INC WELLESLEY, MASS. ARCHITECT · ENGINEER	U.S. ARMY ENGINEER DIV. NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MASS.
<b>NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS</b>	
<b>OLD MARSH POND DAM</b>	
<b>BRISTOL - THOMASTON QUADRANGLES</b>	
<b>CONNECTICUT RIVER BASIN</b>	
<b>STATE - CONN.</b>	
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PHASE I INSPECTION REPORT

OLD MARSH POND DAM CT 00285

SECTION 1 - PROJECT INFORMATION

1.1 General

a. Authority

Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a national program of dam inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Louis Berger & Associates, Inc. has been retained by the New England Division to inspect and report on selected dams in the State of Connecticut. Authorization and notice to proceed was issued to Louis Berger & Associates, Inc. under a letter of 27 October 1978 from Max B. Scheider, Colonel, Corps of Engineers. Contract No. DACW33-78-C-0371 Job Change No. 1 has been assigned by the Corps of Engineers for this work.

b. Purpose

1. Perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.
2. Encourage and assist the States to initiate quickly effective dam safety programs for non-Federal dams.
3. Update, verify and complete the National Inventory of Dams.

1.2 Description of Project

a. Location

Old Marsh Pond Dam is located in the Town of Plymouth, Litchfield County, about 4 miles northwest of the City of Bristol, Hartford County, in Central Connecticut. The dam is reached by proceeding west  $1\frac{1}{2}$  miles from Main Street in Bristol to Hill Street, then north  $1\frac{1}{2}$  miles to Marsh Road and one mile

to the dam. About a third of the reservoir is in  
Middletown and two thirds is in Bristol. The dam is situated  
on Marsh Brook, a tributary of the Poland River which joins  
the Pequabuck River in Terryville. The Pequabuck is a tribu-  
tary of the Farmington River which flows into the Connecticut  
River.

The dam and reservoir are operated as a water supply facility  
for the City of Bristol.

b. Description of Dam and Appurtenances

1. Dam

Marsh Pond Dam is a zoned earthfill embankment about 32 ft.  
high at its maximum section and about 1,100 ft. long. The  
dam is a reconstructed and raised structure built in 1952  
on top of and immediately downstream from an existing dam,  
which was built about 1913. The original dam had a crest  
elevation of about 683; the new dam has its crest at  
elevation 694 MSL.

The original dam was constructed of loamy clay, sand,  
gravel, and boulders, with its 2 to 1 sloped upstream  
face covered with stone paving. A central core wall built  
of boulder concrete on the centerline of the dam was  
carried from about 2.5 ft. below the crest down to bedrock  
foundation. The old dam was left in place, except for  
surface stripping, when the new embankment was added, and  
it now forms the upstream toe of the new dam.

The new dam was built with its centerline 30 ft. down-  
stream from that of the original dam. A new core wall  
was constructed of reinforced concrete with its center-  
line one ft. upstream from the new dam centerline, ex-  
tending from 3 ft. below crest level to firm rock founda-  
tion. To reach bedrock a 7 ft. wide vertical sheeted  
trench was excavated through the downstream face of the  
old dam. After the core wall was completed the trench  
was backfilled with impervious material.

The new dam has a 33 ft. top width at elevation 694, with  
a 1½ to 1 upstream slope intersecting the crest level of  
the old dam, and with heavy random riprap placed on the  
old dam face on about a 2 to 1 slope below the old crest  
level. The downstream face of the new dam is at 1½ to 1  
slope from the crest down to elevation 683, and then at  
2 to 1 slope below that level. The surface of the slope  
is covered with loam soil and is grassed.

The central body of the new dam is compacted impervious embankment. A heavy random riprap on 1½ to 1 covers the upstream face above the old dam. The downstream portion of the dam below elevation 683, with a thickness of 17.5 ft. at that level increasing to as much as 30 ft. at the base of the dam, is composed of pervious material. A graded sand and gravel filter separates this pervious fill from the foundation. Below elevation 668.7 this horizontal filter terminates in a 5 ft. deep toe trench and intercepting drain located along the toe of the dam. A rock-filled toe berm 5 ft. wide covers the intercepting drain.

The foundation of the main portion of the dam and right abutment is overburden of varying thickness, overlying micaceous gneiss and schist ranging from decomposed to firm. The core wall along this portion of the dam was extended at least 2 ft. into sound rock. On the left abutment east of the spillway the bedrock falls away steeply and the core wall was carried through overburden only to what was considered to be impervious stratum. The core wall extends the entire length of the dam below elevation 691. Plans and profiles of the dam and structures are included in Appendix B.

## 2. Closure Dike

A dike to close off a low saddle is located along the reservoir rim about 1,000 ft. to the right of the dam. The dike is a homogeneous embankment about 14 ft. high above natural ground and about 526 ft. long. The dike, which was built in 1952, replaces a lower embankment closing off the saddle, and was located just downstream so that its upstream portion covers a portion of the old embankment. A deep cutoff trench was excavated and back-filled under the dike, extending to bedrock along the entire length of the embankment. The cutoff has a 10 ft. bottom width and 1 to 1 side slopes, and at its deepest was excavated to more than 30 ft. below ground surface. The dike was built to crest elevation 693, one ft. below the crest level of the main dam. The dike has a top width of 20 ft., with 2 to 1 upstream and downstream slopes. The upstream slope is protected with an 18 in. layer of riprap, placed on a gravel and sand filter. A 5 ft. deep downstream toe drain trench filled with coarse stone and a horizontal filter blanket are provided along the downstream toe of the dike. Plans of the dike are included in Appendix B.

### 3. Spillway

When the dam was raised the old spillway was abandoned and a new structure was built to the left, higher on the abutment. The spillway is an ungated overflow crest and converging curved chute structure, terminating with a hydraulic jump stilling basin. The ogee crest is 60 ft. in length, with crest level at elevation 687.7, or 6.3 ft. below the top of the dam.

The ogee crest is an overflow gravity dam which is built up from bedrock, resulting in a structure having a maximum height of about 27 ft. This dam has a vertical upstream face and a downstream face with an average slope of 0.76 to 1. The top of this dam is ogee and curved to conform to the underside of a nappe over a sharp crested weir, shaped for a design head of about 2.7 ft.

The channel downstream from the crest turns 22.5 degrees to the right and converges from the 60 ft. crest width to a 25 ft. width. The 25 ft. chute length continues about 130 ft. to a 45 ft. long stilling basin with its floor at elevation 650. The channel walls are 7.25 ft. high below the converged section, 3.5 ft. high along the chute and 12 ft. high at the stilling basin. A 60 ft. span steel beam bridge with concrete deck crosses the spillway at the crest of the dam. Details of the spillway design are included in Appendix B.

### 4. Outlets

The low level outlet consists of a 24 in. dia. cast iron pipe which was constructed under the old dam, and which then connects directly into a 16 in. dia. line which leads downstream to Bristol Reservoir No. 1. The invert of the inlet to the 24 in. dia. pipe is at elevation 663.7. A 16 in. dia. cast iron high level inlet, with its invert level at elevation 678.3, is carried through the new embankment and connects to the 16 in. dia. line downstream from the toe of the dam. Control for the low level line is by a 24 in. dia. valve, reached from a submerged gate well near the upstream toe, and by a 16 in. dia. gate valve operated from a gate chamber near the downstream toe. The high level outlet is controlled by a 16 in. dia. gate valve operated from a gate chamber located near the upstream edge of the crest of the dam. All outlet flows are carried directly to Reservoir No. 1 via a 16 in. dia. delivery pipe about one mile in length. A 6 in. dia. blowoff line takes off from the 16 in. dia. line about 100 ft. downstream from the toe of the dam. This blowoff

line is carried into the spillway stilling basin and is controlled by a 6 in. dia. gate valve.

Details of the outlet pipes are shown on drawings included in Appendix B.

c. Size Classification

Old Marsh Pond Dam is about 32 ft. high, impounding a maximum storage of about 1,800 acre-feet to spillway crest level and about 3,100 acre-feet to the top of the dam. In accordance with the size and capacity criteria given in the Recommended Guidelines for Safety Inspection of Dams, storage governs and therefore the project is classified as intermediate in size.

d. Hazard Classification

A breach failure of Old Marsh Pond Dam would release water down Marsh Brook and into the Poland and Pequabuck Rivers. These streams traverse through the urbanized areas of East Plymouth and Terryville in Plymouth, and through the City of Bristol. There are a number of homes in East Plymouth near stream level along Marsh Brook; major highways, including State Route 72 and part of U. S. Route 202, follow along the Poland and Pequabuck Rivers; and there are a number of highway crossings of these rivers. Consequently, Old Marsh Pond Dam has been classified as having a high hazard potential in accordance with the Recommended Guidelines for the Safety Inspection of Dams.

e. Ownership

Old Marsh Pond Dam is owned by the City of Bristol, Connecticut.

f. Operator

John Burns, Superintendent  
Water Department  
119 Reservoir Avenue  
Bristol, CT 06010

Telephone: (203) 582-7431

g. Purpose of Dam

Old Marsh Pond reservoir is operated in conjunction with other water storage facilities to supply municipal water to the City of Bristol.

#### h. Design and Construction History

The original Old Marsh Pond Dam was built in 1913. The engineer in charge of constructing the dam was Mr. Nicholas Fogg, employed by the firm of Sperry and Buell. The dam was built for Andrew Terry Co. of Terryville in the Town of Plymouth, who used it as a power generation facility for their manufacturing plant. In 1944 the dam and water rights were acquired by the City of Bristol Water Commission, to supplement the City's water resources. The original dam and reservoir were operated until about 1950, when the reservoir was raised about 8 ft. by the construction of the new dam, which, being 11 ft. higher, provided for an additional 3 ft. of surcharge.

The design of the dam enlargement was prepared by Metcalf and Eddy, Engineers, of Boston, MA.

#### i. Normal Operational Procedure

There are no written operating procedures. All withdrawals from Old Marsh Pond reservoir are transported by pipeline to Bristol Reservoir No. 1 about one mile downstream, and day to day regulation of withdrawals is not required. Water Department personnel are available for regulating reservoir releases.

### 1.3 Pertinent Data

#### a. Drainage area

The drainage area contributing to the Old Marsh Pond reservoir encompasses about 2.34 square miles. The reservoir extends along nearly the full length of the area, with contributing inflows draining off the slopes to the sides through five small inlet streams. These tributary inlets vary from 3,600 ft. to 6,000 ft. in length, with gradients varying from 161 ft. per mile to 267 ft. per mile.

The drainage area measures about 2.4 miles in length and 1.6 miles in maximum width. The eastern edge of the area is relatively open and occupied by a golf course; the remaining area is heavily wooded.

#### b. Discharge at Damsite

##### 1. Outlet works

Outlet releases are limited to the discharge which can be carried through the 16 in. dia. line to the Bristol Reservoir No. 1. A 6 in. dia. blowoff pipe is provided in this

line, controlled by a 6 in. dia. valve. The blowoff line empties into the spillway stilling basin. It is estimated that with all valves open the capacity of the line will be about 9 cfs.

2. Maximum Flood at Damsite

No records are available of flood inflows into Old Marsh Pond reservoir, nor of spillway releases and surcharge heads during such inflows.

3. Ungated Spillway Capacity

The spillway at Old Marsh Pond Dam is an ungated ogee overflow 60 ft. in length, which appears to have been designed for a capacity of about 900 cfs at 2.4 ft. surcharge head. The spillway control will be able to handle about 3,500 cfs with reservoir to top of dike, elevation 693; and about 4,000 cfs with reservoir to top of dam, elevation 694. Discharge curves and computations are shown on Figure 1 and sheets D-1 to D-3, Appendix D.

The spillway chute and stilling basin have been designed to provide for the 900 cfs flow, with adequate sidewall freeboards to prevent an overtopping of the walls. At a 1,400 cfs flow, conjugate depth in the hydraulic jump basin will be equal to the basin wall height; and flows in excess of 1,400 cfs will overtop the basin walls. At about a 2,000 cfs flow, because of the sharp convergence of the left wall into the general direction of flow, it is expected that the resulting wave will ride up and overtop the wall. At about a 2,700 cfs discharge, the flow depth will be equal to the height of the chute walls, without considering wave action and swell owing to air entrainment. At a 4,000 cfs discharge, conjugate depth will be 4 ft. higher than the basin walls. It is thus seen that although the spillway overflow can accommodate 4,000 cfs with the reservoir lower than the top of the dam, the spillway chute and stilling basin can handle no more than 1,400 cfs without incurring damage from an overtopping of its walls. An overtopping would probably wash out the backfill behind the walls and undermine their foundation, thus causing a failure of the spillway chute. Such a failure could threaten the lower portion of the main dam embankment. Computations of spillway channel flow are shown on Sheets D-4 and D-5.

4. Ungated Spillway Capacity at Test Flood Elevation

At the test flood elevation of 694.0, the discharge through the spillway will be 3,940 cfs.

### Flashboarded Spillway Capacity

With the intent of increasing the reservoir yield by capturing and storing runoffs above spillway crest level, 4 in. high flashboards are installed on the crest each spring and removed in early fall. The flashboards are held in place by angle iron frames, which are inserted into the crest at their bottom and then supported at their top by bolting onto the upstream bridge beam.

With the flashboards in place, the spillway capacity for heads up to reservoir level 692 will be reduced by more than half the capacity without the flashboards; for a head to reservoir level 694 the spillway capacity will be 55 percent of that of an unobstructed crest. A discharge curve for flow over the crest with the flashboards installed is shown on Fig. 1, Sheet D-1, Appendix D.

#### 6. Total Project Discharge at Test Flood Elevation

At the test flood elevation of 694.0, the discharge over the dike will be 1,475 cfs, giving a total project discharge of 5,415 cfs.

#### c. Elevation (ft. above MSL)

1. Top of dam	694.0
2. Maximum pool-design surcharge	691.0
3. Spillway crest (ungated)	687.7
4. Diversion outlet invert	663.7
5. Streambed at centerline of dam	663 ±

#### d. Reservoir

1. Length of pool (normal W.S.)	7,300 ft.
2. Average width of pool (normal W.S.)	1,100 ft.

#### e. Storage (acre-feet)

1. At normal storage pool (El. 687.7)	1,808
2. At design surcharge pool (El. 691)	2,454
3. At top of dike (El. 693)	2,876
4. At top of dam (El. 694)	3,094

#### f. Reservoir surface (acres)

1. At top of dam (El. 694)	220
2. At top of dike (El. 693)	216
3. At design surcharge pool (El. 691)	206
4. At top of flashboards (El. 688.87)	192
5. At spillway crest (El. 687.7)	183

g. Dam

1. Type - Zoned earthfill embankment
2. Length 1,100 ft.
3. Height 32 ft.
4. Top width 33 ft.
5. Side slopes -  $1\frac{1}{2}$  to 1 and 2 to 1 upstream;  $1\frac{1}{2}$  to 1 and 2 to 1 downstream.
6. Zoning - Concrete core wall; impervious fill for upstream zone and center portion of downstream zone; pervious material at outer portion of downstream zone; horizontal filter under downstream zone; rock fill toe; upstream rock fill facing.
7. Impervious core - Concrete core wall to bedrock along main dam, to impervious stratum along east abutment; core wall carried to within 3 feet of top of dam.
8. Cutoff - core wall in trench to bedrock or to impervious stratum.
9. Grout curtain - none
10. Other - Horizontal filter under downstream pervious zone of dam.

h. Closure Dike

1. Type - Homogeneous earthfill embankment.
2. Length 526 ft.
3. Height 14 ft.
4. Top width 20 ft.
5. Side slopes - 2 to 1 upstream and downstream
6. Zoning - Compacted earthfill. Most impervious material upstream, less impervious material downstream.
7. Impervious core - none
8. Cutoff - Deep trench to bedrock backfilled with impervious fill material. Cutoff trench to depths up to 35 ft. maximum, 10 ft. wide at bottom.
9. Grout curtain - none
10. Other - Upstream slope riprap - 18 in. thick on 6 in. gravel and sand filter layers. 5 ft. deep drain trench under downstream toe backfilled with coarse stone. Rockfill at downstream toe.

i. Spillway

1. Type - Ungated overflow crest and converging chute structure, terminating with hydraulic jump stilling basin.
2. Length of weir 60 ft.
3. Crest elevation 687.7 MSL

4. Gates - None, but provision for 14 in. flashboards.
5. Upstream channel - Inlet directly from reservoir through upstream face of dam.
6. Downstream channel - Converging curved chute and downstream hydraulic jump basin.
7. General - Crest control adequate to release up to 4,000 cfs before overtopping of dam. Chute and stilling basin adequate to discharge only 1,400 cfs before overtopping of walls takes place.

j. Regulating Outlets

1. Invert - Low-level at elevation 663.7  
High level at elevation 678.3
2. Size - Low level - 24 in. dia. connected to 16 in. dia. line to downstream reservoir.  
- High Level - 16 in. dia. connecting into 16 in. dia. line to downstream reservoir.
3. Control mechanism - Low level: 24 in. dia. valve near upstream end and 16 in. dia. valve near downstream toe of dam.  
- High level: 16 in. dia. valve near upstream edge of crest.

## SECTION 2 - ENGINEERING DATA

### 2.1 Design

1943 correspondence in the files of the State Department of Environmental Protection mentions the availability of plans of the original dam, but these have not been retrieved. It is presumed that they were available to the engineering firm of Metcalf and Eddy (now Metcalf and Eddy, Inc.) of Boston, MA, who prepared the detailed plans for the enlarged dam and dike in 1950. Reduced scale copies of those plans which are pertinent to an assessment of dam safety are included in Appendix B. Neither design data nor design criteria leading to this design have been retrieved.

### 2.2 Construction

No information has been recovered regarding construction of the original dam in 1913. No construction reports or histories of construction have been found documenting construction details or results of the 1952 alterations and additions. The design drawings included in Appendix B have been revised to show as-built details and constitute the record plans for this work.

### 2.3 Operation

The reservoir is operated by personnel of the City of Bristol Water Department, in conjunction with other reservoir storage facilities in their water supply system. All releases are transported to a lower reservoir and day to day operation of the outlet is not required. Flashboards on the spillway crest are installed in the spring and removed in early fall. No operation or maintenance criteria, or guideline manuals have been prepared for this installation.

### 2.4 Evaluation

#### a. Availability

The plans of the enlargement of the dam and dike provide insufficient engineering data for an assessment to be made of the structural stability and safety of the embankments.

#### b. Adequacy

The lack of in-depth engineering data precludes a definitive review and assessment of this dam. The evaluation is based primarily on visual inspection and engineering judgment.

c. Validity

The validity of the engineering data acquired covering the dam and dike is considered acceptable and is not challenged.

## SECTION 3 - VISUAL INSPECTION

### 3.1 Findings

#### a. General

The visual inspection of Old Marsh Pond Dam took place on 9 November 1978. At that time, the reservoir was about 3 ft. below spillway crest level. It was not determined whether storage was being released through the pipeline to Bristol Reservoir No. 1.

Both the main dam and closure dike were judged to be in good condition. There was no evidence of any major maintenance problems. Fairly heavy seepage was noted in the vicinity of the original stream channel below the dam, but its source was not determined.

#### b. Dam

The crest of the dam is a well travelled paved roadway which is in good condition (Appendix C, Photo No. 1).

The riprapped top of the original dam upon which the present dam was superimposed was visible as a berm slightly below the water level. The base of an abandoned intake tower, damaged by ice, was visible in the reservoir opposite the approximate center of the dam.

The upstream face of the dam is riprapped with 6 in. to 18 in. stone almost up to the gravel shoulder. Locally, such as immediately west of the spillway, the riprap has displaced downslope, possibly attributable to surface drainage channelized against the spillway wall. Riprap appears to be less massive than called for on the drawings.

At the left abutment there is an access road to the pumping station and maintenance building, beneath which a culvert passes, draining to a low area at the toe of the dam. Here, the ground is boggy and displays characteristic marsh growth.

Opposite the highest section of the dam in the original stream channel, about 200 ft. west of the spillway, prominent seepage issues from an area about 10 ft. wide in the vicinity of the toe, where a considerable amount of peastone has been placed, and flows southerly some 200 ft. to the end of the spillway

. When it reaches the end of the spillway flow issues from a boggy plateau and cascades over the retaining wall adjacent to the spillway stilling basin. Additional seepage comes from the side of the wall. The combined flow was estimated to be about 5 gpm. This wall is badly deteriorated, probably due to freezing and thawing action from the seepage overtopping the wall. The line of seepage from the dam coincides with the alignment of the water supply line, and a swath some 20 ft. wide has been cut in the growth for the full length of the pipe. (Appendix C, Photo Nos. 2, 3 & 4) According to the Water Department's representative, this marshy area and its seepage was present, in about the same condition, as long ago as 1968.

There is some evidence of rodent infestation at mid-dam, on the lower part of the downstream slope, with one hole having recently been filled by the maintenance crew. (Appendix C, Photo No. 5) The downstream slope is grass covered and kept mowed.

c. Closure Dike

The upstream face of the dike is riprapped with large stone in good condition. The crest and downstream slope are clear of excessive growth. No seepage was apparent.

d. Spillway

Except as noted below, the spillway alignment appears to be good and concrete surfaces appear to be in good condition. No cracks or spillings of consequence were detected (Appendix C, Photo No. 6).

At the left stilling basin wall, an apparent leaning in of the wall has taken place, indicated by offsets at the joints of the two 12 ft. high wall panels. A 2 in. offset outward of the upper panel was measured at Station 0+76 (see Sheet 15 of 19, Appendix B); an outward offset of 2 in. of the lower panel was measured at Station 0+95; and an offset outward of 3½ in. of the lower panel was measured at Station 1+18. In an apparent belief that the walls were unstable and were being pushed over by backfill loadings in excess of design loads, in 1968 four steel beams were placed across the basin at the tops of the walls to act as struts (Appendix C, Photo No. 7). It was noted that granular soil is deposited on the channel bottom, which may have passed through the weep holes from the back of the wall, where loss of ground is evident.

An examination of the plans shows that the two stilling basin wall panels are cantilever walls, with a thin face wall cantilevered off the base. The wall panels upstream of the two cantilever sections are of much more rigid gravity wall construction and the end wall section downstream is rigidly supported by the buttress forming the control opening at the end of the stilling basin. It would thus be expected that the tops of the cantilever wall sections would deflect inwards because of cantilever action, while the sections above and below, being of more rigid construction, would deflect less. The left walls have deflected more than the right walls, which could be explained by greater backfill loadings occasioned by freeze and thaw action of water, which collected between the wall and backfill contact. A cursory examination showed no signs of distress in the walls and it is believed that they are in no danger of failing, either with or without the steel beam struts.

As was discussed in Section 1.3b, when spillway flows approach about 900 cfs, there is great danger of an overtopping of the left side chute wall where it converges into the direction of flow below the crest; and at the stilling basin, where the theoretical conjugate depth will be 10 ft., or to an average water surface 2 ft. below the tops of the walls (Appendix C, Photo No. 8). At flows exceeding about 1,400 cfs, the stilling basin walls will be overtopped; at about 2,700 cfs flow through the spillway, all walls will be overtopped.

Although no flashboards were on the spillway crest at the time of the inspection, outline stains of the boards were visible on the crest and holes have been drilled into the crest where such flashboards have been installed. According to Water Department personnel, these flashboards are installed each year in early spring and remain on the crest until fall, in order to capture additional runoff storage above spillway crest level and thereby increase the yield of the reservoir. The flashboards are 14 in. high, bolted to angle iron frame and supports inserted into the crest at their lower end and bolted to the upstream bridge beam at their top.

The use of flashboards on the spillway crest will significantly reduce the ability of the project to handle the larger magnitude floods, and is not consistent with the original design intent of the selected spillway capacity and reservoir surcharge and freeboard storage space. Once installed, it does not appear that the flashboards could be quickly removed in anticipation of a severe storm; and if suddenly removed while being overtopped, the flood wave occasioned by their removal would cause a severe flood wave down river, which would not be the case if gradual spills were released over the uncontrolled crest.

The inspection team was informed that in the past four years, only once did the reservoir rise above the tops of the boards, when several inches spilled over. In three of the last four years the reservoir storage reached above spillway crest level.

The concrete in the upstream curb of the spillway bridge deck is severely deteriorated, exposing the reinforcement. The original plywood forms on the underside of the deck between the steel beams have not been removed. These appear to trap water and roadway salts, leading to accelerated deterioration of the concrete. The bridge beams are badly rusted, presumably from the entrapment and dripping of water and roadway salts. It was also noted that the expansion joints at both bridge seats were open about 1½ in. in rather cool weather.

e. Reservoir Area

The reservoir shoreline and slopes upstream from the dam on both abutments are stable, with no evidences of slides or sloughing. The reservoir is in a restricted water supply preserve which is generally forested and no homes are constructed along the shoreline. There would be no damage upstream from the dam owing to a reservoir rise within the surcharge and freeboard space of the reservoir.

f. Downstream Channel

Spillway releases empty into Marsh Brook, which traverses a narrow valley for approximately one mile through the Town of East Plymouth, to merge with the Poland River near Terryville. Immediately downstream of the spillway stilling basin, the channel is somewhat overgrown with brush and saplings.

The Poland River continues through Terryville about one-half mile to merge with the Pequabuck River, which continues eastward through the City of Bristol. Many homes have been built near stream level along Marsh Brook in East Plymouth, and State Highway 72 follows along the left bank of Poland River for the entire reach from the Marsh Brook confluence to the Pequabuck. A constricted waterway exists under US Highway 6 and 202 crossing near the Poland-Pequabuck confluence, to limit river discharges at that point.

Discharges owing to an overtopping of the dike to the northwest of the dam would flow down a small tributary channel to the west of the main dam, which joins Marsh Brook about 1,500 ft. downstream from the dam. About five homes are built within the flow-way of this tributary channel, which would be affected by a spill over the dike.

### 3.2 Evaluation

The visual inspection of the dam and dike adequately revealed key characteristics as they may relate to its stability and integrity, permitting an assessment to be made of those features affecting the safety of the structure. The Old Marsh Pond Dam, the dike, and the appurtenant works are judged to be in generally good condition.

## SECTION 4 - OPERATIONAL PROCEDURES

### 4.1 Procedures

The Old Marsh Pond Dam is operated by personnel of the Bristol Water Department. Reservoir operation entails mainly the release of stored water from the reservoir as water supply needs warrant. Flashboards are fixed to the spillway each spring and removed each fall. No documented operating procedures have been prepared.

### 4.2 Maintenance of Dam and Dike

Little maintenance is required except for the periodic cutting of brush and other growth on the dam embankment, and restoration of displaced riprap. No documented maintenance instructions have been prepared, but routine maintenance appears to be regularly carried out.

### 4.3 Maintenance of Operating Facilities

All gate valves are said to be serviceable and inspected regularly, except for the upstream 24 in. dia. valve, which is normally submerged and no longer used. No specific maintenance program is in effect.

### 4.4 Warning System

There is no formal warning system or program at this dam.

### 4.5 Evaluation

The Old Marsh Pond Dam is of relatively modern construction, with simple operating devices, and therefore does not require detailed operating procedures. Maintenance involves periodic growth removal from the dam and dike, and surveillance regarding seeps, slope damage, animal burrows, etc. Inspection observations noted that the facility appears to be generally well maintained. A formal warning and emergency evacuation system should be developed.

## SECTION 5 - HYDRAULIC/HYDROLOGIC

### 5.1 Evaluation of Features

#### a. Design Data

##### 1. Reservoir Area and Capacity

The active reservoir capacity to normal water surface elevation 687.7 is indicated by the Bristol Water Department to be 589 million gallons, or about 1,808 acre-feet. The surface area measured from contours on the USGS map is about 183 acres. From these values the reservoir capacities below normal level were reconstructed and computed, and are shown on Figure 2, Sheet D-6 and Sheet D-7, Appendix D. For surcharge storage above normal reservoir level, areas were planimetered from contours delineated on the USGS 2,000 feet per inch quadrangle sheets. Area and capacity curves and tables, for use in flood routings, are also shown on Sheets D-6 and D-7.

##### 2. Flood Hydrology

Old Marsh Pond Dam is about 32 ft. high and impounds about 3,100 acre-feet of storage to top of dam, and as noted in Section 1.2c, is classified as intermediate in size. As noted in Section 1.2d, the hazard potential is classified as high. The Recommended Guidelines for Safety Inspection of Dams require that for hydraulic evaluation the dam adequacy be tested for floods of PMF magnitude.

Precipitation data was obtained from Hydrometeorological Report No. 30, which for the Connecticut area approximates 24.3 in. of 6 hour point rainfall over a 10 square mile area. This value was then reduced by 20 percent to allow for basin size, shape and fit factors. The 6-hour rainfall-duration curve of a total of 19.2 inches was then distributed and rearranged as suggested in Design of Small Dams. A constant loss factor of 0.1 inch per hour was deducted from the precipitation values to give the excess rainfall used to prepare an inflow hydrograph.

To prepare the inflow hydrograph a triangular incremental unitgraph was adopted, using a computed lag time value of about 1.5 hours to derive a time-to-peak for the triangular hydrograph of 1.5 hours (see computations on

Sheets D-8 and D-9, Appendix D). A PMF inflow hydrograph is shown on Figure 3 Sheet D-10, Appendix D, indicating a peak inflow of about 8,500 cfs or a CSM of about 3,600.

Flood routings were made of the PMF for two premises: first, for the present condition with the top of the dike at elevation 693.0; and second, for the condition where the dike had been raised 1.3 ft. so that its overtopping would not occur. Results of the routings are shown on Figure 4, Sheet D-11, and are summarized as follows:

	Present Condition Top of Dike at Elev. 693.0	Dike Raised With Top to Elev. 694.3
Maximum reservoir water surface	694.0	694.25
Maximum discharge thru spillway	3,940 cfs	4,175 cfs
Maximum total discharge over dam	0	306 cfs
Maximum unit discharge over dam	0	0.35 cfs/ft.
Total volume discharged over dam	0	12 A.F
Duration of overtopping of dam	0	1.4 hours
Maximum total discharge over dike	1,475 cfs	0
Maximum unit discharge over dike	2.8 cfs/ft.	0
Total volume discharged over dike	172 A.F	0
Duration of overtopping of dike	2.7 hours	0

From the above it can be seen that, with present conditions, for a PMF event the dike will be overtopped for a period of almost 3 hours, with a discharge over the dike of up to about 1,500 cfs and a total release of about 170 acre-feet; all assuming that the dike does not fail during the overtopping.

A flood routing of a 0.75 PMF through the reservoir and spillway results in a maximum surcharge to elevation 693.0, the top of the dike. This flood routing is shown on Figure 5, Sheet D-12. Floods in excess of this magnitude will result in an overtopping of the dike.

If the dike was raised about 1.3 feet to elevation 694.3 it would not be overtopped during a PMF event, while the dam would be overtopped by only 0.25 feet, with about a 300 cfs overflow for a total release of only about 12 acre-feet. In effect, the surcharge storage space and spillway capacity could handle about 95 percent of a PMF without an overtopping of the dam and dike. It is presumed that the

thinking behind the designer's decision to construct the dike 1 ft. lower than the dam was to provide an emergency spillway and thereby avoid an overtopping of the main dam. From the flood routing results it would appear that an overtopping of the dike could be much more serious than an overtopping of the main dam, which would point to the desirability of consideration of raising the dike to prevent such an overtopping.

It is to be noted that the dam has two core walls extending to bedrock across its entire length, with the top of the higher core wall being 3 ft. below crest level. If the crest of the dam were to erode owing to an overtopping, it would not be expected that a sudden failure for the entire height would occur, but rather at the failure would be slowed by the core wall. On this premise, it would be safer that the dam rather than the dike be overtopped in the event of a maximum flood.

The installation of the 14 in. flashboards to gain additional storage within the surcharge storage space reduces the capability of the spillway and remaining surcharge to accommodate higher magnitude floods. On Figure 7, Sheet D-14, Appendix D, a flood routing of a 0.5 PMF shows that, if the flashboards are left in place and the reservoir is at the top of the flashboards at the start of the flood event, a maximum reservoir level of about 692.9 is obtained, just short of the top of the dike. On this basis, it would appear that if the dam is to be safe for handling floods up to a PMF event, the storage of inflows above spillway crest and the use of the flashboards during potential flood periods should be discontinued.

While the spillway crest has sufficient capacity to discharge about 4,000 cfs with reservoir level to the top of the dam, the spillway chute and stilling basin have apparently been designed for about 900 cfs, with what was considered adequate freeboard to safely confine outflows within the chute and basin waterway. As noted in Section 3.1d, computations show that the stilling basin will flow full to top of walls at a discharge of 1,400 cfs, and that an overtopping of the chute walls would occur when discharges exceed about 2,000 cfs. Shown on the following page, as plotted from Figure 6, Sheet D-13, are the spillway outflows resulting from various magnitude flood routings, and the expected consequences resulting at the spillway chute and basin from such outflows:

<u>% of PMF</u>	<u>Spillway Outflow cfs</u>	<u>Spillway Channel Condition*</u>	<u>Remarks</u>
28	900	3 ft. or more wall freeboard in channel. 2.2 ft. wall freeboard in stilling basin.	Chute and basin will handle outflow with no wall overtopping threatened
40	1,400	About 1.5 ft. freeboard along chute walls. Stilling basin walls will overtop.	Stilling basin threatened with failure.
55	2,000	Left channel wall below crest will be overtopped. Stilling basin walls will be overtopped.	Overtopping of left chute wall will threaten dam left of spillway with washout and failure.
67	2,700	All walls will be overtopped, up to 4 feet at stilling basin.	Probable complete washout of chute and basin and undermining of dam embankment.

\*See channel hydraulic computations, Sheets D-4 and D-5, Appendix D.

b. Experience Data

No records have been retrieved in regard to past operation of the reservoir, or of surcharge encroachments and spills through the spillway. The maximum past inflows are not known.

c. Visual Observations

There is no visible evidence, either along the reservoir or in the downstream channel, to indicate high water levels or signs of major spillway outflows.

d. Overtopping Potential

For a test flood of full PMF, the reservoir surcharge will reach to elevation 694.0, the top of the main dam. At this head, the dike 1,000 feet north of the right abutment of the dam will be overtopped by 1 ft., with a maximum release over the dike of about 1,475 cfs. This amount of overtopping could cause serious erosion of the dike, and a breach failure would be a distinct possibility.

e. Drawdown Capacity

Drawdown of the reservoir is possible only through the 16 in. dia. line which leads to Bristol Reservoir No. 1. On the basis of an average discharge of about 8 cfs, evacuation would be limited to about 16 acre-feet per day. To empty the 1,800 acre-feet of active storage below spillway crest would require almost 4 months, assuming no inflow in the interim. In the event of an emergency, a rapid drawdown could be carried out only by a controlled breaching of the dam or dike.

f. Downstream Hazard

As discussed in Section 5.d, for a PMF test flood the dike will be overtopped by 1 ft. and a breach failure would be likely. The dike could also conceivably fail structurally, such as by piping or sloughing. On the basis of a trapezoidal gap failure, with a bottom width of 20 ft. at the level of the bottom of the dike, and with slopes at about 1.4 to 1 (angle of repose), a sudden release of a flood wave approaching 7,600 cfs could result. This outflow would reduce to about 4,000 cfs in about 80 minutes during which time about 650 acre-feet of storage would be spilled down Marsh Brook. (See computations on Sheet D-15, Appendix D.)

If the dam were to be overtopped and fail, or to be breached due to structural failure, and the breach is a rectangular gap (assuming 50 ft. of core wall collapses), a sudden release of a flood wave of about 20,000 cfs could result. This outflow would reduce to about 10,000 cfs in 80 minutes during which time about 1,700 acre-feet of storage would be spilled down Marsh Brook. (See computations on Sheet D-15, Appendix D.) It should be noted, however, that the dam has two core walls to varying levels, and it is therefore unlikely that a sudden breach such as is demonstrated above would develop from either an overtopping or from a piping or sloughing failure.

As noted in Sections 1.2d and 3.1f, a number of homes are located along the lower reach of Marsh Brook and a major State Highway traverses the Poland and Pequabuck river below the Marsh-Poland confluence. Stage-discharge computations at a stream section downstream from the populated area on Marsh Brook and below the confluence of the Poland-Pequabuck Rivers show that flood stages of up to 20 ft. could prevail for a 20,000 cfs flood wave from the reservoir. (See computations on Sheet D-16.) Since the river channel storage in this reach of Marsh Brook and Poland River are small, the flood wave would be only slightly diminished until it passed a considerable distance down the Pequabuck River.

Delineated on Figure 8, Sheet D-17, (quad sheet graphic) are the areas which could be flooded by a breach failure of the dam or dike described above.

## SECTION 6 - STRUCTURAL STABILITY

### 6.1 Evaluation of Structural Stability

#### a. Visual Observation

The field investigations of the embankment revealed no significant displacement or distress which would warrant the preparation of slope stability computations based on assumed soil properties and engineering factors.

However, while the dam is in good condition, the visual observations described in Section 3 indicate that attention should be given to a few items, which are noted in Section 7.3.

#### b. Design and Construction Data

Plans for construction of the original dam are unavailable, although the owners of the dam in 1943, the Andrew Terry Co., were then in possession of a plan of the dam prepared by Sperry and Buel, "engineers in charge of building the dam". In 1950, plans for the Bristol Board of Water Commissioners were prepared by Metcalf and Eddy to raise the dam about 10 ft. with an improved road on the crest, and to construct a new closure dike.

The design of the new dam and dike appear generally consistent with good earth dam embankment design practice at the time. The dam has a concrete core wall anchored 2 ft. deep in sound rock or impervious stratum, an ogee spillway and discharge channel, and intercepting toe drains. The dam is of impervious material, with a pervious shell downstream on a coarse sand filter. Hand placed 18-in. riprap was called for on the crest of the old dam, with heavy random riprap above that elevation. The upstream slope of the old dam was flattened and riprapped prior to superimposition of the new construction.

An apparent leaning in of the spillway stilling basin walls led to the installation, in 1968, of four steel beam struts between the tops of the walls. Since the plans show these walls to be of cantilever design, whereas the adjacent chute walls are of gravity design, it is believed that some differential deflection can be expected and that these walls are in no danger of failing, either with or without the struts.

Pertinent drawings are included in Appendix B.

c. Operating Records

Records are kept by the Bristol Water Department. A rain gauge is maintained at the filter plant about 1 mile downstream from the dam.

d. Post Construction Changes

Designs for the raising of the original dam and reconstruction of the original dike were well conceived, were in accord with accepted practice of the period, and would not adversely affect stability or integrity.

e. Seismic Stability

The dam is located in Seismic Zone No. 1, and in accordance with Phase I guidelines, does not warrant seismic analysis.

## SECTION 7 - ASSESSMENT, RECOMMENDATIONS & REMEDIAL MEASURES

### 7.1 Dam Assessment

#### a. Condition

On the basis of the Phase I visual examination, both Old Marsh Pond Dam and the closure dike appear to be in good condition and functioning adequately. The deficiencies revealed are not of major concern, but tend to indicate that a small amount of additional routine maintenance is required, and that the fairly heavy seepage should be monitored frequently.

The closure dike is 1 ft. lower than the main dam. The reservoir cannot be drawn down rapidly because the blowoff on the 16 in. dia. outlet pipe to the filter plant is only 6 in. dia. The spillway crest capacity is only sufficient to accommodate about 75% of the PMF test flood without overtopping the dike.

At the test flood outflow, the spillway chute and stilling basin walls will all be overtopped. At a 0.5 PMF outflow, the stilling basin walls will be overtopped and the left channel wall below the crest will be on the verge of overtopping. Overtopping of the walls would result in washouts of backfill along the walls, and could result in the collapse of the entire spillway chute and stilling basin. A washout of the spillway channel so close to the dam could threaten a washout of the dam for inflows of much less than the test flood.

#### b. Adequacy of Information

The information recovered is considered adequate for the purpose of making an assessment of the performance of the dam.

#### c. Urgency

The recommendations and remedial measures enumerated below should be implemented by the owner within two years after receipt of the Phase I Inspection Report.

#### d. Need for Additional Investigation

The visual inspection identified a number of potential problems. Additional investigations are required as recommended in Para. 7.2.

## 7.2 Recommendations

It is recommended that the owner should retain the services of a competent registered professional engineer to make investigations and studies of the following items, and, if proved necessary, design appropriate remedial works:

1. Review the situation regarding the dam overtopping versus the dike overtopping, and determine whether the dike should be raised.
2. Review spillway flow conditions below the crest of the dam at the converging wall, and determine whether modifications are required to improve flow and increase carrying capacity in the chute, and to forestall failure from wall overtopping.
3. Review the use of flashboards on the spillway crest and determine the feasibility of either eliminating their use altogether, or modifying them to facilitate quick removal in anticipation of a storm.
4. Determine the source of leakage producing the swampy area below the dam. Since the thread of the seepage stream coincides with the line of the outlet pipe and its associated gates and valves, it is advisable to inspect carefully each valve and gate chamber, including those abandoned, for possible contributing sources to the leakage. While it is probable that most of the seepage issues from the rock toe drain, faced by the supplementary peastone, the presence of other channels which may have been induced by reconstruction cannot be discounted. Confining the seepage to a constructed, well-defined channel would be useful for monitoring, would improve soil conditions at the toe, and would prevent further damage from freeze-thaw cycles to the wall which extends from the right stilling basin wall.
5. Investigate possible causes of rotation and/or deflection of spillway chute wall, and examine corrective alternatives.
6. Provision of a new outlet facility to permit rapid drawdown of the reservoir.

### 7.3 Remedial Measures

The owner should take the following actions:

1. Monitor on a monthly basis the fairly heavy seepage at the center of the dam, noting changes in volume and turbidity.
2. Monitor once per month the marshy area to the right of the maintenance building access ramp, at the toe of the slope.
3. Restore riprap in the vicinity of the spillway's upstream walls.
4. Control growth on the right downstream slope.
5. Fill animal burrows, and patrol the downstream slope frequently to detect and eliminate future infestations.
6. Repair the top of the concrete wall which extends from the right stilling basin wall.
7. Remove the plywood forms from the underside of the bridge deck; make all necessary repairs; paint all steelwork.

#### a. Operation and Maintenance Procedures

The owner should institute procedures for a biennial periodic technical inspection of the dam and appurtenant works, with supplementary inspections for any suspect items. A checklist for periodic inspections should be developed and records should be kept of all maintenance and repair work performed. A formal surveillance, flood warning and emergency evacuation plan should also be developed.

### 7.4 Alternatives

There appear to be no practical alternatives to the above recommendations.

APPENDIX A  
VISUAL INSPECTION CHECKLIST

VISUAL INSPECTION  
PHASE I

Identification No. CT 00285      Name of Dam: Old Marsh Pond Dam

Date of Inspection: 9 November 1978

Weather: Clear      Temperature: 55°F

Pool Elevation at Time of Inspection: 684 MSL

Tailwater Elevation at Time of Inspection: Not applicable

INSPECTION PERSONNEL

Pasquale E. Corsetti	Louis Berger & Associates, Inc.	Acting Proj. Manager
Carl J. Hoffman	Louis Berger & Associates, Inc.	Hydraulics, Structures
Thomas C. Chapter	Louis Berger & Associates, Inc.	Hydrology, Soils
James H. Reynolds	Goldberg Zoino Dunicliff & Associates, Inc.	Soils

OWNER'S REPRESENTATIVE

John M. Knibbs	City of Bristol	Assistant Superintendent, Water Department
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VISUAL INSPECTION CHECKLIST

Identification No. CT 00285      Name of Dam: Old Marsh Pond Dam      Sheet 1

VISUAL EXAMINATION OF

OBSERVATIONS AND REMARKS

EMBANKMENT

Vertical alignment and movement

Alignment of dam and dike good. No movement observed.

Horizontal alignment and movement

Alignment of dam and dike good. No movement observed.

Unusual movement or cracking at or near the toe

None observed.

Surface cracks

None observed.

Animal burrows and tree growth

Two fairly large burrows about mid-dam, mid-slope on d/s face.

Sloughing or erosion of slopes

Minor sloughing on upstream face near spillway walls.

Riprap slope protection

Generally good condition. Some displaced downslope at spillway walls.

VISUAL INSPECTION CHECKLIST

Identification No. CT 00285

Name of Dam: Old Marsh Pond Dam

Sheet 2

VISUAL EXAMINATION OF

OBSERVATIONS AND REMARKS

Seepage

Marshy area, toe of dam, right of access ramp. Heavy seepage from 10 ft. zone at toe drain, 150 ft. right of spillway; marshy zone with free water extends to end of spillway channel; total combined flow about 5 gpm.

Piping or boils

None noted.

Junction of embankment and abutment, spillway and dam

Minor sloughing and displacement of riprap each side of spillway.

Foundation drainage

Granular filter zone under permeable shell; intercepting granular drains; cross underdrains under spillway channel; toe drains.

OUTLET WORKS  
Approach channel

None.

Outlet conduit concrete surfaces

None.

Intake structure

Concrete valve chamber on u/s slope, condition good.

VISUAL INSPECTION CHECKLIST

Identification No. CT 00285

Name of Dam: Old Marsh Pond Dam

Sheet 3

VISUAL EXAMINATION OF

OBSERVATIONS AND REMARKS

Outlet structure

None.

Outlet channel

None (16 in. dia. buried pipe).

Drawdown facilities

16 in. dia. pipe to Reservoir No. 1 one mile d/s, with 6 in. dia. blowoff to spillway stilling basin.

SPILLWAY STRUCTURES  
Concrete weir

Good condition.

Approach channel

None.

Discharge channel

Converging chute with concrete walls and retarding sill; good condition.

Stilling basin

Hydraulic jump type with concrete walls which have deflected inwards and are braced with 4 steel beam struts.

VISUAL INSPECTION CHECKLIST

Identification No. CT 00285

Name of Dam: Old Marsh Pond Dam

Sheet 4

VISUAL EXAMINATION OF

OBSERVATIONS AND REMARKS

Bridge and piers

Plywood deck forms not removed; concrete curbs deteriorated; exp. joints open 1½"; steel work corroded.

Control gates and operating machinery

None.

INSTRUMENTATION

Headwater and tailwater gages

None.

Embankment instrumentation

None.

Other instrumentation

None.

RESERVOIR

Shoreline

Gentle slopes, wooded, stable.

Sedimentation

None observed.

Upstream hazard areas in event of backflooding

None.

VISUAL INSPECTION CHECKLIST

Identification No. CT 00285 Name of Dam: Old Marsh Pond Dam Sheet 5

VISUAL EXAMINATION OF OBSERVATIONS AND REMARKS

Alterations to watershed affecting runoff None apparent.

DOWNSTREAM CHANNEL  
Constraints on operation of dam None.

Valley section Wide, natural.

Slopes Gentle, wooded.

Approx. No. of homes/population At least 15 homes in E. Plymouth and Terryville close to channel.

OPERATION & MAINTENANCE FEATURES  
Reservoir regulation plan, normal conditions No formal plan. 14 in. flashboards added in spring, removed in fall, for additional storage.

Reservoir regulation plan, emergency conditions No formal plan.

Maintenance features Brush cut, grass mowed, animal burrows filled, etc.

APPENDIX B  
PLANS & RECORDS

Plans obtained from Water Department, City of Bristol, CT:

Plan, Profile & Typical Section of Dam	Sheet 2 of 19
Existing Low-Level Water Supply Main and Existing Spillway	Sheet 6 of 19
Bridge & Adjacent Spillway; General Plan	Sheet 8 of 19
Bridge & Adjacent Spillway; Plan & Elevation of East Abutment	Sheet 9 of 19
Bridge & Adjacent Spillway; Misc. Details	Sheet 13 of 19
Spillway Channel & Walls; Plan, Profile & Typical Section	Sheet 15 of 19
Spillway Channel & Walls; Misc. Details Vicinity of Outlet	Sheet 16 of 19
Plan, Profile & Sections of Dike	Sheet 18 of 19
Borings & Test Pits	Sheet 19 of 19

Memorandum to General Wadhams:

October 19, 1943  
William S. Wise

Re: Old Marsh Pond  
Plymouth-Bristol  
Owned by Andrew Terry Co.  
Terryville, Conn.

On October 18, I discussed with Mr. George Clark of the Andrew Terry Co. the proposal for using this pond for additional water supply for the City of Bristol. We looked over Old Marsh Pond and also the proposed site for a new reservoir for Bristol.

Mr. Clark's story regarding this proposal is as follows:

A few weeks ago Mr. Wooding of the Bristol Water Co. discussed with Mr. Clark the building of a new reservoir which is very badly needed to supplement their water supply. The site has been selected, (I do not know by whom), and the estimated cost of building the dam was \$275,000 pre-war figures. Mr. Clark did not know whether this included clearing of the reservoir site. Mr. Clark was very much disturbed by the suggestion of spending \$275,000 and more for the construction of a reservoir on a small brook which is dry most of the summer; it certainly was during our visit. Mr. Clark therefore suggested the purchase of Old Marsh Pond from the Andrew Terry Co. because of its excellent location and good condition and because it is no longer used by this company for power. Mr. Wooding indicated that this pond had never been considered for this purpose.

Mr. Clark has a plan of this pond showing all the properties around it. The company owns all of the property except two or three small pieces. They apparently do not own all of the watershed above it but this is all undeveloped land.

At present the gate to the pond is partly open and the City of Bristol has built a small temporary dam about a mile downstream from which the water is pumped into one of the reservoirs. So, the city at present is using Old Marsh Pond water by consent of the Andrew Terry Co.

Old Marsh Pond water surface covers an area of about 100 acres and has a storage capacity of about 166,000,000 gals. The dam and spillway was raised four feet in 1912. At that time, the core wall was designed so that it could be raised another four feet when desired. This increase would probably add another 220,000,000 gallons storage. The raising of the dam would not be very expensive.

This pond, according to Mr. Clark, has a drainage area of about 2 1/2 sq. mi. but apparently is well fed by springs because the water never drops much even during dry weather. This year the water has been flowing over the spillway until the latter part of July, and now with the gate partly open, for some time,

176,418,000 gal

10/21/43 Mr. Geo. Clark and Mr. Budgett came in to discuss the pond. Mr. Clark stated he should have an appraisal made. We are to look for plans of Marsh Pond at Ubrava.

is only about 8" below the ...

The pond is only about 1/2 mile from Bristol No.2 Reservoir and, according to Mr. Clark, at an elevation so that the water could be discharged into No. 2 by gravity.

The proposed new dam is located on a small stream, apparently with a small drainage area, according to appearances. It was estimated that it would have a storage capacity of 225,000,000 gals. when full.

Mr. Clark employed Mr. Blodgett for legal advice concerning the proposal to use Old Marsh Pond and also try to get some idea of its value. Mr. Blodgett suggested getting an appraiser from Philadelphia to determine its value. This of course would be more than an ordinary appraisal job because its value would be based on the weighted advantages, disadvantages and cost of developing a new supply.

"It is unsafe to make any recommendations concerning this problem from such a very rough investigation." But, on the surface, it would appear that the use of Old Marsh Pond for water supply should be given some consideration if it has not yet been done. The advantages would appear to be - from this rough investigation:

- (1) It could be put into use in a comparatively short time.
- (2) The pond has been in use for a long time so that it has been seasoned and the water has lost the high color, usual in a new reservoir.
- (3) It now has a storage capacity nearly equal to the proposed new reservoir and if the dam was raised it would have a total capacity of 385,000,000 gallons as compared to 225,000,000 gallons for a new one.
- (4) It might be purchased and the dam raised for less cost than a new one.
- (5) Apparently the watershed would not be difficult to control from a sanitary standpoint.

At present the Andrew Terry Co. permits bathing in the pond just as a courtesy to local people. Two of the small property owners rent boats for fishing and recreation. The Fish and Game Commission stocks the pond but the lease expires with termination of present ownership.

The only conclusion that can be drawn is that if this pond has never been considered as a source of additional water supply, it has merits worthy of consideration. There may be disadvantages that have not appeared in the discussion.

Respectfully submitted,

William S. Wise, Chief Engineer

EDGAR L. POND  
PRESIDENT

GEORGE C. CLARK  
VICE PRESIDENT & TREAS.

DWIGHT W. POND  
SECRETARY

# The Andrew Terry Co.

MANUFACTURERS OF

## ELECTRIC CONDUIT FITTINGS

Terryville, Conn.

December 11, 1943

General Sanford H. Wadhams  
Room 317, State Office Building  
Hartford, Conn.

Dear General Wadhams:

I was much interested in your letter of the 10th and wish to thank you for it and also thank you for your efforts in having this matter placed on the agenda for the special session of the General Assembly.

We have handed a plan of a dam to Bristol which we think is the correct plan of the dam at the Old Marsh. At any rate, they can ascertain the correctness of same by getting in touch with Nicholas Fogg of Niantic, formerly employed by Sperry and Buel, and who was the engineer in charge of the building of the dam.

Yours very truly,

ANDREW TERRY CO.



George C. Clark  
President

GCC:mfb

**RECEIVED**  
DEC 13 1943

STATE WATER COMMISSION

12/13/43

*Note: If Bristol purchases Old Marsh Pond it seems probable that the Board of Supervision of Dams will ask for approval to raise the dam by perhaps four feet. In that case the plans showing details of construction will be very helpful.*

*(Signature)*

EDGAR L. BOND  
PRESIDENT

GEORGE C. CLARK  
VICE PRESIDENT & TREAS.

DWIGHT W. BOND  
SECRETARY

# The Andrew Terry Co.

MANUFACTURERS OF

**ELECTRIC CONDUIT FITTINGS**

**Terryville, Conn.**

**RECEIVED**

DEC 20 1943

STATE WATER COMMISSION

December 18, 1943

General Sanford H. Wadhams  
Room 317, State Office Building  
Hartford, Conn.

Dear General Wadhams:

The Superintendent of the Bristol Water Company dropped into my office yesterday and said that they had found a detailed plan of the dam at the Old Marsh in the effects of Mr. Buell.

Metcalf and Eddy had looked the thing over and the only fault they find is a small growth of underbrush on the lower side of the dam.

The only thing holding up the deal now is an infected toe of Bristol's Corporation counsel. We believe that a certain degree of professional efficiency will obliterate all these defects soon.

Yours very truly,

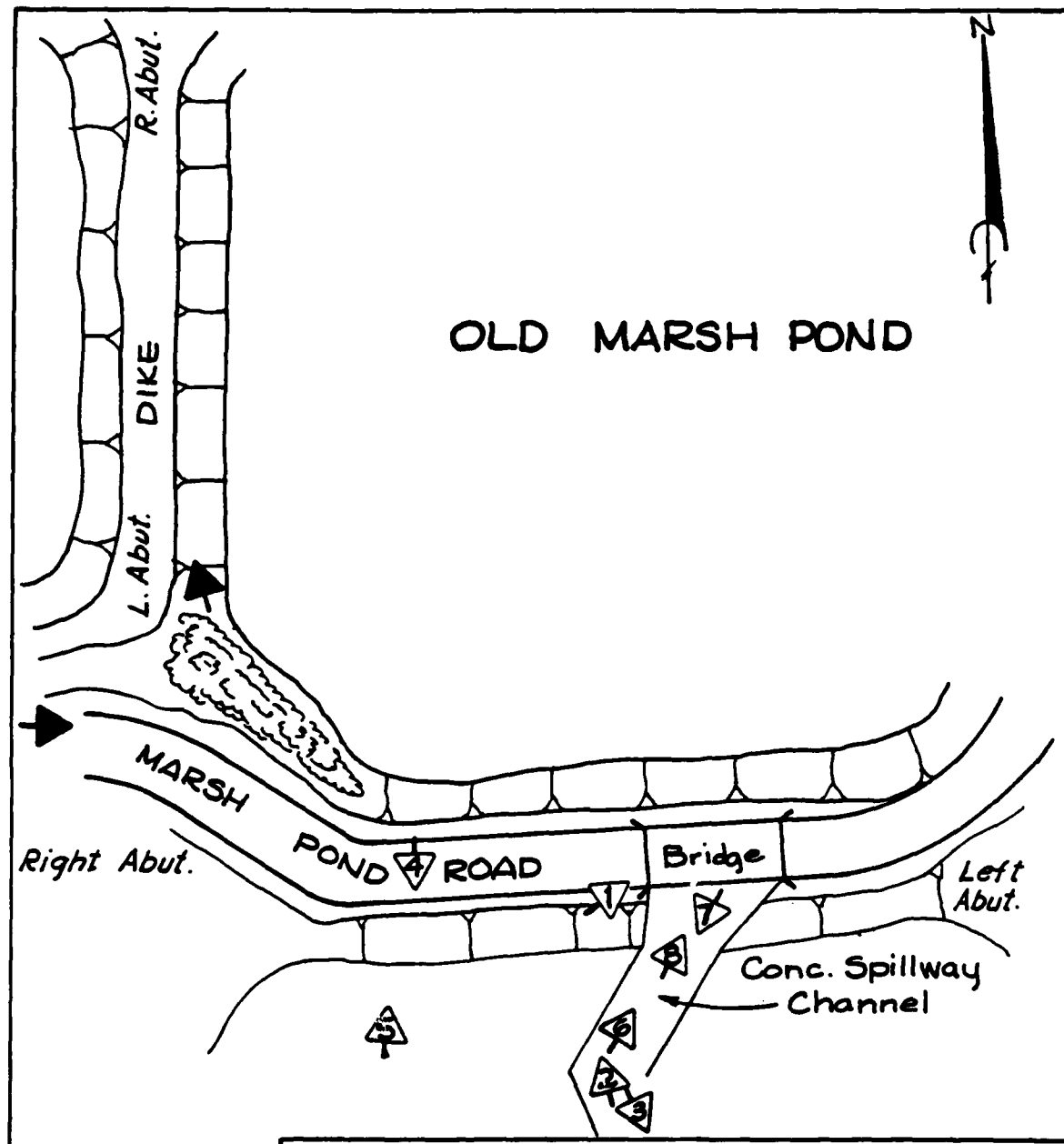
ANDREW TERRY CO.





George C. Clark  
President

GCC:mfb

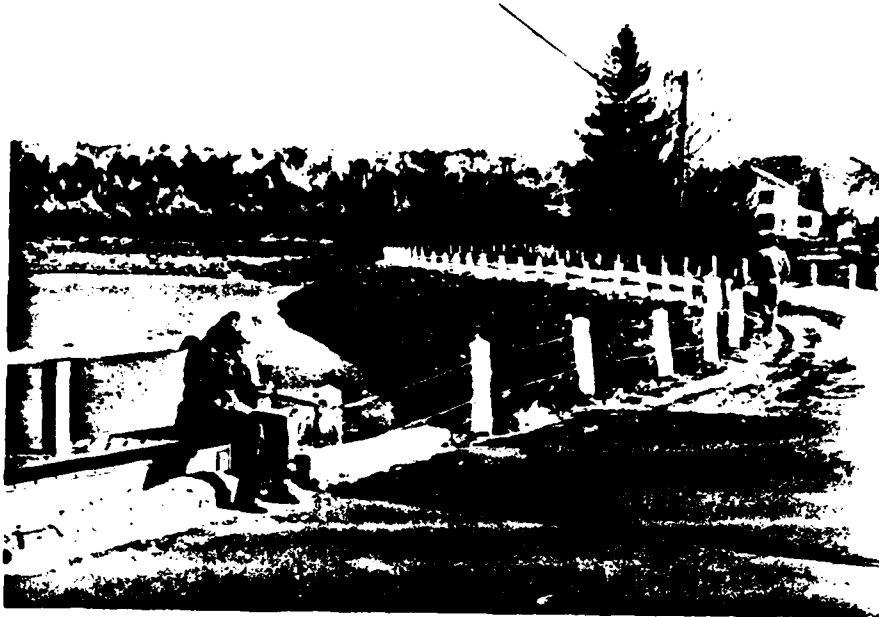
APPENDIX C  
SELECTED PHOTOGRAPHS



 Appendix "C" Photos  
 Overview Photos

LOUIS BERGER & ASSOC., INC WELLESLEY, MASS. ARCHITECT · ENGINEER	U.S. ARMY ENGINEER DIV. NEW ENGLAND CORPS OF ENGINEERS WALTHAM, MASS.
<b>NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS</b>	
<b>OLD MARSH POND DAM</b>	
<b>SKETCH PLAN SHOWING LOCATION &amp; ORIENTATION OF PHOTOS</b>	
<b>STATE - CT.</b>	
	SCALE 1:24000
	DATE

OLD MARSH POND DAM

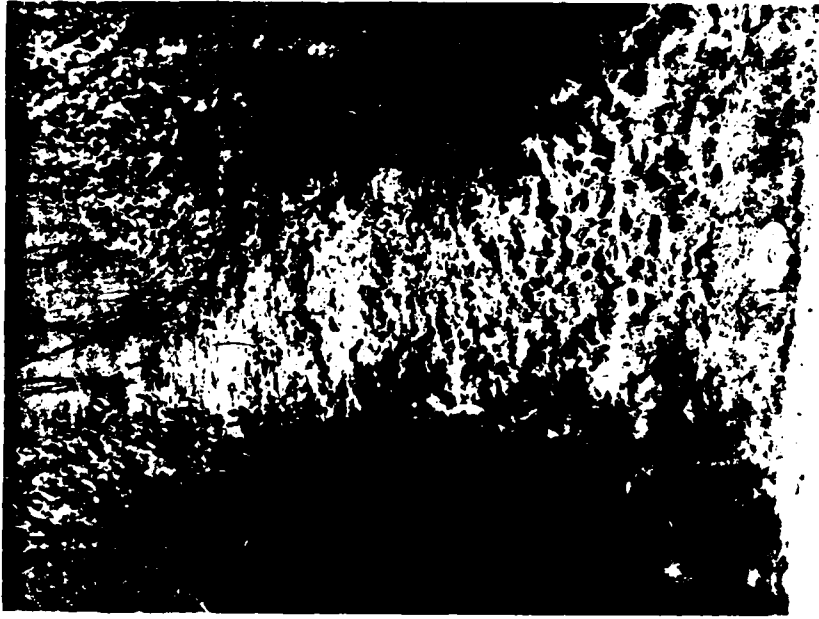


1. Upstream slope left of spillway bridge.



2. Seepage over wall below stilling basin.

OLD MARSH POND DAM



4. Valve chamber at downstream toe and seepage in old stream bed.



3. Downstream channel from spillway stilling basin.

OLD MARSH POND DAM

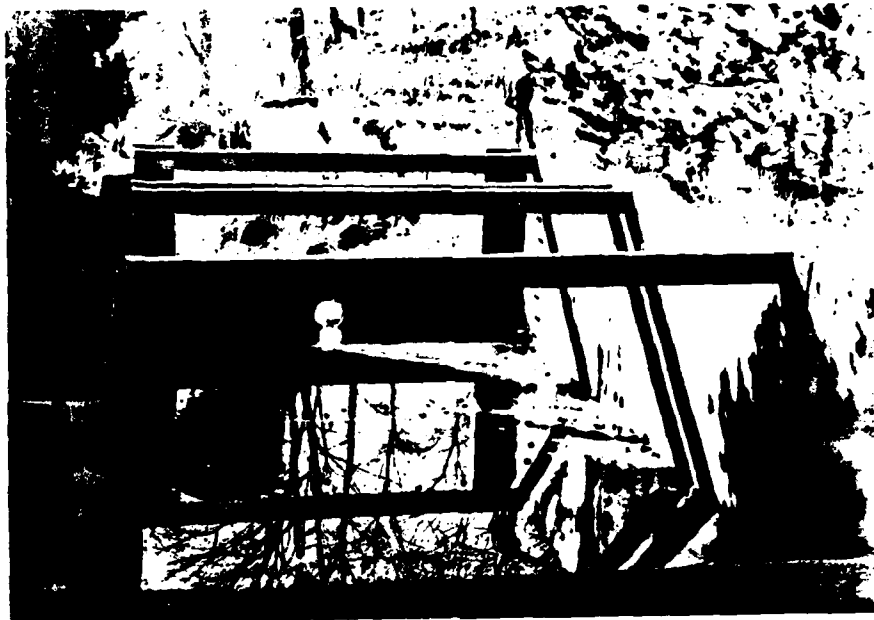


5. Animal burrow on downstream face.



6. Spillway channel and stilling basin.

OLD MARSH POND DAM



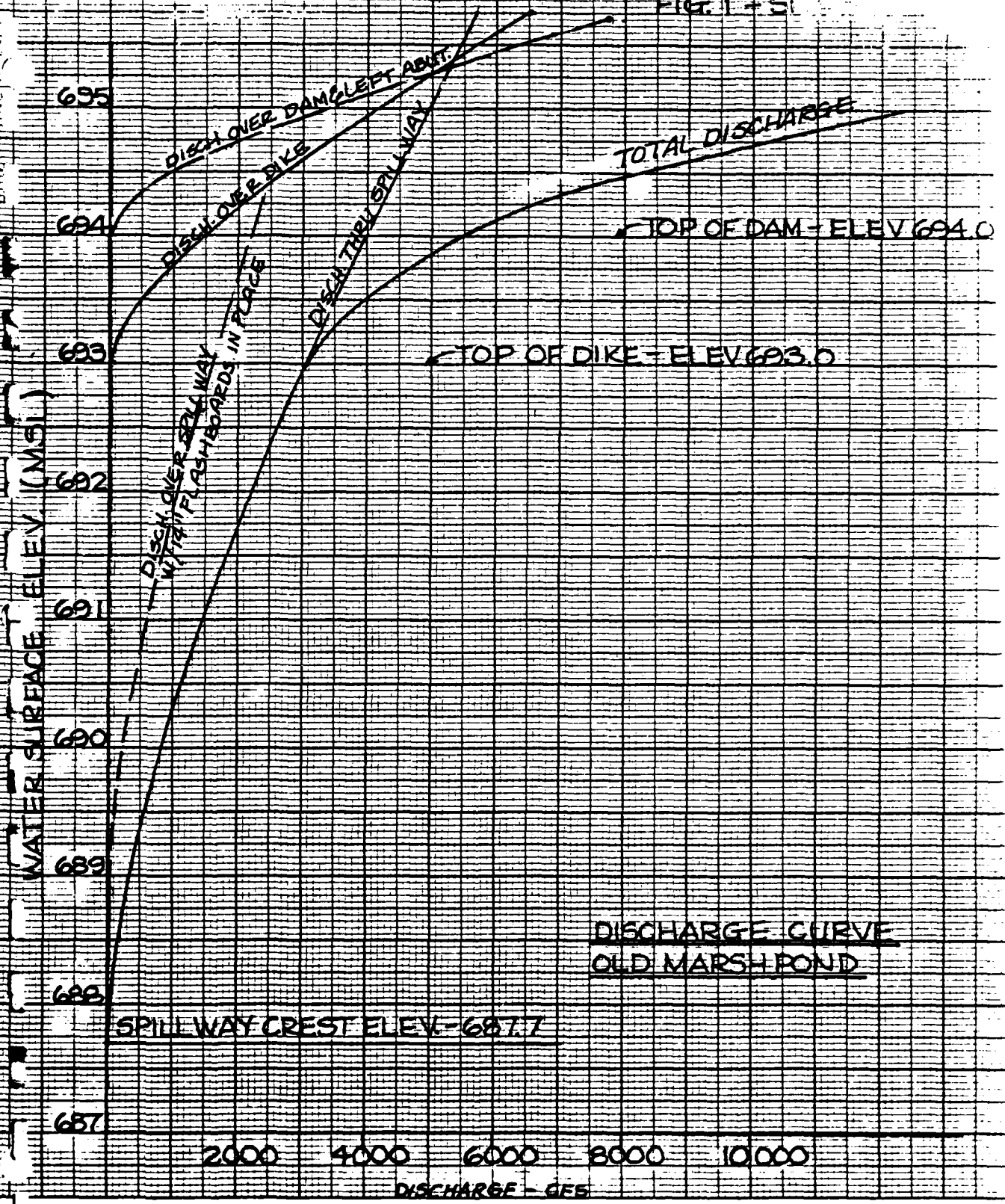
7. Spillway stilling basin.



8. Spillway channel with retarding sill.

APPENDIX D  
HYDROLOGIC & HYDRAULIC COMPUTATIONS

FIG. 1 - S.I.



K&E STANDARD CROSS SECTION  
10 X 10 TO THE HALF INCH

D-1

BY REC DATE 12/14/70

BERGER & ASSOCIATES INC.

SHEET NO. 2-2 OF

CHKD. BY DATE

INSPECTION

PROJECT W.189

PROJECT OLD MARSH PCND #15

Assum  $r_1 = 3'$   $r_2 = 8'$

Assum  $C = 2.4$   $q = CH^{3/2} = 20.55/4$

Capit  $V = 2.5$   $h_a = 0.10$

$\frac{h_a}{H_0} = \frac{0.1}{3} = 0.033$

From Fig 247  $n = 1.765$   $k = .57$   $x = 4.5$   $y = 3.58$

$$\frac{3.58}{H_0} = .57 \left( \frac{4.5}{H_0} \right)^{1.765}$$

$$0.63 = \frac{4.5}{H_0} \left( \frac{4.5}{H_0} \right)^{1.765}$$

$$6.63 = \frac{(4.5)^{2.765}}{H_0^{2.765}}$$

$$H_0^{2.765} = 2.145$$

$$H_0 = 2.71$$

∴ Design head = 2.71 Say 2.70' @ EL 690.4

Fig 249 22 Small Dams

$\frac{P}{H_0} = \frac{5}{2.7} = 1.85$  Coeff for design head = 3.93

Fig 251 For 3:3 slope reduction  $Q = 3.93 \cdot H_0^{3/2} = 3.90$

Spillway  $L = 60'$

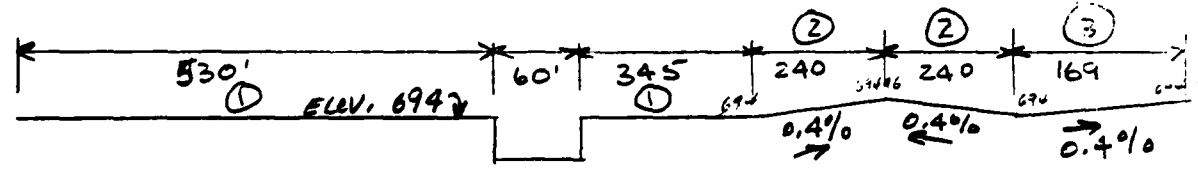
Elev.	H	$\frac{H_2}{H_0}$	$\frac{C}{C_0}$	$C$	A.Q	-Main Dam $k = 875$	Left Abut	Dike	Total		
687.7	0	-	-	-	-	4	0	0	0		
688.2	0.5	.20	.85	3.34	71				71		
688.7	1.0	.4	.9	3.52	212				212		
689.2	1.5	.6	.94	3.70	408				408		
689.7	2.0	.8	.97	3.81	647				647		
690.2	2.5	1.0	1.0	3.93	932				932		
690.7	3.0	1.2	1.03	4.05	1263				1263		
691.2	3.5	1.4	1.05	4.13	1623				1623		
691.7	4.0	1.6	1.07	4.15	1992				1992		
692.2	4.5			4.15	2377				2377		
692.7	5.0			4.15	2784				2784		
693.7	6.0			4.15	3660			863	4523		
694	6.3			4.15	3937	0	0	1473	5410		
694.7	7.0				4612	0.7	2.5	1435	426	3265	9312
695.7	8.0				5634	1.7	2.8	5431	2307	6534	17599

BY: PEC DATE: 1/8/79  
 CHKD. BY: DATE: \_\_\_\_\_  
 SUBJECT: \_\_\_\_\_

LOUIS BERGER & ASSOCIATES INC.  
 OLD MARSH POND

SHEET NO. \_\_\_\_\_  
 PROJECT: \_\_\_\_\_

DISCHARGE - Cont.



ELEV	H	C	S/FT	② - Twice AV. Q/FT DQ			③ - AV. Q/FT DQ			Dike L=526		
				L	AV. Q/FT	DQ	L	AV. Q/FT	DQ	H	C	DQ
687.7												
688.2												
688.7												
689.2												
689.7												
690.2												
690.7												
691.2												
691.7												
692.2												
692.7									665.0			
693.7									0.7	2.8	863	
694.0	0	2.8	0				0		1.0	2.8	1473	
694.7	0.7	2.8	1.64	175	0.82	287	169	0.82	139	1.7	2.8	3265
695.7	1.7	2.8	6.21	240	3.93	1643	169	3.93	664	2.7	2.8	6534

BY LOI DATE 1-19-79

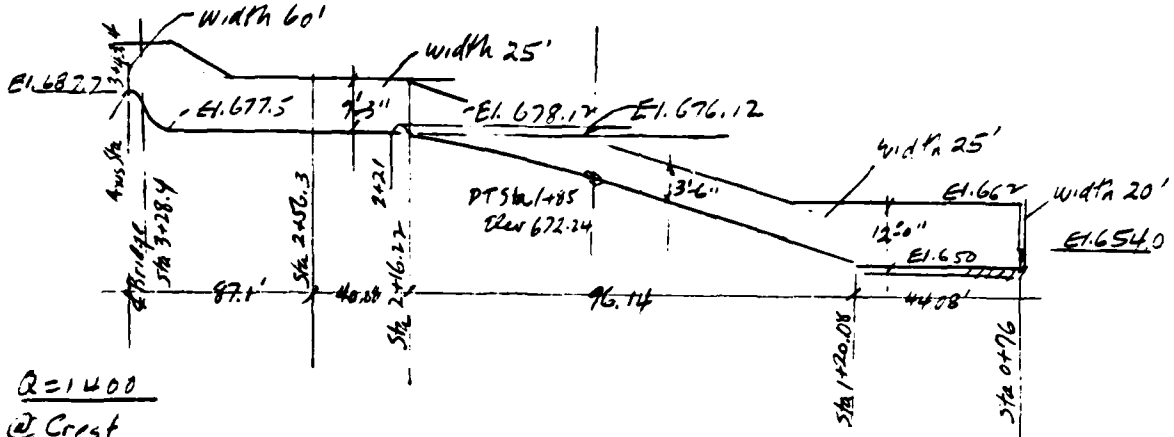
LOUIS BERGER & ASSOCIATES INC.

PROJECT NO. D-4

CHKD. BY \_\_\_\_\_ DATE \_\_\_\_\_ INSPECTION OF DAMS - CONN. & R.I.

PROJECT \_\_\_\_\_

SUBJECT OLD MARSH POND DAM - SPILLWAY HYDRAULICS



Q = 1400

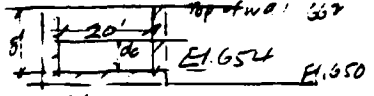
@ Crest

H = 3.3' Res. WS EL 691.0

@ Sta 2+22 Bottom EL 678.17  $d+1.2h_v = 691 - 678.17 = 12.88$   $w = 25'$   
 $d = 2.36$   $A = 59.0$   $v = 23.73$   $h_v = 8.74$   $1.2h_v = 10.49$   $d+1.2h_v = 12.85$  OK  
 Wall ht. 5'-3" Freeboard = 2.89'

@ Sta 1+85 Bottom EL 672.24  $d+1.2h_v = 691 - 672.24 = 18.76$   $w = 25'$   
 $d = 1.86$   $A = 46.5$   $v = 30.11$   $h_v = 14.07$   $1.2h_v = 16.89$   $d+1.2h_v = 18.75$  OK  
 Wall ht 3'-6" Freeboard 1.64'

@ Stilling basin

Down stream end sill   
 Critical flow over end sill Gradient EL. 662

$d_c = 2h_{vc}$   $d_c + h_{vc} = 8'$   $h_{vc} = 2.67$   $v_c = 13.10$   $A = 533 \times 26 = 106.7$   
 $Q_c = 1397$  cfs

Depth in basin upstream  $d+h_v = 12'$   
 $d = 11.65'$   $A = 291.21$   $v = 4.81$   $h_v = 0.35'$  OK

Conjugate depth of Jump.  $H_T = 691 - 662 = 29'$   
 $q = \frac{1400}{25} = 56$  cfs/ft.  $d_2 = 12'$  TWEL 662.

Q = 4000

@ Crest

H = 6.3 Res WS EL 694.0

@ Sta 2+22 Bottom EL 678.12  $d+1.2h_v = 694 - 678.12 = 15.88$   $w = 25'$   
 $d = 7.51$   $A = 187.75$   $v = 21.30$   $h_v = 7.05$   $1.2h_v = 8.46$   $d+1.2h_v = 15.96$  OK  
 Wall ht = 5'-3" Overtopping 2.25'

@ Sta 1+85 Bottom EL 672.24  $d+1.2h_v = 694 - 672.24 = 21.76$   $w = 25'$   
 $d = 5.40$   $A = 135$   $v = 29.63$   $h_v = 13.63$   $1.2h_v = 16.36$   $d+1.2h_v = 21.76$  OK.  
 Wall ht = 3'-6" Overtopping 1.96'

DATE 1-9-79

LOUIS BERGER & ASSOCIATES INC.

SHEET NO. D-5 OF

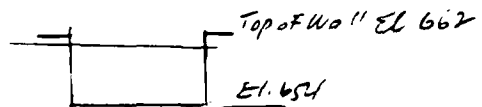
DATE

INSPECTION OF DAMS - CONN. & R.I.

PROJECT

OLD MARSH POND DAM - SPILLWAY HYDRAULICS

Q = 4000 cfs  
 @ Stilling basin



Downstream end sill

$Q_c = 4000$      $Q_c = 3.087 b H_{Ec}^{3/2}$      $H_{Ec} = \left( \frac{4000}{20 \times 3.087} \right)^{2/3} = 16.1'$   
 In basin  $d + h_v = 16.1$      $d = 14.0$      $A = 350$      $v = 11.43$      $h_v = 2.03$      $d + h_v = 16.33$  ck

Conjugate depth of jump  $H_T = 694 - 664 = 30'$

$q = \frac{4000}{25} = 160 \text{ cfs/ft}$      $d_2 = 21'$   
 $\therefore$  TW depth =  $\frac{14}{21} = 0.67\%$  of conj. depth.  
 Sweepout of basin is probable.

Q = 4700 cfs. (Max outflow for PMF)

Downstream end sill @ Critical depth

$H_{Ec} = \left( \frac{4700}{20 \times 3.087} \right)^{2/3} = 18.0'$   
 @ EL 672

In basin  $d + h_v = 18'$      $d = 15.8$      $A = 395$      $v = 11.9$      $h_v = 2.2$   
 Basin WS EL 665.8

Conjugate depth of jump  $H_T = 694.25 - 665.8 = 28.45$

$q = \frac{4700}{25} = 188$      $d_2 = 22$   
 $\therefore$  TW depth =  $\frac{15.8}{22} = 0.72\%$  of conj. depth  
 Sweepout of basin is probable.

SPILLWAY DISCHARGE OVER 14" FLASHBOARDS ON CREST

ELEVATION	H	C	Q cfs
687.7	0		
688.87	0		0
689.0	0.13	3.30	9
690.0	1.13	3.30	238
691.0	2.13	3.28	612
692.0	3.13	3.24	1076
693.0	4.13	3.20	1611
694.0	5.13	3.16	2203.

3-27

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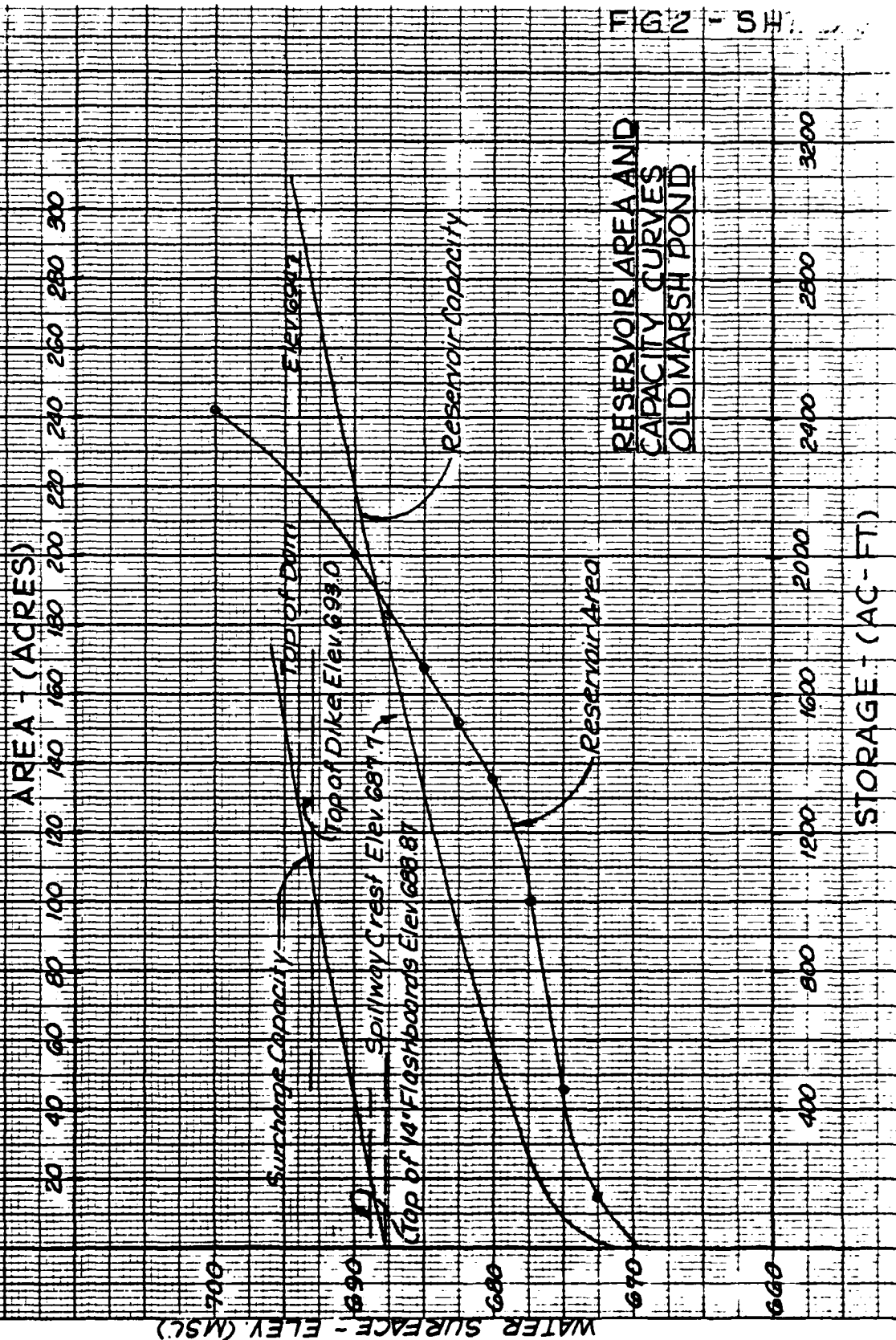


FIG. 54

RESERVOIR AREA AND  
CAPACITY CURVES  
OLD MARSH POND

STORAGE - (AC - FT)

WATER SURFACE - ELEV. (MSL)

MUFFEL & ESSER CO.  
MADE IN U.S.A.

D-6

BY Q/M DATE 11-10-28

**LOUIS BERGER & ASSOCIATES INC.**

SHEET NO. 2-7 OF

CHKD. BY \_\_\_\_\_ DATE \_\_\_\_\_

INSPECTION OF DAMS - COONT P.I.

PROJECT \_\_\_\_\_

SUBJECT OLD MARSH POND RESERVOIR - AREA - CAPACITY and DISCH. CURVES

**RESERVOIR AREA - CAPACITY**

Res Area 183 ac @ El 627.7  
201 ac @ El 627.7  
242 ac @ El 627.7

Elev.	Area (Acres)	Avg. Area (acres)	Height (ft)	Incremental Storage (ac-ft)	Cumulative Storage (ac-ft)	Surcharge Storage (ac-ft)	REMARKS
670	0	0	0	0	0		24" $\phi$ invert.
672.5	15	7.5	2.5	19	19		
675	46	30.5	2.5	76	95		
677.5	100	73	2.5	183	278		
680	136	118	2.5	295	573		
682.5	152	144	2.5	360	933		
685	168	160	2.5	400	1334		
687.7	183	175.5	2.7	474	1808	0	Spillway Crest
688	186	184.5	0.3	55.4	1863	55.	
689	193	190.0	1	190.0	2053	245.	
690	201	197.0	1	197.0	2250	442.	
691	206	203.5	1	203.5	2454	645.	
692	211	208.5	1	208.5	2662	854.	
693	216	213.5	1	213.5	2876	1068	Top of DIKE
694	220	218.0	1	218.0	3094	1286	TOP OF DAM
695	224	222	1	222	3316	1508.	
696	228	226	1	226	3542	1734.	
700	242						

14" Flashboards El 687.7 to 688.8

$1.17 \times 187.5 = 219$  AF storage reduction

Spwy Q over flashboards

$C = 3.33$  To El. 694  $H = 5.13$   $Q = 2321 < 3937$

Drainage area 2.34 sq. mi.  
 Res. area = 183 acres = 12% of total DA  
 Reservoir capacity to normal storage level 1808 AF.  
 Spillway crest El. 687.7 L = 60'  
 Top of dam El. 694.  
 (See memo Oct 19, 1943 for data on storage capacity, area, etc.)

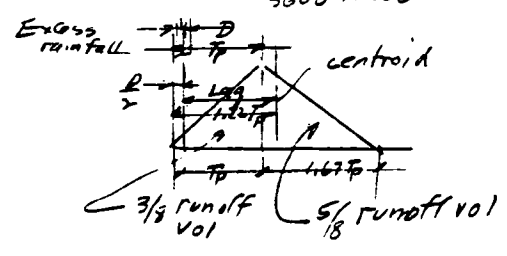
Drainage Area

Stream ③ L = 6000' = 1.14 mi  
 Max elev. = 989 Res El. 688 H = 300' } Tributary ①  
 $S = \frac{300}{1.14} = 263 \text{ ft/mi.}$

Stream 5 L = 6000 = 1.14 El. 935 - 680 =  $\frac{255}{1.14} = 224 \text{ ft/mi}$  } Tributary ②

Average S = 244'/mi K = 425 (Curve B K = 2.5 to 5)  
 Assume K = 425  
 $Lag = K \left( \frac{L L_c}{\sqrt{S}} \right)^{.33} = 425 \left( \frac{1.14 \times 1.14}{\sqrt{244}} \right)^{.33} = 1.48 \text{ Say } 1.50$

Check for velocity.  
 $v = \frac{6000}{3600 \times 1.50} = 1.11 \text{ ft/sec.} \approx 1.0 \text{ OK}$



$\frac{D}{2} + Lag = 1.22 T_p$   
 $T_p = \frac{Lag}{1.22} + \frac{D}{2 \times 1.22} = .82 Lag + .42 D$   
 $D = 0.5$   
 $T_p = .82 \times 1.5 + .42 \times 0.5 = 1.44$   
 Say  $T_p = 1.5 \text{ hrs.}$

If:  $T_p = 1.5$  Then:  $T_p(1.67) = 1.5(1.67) = 2.51$

Tributary streams into reservoir

Tributary	Length	Drop	Slope/mi	SL
①	3600	182	267	961,200
②	4000	122	161	544,000
③	6000	300	263	1,578,000
④	4200	132	166	697,200
⑤	6000	252	210	1,344,200
Average	4760			$\frac{1,049,920}{4760} = 220 \text{ ft/mi}$

BY DEL DATE 1/5/79 LOUIS BERGER & ASSOCIATES INC. SHEET NO. 11-9 OF  
 CHKD. BY \_\_\_\_\_ DATE \_\_\_\_\_ INSPECTION OF DAMS PROJECT \_\_\_\_\_  
 SUBJECT OLD HARSH POND - HYDROLOGY

Calculate  $Q_p$ :

$$Q_p = \frac{484 \text{ AQ}}{T_p} = \frac{484 (2.34)(1)}{1.5} = 755 \text{ cfs}$$

Rainfall

PMF = 24" x 0.8 (Fit Factor) = 19.2" for 6 hrs.  
 Adjust for Filtration Loss  $\frac{1}{3}$  19.2 - 0.4 = 18.8"

Time	Rain Dist (inches)	$Q_p$	Begin T	Tot Ret	Tot Ecl
0					
0.5	0.75	566	0	1.5	4.01
1.0	0.75	566	0.5	2.0	4.51
1.5	0.75	566	1.0	2.5	5.01
2.0	0.75	566	1.5	3.0	5.51
2.5	0.94	710	2.0	3.5	6.01
3.0	1.13	853	2.5	4.0	6.51
3.5	3.57	2695	3.0	4.5	7.01
4.0	5.64	4258	3.5	5.0	7.51
4.5	1.69	1276	4.0	5.5	8.01
5.0	1.32	997	4.5	6.0	8.51
5.5	0.75	566	5.0	6.5	9.01
6.0	0.75	566	5.5	7.0	9.51

FIGURE 4 Sheet I

OLD MARSH POND RESERVOIR  
FLOOD ROUTINGS OF PAMETHRU RESERVOIR AND SPILLWAY

case ① Dike constructed to EL. 693.0

case ② Dike raised to Elev. 694.3

$$T^2 = \frac{121 \times 400}{2000} = 24.2$$

K&M STANDARD CROSS SECTION  
10 X 10 TO THE HALF INCH

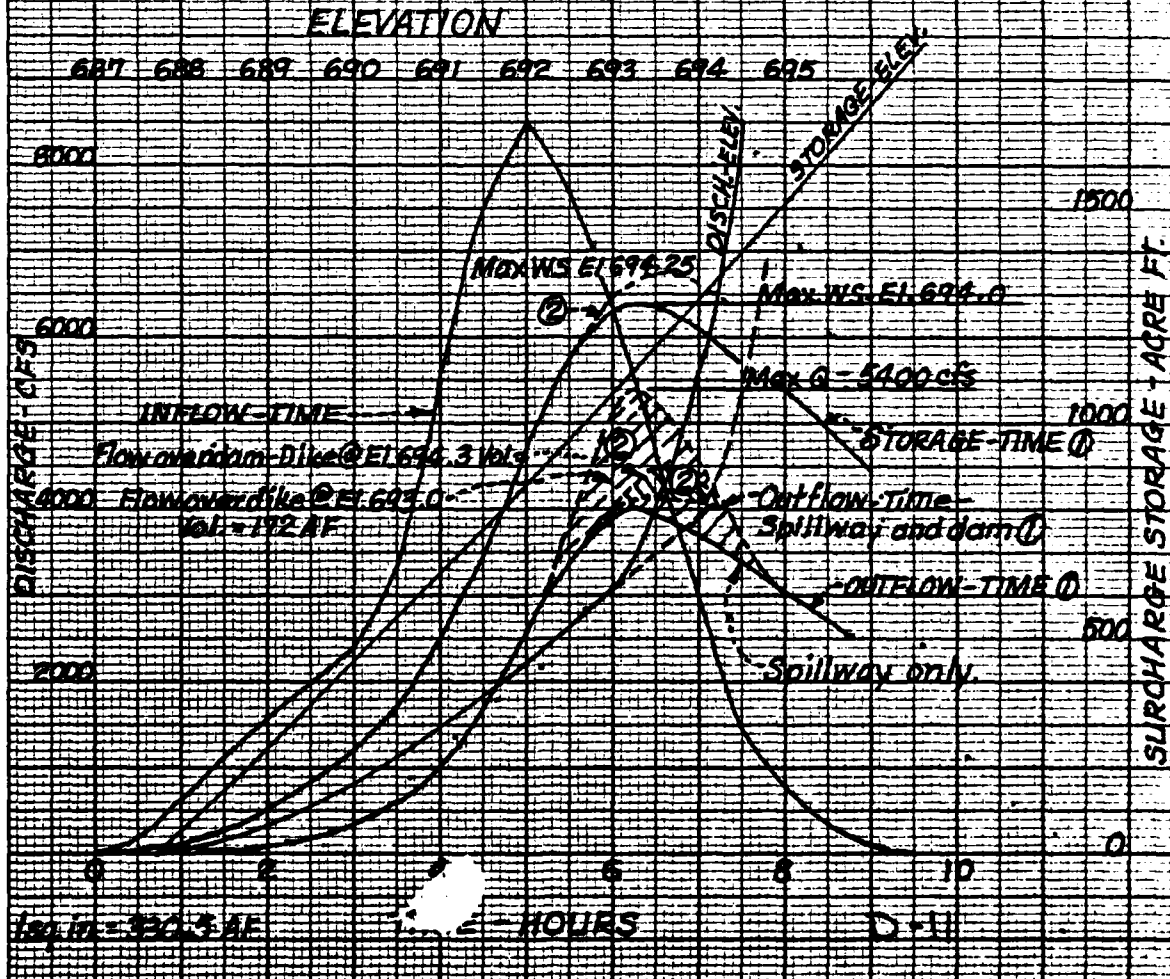


FIG. 5 - SHT D-12

S.D. MARSH POND RESERVOIR  
FLOOD ROUTING OF 0.75 PMF THRU RESERVOIR AND SPILLWAY

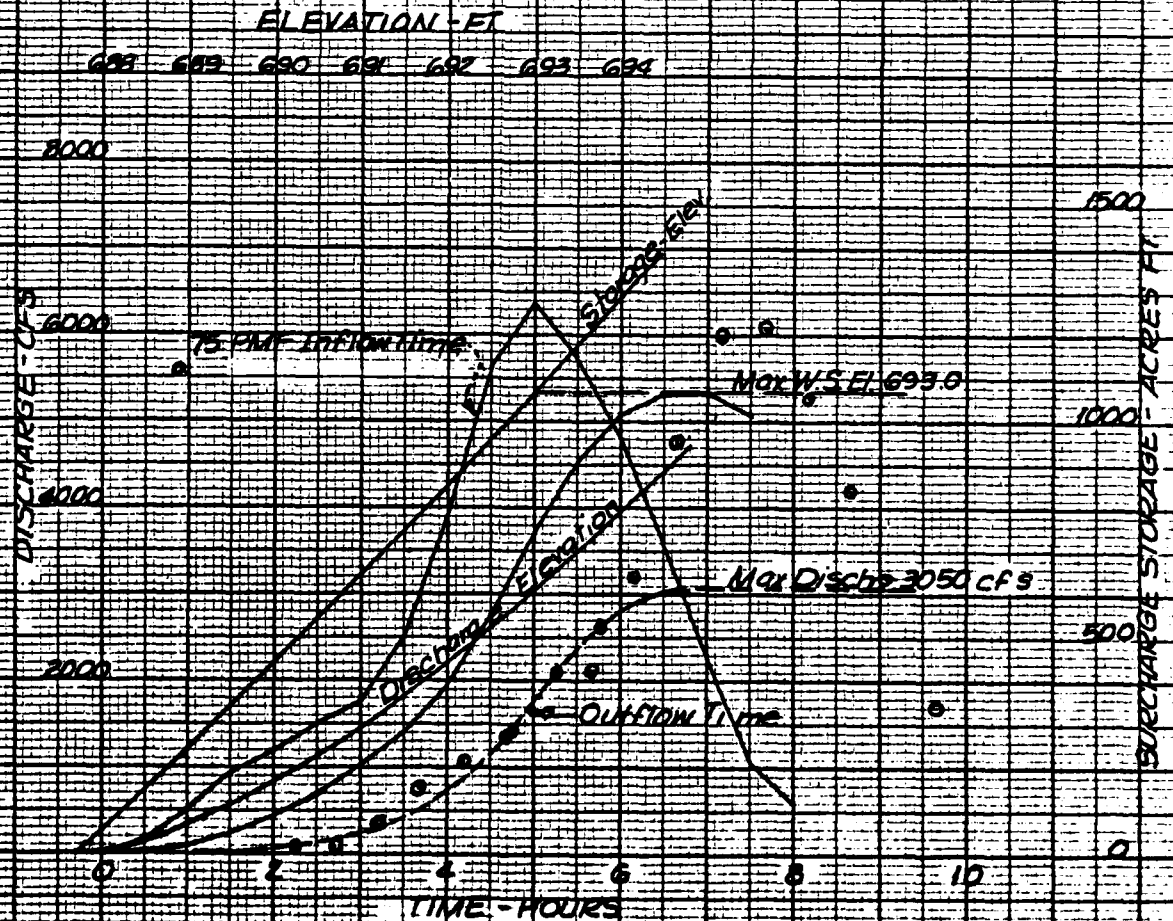
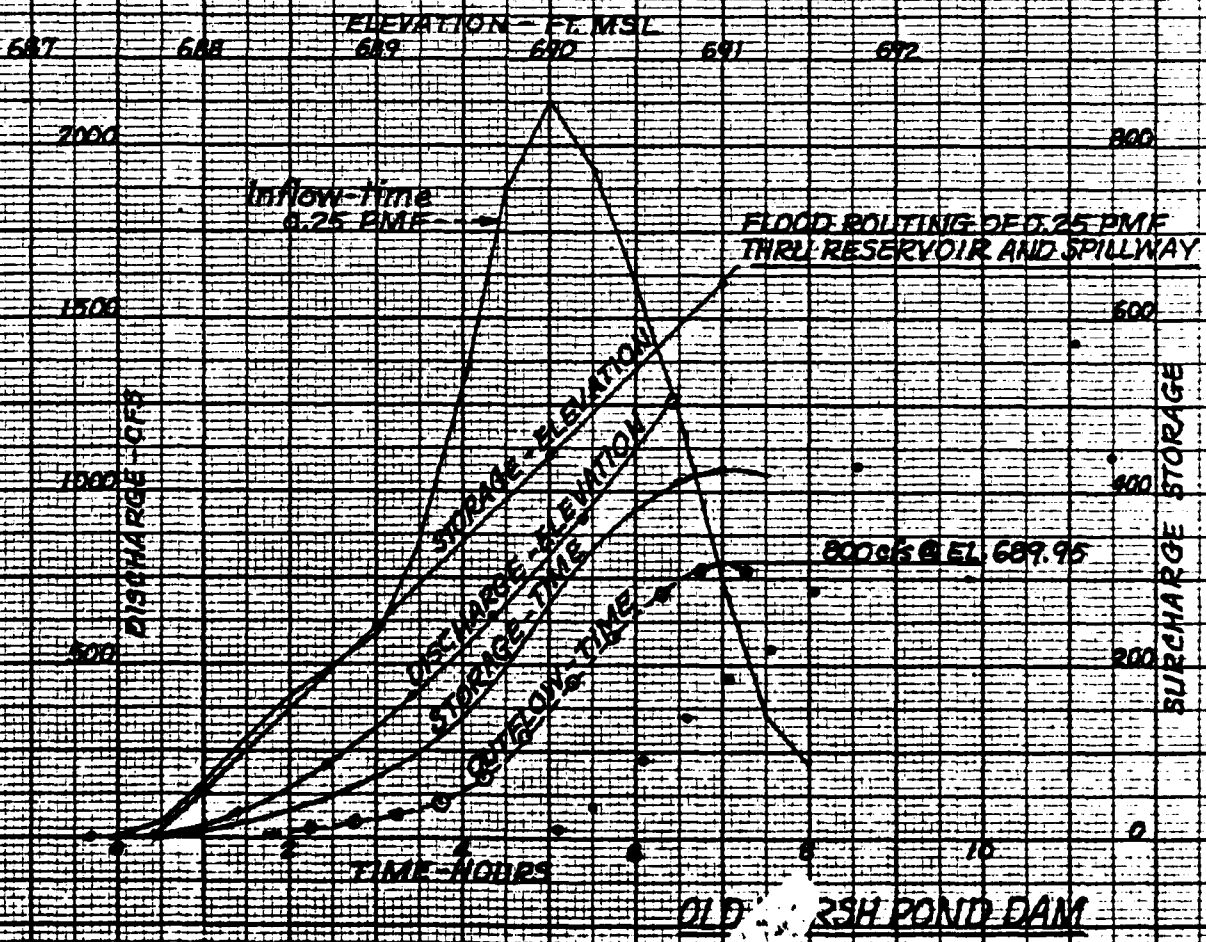
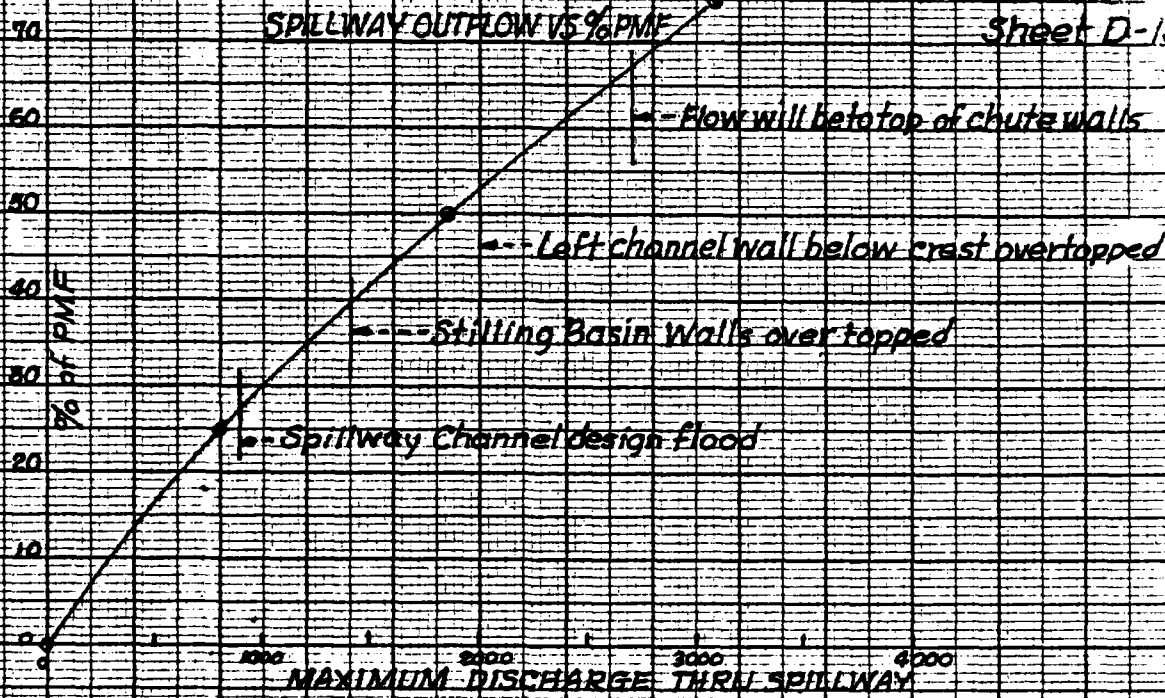
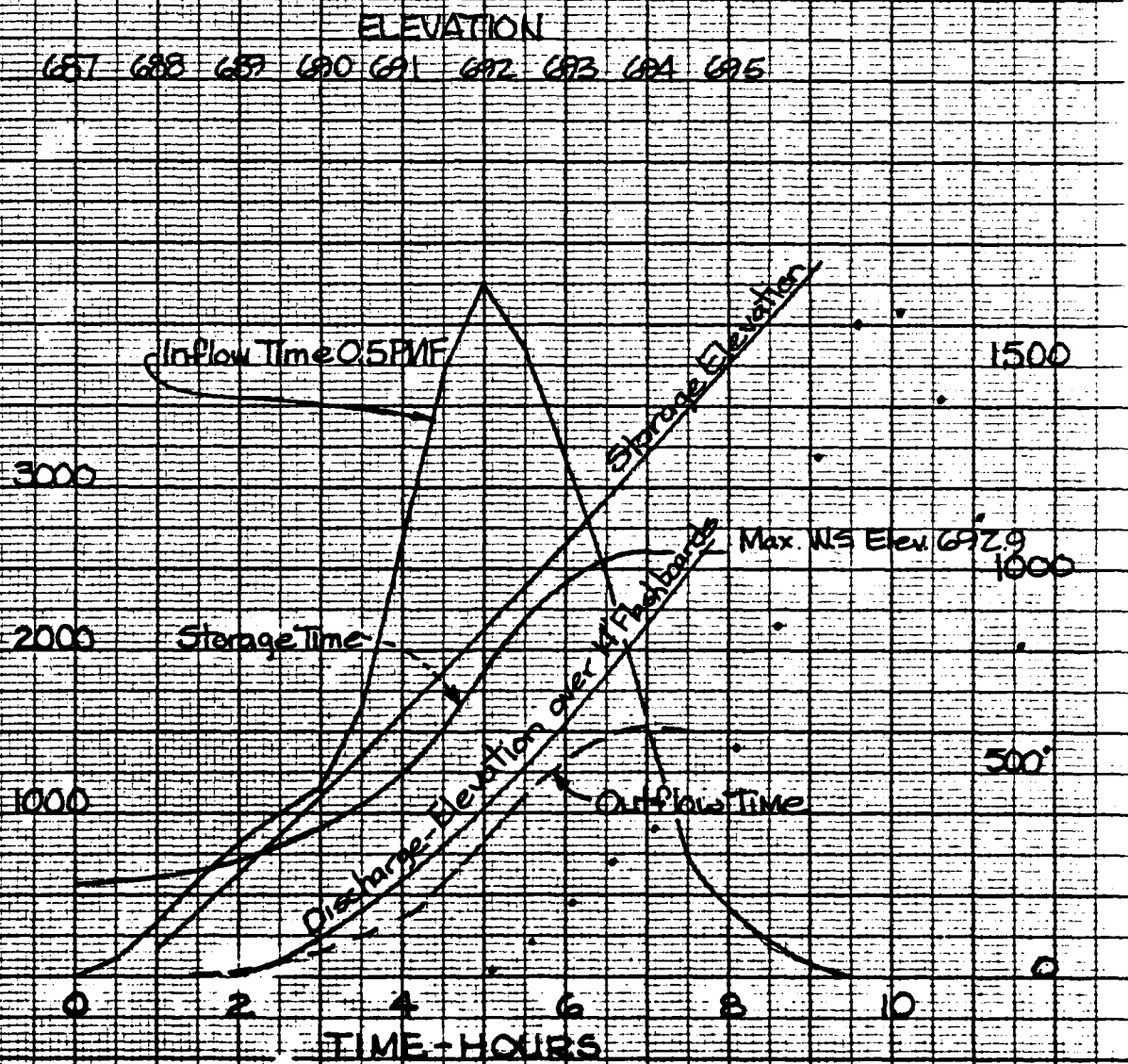


FIGURE  
Sheet D-13



OLD RSH POND DAM

OLD MARSH POND RESERVOIR  
FLOOD ROUTING OF 0.5 PMF THRU RESERVOIR  
AND SPILLWAY WITH 14" FLASHBOARDS INSTALLED  
AND RESERVOIR TO TOP OF FLASHBOARDS AT  
START OF FLOOD EVENT.



BY: CB DATE: 1-21-79

**LOUIS BERGER & ASSOCIATES INC.**

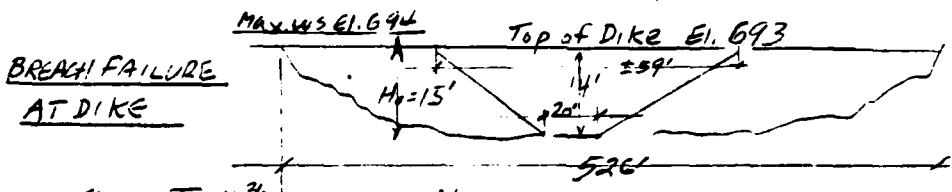
SHEET NO. D-15 OF

CHKD. BY: \_\_\_\_\_ DATE: \_\_\_\_\_

INSPECTION OF DAMS - CONN. & R. I.

PROJECT

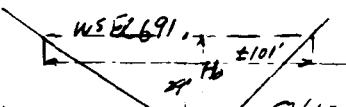
SUBJECT: OLD MARSH POND DAM - HYDRAULICS, OF BREACH FAILURE



$$Q = \frac{K}{2.7} \times W \sqrt{g} H_o^{3/2} = 1.68 W H_o^{3/2}$$

Elev	H	$\frac{Q}{W}$	$Q_{CR}$	$\frac{L}{H}$	$Q_{CR}$	$EQ$	Spillway Q	Total Outflow	Average outflow CF	Storage AF	Evacuation time - hrs
694	15	97.6	1952	25.2	2459	4112	3500	7612			
691	12	69.8	1396	16.8	1173	2568	1500	4068	5840	640	1.33
688	9	45.4	908	12.6	572	1480	50	1530	2800	591	2.56
685	6	24.7	494	8.4	207	702	0	702	1116	529	5.75
682	3	8.7	174	4.2	37	210	0	210	456	473	12.57
679	0	0	0	0	-	0	0	0	105	406	46.87
										2639 AF	169 hrs
											7 days

**TRAPEZOIDAL BREACH FAILURE AT DAM**



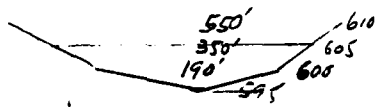
ELEV	H	$\frac{Q}{W}$	$Q_{CR}$	$\frac{L}{H}$	$Q_{CR}$	$EQ$	Spillway Q	Total Q	Average outflow	Storage	Evac time hrs
694	32	304.1	6082	448	13503	14385	0	14385			
691	29	262.4	5248	406	10633	15900	1500	17400	26402	640	0.38
688	26	222.7	4454	36.4	8106	12560	50	12610	15005	591	0.48
685	23	185.3	3706	32.2	5967	9673	0	9673	11142	529	0.58
682	20	150.3	3006	28.0	4208	7214	0	7214	8413	473	0.68
679	17	117.8	2356	23.8	2804	5160	0	5160	6187	406	0.78
676	14	88.0	1760	19.6	1725	3485	0	3485	4322	297	0.90
673	11	61.3	1226	15.4	944	2170	0	2170	2827	134	0.57
670	8	38.0	760	11.2	426	1186	0	1186	1678	19	0.14
667	5	18.8	376	7	132	508	0	508	847	0	0
662	0	0	0	0	0	0	0	0	254	0	0
										2439 AF	4.23 hrs

**50' RECTANGULAR BREACH FAILURE AT DAM (50' of core wall collapses.)**

ELEV	H	$\frac{Q}{W}$	$Q_{CR}$	Spillway Q	Disch. over spillway	Total Outflow
694	32	304.1	15200	4000	1500	20700
690	28	248.9	12450	850	0	13300
685	23	185.3	9260	0	0	9260
680	18	128.3	6420	0	0	6420
675	13	78.7	3940	0	0	3940
670	8	38.0	1900	0	0	1900

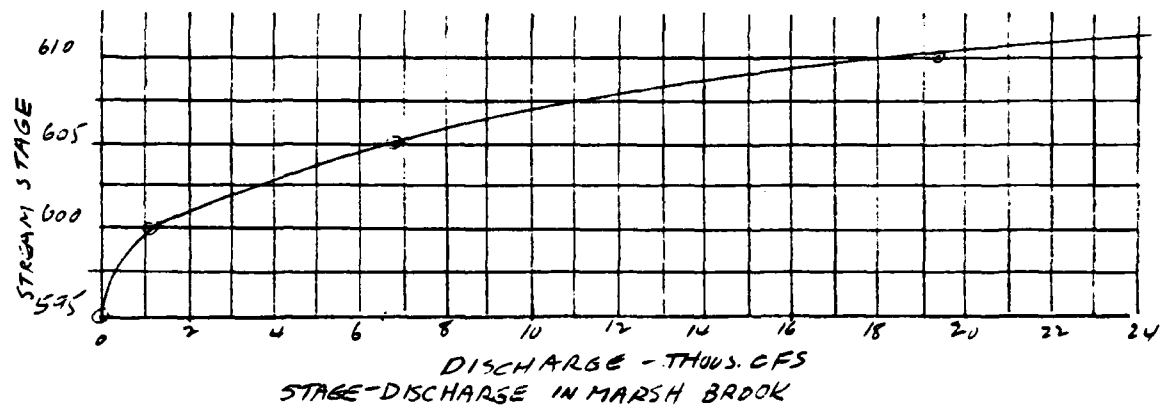
STAGE-DISCHARGE - MARSH BROOK IN EAST PLYMOUTH 5450' Below Dam

$s = \frac{12}{2500} = .0048 \quad s^{1/2} = .0632$   
 $n = 0.075$   
 $Q = \frac{1.486}{n} A r^{2/3} s^{1/2} = 1.252 A r^{2/3}$

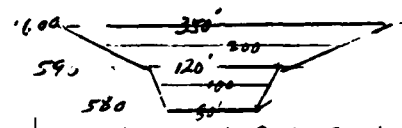


ELEV.	Area	W.P.	r	r <sup>2/3</sup>	Q
595	0	0			0
600	475	475	190.3	2.50	1094
605	1350	1825	350.6	5.21	6863
610	2250	4075	550.8	7.40	19372

$\frac{4075 \times 4500}{3500} = 420A =$

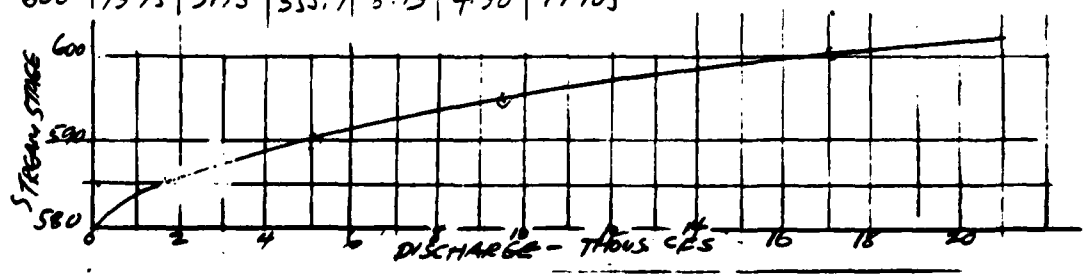


STAGE DISCHARGE - BELOW CONFLUENCE OF POLAND & PEQUABUCK RIVERS



$n = 0.075$   
 $s = \frac{14}{3100} = .0045 \quad s^{1/2} = 0.0672$   
 $Q = 1.252 A r^{2/3}$

EL.	Area	W.P.	r	r <sup>2/3</sup>	Q
580	0	0			0
585	450	450	102.4	2.68	1512
590	550	1000	124.8	3.01	5013
595	800	1800	205.4	4.25	9579
600	1375	3175	355.7	4.30	17105



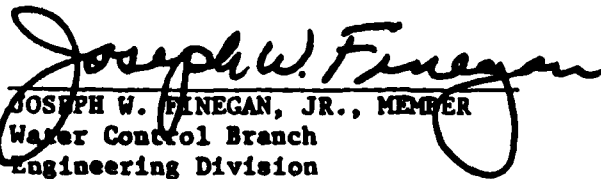
APPENDIX E

INFORMATION AS CONTAINED IN THE  
NATIONAL INVENTORY OF DAMS

# FEMA INVENTORY OF DAMS IN THE UNITED STATES

STATE	DIVISION	COUNTY	COUNTY DIST.	COUNTY COMB.	NAME	LATITUDE (NORTH)	LONGITUDE (WEST)	REPORT DATE
PA	1	1	1	1	MANSH ROAD DAM	31 11 00	75 59 00	5 30 79
POPULAR NAME		NAME OF IMPROVEMENT						
OLD MANSH ROAD								
REGION	BASIN	RIVER OR STREAM	NEAREST DOWNSTREAM CITY - TOWN - VILLAGE		DIST. FROM DAM (MILE)	POPULATION		
1	1	PAST PLY CREEK	PAST PLY		1	2500		
TYPE OF DAM	YEAR COMPLETED	PURPOSES	STRUC. HEIGHT	HYDRAU. HEIGHT	NEARBY CAPACITIES	DRAINAGE AREA (SQ. MI.)	DRAINAGE POPULATION	DRAINAGE DIST. (MILE)
			12	12				
REMARKS								
D-S SPILLWAY HAS LENGTH TYPE ...								
OWNER		ENGINEERING BY		CONSTRUCTION BY		MAINTENANCE		
DESIGN		CONSTRUCTION		REGULATORY AGENCY		INSPECTION DATE		
INSPECTED BY		INSPECTION DATE		AUTHORITY FOR INSPECTION		REMARKS		

This Phase I Inspection Report on Old Marsh Pond Dam has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

  
JOSEPH W. FINEGAN, JR., MEMBER  
Water Control Branch  
Engineering Division

  
CARNEY M. TERZIAN, MEMBER  
Design Branch  
Engineering Division

  
JOSEPH A. MCELROY, CHAIRMAN  
Chief, NED Materials Testing Lab.  
Foundations & Materials Branch  
Engineering Division

APPROVAL RECOMMENDED:

  
JOE B. FRYAR  
Chief, Engineering Division



DEPARTMENT OF THE ARMY  
NEW ENGLAND DIVISION, CORPS OF ENGINEERS  
424 TRAPELO ROAD  
WALTHAM, MASSACHUSETTS 02154

REPLY TO  
ATTENTION OF:

OCT 2 1979

NEDED-E

Mr. John Burns, Superintendent  
Water Department  
119 Reservoir Avenue  
Bristol, CT 06010

Dear Mr. Burns:

Forwarded herewith for your information and use is a copy of the Phase I Inspection Report on the Old Marsh Pond Dam. This inspection was made under the authority of Public Law 92-367 by the firm of Louis Berger & Associates, Inc. Wellesley, Mass. 02181 under the direction and supervision of the Corps of Engineers. Copies of the finished report have been forwarded to the Governor and the Department of Environmental Protection, the cooperating agency for the State of Connecticut.

Section 7 of the report contains an evaluation and recommendations. If you have any questions concerning this report, we suggest that you contact the Department of Environmental Protection first. Then, if there are further questions contact the Project Management Branch, Engineering Division of this office. We thank you for your cooperation and assistance in carrying out this program.

Sincerely,

*George I. Sarantis*

for  
JOE B. FRYAR  
Chief, Engineering Division

Incl  
As stated

# DISPOSITION FORM

For use of this form, see AR 340-15, the proponent agency is TAGCEN.

REFERENCE OR OFFICE SYMBOL  NEDEL-E	SUBJECT  Dam Inspection Final Report		
TO  Chief, Design Branch Chief, F & M Branch Chief, Water Control Branch	FROM  Chairman, Dam Safety Review Board	DATE 8 Mar 1979	CMT 1
<p>1. Attached is a single copy of the final report for <u>Old Marsh Pond</u> Dam, Identity No. <u>CT00285</u>.</p> <p>2. Please ascertain that the report is acceptable in accordance with your Branch comments or instructions given to the Architect-Engineer at the Review Board Meeting.</p> <p>3. If acceptable, retain the copy for your files and be prepared to sign the (master) approval sheet on <u>15 Mar</u>.</p> <p>4. If the report requires further work or correction, notify the undersigned as soon as the determination is made.</p> <p>5. The cost code for this review is ABA0/0700000000.</p>			

*Finegan*  
FINEGAN



DEPARTMENT OF THE ARMY  
NEW ENGLAND DIVISION, CORPS OF ENGINEERS  
424 TRAPELO ROAD  
WALTHAM, MASSACHUSETTS 02154

REPLY TO  
ATTENTION OF:

NEDED

OCT 2 1979

Honorable Ella T. Grasso  
Governor of the State of Connecticut  
State Capitol  
Hartford, Connecticut 06115

Dear Governor Grasso:

I am forwarding to you a copy of the Old Marsh Pond Dam Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. This report is presented for your use and is based upon a visual inspection, a review of the past performance and a brief hydrological study of the dam. A brief assessment is included at the beginning of the report. I have approved the report and support the findings and recommendations described in Section 7 and ask that you keep me informed of the actions taken to implement them. This follow-up action is a vitally important part of this program.

A copy of this report has been forwarded to the Department of Environmental Protection, the cooperating agency for the State of Connecticut. In addition, a copy of the report has also been furnished the owner, City of Bristol, Connecticut 06010.

Copies of this report will be made available to the public, upon request, by this office under the Freedom of Information Act. In the case of this report the release date will be thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Department of Environmental Protection for your cooperation in carrying out this program.

Sincerely,

*Max B. Scheider*

MAX B. SCHEIDER  
Colonel, Corps of Engineers  
Division Engineer

Incl  
As stated