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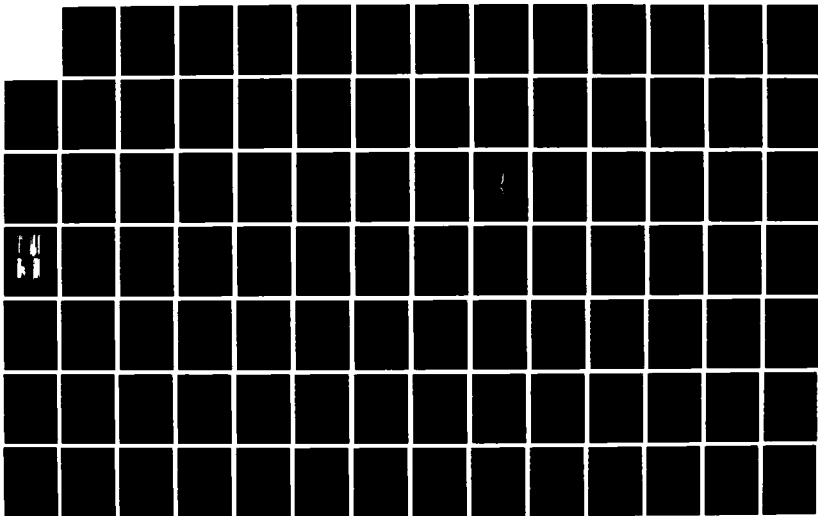
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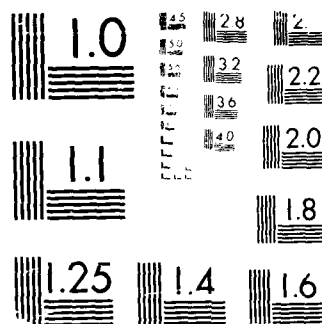
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A COMPREHENSIVE STUDY
ON DAMAGE TOLERANCE PROPERTIES OF
NOTCHED COMPOSITE LAMINATES

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19. ABSTRACT (Continue on reverse if necessary and identify by block number) This final report contains the results of an investigation on matrix-related damage mechanisms in notched composite laminates. The theoretical approach taken follows the principles of micromechanics and the mechanics of brittle fracture at the descriptive level considered valid for the so-called ply-elasticity. Namely, the laminate is basically treated as a 3-dimensional elastic solid which is made of distinctly anisotropic layers. Brittle fracture can initiate and propagate within any later having a weaker axis of material anisotropy, and within any one of the weaker layer interfaces due to the 3-dimensional interlaminar stresses. Owing to the particular microstructure of the laminate, growth of such sublaminate cracks constitutes a load- or time-dependent evolutionary process. A computer simulation methodology is developed to describe the modes and the extent of damage caused initially by the presence of the notch, and subsequently by the damages themselves. Experiment using a graphite-epoxy laminate is then conducted to validate the simulation results.			
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- I "Fracture due to A Kinked Crack in Unidirectional Fiber Reinforced Composites"
Paper presented at the ASME Winter Annual Meeting, Boston, 1987; also in
Damage Mechanics in Composites, AD-12, ASME, 1987. pp. 73-81.
- II "A Criterion for Mixed-Mode Matrix Cracking in Graphite-Epoxy Composites"
Paper presented at the ASTM 9th Symposium on Composites, Reno, 1988; to
appear in ASTM STP.
- III "Three-Dimensional Simulation of Crack Growth in Notched Laminates"
Paper presented at the 2nd Annual Meeting, Society for composites, Univ. of
Delaware, 1987; also in Proceedings of the American Society for Composites,
1987. pp. 444-457.
- IV "Simulation of Matrix Cracks in Composite Laminates Containing a Small Hole"
Paper presented at the ASME Winter Annual Meeting, Boston, 1987; Also in
Damage Mechanics in Composites, AD-12, ASME, 1987. pp. 83-91.
- V "3 D Finite Element Crack Simulation Code - User's Guide and Source Code"

FOREWORD

This is the final report for a comprehensive study on damage tolerance properties of notched composite laminates under the Air Force Grant AFOSR-84-0334. The grant was awarded to Dr. A. S. D. Wang of Drexel university with the initial grant period covering from 30 September 1984 to 31 December 1986. However, during the period from 1 September 1986 to 31 August 1987, Dr. worked at the AFOSR as visiting scientist under the Intergovernment Personnel Loan Program; Dr. C. W. Lau then served as an interim principal investigator, with the termination date of the grant extended to 31 December 1987.

The research was performed by Dr. A. S. D. Wang and his assistants: Dr. E. S. Reddy, Drexel University post-doctoral fellow, Dr. W. Binienda and Mr. Y. Zhong, Drexel University graduate students.

Major David A. Glasgow and Lt. Col. George K. Haritos of AFOSR served successively as technical monitors during the course of this research.



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INTRODUCTION

Objectives of Research.

The main objective of this research is to investigate matrix-related damage mechanisms in composite laminates that have a through-the-thickness line-notch or a small hole. A computer simulation methodology is then developed to describe the modes and the extent of damage growth caused initially by the presence of the notch (or hole), and subsequently by the damages themselves.

Theoretical Approach.

The theoretical approach taken in this endeavor followed the principles of micromechanics and the mechanics of brittle fracture at the descriptive level considered valid for the so-called ply-elasticity. Namely, the laminate is basically treated as a 3-dimensional elastic solid which is made of distinctly anisotropic layers. While each layer is assumed homogeneous and endowed with a set of effective elastic constants (see, e.g. [1]), brittle fracture can initiate and propagate within any layer having a weaker axis of material anisotropy, and within any one of the weaker layer interfaces due to the 3-dimensional interlaminar stresses.

Since the propagation modes and the growth behaviors between fracture in a layer and fracture in a layer-to-layer interface differ fundamentally owing to the particular microstructure of the laminate, growth of damages in the form of sublaminar cracks constitute a load-time dependent evolutionary process. The general premise of ply-elasticity and theory of brittle fracture on which a simulation model is based has recently been discussed in detail by Wang [2].

Crack Growth Simulation.

With laminates having a through-the-thickness line-notch or a small hole, stress

concentrations and hence sublaminar damages near the notch (or hole) are expected when the laminate is loaded by externally applied load. In order to simulate the damage initiation and damage growth as a function of the applied load, a 3-dimensional analysis of the stress field near the notch or hole must be first performed. Such a stress field, however, contains regions of stress concentration caused not only by the notch (or hole) itself in the usual sense, but also by the interaction of the free edges of the notch (or hole) with the layer interfaces known as free-edge effect [3].

In addition, if one or more sublaminar cracks have already initiated near the notch (or hole), the stress field disturbed by the presence of these cracks and the new conditions for these cracks to grow must be continuously analyzed.

Clearly, to effectively analyze such a complex system requires, as a prerequisite, an efficient and accurate finite element computational routine on one hand, and a set of physically consistent material conditions that govern the various crack growth behaviors on the other. Of course, the finite element routine must be developed in accordance within the basic confines of ply-elasticity and the theory of fracture mechanics. Similarly, material conditions governing the various crack growth behaviors must be determined independent of the laminate geometry, both in its overall shape and its lamination structure.

Finally, the simulation methodology must be validated by experiment in which actual growth of sublaminar damages is recorded as a function of the applied load. The recorded damage must be measured in quantity units consistent with those simulated numerically so that a direct comparison between the two can be made.

Major Tasks Performed.

Within the context of the forgoing discussions, the following major tasks have been performed during the course of the research:

1. Development of a 3-dimensional finite element code based on ply-elasticity and the linear theory of fracture mechanics. The code is capable of simulating the initiation and growth mechanisms of sublaminar cracks

expected to occur in certain notched laminates when they are specifically loaded.

2. Development of rigorous solutions based on anisotropic elasticity and fracture mechanics for a crack problem similar to that anticipated to occur in laminates but mathematically tractable without compromising accuracy. The same problem is then analyzed by means of the developed finite element code. Comparison of results from the two independent solution methods adjudicates the general accuracy of the finite element method.

3. Experiment to establish material conditions that govern the initiation and growth behaviors of the kinds of cracking anticipated to occur in notched laminates. This is accomplished by testing a family of specially designed specimens in which the anticipated cracking occurred, and by simulations of the observed cracking using the developed finite element code.

4. Validation of the simulation method by testing actual laminates that have through-the-thickness line-notches or small holes. Comparisons are then made between the test results and the simulation results, which display the adequacy and/or limitations of the simulation methodology.

In the next section, specifics in each of the tasks are discussed in more detail along with highlights of the results obtained therein. The actual results and the manner in which these results are obtained have been reported in open literature. Four full-length papers and one computer code with user's guide are appended to this report for reference.

The last section outlines a set of concluding remarks pertinent to the major themes of this research.

SPECIFIC TASKS AND RESULTS

3-D Finite Element Code.

As mentioned, the finite element code is developed on the basis of ply-elasticity and the theory of linear fracture mechanics. Its main functions are

1. To compute the 3-D stress field in a laminate of given lamination structure, overall laminate shape, manner of loading, the exact geometry and location of the notch. Because of the expected stress concentrations near the notch region, the code is capable of generating the desired mesh in the region around the notch. The computed stress field provides 6 stress components at any point. In general, stress distribution on any specified plane can be displayed graphically in various isometric forms.
2. To compute the strain energy release rates at any crack-tip with specified direction of propagation. If one or more cracks are already present near the notch, the code can compute the associate stress field as well as the strain energy release rate at one of the crack tip. In cases where the crack may propagate in mixed-modes, then the energy release rate corresponding to each mode can also be calculated. The calculated strain energy release rates are expressed in terms of the appropriately unit for the applied load.

Input data required to run the code include the geometry for the overall laminate specimen shape, the applied load and boundary conditions, the laminate stacking sequence and fiber orientations, the effective elastic constants (including thermal expansion coefficients if appropriate) for each of the laminating layers relative to their respective principal material axes, the location, size and orientation of the notch, and the suspected matrix crack or delamination near the notch.

Appendix V contains the user's guide in which a considerable detail about the code

is discussed. To help run the code, illustrative examples are provided with explanations and actual input/output results. A list of the source code, written in Fortran-IV, is also included.

Assessing the Accuracy of the Finite Element Method.

As the developed finite element code is to be used to compute both the stress fields and the fracture quantities for small cracks in layered, anisotropic solids, an effort is made to assess the numerical accuracy the code can provide. To this end, a problem of an ideal overall configuration and loading condition is treated rigorously on the basis of the anisotropic theory of elasticity and fracture mechanics.

The specific problem treated is a unidirectional laminate of infinite domain as illustrated in Figure 1. The laminate contains initially a kink crack and is loaded in uniform tension applied off-axis, making an arbitrary angle θ with the fibers. The base of the kink crack is normal to the applied tension while the kink itself is in the fiber direction. Thus, the problem is one that involves self-similar, mixed-mode fracture at the kink tip. Within the framework of elasticity theory and linear fracture mechanics, the problem can be formulated exactly and solved rigorously by means of singular integrals and the boundary collocation method.

Solutions to this rigorously formulated problem serve as branch mark from which the finite element solutions can be compared. As it turns out, it is possible to tune the finite element shape and mesh selections in order to yield as accurate numerical results as the rigorous solutions.

Detailed development of this effort has been published in the paper entitled "Fracture due to A Kink Crack in Unidirectional Fiber reinforced Composites." This paper is appended here as Appendix I.

Establishing A Mixed-Mode Fracture Criterion.

Another essential element in the present effort to simulate mixed-mode sublaminate crack is to ascertain the material condition under which the crack propagates. The problem

is complicated by the fact that fracture of different modes often involves different mechanisms at the microscale, which in turn result in different material conditions for propagation. For fracture propagating in arbitrary combination of modes, a general set of conditions is needed. This, however, is not always possible without actually specifying the material.

In the present work, the AS4-3501-06 graphite-epoxy composite system is used in all experiments and simulations. To establish the desired mixed-mode fracture criterion for matrix cracks in this material, a test specimen is designed which can yield crack propagation under 28 different mixed-mode conditions. The test specimen is shown in Figure 2.

It is an off-axis unidirectional tensile coupon with a pair of side notches cut normal to the applied tension. At the critical loading, a kink crack is initiated at one of the notch tips and is propagated along the fiber direction in mixed-mode. By varying the off-axis angle θ and the notch depth, the nature of the mode-mix as well as the critical conditions can thus be altered.

Correlation between experiment and finite element analysis concludes that a useful criterion governing mixed-mode fracture in this material appears to be the total strain energy release rate that exists at the crack tip.

The details of this subject have been included in the paper entitled "A Criterion for Mixed-Mode Matrix Cracking in Graphite Epoxy Composites." This paper is appended here in Appendix II.

Simulation of Matrix Cracks in Notched Laminates.

For simulation of matrix crack growth in laminates, the graphite-epoxy (AS4-3501-06) $[0_2/90_2]_S$ laminate coupon is chosen. The dimension of the actual coupon is 1" wide and 9" long; it is notched in two different forms: (a) a pair of side notches and (b) a small center hole. The applied load is uniaxial tension. Under the applied loading, both in-ply matrix cracks and interply delaminations are expected to occur and grow with the increasing load. In particular, these cracks can occur interactively. It should also be emphasized that in all cases the resulting sublaminates cracks propagate in mixed-modes of various degree of mode-mix.

Evolution of the matrix cracks and delamination in the specimen is both recorded in

experiment and simulated independently by the finite element routine.

Results from this part of the study have been reported in two papers entitled "Three-Dimensional Simulation of Crack Growth in Notched Laminates," and "Simulation of Matrix Cracks in Composite Laminates Containing A Small Hole." These papers are appended here as Appendix II and Appendix IV, respectively.

CONCLUSIONS

In this research program, a simulation method is developed to describe the evolution of matrix cracks in the vicinity of notches in composite laminates. The method is based on a generic approach of the problem in which actual cracking mechanisms are closely modeled. Still, these mechanisms are extremely complex and the simulation has to resort to some degree of idealization. This then causes discrepancies between the simulation and experiment, as is evident by the results reported in the papers appended herein. It is conceivable that these difficulties could be considerably removed if more is known about the interactive mechanisms of the various cracks at the microscopic scale and if a more realistic simulation technique becomes available.

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- [1] Tsai, S. W. and Hahn, H. T., "Introduction to Composite Materials," Technomic Pub., Lancaster, Pa. 1980.
- [2] Wang, A. S. D., "Fracture Mechanics of Sublaminar Cracks in Composite Materials," Composite Technology Review, Vol. 6, 1984, pp. 45-62.
- [3] Wang, A. S. D. and Crossman, F. W., "Some New Results on Edge Effects in Symmetric Composite Laminates," Journal of Composite Materials, Vol. 11, 1977, pp. 92-108.

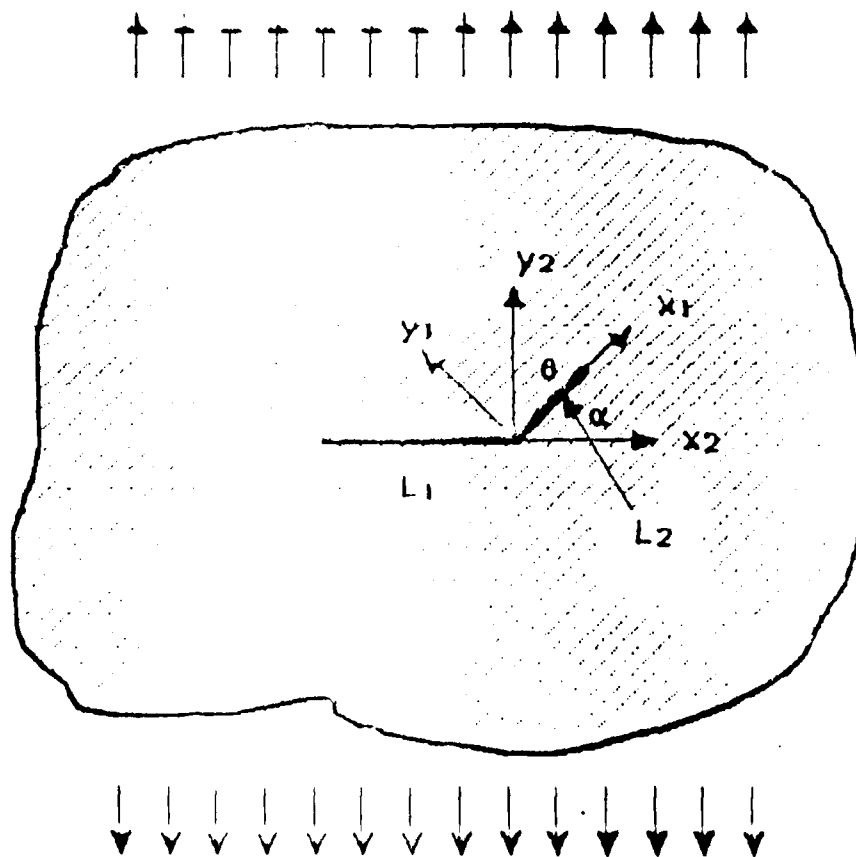


Figure 1. Kink crack in an infinite unidirectional laminate subjected to uniform tension.

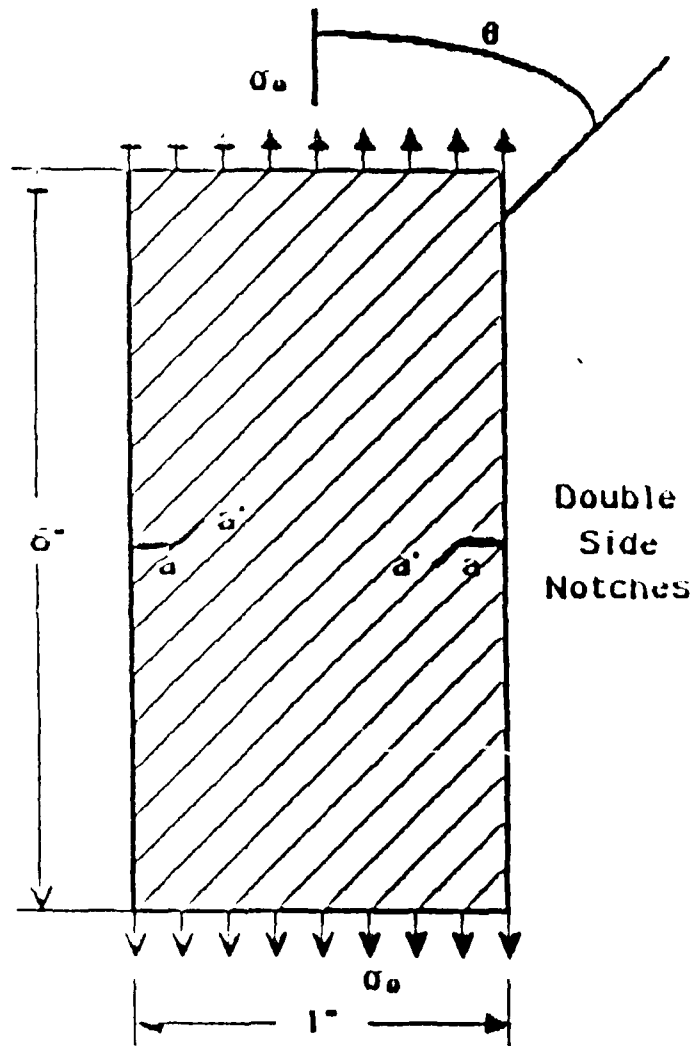


Figure 2. Geometry of test specimen used to establish mixed-mode fracture criterion.

**A COMPREHENSIVE STUDY
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NOTCHED COMPOSITE LAMINATES**

Appendix I

**Fracture due to A Kinked Crack
in Unidirectional Fiber Reinforced Composites"**

Paper presented at the ASME Winter Annual Meeting, Boston, 1987;
also in Damage Mechanics in Composites, AD-12, ASME, 1987. pp. 73-81.

FRACTURE DUE TO A KINKED CRACK IN UNIDIRECTIONAL FIBER REINFORCED COMPOSITES

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ABSTRACT

This paper presents an analysis for a kinked crack in a unidirectionally fiber reinforced composite plate. The plate is assumed infinite and contains a through-thickness crack of initial length L_1 , which makes an angle θ with the direction of fibers. When the plate is subjected to a far-field uniform tensile stress normal to the crack, the crack can only propagate in the preferential direction of fibers due to the weak strength of the fiber-matrix interface. The result is a kinked crack propagating in mixed mode, with the degree of modal mixture depending on the angle θ and the ratio between the length of the kink L_2 and the length of the initial crack L_1 .

To determine the parameters relevant to mixed-mode fracture at the tips of the kinked crack, the problem is formulated in terms of singular integral equations with generalized Cauchy kernels. The resulting system of equations is then solved numerically employing a Gaussian quadrature and the collocation method. Stress intensity factors, k_I and k_{II} , and the strain energy release rates, G_I and G_{II} , of the kinked crack are obtained for various values of θ and L_2/L_1 ratios.

1. INTRODUCTION

Failure in fiber reinforced polymeric composites frequently occurs in the form of matrix cracks due to weak fiber/matrix interface strength. Depending on the local fiber geometry, a matrix crack may propagate in the preferential fiber direction under mixed-mode conditions. Invariably, the relevant fracture parameters which govern matrix crack propagation are dominated by the anisotropic properties of the material. This makes it necessary to formulate an anisotropic criterion for fracture propagation.

Within the frame work of the original Griffith theory for brittle fracture, a number of mixed-mode crack propagation criteria have been used for various types of

materials, including fiber reinforced composites [1-6]. Sih [5,6], for example, proposed a criterion based on the local strain energy density. Others have used criteria in the general form of $f(k_I, k_{II}) = k_{eff}$. In the experiment by Wu [3], who tested notched balsa wood and unidirectional fiber glass reinforced composite plates, the fracture criterion $(k_I/k_{IC}) + (k_{II}/k_{IIC})^2 = 1$ was shown to apply.

In a series of recent papers by Wang, Crossman, et. al. [7-10], the critical energy release rate G_{IC} was used as a criterion for the initiation and propagation of mode-I cracks in multi-layered laminates. When the crack is blunted by a local fiber or layer interface, the crack would kink and a mixed-mode or shear-dominated fracture would result. In this case, the total critical energy release rate $(G_T)_C$ has been employed as a criterion [11].

Regardless of the form of the fracture criteria, it is essential to treat the crack conditions correctly and determine the associated fracture parameters accurately.

Fracture problems in homogeneous anisotropic materials have been rigorously studied, see e.g. [12-15]. But for fracture in fibrous composites, material inhomogeneity and the associated microstructure often prevent an analytical solution. A numerical technique such as the finite element method is employed, without a rigorous interrogation of the fracture conditions near the crack tip.

This paper treats a kinked crack in a unidirectionally fiber reinforced composite plate. The plate is assumed to contain a through-thickness crack of initial length L_1 , which makes an angle θ with the direction of fibers. When the plate is subjected to a uniform far-field tensile stress normal to the crack, the crack can only propagate in the direction of the fiber because of the weak strength of the fiber-matrix interface. Thus, a kinked crack is induced propagating in mixed mode. Clearly, the nature of the propagation depends on the kink angle θ , the lengths of the kink L_2 and the main crack L_1 .

To determine the parameters relevant to mixed-

mode fracture at the tips of the kinked crack, first the problem of two separate cracks embedded in an infinite orthotropic plate is considered. Namely, one crack is the main crack of length L_1 and the second crack of length L_2 is assumed to lie along the direction of fibers. The line of L_2 intersects the line of L_1 at the origin of the x - y coordinates as shown in Figure 1. Using the crack surface derivatives as unknown, the problem is formulated on the basis of two-dimensional theory of elasticity and the field equations are expressed in terms of singular integrals with Cauchy type kernels. The system of integral equations is then solved numerically by employing a Gaussian quadrature and the collocation method.

Next, the actual kinked crack is considered. This is accomplished by letting the approaching tips of the kink and the main cracks to touch each other at the intersect of the two crack lines. In this configuration, the singular integral equations are still valid but some of the kernels become singular, giving rise to generalized Cauchy kernels. In fact, it is shown that at the point of touch the stresses are singular and the power of singularity is different from $1/2$. Thus, for the kinked crack geometry, a set of singular integral equations with singular kernels is solved. Stress intensity factors, k_1 and k_2 , and strain energy release rate components G_I and G_{II} , at the tips of the kinked crack are obtained for various values of θ and L_2/L_1 ratios. Note that the problem of the plate containing only the main crack corresponds to $L_2 \rightarrow 0$.

2. FORMULATION OF THE PROBLEM

As stated previously, the problem at hand is a kinked crack in an infinite plate, and it is treated first by considering two separate cracks as depicted in Figure 1. Let the plate be orthotropic with principal directions x_1 and y_1 . The far-field uniform tension is applied in the direction of y_2 which makes angle θ with y_1 . The main crack of length L_1 lies on the x_2 axis, while the inclined crack of length L_2 (the future kink) lies on the x_1 axis (which is the direction of the fibers). The stress fields for the individual cracks are first solved, and the stress field for the interacting cracks is then obtained by superposition. A brief outline of the solution procedures is given below; details are contained in Reference [16].

Crack Parallel to the Fibers.

For the crack parallel to the fibers, the governing field equation is expressed in terms of the stress function $F_1(x_1, y_1)$ in the principal coordinates (x_1, y_1) :

$$\frac{\partial^4 F_1}{\partial x_1^4} + \beta_2 \frac{\partial^4 F_1}{\partial x_1^2 \partial y_1^2} + \beta_1 \frac{\partial^4 F_1}{\partial y_1^4} = 0 \quad (1)$$

where

$$\beta_1 = \frac{a_{11}}{a_{22}}; \quad \beta_2 = \frac{2a_{12} + a_{66}}{a_{22}} \quad (2)$$

and

$$a_{11} = \frac{1}{E_{LL}}; \quad a_{12} = -\frac{\nu_{LT}}{E_{LL}}; \quad a_{22} = \frac{1}{E_{TT}}; \quad a_{66} = \frac{1}{G_{LT}} \quad (3)$$

E_{LL} , E_{TT} , G_{LT} , ν_{LT} being the engineering elastic constants for the orthotropic material.

Fourier transformation of the stress function $F_1(x_1, y_1)$ can be defined as:

$$F_1(x_1, y_1) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \phi_1(s, y_1) e^{-isx_1} ds \quad (4)$$

Substituting equation (4) in (1), Ordinary Differential Equation (ODE) with constant coefficients is obtained. The solution of such equation can be expressed as:

$$\phi_1(s, y_1) \sim e^{\omega s y_1} \quad (5)$$

so the following characteristic equation is obtained:

$$\beta_1 \omega^4 - \beta_2 \omega^2 + 1 = 0 \quad (6)$$

The roots of equation (6) are: $\omega_1, -\omega_1, \omega_2, -\omega_2$, such that $\text{Re}(\omega_1) > 0$ and $\text{Re}(\omega_2) > 0$.

Taking into consideration the fact that the stress and displacements must vanish at infinity, the stress function may then be written as:

$$F_1(x_1, y_1^+) = \frac{1}{2\pi} \int_{-\infty}^{\infty} [A e^{\omega_1 |s| y_1} + B e^{\omega_2 |s| y_1}] e^{-isx_1} ds, \quad y_1 > 0 \quad (7)$$

$$F_1(x_1, y_1^-) = \frac{1}{2\pi} \int_{-\infty}^{\infty} [C e^{\omega_1 |s| y_1} + D e^{\omega_2 |s| y_1}] e^{-isx_1} ds, \quad y_1 < 0$$

Using the continuity of stress at $y_1=0$ and introducing the following crack surface displacement derivatives as the new unknowns,

$$f_1(x_1) = \frac{\partial}{\partial x_1} [u(x_1, 0^+) - u(x_1, 0^-)] \quad x_c < x_1 < x_d \quad (8)$$

$$f_2(x_1) = \frac{\partial}{\partial x_1} [v(x_1, 0^+) - v(x_1, 0^-)] \quad (9)$$

the stresses may be expressed as:

$$\sigma_{x_1 x_1} = \frac{1}{2\pi(\omega_1^2 - \omega_2^2)a_{11}} \int_{x_c}^{x_d} \left\{ \frac{f_1(t_1)\omega_1^3 y_1 + f_2(t_1)(t_1 - x_1)\omega_1}{\omega_1^2 y_1^2 + (t_1 - x_1)^2} - \frac{f_1(t_1)\omega_2^3 y_1 + f_2(t_1)(t_1 - x_1)\omega_2}{\omega_2^2 y_1^2 + (t_1 - x_1)^2} \right\} dt_1 \quad (10)$$

$$\sigma_{y_1 y_1} = \frac{-1}{2\pi(\omega_1^2 - \omega_2^2)a_{11}} \int_{x_c}^{x_d} \left\{ \frac{f_1(t_1)\omega_1 y_1 + f_2(t_1)\frac{t_1 - x_1}{\omega_1}}{\omega_1^2 y_1^2 + (t_1 - x_1)^2} - \frac{f_1(t_1)\omega_2 y_1 + f_2(t_1)\frac{t_1 - x_1}{\omega_2}}{\omega_2^2 y_1^2 + (t_1 - x_1)^2} \right\} dt_1 \quad (11)$$

$$\tau_{x_1 y_1} = \frac{1}{2\pi(\omega_1^2 - \omega_2^2)a_{11}} \int_{x_a}^{x_b} \left\{ \frac{f_1(t_1)\omega_1(t_1 - x_1) - f_2(t_1)\omega_1 y_1}{\omega_1^2 y_1^2 + (t_1 - x_1)^2} - \frac{f_1(t_1)\omega_2(t_1 - x_1) - f_2(t_1)\omega_2 y_1}{\omega_1^2 y_1^2 + (t_1 - x_1)^2} \right\} dt_1 \quad (12)$$

For more details about the formulation one may refer to [16].

Crack Making an Angle θ with the Fibers.

In this configuration the crack is assumed to lie along the x_2 axis. For a formulation using the x_2 - y_2 coordinate system the material has to be taken as fully anisotropic, giving the following governing equation in terms of the Airy Stress Function $F_2(x_2, y_2)$:

$$\frac{\partial^4 F_2}{\partial x_2^4} + \gamma_1 \frac{\partial^4 F_2}{\partial x_2^2 \partial y_2^2} + \gamma_2 \frac{\partial^4 F_2}{\partial x_2^2 \partial y_2^2} + \gamma_3 \frac{\partial^4 F_2}{\partial x_2 \partial y_2^3} + \gamma_4 \frac{\partial^4 F_2}{\partial y_2^4} = 0 \quad (13)$$

where

$$\begin{aligned} \gamma_1 &= -\frac{2b_{26}}{b_{22}}; & \gamma_2 &= \frac{2b_{12} + b_{66}}{b_{22}}; \\ \gamma_3 &= -\frac{2b_{16}}{b_{22}}; & \gamma_4 &= \frac{b_{11}}{b_{22}}; \end{aligned} \quad (14)$$

and

$$\begin{aligned} b_{11} &= a_{11} \cos^4 \theta + (2a_{12} + a_{66}) \sin^2 \theta \cos^2 \theta + a_{22} \sin^4 \theta \\ b_{22} &= a_{11} \sin^4 \theta + (2a_{12} + a_{66}) \sin^2 \theta \cos^2 \theta + a_{22} \cos^4 \theta \\ b_{12} &= a_{12} + (a_{11} + a_{22} - 2a_{12} - a_{66}) \sin^2 \theta \cos^2 \theta \\ b_{66} &= a_{66} + (a_{11} + a_{22} - 2a_{12} - a_{66}) \sin^2 \theta \cos^2 \theta \\ b_{16} &= [a_{22} \sin^2 \theta - a_{11} \cos^2 \theta + \frac{1}{2}(2a_{12} + a_{66}) \cos 2\theta] \sin 2\theta \\ b_{26} &= [a_{22} \cos^2 \theta - a_{11} \sin^2 \theta - \frac{1}{2}(2a_{12} + a_{66}) \cos 2\theta] \sin 2\theta \end{aligned} \quad (15)$$

Again following the same procedure, the stress can be expressed in terms of the crack displacement derivatives $f_3(t_2)$ and $f_4(t_2)$ as follows:

$$\begin{aligned} \sigma_{x_2 y_2} &= \frac{1}{2\pi} \int_{x_a}^{x_b} \left[\frac{R_1 f_3(t_2) - R_2 f_4(t_2)}{y_2(a+ib) + i(t_2 - x_2)} + \frac{R_3 f_4(t_2) - R_4 f_3(t_2)}{y_2(c+id) + i(t_2 - x_2)} \right. \\ &\quad \left. + \frac{R_5 f_3(t_2) - R_6 f_4(t_2)}{y_2(a-ib) - i(t_2 - x_2)} + \frac{R_7 f_4(t_2) - R_8 f_3(t_2)}{y_2(c-id) - i(t_2 - x_2)} \right] dt_2 \end{aligned} \quad (16)$$

$$\begin{aligned} \sigma_{y_2 y_2} &= \frac{1}{2\pi} \int_{x_a}^{x_b} \left[\frac{R_9 f_3(t_2) - R_{10} f_4(t_2)}{y_2(a+ib) + i(t_2 - x_2)} + \frac{R_{11} f_4(t_2) - R_{12} f_3(t_2)}{y_2(c+id) + i(t_2 - x_2)} \right. \\ &\quad \left. + \frac{R_{13} f_3(t_2) - R_{14} f_4(t_2)}{y_2(a-ib) - i(t_2 - x_2)} + \frac{R_{15} f_4(t_2) - R_{16} f_3(t_2)}{y_2(c-id) - i(t_2 - x_2)} \right] dt_2 \end{aligned} \quad (17)$$

$$\begin{aligned} \tau_{x_2 y_2} &= \frac{1}{2\pi} \int_{x_a}^{x_b} \left[\frac{R_{17} f_3(t_2) - R_{18} f_4(t_2)}{y_2(a+ib) + i(t_2 - x_2)} + \frac{R_{19} f_4(t_2) - R_{20} f_3(t_2)}{y_2(c+id) + i(t_2 - x_2)} \right. \\ &\quad \left. + \frac{R_{21} f_3(t_2) - R_{22} f_4(t_2)}{y_2(a-ib) - i(t_2 - x_2)} + \frac{R_{23} f_4(t_2) - R_{24} f_3(t_2)}{y_2(c-id) - i(t_2 - x_2)} \right] dt_2 \end{aligned} \quad (18)$$

where

$$f_3(x_2) = \frac{\partial}{\partial x_2} [u(x_2, 0^+) - u(x_2, 0^-)] \quad (19)$$

$$x_a < x_2 < x_b$$

$$f_4(x_2) = \frac{\partial}{\partial x_2} [v(x_2, 0^+) - v(x_2, 0^-)] \quad (20)$$

and R_i $i=1,2,\dots,24$ are given in [16]. Here it must be noted that the formulation leading to expressions (16)-(18) is quite lengthy and tedious. The intermediate steps can be found in [16].

The Integral Equations.

Stress field for two-crack system is generated by superimposing the two solutions briefly described in two previous sections. It is noted that the stresses are given in different coordinate systems. Therefore the following coordinate transformations are used:

$$x_2 = x_1 \cos \theta - y_1 \sin \theta \quad (21)$$

$$y_2 = x_1 \sin \theta + y_1 \cos \theta$$

or

$$x_1 = x_2 \cos \theta + y_2 \sin \theta$$

$$y_1 = -x_2 \sin \theta + y_2 \cos \theta \quad (22)$$

The total solution for the stress field can be expressed in either (x_1, y_1) or (x_2, y_2) . Let superscript (T) be used to denote the total stresses in either system. To satisfy the boundary conditions along $y_2=0$ and $y_1=0$ we may write:

$$\sigma_{y_2 y_2}^T = -\sigma_0$$

$$x_a < x_2 < x_b \quad (23)$$

$$\tau_{x_2 y_2}^T = 0$$

and

$$\begin{aligned}\sigma_{y_1 y_1}^T &= -\sigma_0 \cos^2 \theta \\ \tau_{x_1 y_1}^T &= -\sigma_0 \sin \theta \cos \theta\end{aligned}\quad x_c < x_1 < x_d \quad (24)$$

By means of a normalization procedure by substituting the following:

$$\begin{aligned}t_2 &= \frac{\tau_2(x_b - x_a)}{2} + \frac{x_b + x_a}{2} \\ x_2 &= \frac{s_2(x_d - x_c)}{2} + \frac{x_d + x_c}{2} \quad -1 < \tau_2, s_2 < 1 \quad (25) \\ dt_2 &= \frac{x_b - x_a}{2} d\tau_2\end{aligned}$$

and

$$\begin{aligned}t_1 &= \frac{\tau_1(x_d - x_c)}{2} + \frac{x_d + x_c}{2} \\ x_1 &= \frac{s_1(x_d - x_c)}{2} + \frac{x_d + x_c}{2} \quad -1 < \tau_1, s_1 < 1 \quad (26) \\ dt_1 &= \frac{x_d - x_c}{2} d\tau_1\end{aligned}$$

(23) and (24) lead to the following system of Cauchy type singular integral equations:

$$\begin{aligned}C_{12} \int_{-1}^1 \frac{f_2(\tau_1)}{\tau_1 - s_1} d\tau_1 + \int_{-1}^1 K_{13} f_3(\tau_2) d\tau_2 + \int_{-1}^1 K_{14} f_4(\tau_2) d\tau_2 \\ = -\sigma_0 \cos^2 \theta\end{aligned} \quad (27)$$

$$\begin{aligned}C_{21} \int_{-1}^1 \frac{f_2(\tau_1)}{\tau_1 - s_1} d\tau_1 + \int_{-1}^1 K_{23} f_3(\tau_2) d\tau_2 + \int_{-1}^1 K_{24} f_4(\tau_2) d\tau_2 \\ = -\sigma_0 \sin \theta \cos \theta\end{aligned} \quad (28)$$

$$\begin{aligned}C_{33} \int_{-1}^1 \frac{f_3(\tau_2)}{\tau_2 - s_2} d\tau_2 + C_{34} \int_{-1}^1 \frac{f_4(\tau_2)}{\tau_2 - s_2} d\tau_2 + \int_{-1}^1 K_{31} f_1(\tau_1) d\tau_1 \\ + \int_{-1}^1 K_{32} f_2(\tau_1) d\tau_1 = -\sigma_0\end{aligned} \quad (29)$$

$$\begin{aligned}C_{43} \int_{-1}^1 \frac{f_3(\tau_2)}{\tau_2 - s_2} d\tau_2 + C_{44} \int_{-1}^1 \frac{f_4(\tau_2)}{\tau_2 - s_2} d\tau_2 + \int_{-1}^1 K_{41} f_1(\tau_1) d\tau_1 \\ + \int_{-1}^1 K_{42} f_2(\tau_1) d\tau_1 = 0\end{aligned} \quad (30)$$

These equations must be solved with the following single-valuedness conditions which complete the formulation of

the problem:

$$\int_{-1}^1 f_1(\tau_1) d\tau_1 = 0 \quad (31)$$

$$\int_{-1}^1 f_3(\tau_2) d\tau_2 = 0 \quad (32)$$

$$\int_{-1}^1 f_2(\tau_1) d\tau_1 = 0 \quad (33)$$

$$\int_{-1}^1 f_4(\tau_2) d\tau_2 = 0 \quad (34)$$

The expressions for the kernels K_{ij} are functions of material constants and crack geometry [16].

The system of integral equations (27-34) can be solved by using one of the Gaussian quadrature technique [17],[18]. It should be noted that this system of integral equations contain Cauchy type kernels, so the stress and strains will have a square-root singularity and one may therefore use the classical definition of stress intensity factors to evaluate them at the crack tips [12-14].

Solution for the Kinked Cracked.

The geometry of interest is that of a kinked cracked. We can arrive at that configuration by letting $x_b=0$ and $x_c=0$. In this case the integral equations (27-30) remain valid but some of the kernels become singular while approaching the tips, giving rise to a singularity of unknown power β at the apex. The singularity β can be derived by requiring the displacements of common end to match what giving the following transcendental characteristic equation:

$$\begin{aligned}-\pi^4 \cos^4 \pi \beta C_{12} C_{21} C_{33} C_{44} - \frac{\pi^2}{4} \cos^2 \pi \beta C_{12} C_{34} A_{23} A_{41} \\ - \frac{\pi^2}{4} \cos^2 \pi \beta C_{12} C_{43} A_{24} A_{31} + \frac{\pi^2}{4} \cos^2 \pi \beta C_{12} C_{33} A_{24} A_{41} \\ + \frac{\pi^2}{4} \cos^2 \pi \beta C_{12} C_{44} A_{23} A_{31} + \pi^4 \cos^4 \pi \beta C_{12} C_{21} C_{33} C_{44} \\ + \frac{\pi^2}{4} \cos^2 \pi \beta C_{21} C_{44} A_{13} A_{32} + \frac{1}{16} A_{13} A_{24} A_{31} \\ - \frac{1}{16} A_{13} A_{24} A_{32} A_{41} - \frac{\pi^2}{4} \cos^2 \pi \beta C_{21} C_{34} A_{13} A_{42} \\ - \frac{\pi^2}{4} \cos^2 \pi \beta C_{21} C_{43} A_{14} A_{32} - \frac{1}{16} A_{14} A_{23} A_{31} A_{42} \\ + \frac{1}{16} A_{14} A_{23} A_{32} A_{41} + \frac{\pi^2}{4} \cos^2 \pi \beta C_{21} C_{33} A_{14} A_{42} = 0\end{aligned} \quad (35)$$

For the details of the derivation and definition of A_{kl} and C_{mn} one may again refer to [16].

For the same reason two conditions from (31-34) are replaced by:

$$x_d \int_{-1}^1 f_2(\tau_1) d\tau_1 = x_a \int_{-1}^1 [f_3(\tau_2) \sin \theta - f_4(\tau_2) \cos \theta] d\tau_2 \quad (36)$$

$$x_d \int_{-1}^1 f_1(\tau_1) d\tau_1 = -x_a \int_{-1}^1 [f_3(\tau_2) \cos \theta + f_4(\tau_2) \sin \theta] d\tau_2 \quad (37)$$

The singular integral equations have generalized Cauchy kernels and may be solved by using a Gauss-Jacobi [17] or Lobatto-Jacobi quadrature technique [22]. The stress intensity factors at the crack tips can again be derived using their classical definitions.

The Strain Energy Release Rate

From the fracture point of view, perhaps the most important physical quantity is the strain energy release rate G . Using the usual definition [26], it can be written that:

$$G = \frac{d}{da} (U - V) \quad (38)$$

at $x = x_d$, we may write:

$$dU - dV = \frac{1}{2} \int_{x_d}^{x_d + da} \left[\sigma_{y_1 y_1}(x_1, 0) [v(x_1 - da, 0^+) - v(x_1 - da, 0^-)] + \tau_{x_1 y_1}(x_1, 0) [u(x_1 - da, 0^+) - u(x_1 - da, 0^-)] \right] dx_1 \quad (39)$$

The expressions of normal and shear stresses can be found using definition for stress intensity factors. Thus,

$$\sigma_{y_1 y_1}(x_1, 0) = \frac{k_1(x_d)}{\sqrt{2(x_1 - x_d)}} + \text{higher order terms} \quad (40)$$

$$\tau_{x_1 y_1}(x_1, 0) = \frac{k_2(x_d)}{\sqrt{2(x_1 - x_d)}} + \text{higher order terms} \quad (41)$$

To obtain the asymptotic expressions for $[u(x_1, 0^+) - u(x_1, 0^-)]$ and $[v(x_1, 0^+) - v(x_1, 0^-)]$, we can use equations (8) and (9). Following the procedure of derivation as in [27,28], it can be shown that:

$$G_I = \frac{1}{4} \frac{k_1^2(x_d)}{C_{12}} \quad (42)$$

$$G_{II} = \frac{1}{4} \frac{k_2^2(x_d)}{C_{21}} \quad (43)$$

$$\text{and} \quad G = G_I + G_{II} \quad (44)$$

For all the details one may refer to [16].

3. RESULTS AND DISCUSSION

The important results are those pertaining to the kinked crack case. Here for conciseness only this case is studied in details. To determine the stress intensity factors one must first obtain the singularity β by solving equation (35), so certain material properties of orthotropic plate have to be used. The singularity β for an isotropic wedge is given in [19]. Similar results are reported for an orthotropic wedge in [20] and [21]. The numerical values of β obtained from equation (35) for the special cases of isotropic and orthotropic materials compared closely with those computed in [19-21]. Figure 2 shows the variation of the stress singularity power ($-\beta$) with the angle θ for an isotropic and orthotropic material. For the orthotropic case the material properties are listed in Table 1.

As expected for $\theta=0$ (i.e. for a half plane) there is no singularity ($\beta \geq 0$) and the singularity increases with increasing wedge angle. The value of β must eventually reach the value -0.5 (the well-known square-root singularity) for a crack (i.e. when $\theta=180^\circ$). It is interesting to note that for some orthotropic materials the stresses may not be singular even if the wedge angle is larger than 180° .

The stress intensity factors are obtained by solving equations (27-30) in conjunction with equations (36) and (37). Since the integral equations have generalized Cauchy kernels, the collocation methods described in [17] and [22,23] are used. In the results given subsequently, the stress intensity factors are normalized with respect to the uniaxial load σ_0 and the square-root of their respective half crack length. To check the accuracy of the technique the results are first compared with the solutions of special cases that exist in the literature. Table 2. shows the comparison of the mixed-mode stress intensity factors at the tips of a kinked crack embedded in an infinite isotropic plate with those found in [24-25].

As one may infer from the Table, the results compare rather well. The stress intensity factors at the tips of a kinked crack are given in Figures 3-6. Figures 3 and 4 show the variation of the normalized stress intensity factors with respect to crack length ratio L_2/L_1 whereas Figures 5 and 6 display the same results with respect to the angle θ . Results are obtained for orthotropic as well as isotropic materials.

It is seen that (Figures 3 and 4) for a fixed angle θ , normalized $k_1(d)$ and $k_2(d)$ decrease with increasing L_2/L_1 , however the strain energy release rates increase with increasing L_2/L_1 (Figures 7 and 8). So there is a very small chance for crack arrest, as it is illustrated for the 30° plate. On the other hand for varying angle θ (Figures 5 and 6), $k_1(d)$ decreases while $k_2(d)$ first increases then decreases with increasing θ . For this case the total strain energy is not a monotonous function of θ (Figures 9 and 10). Thus the resistance to fracture may strongly depend on the direction of reinforcing fibers. It may be seen that (Figure 9) for isotropic materials G is a monotonically decreasing function with increasing θ , while for the orthotropic material used in the calculations (Figure 10), G first decreases then

increases due to strong influence of mode-II component of the strain energy release rate.

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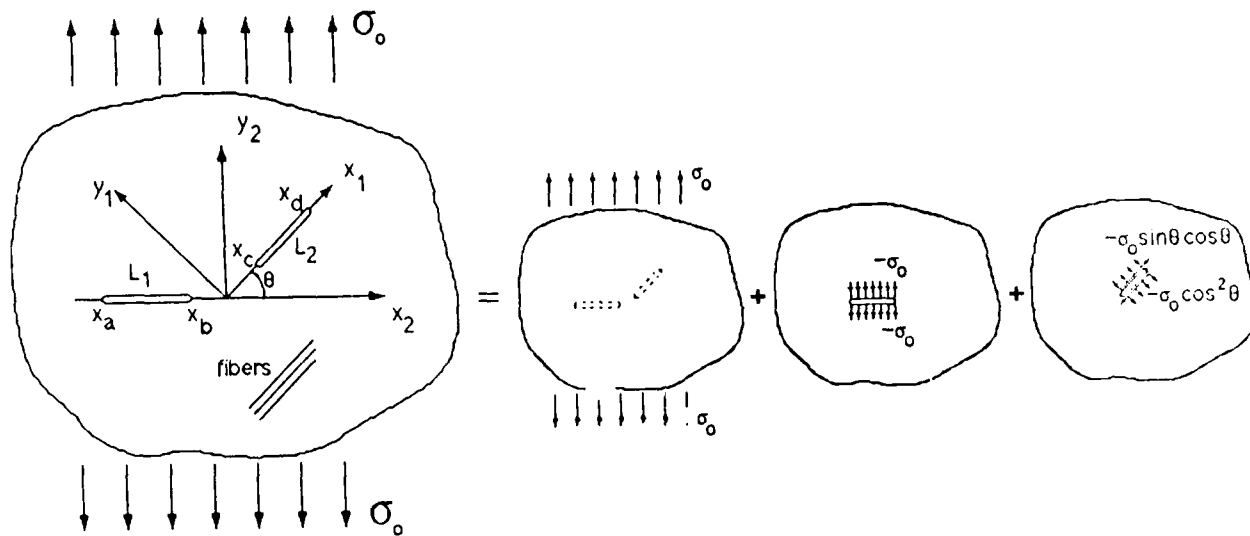


Figure 1. Superposition Scheme for the Infinite Composite Plate with Two Embedded Cracks.

Table 1. Material constants for orthotropic plate.

E_L	21.08 e+06 psi.
E_T	1.5 e+06 psi.
G_{LT}	0.98 e+06 psi.
ν_{LT}	0.3

Table 2. Comparison of present solution with references for the special case of isotropic material.

θ		$k1(a)$	$k2(a)$	$k1(d)$	$k2(d)$
30°	[24]	1.3559	0.0327	1.0873	0.6833
	[25]	1.3508	0.0325	1.0830	0.6804
	Present	1.3421	0.0328	1.0949	0.6855
45°	[24]	1.2902	0.0211	0.7463	0.8405
	[25]	1.2887	0.0208	0.7438	0.8377
	Present	1.2732	0.0217	0.7546	0.8450
60°	[24]	1.2221	-0.0109	0.3900	0.8319
	[25]	1.2194	-0.0116	0.3822	0.8292
	Present	1.2082	-0.0108	0.3941	0.8350

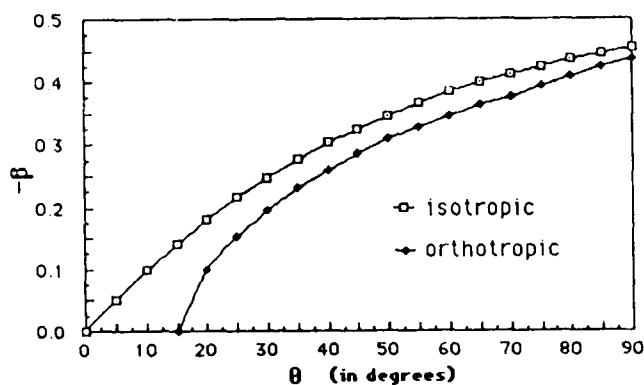


Figure 2. Variation of the Stress Singularity Power ($-\beta$) with the Angle θ .

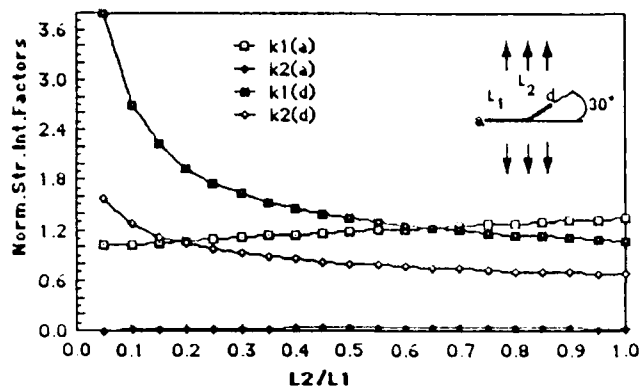


Figure 3. Variation of the Normalized Stress Intensity Factors with L_2/L_1 . Isotropic Case.

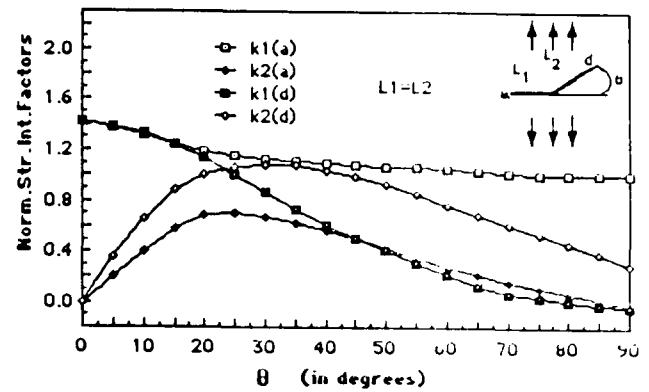


Figure 6. Variation of the Normalized Stress Intensity Factors with the angle θ . Orthotropic Case.

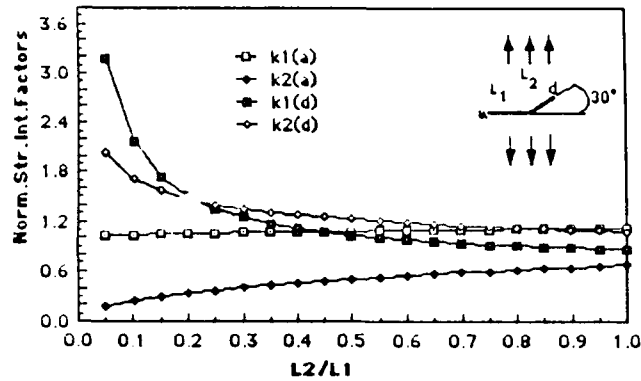


Figure 4. Variation of the Normalized Stress Intensity Factors with L_2/L_1 . Orthotropic Case.

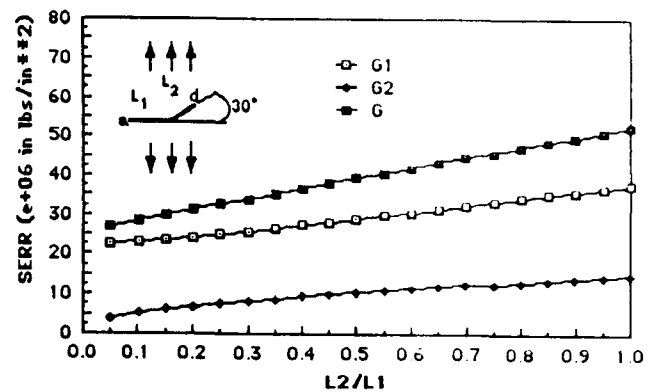


Figure 7. Variation of the Strain Energy Release Rates with L_2/L_1 . Isotropic Case.

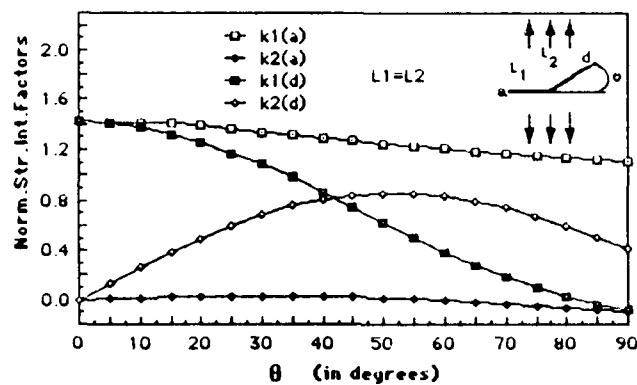


Figure 5. Variation of the Normalized Stress Intensity Factors with the angle θ . Isotropic Case.

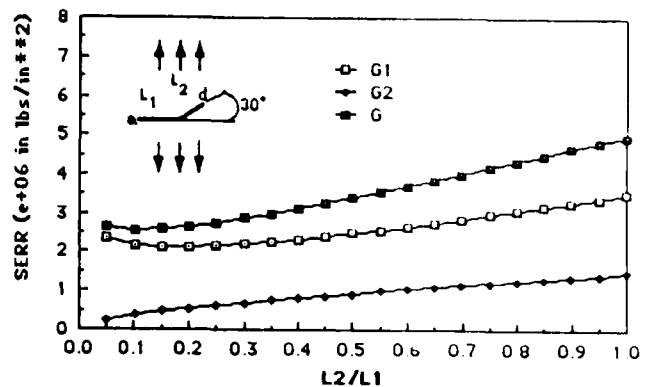


Figure 8. Variation of the Strain Energy Release Rates with L_2/L_1 . Orthotropic Case.

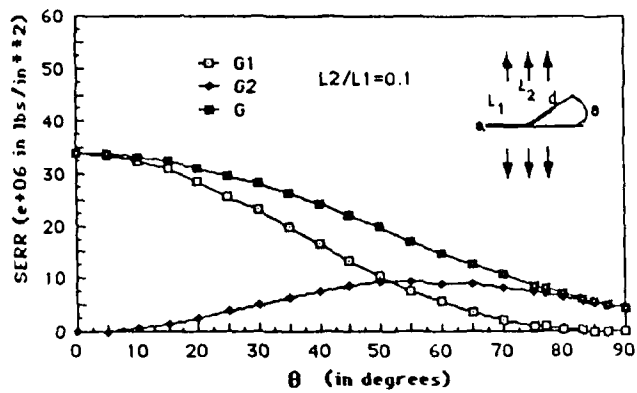


Figure 9. Mixed-Mode Strain Energy Release Rates at the Kink Tip as a Function of Kink Angle θ . Isotropic Case. ($e_x=1$, $L_2 \rightarrow 0$).

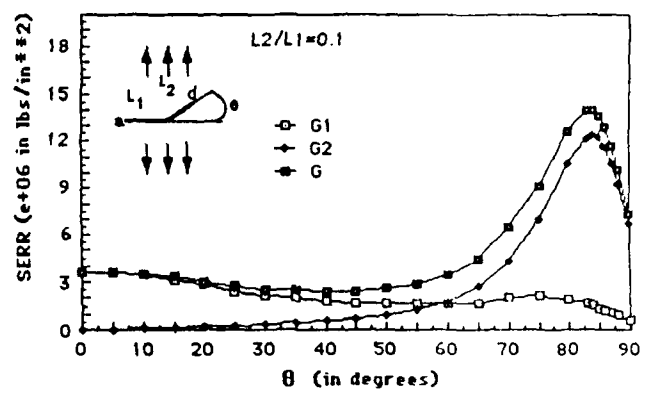


Figure 10. Mixed-Mode Strain Energy Release Rates at the Kink Tip as a Function of Kink Angle θ . Orthotropic Case. ($e_x=1$, $L_2 \rightarrow 0$).

**A COMPREHENSIVE STUDY
ON DAMAGE TOLERANCE PROPERTIES OF
NOTCHED COMPOSITE LAMINATES**

Appendix II

**A Criterion for Mixed-Mode Matrix Cracking
in Graphite-Epoxy Composites**

Paper presented at the ASTM 9th Symposium on Composites, Reno, 1988;
also to appear in ASTM STP.

A CRITERION FOR MIXED-MODE MATRIX CRACKING IN GRAPHITE-EPOXY COMPOSITES

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ABSTRACT: In this paper, mixed-mode matrix fracture in graphite-epoxy composites has been studied. Experimental investigation was conducted on a family of doubly side-notched unidirectional off-axis specimens. By varying the notch depth and the off-axis angle, a total of 28 fracture conditions of differing mixed-mode ratios was produced. Fracture analysis of the test data suggested that the total strain energy release rate is a suitable material condition for mixed-mode matrix cracking in graphite-epoxy composites.

KEYWORDS: graphite-epoxy, mixed-mode matrix fracture, strain energy release rates, finite element analysis, mixed-mode fracture criterion.

Structural composites, notably laminates made of unidirectional tape systems, can sustain extensive matrix cracking before the load carrying fibers fail. Matrix cracking usually occurs at low stress level due to weak interfacial bond strength between matrix and fiber, and between laminating plies. Thus, propagation of matrix cracks in laminates either follows the fiber-matrix interface or the ply-to-ply interface, or both.

Fig. 1 is an x-radiograph taken from a graphite-epoxy [0₂/90₂]s laminate having a center-notch. When the laminate is loaded in uniaxial tension, extensive damage in the form of matrix cracks near the notch can be observed. At this phenomenological scale, matrix cracking can be classified into two major modes. Namely, the intra-ply cracking (fiber-wise splitting) which occurs inside a ply and propagate along the fibers; and the inter-ply cracking

(delamination) which occurs in the interface between two adjacent plies.

In Fig. 1, the four vertical cracks were initiated first near the hole and then propagated along the fibers in the 0^0 -ply. The driving force here is the interfacial shear due to load-transfer from the fiber bundle cut by the hole to the fiber bundle which is uncut. Because of the constraint stemming from bonding between the 0^0 and the 90^0 plies, the vertical splits propagated stably with the applied tension.

As the vertical cracks propagated away from the hole, another mode of load-transfer then took place between the cracked 0^0 -ply and the uncracked 90^0 -ply. Secondary inter-ply stresses along the roots of the vertical cracks were then induced, which then initiated delamination in the $0/90$ interface.

Fracture analysis of the cracked specimen at each major form of cracking reveals that the corresponding crack-tip stress fields are complex and the associated propagation involves both opening and shearing modes.

Model simulation for intra-ply fiber-wise matrix cracking and inter-ply delamination has recently been performed using the strain energy release rate method [1]. This method, when limited to mode-I propagation conditions, has proven useful for modeling brittle matrix cracks in graphite-epoxy systems. In such cases, it is necessary to determine the strain energy release rate G_I at the crack front as driving force, and to validate the corresponding critical strain energy release rate G_{IC} as material resistance [1].

MIXED-MODE FRACTURE CRITERIA

As illustrated in Fig. 1, most matrix cracking in laminates involves mixed opening and shearing modes. However, the applicability of the energy release rate criterion to mixed-mode cracking has not been as firmly established.

Several studies aimed at establishing criteria for mixed-mode matrix

cracking in unidirectional laminates have been conducted in the past using graphite-epoxy composites. Wilkins, et. al. [2] and Ramkumar, et.al [3] used the cracked-lap shear specimen loaded in uniaxial tension to induce mixed mode-I and mode-II delamination between the lap-layer and the substrate layer. By varying the thickness of the lap-layer relative to the substrate layer, mixed-mode ratio, G_{II}/G_I , ranging from 0.35 to 0.45 could be obtained. They observed that the total strain energy release rate $(G_I + G_{II})_C$ obtained under mixed-mode conditions is slightly greater than G_{IC} obtained under pure mode-I conditions. Bradley and Cohen [4] used a cantiliver split-beam specimen loaded by a pair of upward and downward loads applied at the tip of the cantiliver. Variation of the mixed-mode ratio G_{II}/G_I was achieved by changing the ratio of the upward and downward loads. Mixed-mode conditions with G_{II}/G_I ratios ranging from 0 to about 0.6 were produced. They observed that, in composite systems made of brittle matrix, the measured total strain energy release rate $(G_I + G_{II})_C$ increased with G_{II}/G_I ; but it decreased slightly with G_{II}/G_I in systems of ductile matrix. Wang, et. al. [5] used a double side-notched, off-axis unidirectional laminate specimen loaded in axial tension. By varying the off-axis angle from 0° to 90° and the depth of the notches, mixed-mode conditions with G_{II}/G_I ratios ranging from 0 to about 2.5 were achieved. They found that the total strain energy release rate $(G_I + G_{II})_C$ increased with G_{II}/G_I up to about $G_{II}/G_I = 1.5$; it then remained constant for G_{II}/G_I between 1.5 and 2.5.

Russell and Street [6] used specimens of four different configurations and obtained critical strain energy release rates for a wide range of mixed-mode cracking conditions, including pure mode-II cracking. They showed

that the critical strain energy release rates depended on the test specimen and test method used; hence, a general criterion for all the mixed-mode matrix cracking cases tested could not be established.

One possible reason for the lack of a general criterion has been attributed to the manner in which fracture analysis of the test specimens was performed. In the case of a beam-like specimen, the approximate beam theory was employed, while in the case of the plate-like specimen, a finite element plate model was constructed. These analysis methods lacked the required precision to treat complicated singular stress fields, to simulate the actual loading conditions or to properly represent the exact configuration of the cracked specimens. Significant numerical errors could result in the computed fracture quantities, especially for mixed-mode cracking.

Another possible reason stems from uncertainties about the fracture mechanisms associated with pure mode-II cracking. Specifically, ideally pure mode-II cracking is difficult to simulate by tests. In actual experiment, pure mode-II propagation is often accompanied by some amount of friction between the cracked surfaces. The fracture analysis models do not include any such friction mechanisms. A separate criterion may be needed for pure mode-II cracking.

THE PRESENT INVESTIGATION

In this paper, a mixed-mode criterion is suggested for matrix cracks propagating in graphite-epoxy composites. This criterion is based on analysis of test data using specimens of varying cracked configurations, which provide mixed-mode fracture conditions with G_{II}/G_I ratios ranging uniformly from 0 to about 3. The case of predominantly mode-II ($G_{II}/G_I > 3$) or pure mode-II ($G_I = 0$) is excluded. Fracture analysis of the test specimens is performed using a finite element crack growth simulation model, as exact solutions for the test

specimen configurations cannot presently be obtained. The accuracy of the simulation model is, however, adjudicated by comparing results of problems of similar crack configurations whose solutions can also be found rigorously.

Experiment

The specimen used in the experiment is a notched off-axis tension coupon prepared from a unidirectional laminate made of Hercules AS4-3501-06 graphite-epoxy prepreg tape. Fig. 2 depicts the general configuration of the coupon. The overall dimension is 23 cm long and 2.5 cm wide. Excluding the 4 cm end-tabs, the clear section of the coupon is about 15 cm in length. The pair side-notches are introduced at the mid-section by an 8-mil (0.2 mm) thick diamond saw.

The depth of the side-notch a and the off-axis angle θ (between the applied tension and the direction of the fibers) are varied in the test program as follows:

$$\theta = 0^\circ, 5^\circ, 10^\circ, 15^\circ, 20^\circ, 25^\circ, 90^\circ$$

$$a = 2.5 \text{ mm}, 3.2 \text{ mm}, 3.8 \text{ mm}, 4.5 \text{ mm}$$

As depicted in Fig. 3, the coupon can initiate a kink crack (denoted as a') at the side-notch tip and propagate in the fiber direction when the applied tension σ_0 reaches some critical value. The propagation is generally mixed with modes I and II. The degree of mix is determined solely by the angle θ , if the notch depth a is held constant. Conversely, if θ is held fixed, the critical applied tension at the onset of the kink is determined by the notch depth, a .

In this experiment, a total of 28 mixed-mode fracture conditions were created by varying θ and a as mentioned. This has provided fractures with G_{II}/G_I ratios ranging uniformly from 0 to about 3. It should be noted that mixed-mode matrix fracture in such a wide G_{II}/G_I ratio range has not been previously investigated.

In each of the 28 mixed-mode fracture conditions, three to four test specimens were used, with the exception of one case (notch depth = 3.8 mm) where only one specimen was available for some of the off-axis angles.

The tests were conducted in room temperature on a close-loop Instron tester with a load rate of 1800 Kg/min. The critical load at the onset of the kink crack was recorded on a strip chart. Figs. 4,5 and 6 show the experimental plot of critical laminate stress versus the off-axis angle θ at the onset of the kink crack for specimens of side-notches 2.5 mm, 3.2 mm and 4.5 mm deep, respectively. The case for $a = 3.8$ mm is not shown because of insufficient numbers of test specimens.

It is seen from the test results that the critical stress, σ_{cr} , at the onset of the kink decreases sharply with the off-axis angle θ when the notch depth is held constant. Similarly, the critical stress also decreases with the increase of the notch depth, a when the angle θ is held constant.

Post-test SEM examination of the fractured surfaces under 500x to 1000x magnifications revealed extensive fiber breaking in the wake of the kink. Fig. 7 presents two such pictures taken near the kink point. Fiber breaks are visible in all cases. It is believed that the observed fiber breakage is due to the good bond between the matrix and the fiber, resulting in fiber nesting and/or fiber bridging accross the kink path.

Finite Element Analysis

The experimental mixed-mode kinking problem is next simulated by the finite element routine. As mentioned earlier, the simulation model must be adjudicated for it's accuracy. In the interest of conciseness, however, details of this development will not be discussed in this paper. Interested readers are referred to Ref. [7].

Return to the off-axis doubly side-notched coupon section shown in Fig.

2. The unidirectional laminate will be assumed an elastic, homogeneous and

orthotropic plate having constants in the principal material coordinates (L,T) determined as follows:

$$E_L = 145 \text{ Gpa} \quad E_T = 10.3 \text{ Gpa} \quad G_{LT} = 6.7 \text{ Gpa} \quad \nu_{LT} = 0.3$$

Now, let the coupon be loaded by the far-field strain, e_x . At some critical value of e_x , the stresses near one of the side-notch tips are assumed to cause a kink emanating from the notch tip and propagate stably in the direction of the fibers. Of interest is when the length of the kink is small compared to the notch depth a . Then, the mixed-mode strain energy release rates G_I and G_{II} at the kink tip are assumed to control the behavior of the initial kink. The values of G_I and G_{II} are calculated by the finite element routine via a crack-closure technique. These can be conveniently expressed in terms of the applied far-field strain in the form:

$$G_I = C_I(e_x)^2 \quad G_{II} = C_{II}(e_x)^2 \quad (1)$$

where C_I and C_{II} are coefficients from the finite element calculations.

Figs. 8 and 9 show, respectively, the coefficients C_I and C_{II} plotted against the off-axis angle θ , and with the side-notch depth a as an independent parameter. It is seen that the kink is mixed in fracture modes for off-axis angles up to 30° . Beyond 30° , the fracture is essentially mode-I. Variation of the mixed-mode ratio, C_{II}/C_I , with the off-axis angle θ is shown in Fig. 10. This ratio depends principally on θ , and is almost independent of the notch depth a .

Since for each test coupon the critical stress σ_{cr} at the onset of the kink was measured experimentally. The corresponding critical strain $(e_x)_{cr}$ can be calculated by dividing σ_{cr} by the coupon's axial modulus, E_x . Then, using the values of C_I and C_{II} , the critical strain energy release rates $(G_I)_{cr}$ and $(G_{II})_{cr}$ at

the initial kink for each test case can be calculated via Eq. 1.

For test cases where G_I dominated, the deduced $(G_I)_{cr}$ is clearly G_{IC} . However, for the cases where both mode-I and mode-II were present, a combination of $(G_I)_{cr}$ and $(G_{II})_{cr}$ in some form would control the behavior of the kink. Fig. 11 is a diagram depicting the interactions between $(G_I)_{cr}$ and $(G_{II})_{cr}$ determined from all the test cases.

Though the test data show some degree of scatter, the overall trend indicates that the total strain energy release rate $(G_T)_{cr}$ remain more or less a constant. This strongly suggests that $(G_T)_{cr}$ or G_{TC} essentially controls the behavior of the kink, including the special case of mode-I fracture.

Of course, this suggestion is based only on mixed-mode fracture data with G_{II}/G_I ratios ranging from 0 to about 3. In this range, pure mode-II or predominantly mode-II fracture is not included.

It is also noted that, for graphite-epoxy composites, critical strain energy release rate data for matrix fracture have mostly been limited to G_{IC} . Generally, the measured values for G_{IC} lie in the range between 120 to 260 J/m² depending on the material system used. In this study, G_{IC} has the value in the order of 300 J/m². This seems to be on the high side compared to most other accepted values. However, in the present tests, fiber breakage in the wake of matrix cracking was detected in all cases. This could account for the higher measured value for G_{IC} .

CONCLUDING REMARKS

In this paper, mixed-mode matrix fracture in graphite-epoxy composites has been studied using a doubly side-notched, unidirectional off-axis specimen. This specimen has a configuration which is simple to fabricate and versatile in

geometrical variation. As a result, a total of 28 mixed-mode fracture conditions could be produced, which yielded a set of G_{II}/G_I ratios covering uniformly from 0 to about 3.

Based on this data, a more definitive conclusion could be reached regarding the criterion for mixed-mode matrix fracture. Specifically, the total strain energy release rate G_{TC} appears to be a suitable criterion. This criterion, however, may not be applicable to pure mode-II or predominantly mode-II matrix fracture. The latter may involve additional energy dissipating mechanisms such as friction. If so, a separate criterion may be necessary.

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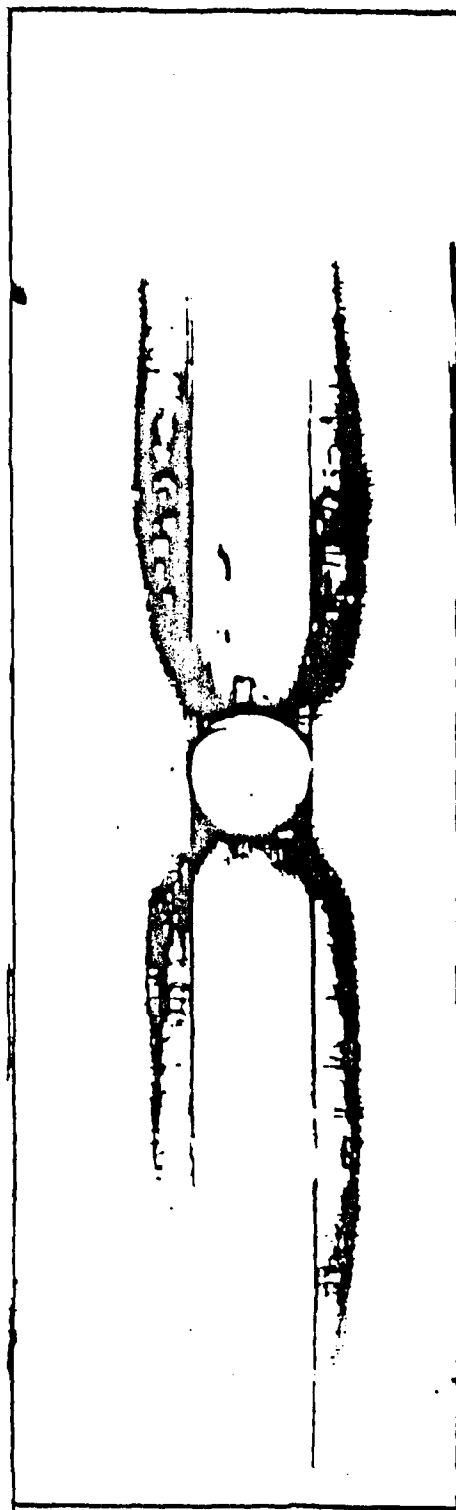


Fig. 1. X-radiograph of matrix crack development in a notched $[0_2/90_2]_s$ graphite-epoxy laminate loaded in axial tension

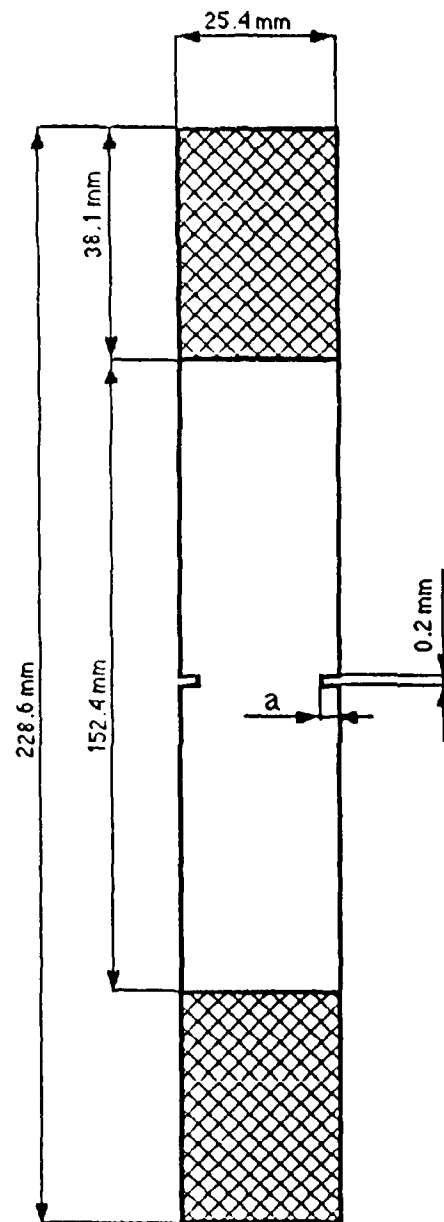


Figure 2 Geometry of the double side-notched specimen.

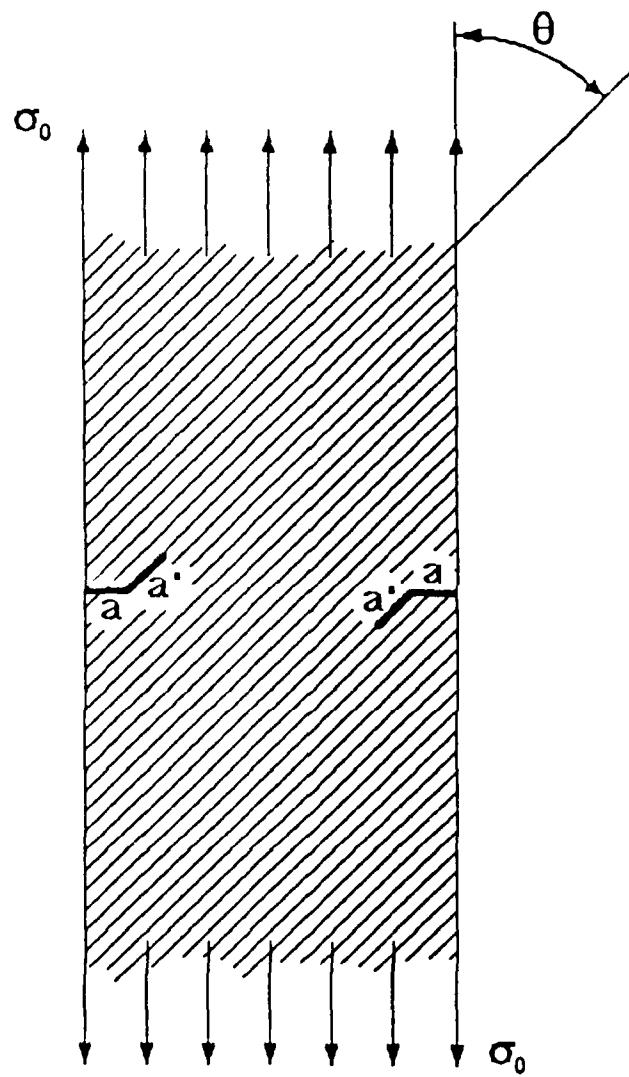


Figure 3 Geometry of kink cracks in the tested specimen.

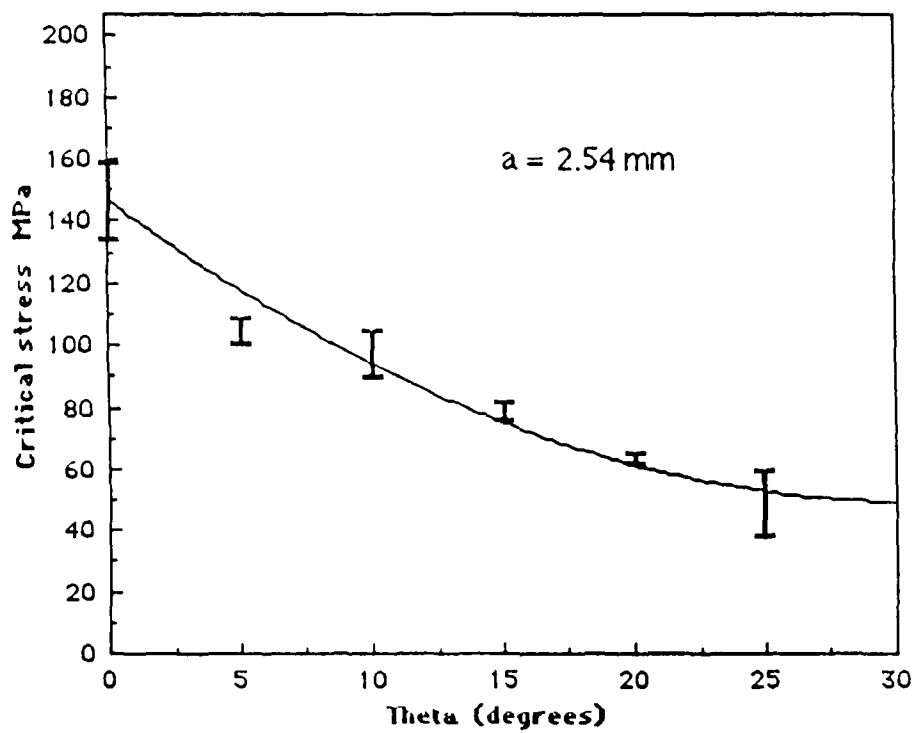


Figure 4 Critical stresses at onset of kink crack. $a = 2.54$ mm.

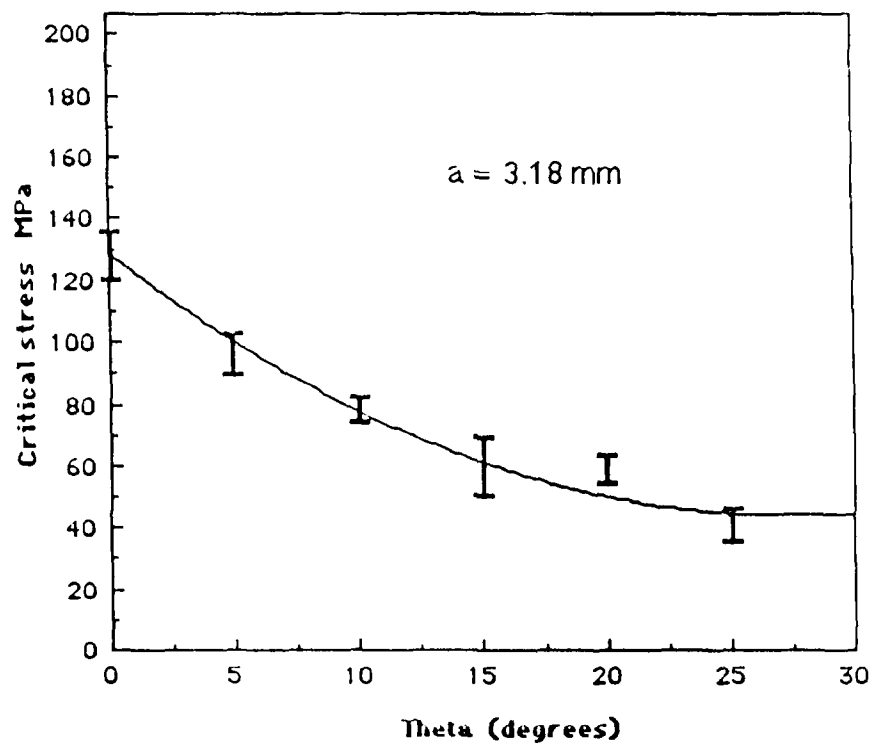


Figure 5 Critical stresses at onset of kink crack. $a = 3.18$ mm.

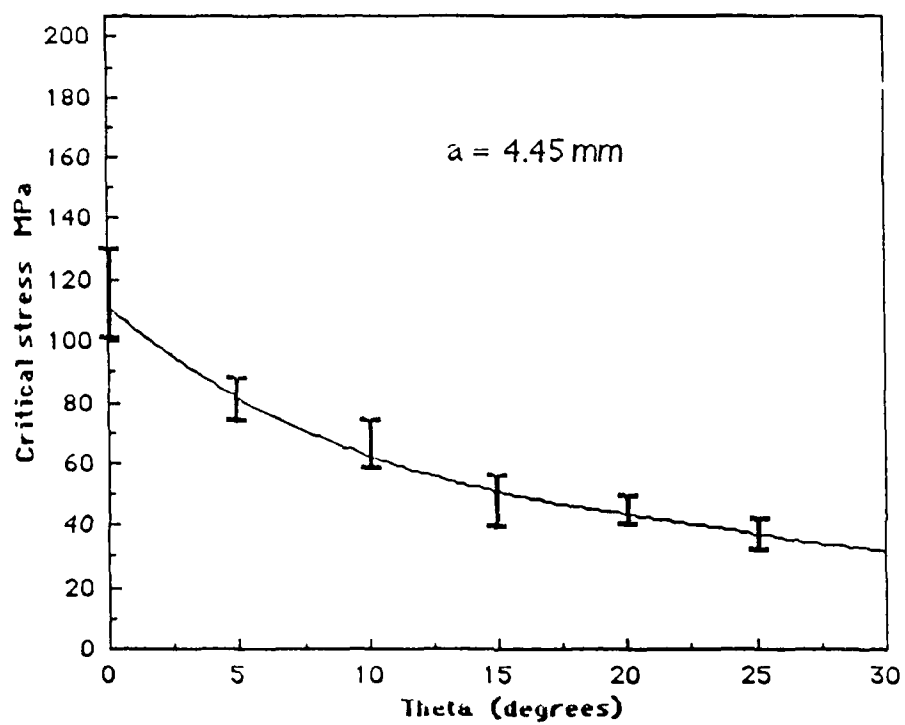


Figure 6 Critical stresses at onset of kink crack. $a = 4.45$ mm.

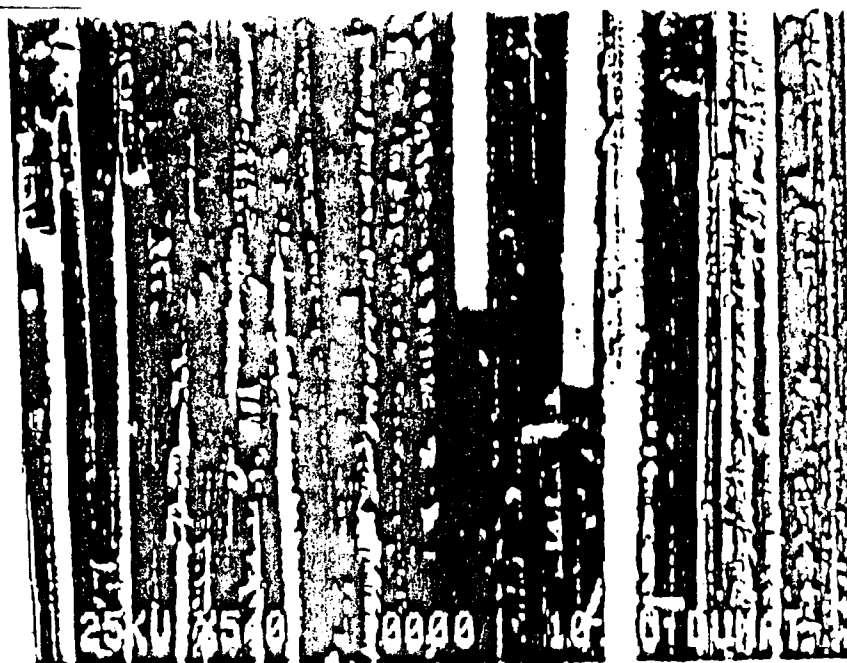


Figure 7 Photomicrographs of fractured surface near kink point. Above: $\theta = 0^\circ$, $a = 3.81$ mm ; below: $\theta = 5^\circ$, $a = 2.54$ mm.

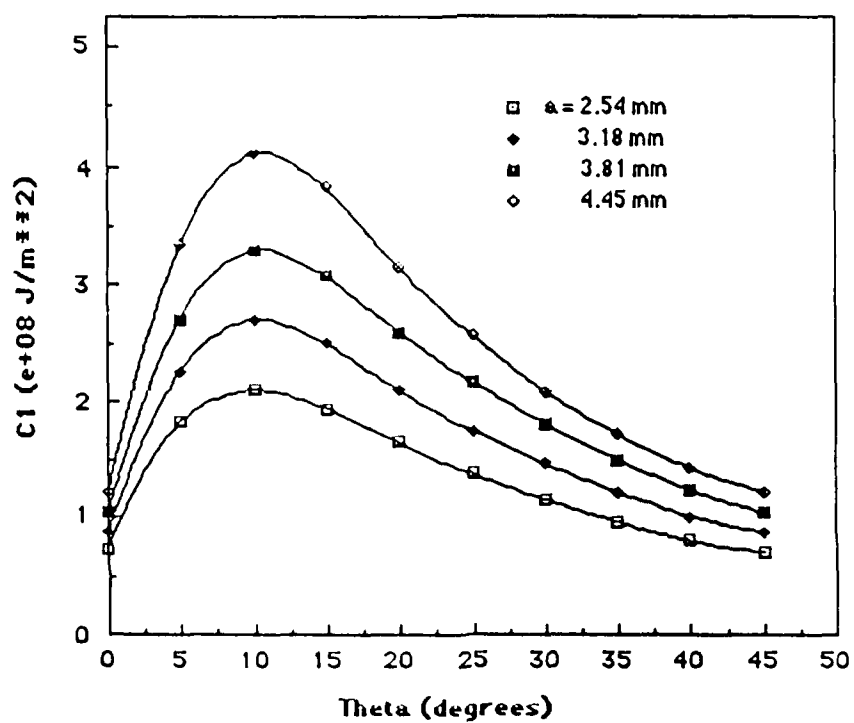


Figure 8 Mode-I strain energy release rate coefficients as function of off-axis angle θ .

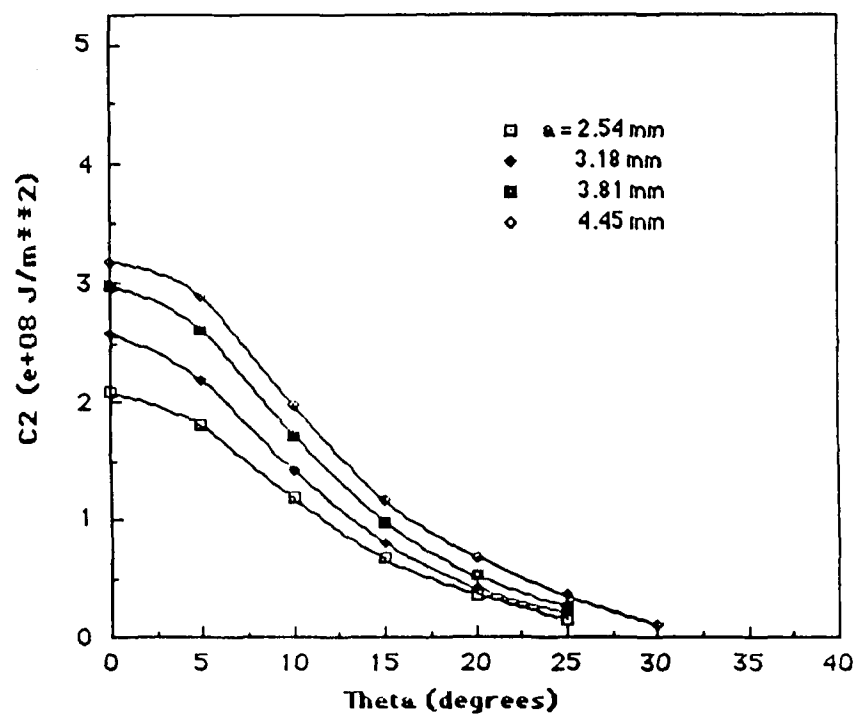


Figure 9 Mode-II strain energy release rate coefficients as function of off-axis angle θ .

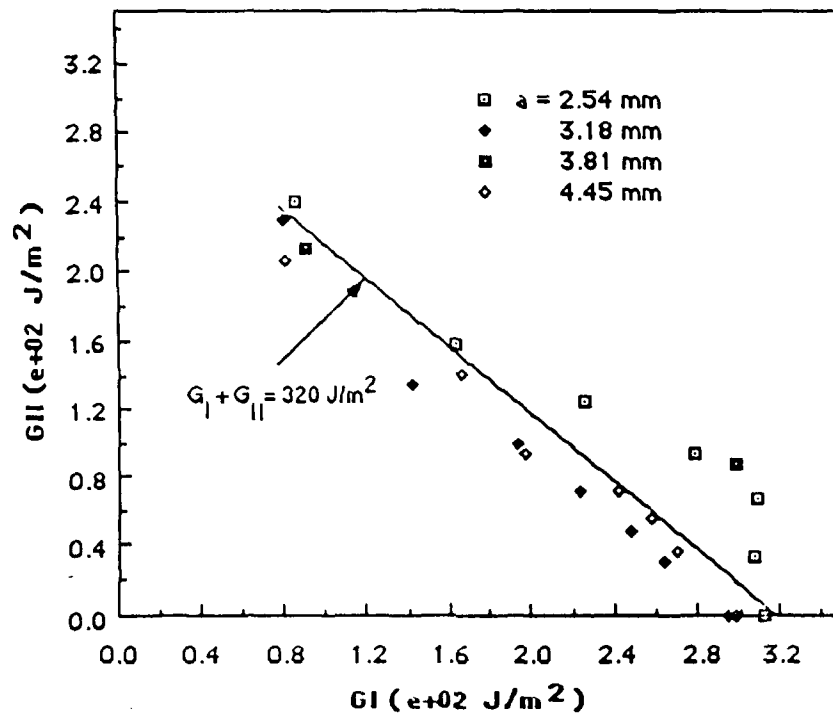


Figure 11 Interaction diagram of mixed-mode strain energy release rate data.

**A COMPREHENSIVE STUDY
ON DAMAGE TOLERANCE PROPERTIES OF
NOTCHED COMPOSITE LAMINATES**

Appendix III

**Three-Dimensional Simulation of Crack Growth
in Notched Laminates**

Paper presented at the 2nd Annual Meeting, Society for composites, Univ. Delaware;
also in Proceedings of the American Society for Composites, 1987. pp. 444-457.

Three-Dimensional Simulation of Crack Growth in Notched Laminates

A. S. D. WANG, E. S. REDDY AND YU ZHONG

ABSTRACT

This paper discusses the matrix cracking sequence in a $[0_2/90_2]_s$ graphite-epoxy laminate with double-side notches. Experiments were performed on specimens loaded in uniaxial, quasi-static tension. The specimens were inspected at ascending load increments by x-radiography for patterns of matrix cracks caused by stress concentration near the notched region.

A numerical procedure based on a 3-D finite element method was then developed to simulate the observed matrix crack initiation, crack interaction and load-dependent crack growth sequence. The simulation begins with an analysis of the 3-D stress field near the notched region. This is followed by a search of possible modes of matrix cracking and the associated condition for propagation. The concept of brittle fracture is invoked to provide the necessary criterion for identifying the appropriate cracking modes and for determining the associated critical loads for their initiation. A comparison between experiment and prediction is presented.

INTRODUCTION

For a class of structural laminates, initial material damage involves two basic forms of matrix cracking [1]. One form is referred to as intraply cracking where a ply, or a layer of several plies of like fiber orientation, suffers a through-the-thickness crack along the fiber direction. Take the $[0_2/90_2]_s$ laminate coupon under uniaxial tension as an example. A fiber-wise crack in the inner 90° -layer, known as transverse cracking, is a case of intraply cracking.

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Similarly, a fiber-wise crack in the outer 0^0 -layer, known as longitudinal splitting, is also a case of intraply cracking. The other basic form is referred to as interply cracking where two adjacent plies suffer a separation in their interface. A delamination in the $0/90$ ply interface of the $[0_2/90_2]_s$ laminate coupon mentioned above is a case of interply cracking. These two basic forms of matrix cracking may occur independently or interactively, depending on the manner of loading and the lamination structure [2]. Generally, one or both of these cracking modes occur before the load-carrying fibers break.

The initiation and growth mechanisms of intraply and interply matrix cracks, when occurring independently, have successfully been described within the frame work of anisotropic ply elasticity and the fracture theory of brittle cracks [3,4]. A 3-dimensional treatment based on the same analysis concept was recently applied to laminates where the two basic cracking modes occur interactively [5]. In these previous studies, the laminate configuration was that of a straight flat coupon, where free-edge effects dominated the mechanisms.

In this paper, we use laminate coupons with double-side notches to study the formation of interactive matrix cracks that emanate from the notch rather than from the free edge. Since the notch is orientated normal to the applied tension, a very strong stress concentration is induced near the notch-tip. Thus, the intensity of concentration is sensitive to the depth of the notch and alters the matrix cracking characteristics.

Experiments were performed on specimens made of a graphite-epoxy laminate in the form of $[0_2/90_2]_s$ tension coupons with side-notches of various depths. For each test specimen, matrix cracking patterns near the notch-tip were inspected by x-radiography at prescribed ascending load increments in order to obtain a load-sequence of the matrix cracking events.

A numerical procedure based on a 3-D finite element method was then developed to simulate the observed load-dependent crack growth. The simulation is based on the strain energy release rate analysis method for non-interactive matrix cracking [3,4] and interactive matrix cracking [5].

A comparisons is made between the predicted load-sequence of events and those recorded experimentally for specimens of different notch depths.

EXPERIMENT

The material used in the experiment was the AS4-3501-06 graphite-epoxy unidirectional system. $[0_2/90_2]_s$ laminate panels were made using an autoclave curing procedure. Test coupons were cut from these laminate panels, with dimensions of 25.4 mm wide and 228.6 mm long; the specimen thickness was about 1.016 mm. Double side-notches were introduced at the mid-section

of the coupon by an 8-mil (0.008 in.) diamond saw. Specimens of four notch depths were so prepared (2.54 mm, 3.175 mm, 3.81 mm and 4.445 mm).

Tensile loading was applied to the test specimen through an Instron tester with the cross-head speed set at 0.25 mm per minute. At prescribed ascending load increments, the specimen was x-radiographed at the notched section in order to determine the developing matrix cracking patterns.

For all the specimens tested, the x-radiographs revealed three major forms of matrix cracking during loading. In order of their occurrence, these include longitudinal splitting in the 0^0 -layer which emanates from the notch-tip, transverse cracks in the 90^0 -layer along with the progression of 0^0 -layer splitting and, at some higher load, $0/90$ interface delamination growing stably along the length of the 0^0 -layer splitting boundary.

Fig. 1 is a sketch of the developing cracking pattern from a specimen with side-notch 3.175 mm deep. It is seen that at the laminate stress of 112 Mpa, a pair of 0^0 -layer splits of measurable length emanated from the notch-tip. Initially, the split at one notch-tip grew upward while the split at the other notch-tip grew downward. The growth was extremely stable. At 172 Mpa, splits in four directions emerged from the notch-tips; and a few 90^0 -layer transverse cracks appeared between the parallel splits. The 0^0 -layer splits grew in length while the 90^0 -layer transverse cracks grew in numbers as the laminate stress increased; see sketch corresponding to 259 Mpa. Then, while the splits were still growing, a measurable $0/90$ interface delamination initiated along the split boundary near the notch-tip, see sketch corresponding to 319 Mpa. The delamination grew stably as the laminate stress increased; see sketch corresponding to 345 Mpa. The specimen ruptured through the notch section at laminate stress well beyond 600 Mpa.

Fig. 2 is a plot of the measured length of the 0^0 -layer split versus the laminate stress, using data from two test specimens having notch depth of 3.175 mm. The scatter in the data is due to variation of the split lengths in four directions. The mean length is taken as the average of the splits in four directions. The laminate stress levels at which 0^0 -layer splitting, 90^0 -layer transverse cracking and $0/90$ interface delamination initiated were all recorded.

Fig. 3 is a plot of the measured $0/90$ interface delamination (in area) versus the laminate stress from the same two test specimens. The delamination area at different load increments were measured from prints of x-radiographs using an Lemont Scientific image analyzer. The procedure involves magnification of the delamination area by a high resolution video camera which traverses the contour of the delamination. The scatter of the measured values is due to the variation in areas from the four branches of delamination. From the plot, onset of $0/90$ interface delamination may be extrapolated. In this case, delamination onset had occurred at about 260 Mpa.

Table 1 summarizes the onset stresses of the three major forms of matrix cracking from specimens of four notch depths. It is seen that for each form of matrix cracking, onset stress decreases with increase of notch depth. This is expected because the deeper the notch the larger is the stress concentration at the notch-tip.

SIMULATIONS

The Finite Element Model. To simulate the specimen used in the experiment, let us consider the $[0_2/90_2]_s$ laminate having double side-notches at regular interval as shown in Fig. 4a. Assume that these double side-notches are spaced so far apart that they do not interact with one another. Then a periodic element of the laminate which contains only one pair of notches is isolated as shown in Fig. 4b. This element thus represents the test specimen. Note that the laminate is symmetric with respect to the laminate mid-plane (the x-y plane), and the y-axis lies in the plane of the notches. Hence, it is sufficient to model one-eighth of the element shown in Fig. 4b. A schematical finite element mesh is shown in Fig. 4c. Due to expected stress concentration near the notch-tip and ply interfaces, a finer mesh is always deployed in these regions.

The finite element routine was developed based on the assumption that the unidirectional ply is an elastic, homogeneous and orthotropic medium. The elastic and other pertinent material constants for the AS-3501-06 system were characterized by routine tests [6], and their values are listed in Table 2. Solutions for stresses and other quantities, such as strain energy release rates, were obtained by employing a 21-node brick element. The actual computation was carried out on VAX-11/750 and Cray X/MP computers. These and other computational details are found in [7].

Notch-Tip Stress Fields. The laminate stress fields were calculated for two types of loading. The first is by prescribing a far-field laminate strain of $\epsilon_x = 10^{-6}$, and the other is by prescribing a uniform temperature change of $\Delta T = -1^\circ\text{C}$. Stresses due to applied laminate tension (by giving a value for ϵ_x) and laminate post-cure cooling (by giving a value for ΔT) can then be obtained by superposition.

Although there are six stress components at each finite element node, it is of interest to examine only those components that are responsible for the observed matrix cracking initiation.

First, let us examine σ_y in the 0° -layer. This stress is thought to cause 0° -layer split, which in fact was observed as the first mode of failure under a very low tensile loading. For the case of $\epsilon_x = 10^{-6}$, σ_y is tensile throughout the thickness of the 0° -layer near the notch region. Its value varies from the top to

the bottom of the layer, with the minimum occurring near the 0/90 interface. Fig. 5a shows the σ_y distribution in the 0°-layer near the 0/90 interface for the specimen having notch depth of 3.175 mm. A sharp rise of σ_y in tension is seen to occur at the notch-tip, displaying a singular behavior. A similarly behaved in-plane shear stress τ_{xy} is also present at the notch-tip; its planar distribution near the 0/90 interface is shown in Fig. 5b. The concentration intensities of σ_y and τ_{xy} at the notch-tip are about the same.

For the case of $\Delta T = -1^\circ\text{C}$, σ_y is also tensile throughout the 0°-layer. Fig. 5c shows the σ_y distribution in the 0°-layer near the 0/90 interface. Here, stress concentration due to the notch is much less. But, by the magnitude of this stress throughout the 0°-layer is quite large. Thus, the combined tensile and thermal loading will cause the 0°-layer splitting to be in mixed modes.

Next, let us examine σ_x in the 90°-layer. This stress causes 90°-layer transverse cracking. Again, for the case of $e_x = 10^{-6}$, this stress is tensile and varies throughout the thickness of the 90°-layer near the notch region, with the minimum occurring near the 0/90 interface and the maximum at the mid-plane. Fig. 6a shows the σ_x distribution in the 90°-layer near the laminate mid-plane for the specimen having 3.175 mm notch depth. It is seen that a sharp tensile stress is again developed at the notch-tip.

Similarly, for the case of $\Delta T = -1^\circ\text{C}$, σ_x in the 90°-layer is also tensile with significant magnitude; but stress concentration caused by the notch is minimal, Fig. 6b. Other stress components also exist in the 90°-layer near the notch-tip; but their magnitudes appear to be negligible.

Finally, the nature of the interlaminar stresses (σ_z , τ_{xz} , τ_{yz}) should be examined because these stresses are responsible for interface delamination. For the same specimen considered under $e_x = 10^{-6}$ loading, its σ_z distribution on the 0/90 interface is shown in Fig. 7a, while distribution on the 90/90 plane is shown in Fig. 7b. It is seen that σ_z can be tensile and of significant magnitude; but it exists only near the notch-tip. As for σ_z caused by thermal cooling, the associated magnitude for σ_z is relatively small. Similarly, the interlaminar shear stresses, τ_{xz} and τ_{yz} also exist with highly localized magnitudes at the notch-tip.

From the above analysis, it appears that 0°-layer splitting and 90°-layer transverse cracking are equally likely to occur, while the likelihood for interface delamination is comparatively smaller. However, judgement regarding relative occurrence of these cracking events cannot be made based on the computed stresses, as they all display some degree of stress concentration. In what follows, we attempt to simulate the onset of the observed cracking modes from a fracture point of view.

Simulation of 0°-Layer Splitting. To simulate the initiation and growth of 0°-layer splitting, we shall assume that 90°-layer transverse cracking will

not simultaneously occur. Then, at the notch-tip, we issue a small 0^0 -layer split of length s_0 as shown by the insert in Fig. 8. This small split represents an effective flaw which exists at the notch-tip and propagates to become a 0^0 -layer split whenever a certain condition is reached. Under a constant far-field strain loading, the split is assumed to propagate stably to reach a length $s > s_0$. Thus, the finite element simulation is to calculate the split-tip stresses and the associated fracture quantity. For the latter, we calculate the split-tip strain energy release rate G as a function of the split length, s .

As was mentioned earlier, the tensile normal stress σ_y and the in-plane shear stress τ_{xy} in the 0^0 -layer are the major stress components causing splitting. Fig. 8 is a plot of the split-tip stress σ_y versus the split length, s , for the specimen having 3.175 mm side-notches subjected to $e_x = 10^{-6}$ loading. It is seen that σ_y is larger when s is small, but it decreases sharply with increase of s . On the other hand, the associated shear stress τ_{xy} (not shown) became relatively more dominant with increasing s .

When subject to thermal cooling of $\Delta T = -1^0\text{C}$, σ_y in the 0^0 -layer is also tensile, see Fig. 5b. But variation of σ_y at split-tip due to growth of split is rather insignificant.

To facilitate a prediction for the load versus split-growth relationship, we then calculate the split-tip strain energy release rate, $G(s)$. This quantity is conveniently expressed in terms of the loads e_x and ΔT [5]:

$$G(s) = [\sqrt{C_e} e_x + \sqrt{C_t} \Delta T]^2 d \quad (1)$$

where ΔT represents thermal cooling and d is a length scale which is set at unity in this study. The coefficients C_e and C_t are functions of s and represents the strain energy release rates, corresponding to $e_x=1$ and $\Delta T=-1^0\text{C}$, respectively.

Figs. 9a and 9b show, respectively, the coefficients C_e and C_t versus the split length s for the specimen with 3.175 mm notch depth. It is seen that C_e and C_t both contain mixed modes. However, C_e is predominantly of mode-II, while C_t predominantly of mode-I. When the two load agencies are combined, as in Eq. (1), a mixed-mode cracking of approximately equal ratio results. Note that the overall strain energy release rate is one which decreases with the split length, s ; This indicates a stable splitting growth, a behavior consistent with that observed in the experiment.

The load versus split-growth relation is derived from the fracture criterion,

$$G(s) = G_c \quad (2)$$

where G_c is the critical strain energy release rate for mixed mode cracking. Assume that the value of ΔT is given. Then, by combining Eqs. (1) and (2) we

obtain the critical laminate strain $(\epsilon_x)_{cr}$ as a function of split length, s .

For the material system used in this study, ΔT and the mixed mode G_c have been determined elsewhere [6]; and their values are listed in Table 2. Thus, the predicted $(\epsilon_x)_{cr}$ can be converted to $(\sigma_x)_{cr}$. For the specimen just considered, the computed $(\sigma_x)_{cr}$ versus s relations is shown by the solid line in Fig. 2. It is seen that the predicted result agrees well with the initial portion of the experimental split-growth data, where the splitting was not yet significantly complicated by the development of 90° -layer transverse cracks. The predicted curve, however, departs away from the observed results as 90° -layer transverse cracks developed in higher density. To include the effects of these transverse cracks on split growth will require a major modification of the simulation model.

The predicted onset stresses for 0° -layer splitting for specimens of four notch depths are listed in Table 1 along with their experimental counterparts. In all cases, the model seems to predict well the initiation of the splitting.

Simulation of $0/90$ Interface Delamination. Delamination of the $0/90$ interface takes place at much higher load. Once initiated, it grows stably along the boundary of the 0° -layer splits. The delamination pattern is shown schematically in Fig. 10. As we have observed in the experiment, 90° -layer transverse cracks actually formed continuously as the $0/90$ interface delamination grew, see Fig. 1. An analytical/computational simulation of this complex interactive cracking phenomenon, though not impossible, is quite tedious and probably not fruitful. Thus, a simplified version is attempted instead. Namely, we shall assume that only 0° -layer splitting precedes the initiation of the $0/90$ interface delamination and the effect of 90° -layer transverse cracking is negligible.

The simulation follows a similar procedure as used for 0° -layer splitting. A set of densely meshed finite elements is deployed near the intended delamination region, double nodes are assigned on the plane of delamination and these are then released in sequence so as to mimic the actual growth pattern observed in the experiment. In the node-releasing process, the fracture energy release rate coefficients, C_e and C_t are computed as functions of the delaminated area [7]. Figs. 11a and 11b show, respectively, the computed C_e and C_t coefficients versus delamination area for the specimen having the notch depth of 3.175 mm.

From the energy release rate curves, it is seen that the delamination is primarily of mode-III under the applied tensile loading ($\epsilon_x=1$), while primarily in mode I and II under thermal cooling ($\Delta T=-1^\circ\text{C}$). Thus, the combined effect is again one of mixed modes. The overall energy release rate, however, decreases sharply with increasing delamination area, indicating a stable growth. This is

also consistent with the behavior observed in the experiment.

Using the computed energy release rate coefficients, stress levels corresponding to the prescribed delamination node-releasing sequence can be predicted by means of Eqs. (1) and (2). For example, in the case of the specimen having notch depth of 3.175 mm, the predicted delamination growth curve is shown by the solid line in Fig. 3. Here, again, the agreement between prediction and experiment is quite close for the initial portion of the delamination growth. Apparently, as the delamination grows larger, many transverse cracks are formed in the 90°-layer; the associated cracking mechanisms then becomes more complicated than the model has portrayed.

The critical stresses for 0/90 delamination in specimens having other notch depths were also computed. These are listed in Table 1 for comparison with their experimental counterparts.

CONCLUSIONS

In this paper, we have presented a method of simulation for matrix cracks that develop in laminate specimens having double side-notches. The analysis entails a 3-D stress analysis and computer simulation of fracture growth near the notched region. The purpose of the study is to understand the damage mechanisms at a level below the lamination structure. The actual specimen chosen for analysis could represent a critical element [8] in a large laminated structure whose global strength and/or fatigue properties are to be evaluated.

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Table 1. Experimental and Predicted (in parenthesis) Onset Stresses

Notch Depth	2.54 mm	3.175 mm	3.81 mm	4.445 mm
0°-layer	100 Mpa	75 Mpa	60 Mpa	60 Mpa
Split	(80 Mpa)	(70 Mpa)	(60 Mpa)	(60 Mpa)
90°-layer				
Transverse	170 Mpa	160 Mpa	150 Mpa	150 Mpa
Crack				
0/90 interface	350 Mpa	260 Mpa	225 Mpa	220 Mpa
Delamination	(300 Mpa)	(230 Mpa)	(200 Mpa)	(180 Mpa)

Table 2. Pertinent Material Constants for AS4-3501-06 UD Ply

$$E_{LL} = 145 \text{ Gpa} \quad E_{TT} = E_{ZZ} = 10.3 \text{ Gpa} \quad G_{LT} = G_{LZ} = 6.8 \text{ Gpa} \quad G_{TZ} = 3.5 \text{ Gpa}$$

$$\nu_L = \nu_{LZ} = 0.3 \quad \nu_{TZ} = 0.54 \quad \alpha_L = 0.4 \times 10^{-6}/^{\circ}\text{C} \quad \alpha_T = \alpha_Z = 28.8 \times 10^{-6}/^{\circ}\text{C}$$

$$\Delta T = -140^{\circ}\text{C} \quad (G_c)_{\text{total}} = 289 \text{ J/m}^2 \quad \text{Ply Thickness} = 0.127 \text{ mm}$$

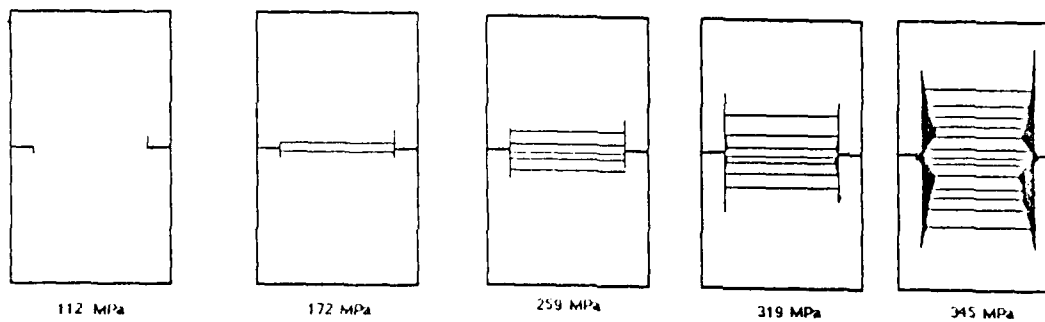


Fig. 1. Development of Matrix Cracks in Specimen of Notch Depth 3.175 mm

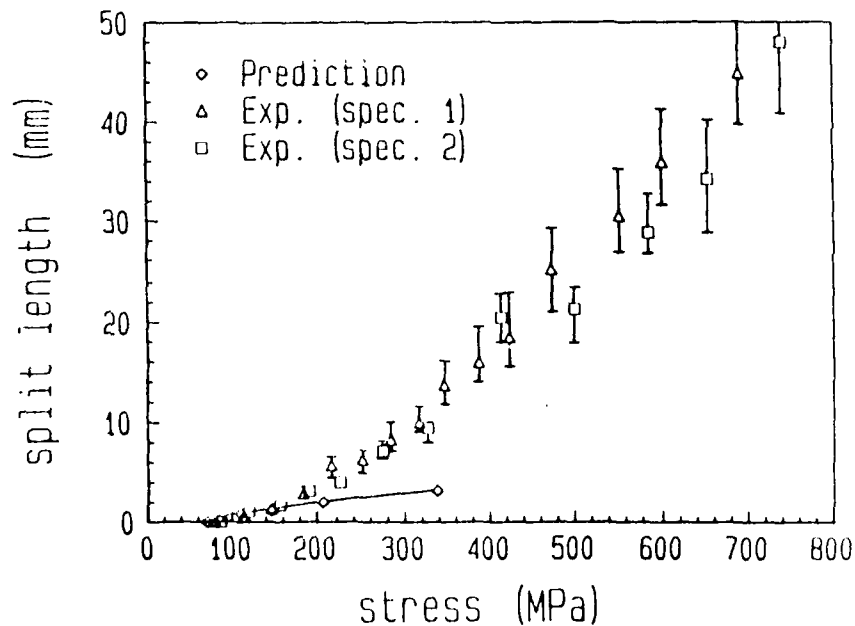


Fig. 2. Split Length Growth versus Applied Tension (Notch Depth, 3.175 mm)

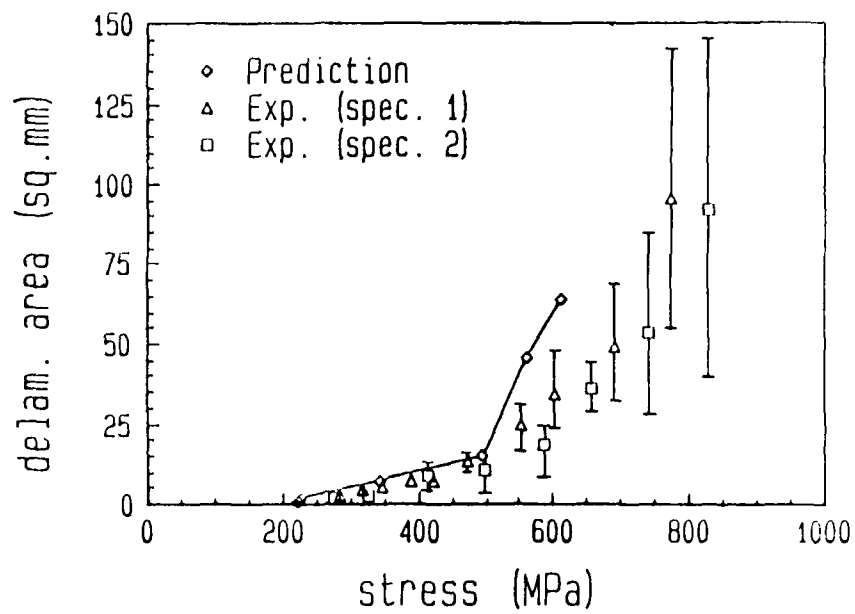


Fig. 3. Delamination Area Versus Applied Tension (Notch Depth 3.175 mm)

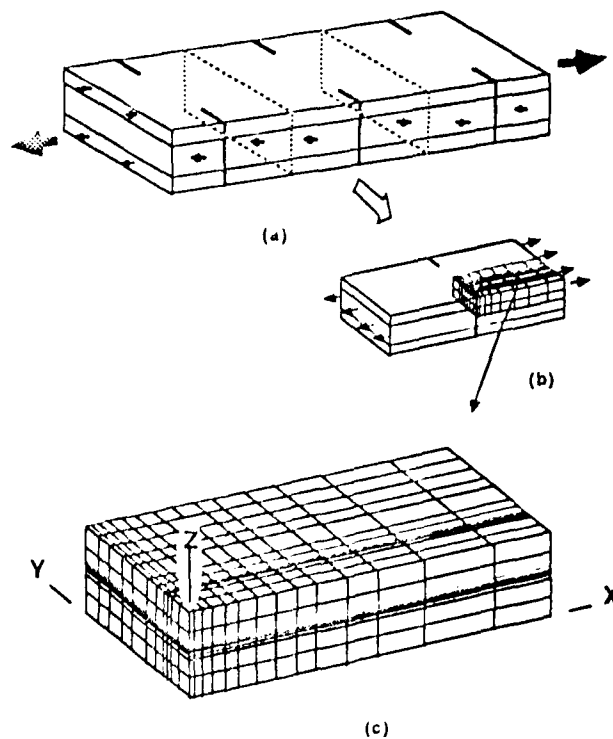
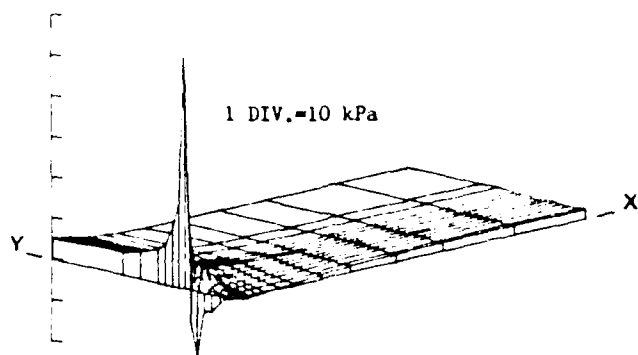
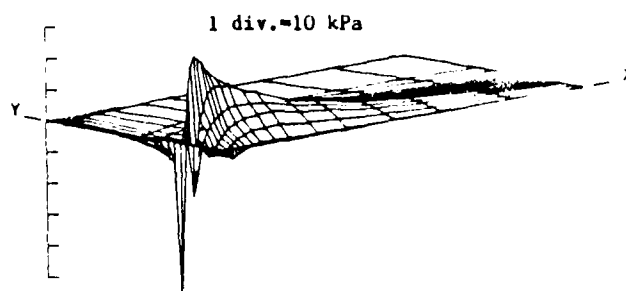


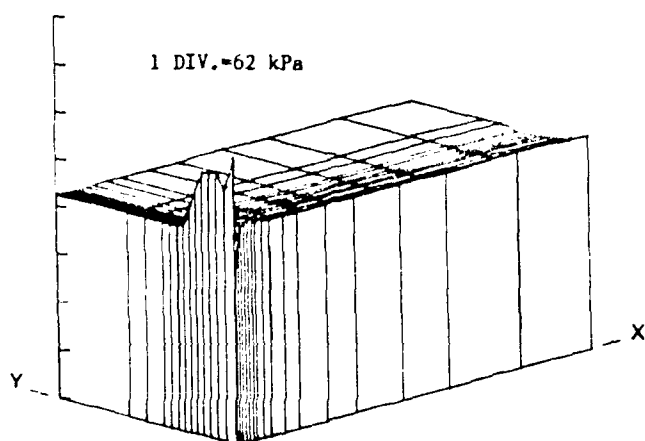
Fig. 4. Finite Element Model for the Double Side-Notched Specimen.



(a)

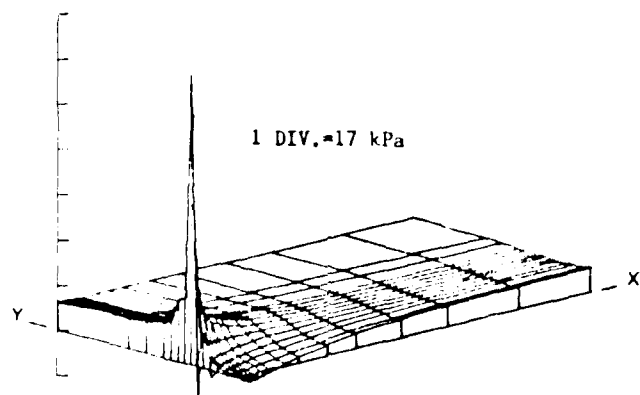


(b)

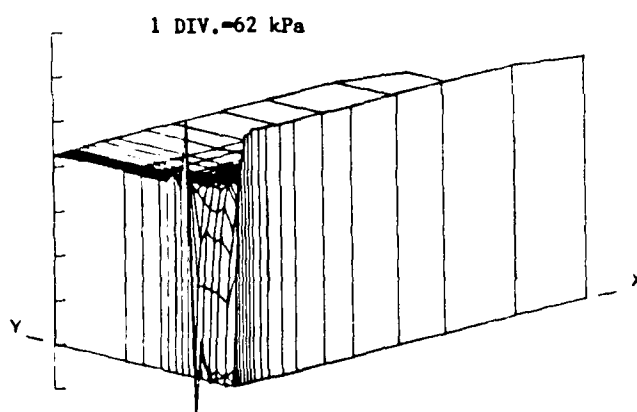


(c)

Fig 5. (a) σ_y Distribution in the 0^0 -Layer ($e_x=10^{-6}$)
(b) τ_{xy} Distribution in the 0^0 -Layer ($e_x=10^{-6}$)
(c) σ_y Distribution in the 0^0 -Layer ($\Delta T=-1^0C$)

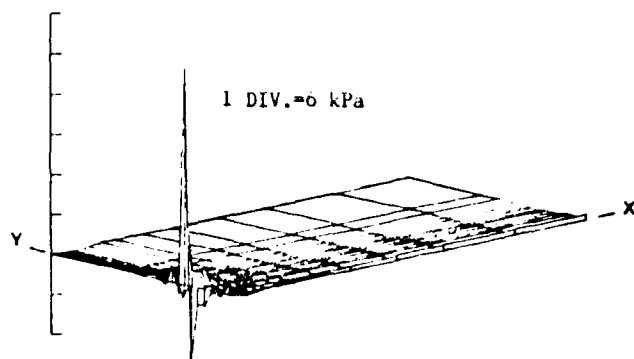


(a)

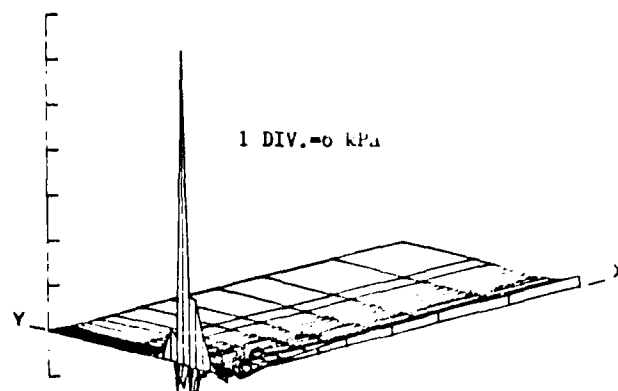


(b)

Fig 6 σ_x Distribution in 90^0 -Layer due to (a) $e_x=10^{-6}$ and (b) $\Delta T=-1^0C$



7(a)



7(b)

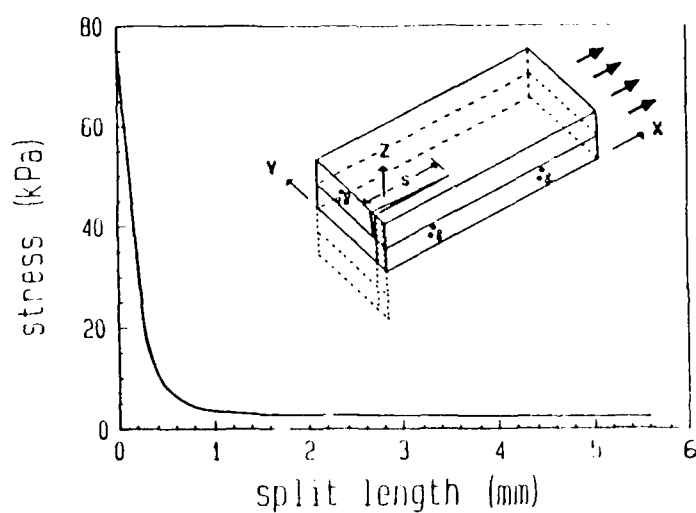
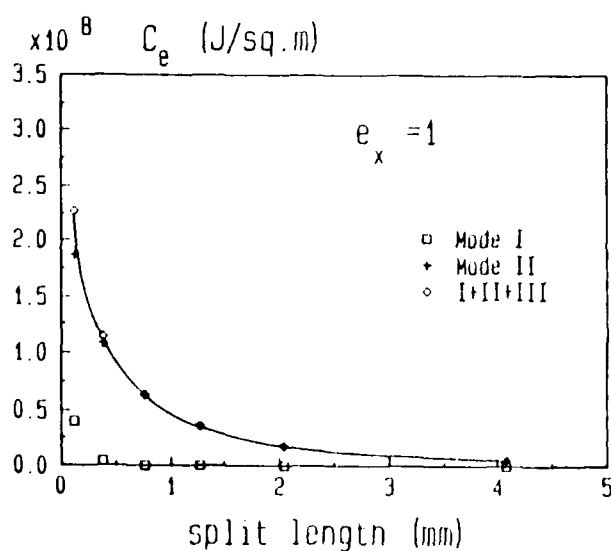


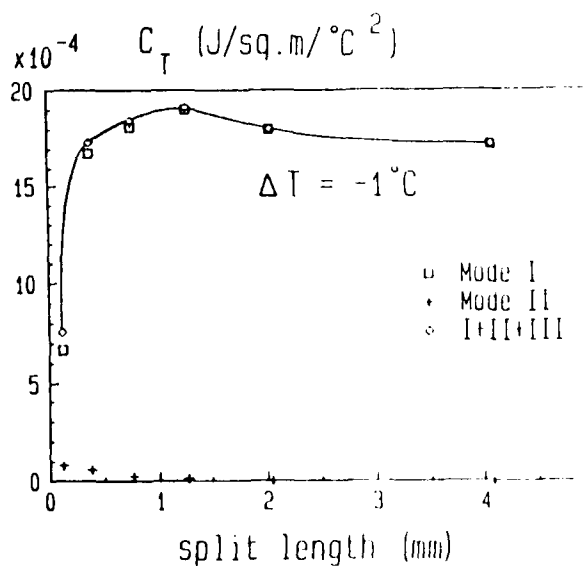
Fig. 7. σ_z Distribution on (a) 0/90 and (b) 90/90 Interface

Fig. 8. σ_y at Split-tip versus Split Length, s

Fig. 9. Energy Release Rates at Split-tip. (a) C_e and (b) C_t

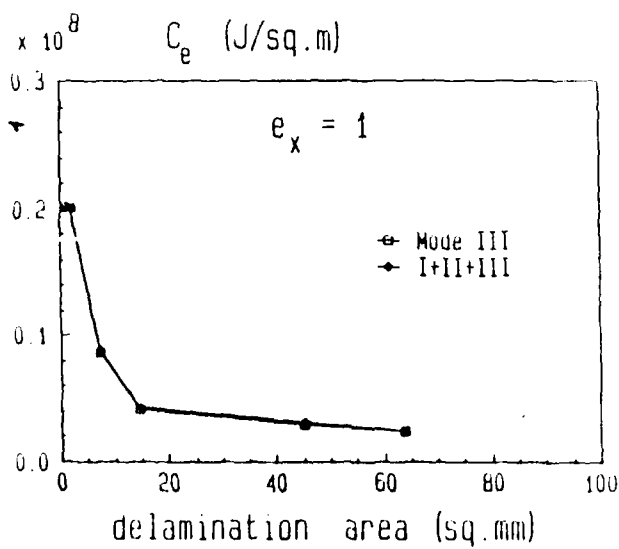
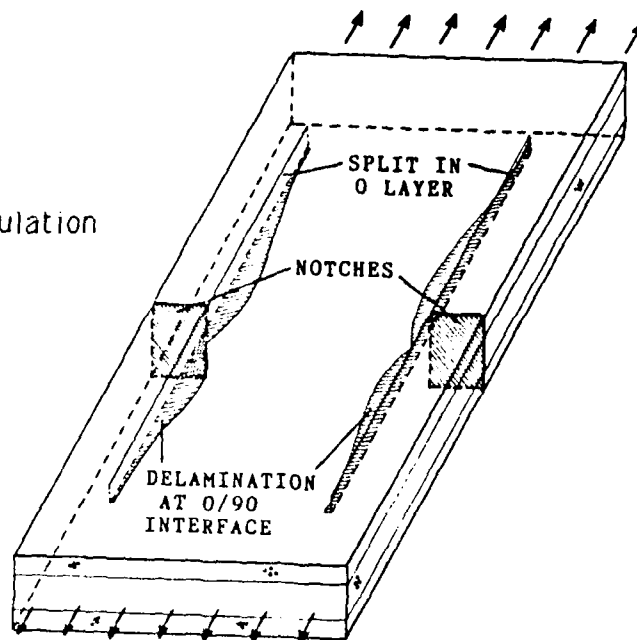


9(a)

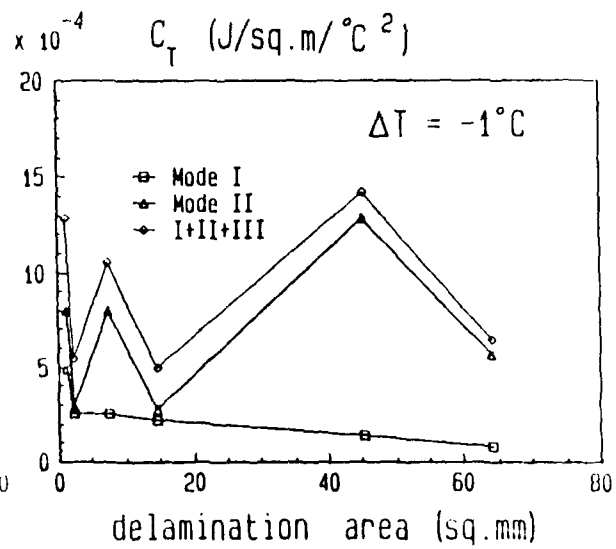


9(b)

Fig. 10. Finite Element Simulation of 0/90 interface Delamination.



(a)



(b)

Fig. 11. Energy Release Rate at Delamination Front: (a) for $e_x=1$, (b) $\Delta T=-1^\circ\text{C}$

**A COMPREHENSIVE STUDY
ON DAMAGE TOLERANCE PROPERTIES OF
NOTCHED COMPOSITE LAMINATES**

Appendix IV

**Simulation of Matrix Cracks in Composite Laminates
Containing a Small Hole**

Paper presented at the ASME Winter Annual Meeting, Boston, 1987;
Also in Damage Mechanics in Composites, AD-12, ASME, 1987. pp. 83-91.

SIMULATION OF MATRIX CRACKS IN COMPOSITE LAMINATES CONTAINING A SMALL HOLE

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ABSTRACT

This paper studies the matrix cracking sequence in $[0_2/90_2]_S$ graphite-epoxy laminates that contain a small central hole. Experiment was performed first using specimens loaded in uniaxial, quasi-static tension, followed by inspections of the specimen at several prescribed loading increments by means of x-radiography. The inspection provides a quantitative measurement and a physical analysis of matrix cracking patterns near the hole.

A numerical procedure based on a three dimensional finite element method was then employed to simulate the observed matrix cracking patterns, including their initiation and growth behaviors. Here, the theory of ply elasticity and the concept of brittle fracture are used as basis for the finite element simulation.

A comparison between the simulated and the experimental results is presented.

1. INTRODUCTION

Failure analysis of fiber-reinforced composites has attracted increased interest in recent years. Application of composites in high-performance aircraft and spacecraft structures has led the researchers to carry out intensive experimental and theoretical studies on failure mechanisms in a variety of composite materials. For a special class of composites, namely, polymeric laminates made by laminating unidirectional continuous fiber systems, failure initiation usually involves some forms of matrix cracking. When viewed at the laminating ply level, these can be classified into two basic forms. One basic form is known as intraply cracking, where a ply or several plies of the same fiber orientation that formed a layer, suffers a through-the-thickness crack along the fiber direction. A simple example of intraply cracking is found in a $[0/90]_S$ type laminate under axial tension, in which the inner 90° -layer suffers multiple transverse cracks. The other basic form is interply cracking,

where two adjacent plies in the laminate suffer a separation in their interface. Free edge delamination in a $[\pm 45/0/90]_S$ laminate loaded in axial tension, for instance, provides a case of interply cracking.

Studies of the individual growth mechanisms of the two basic forms of matrix cracking have been extensively reported in the literature (see, e. g. [1,2]). Interactions of the two basic forms of cracking were examined partially in [3,4]. The problems studied in [1-3] concerned cracking development in plain laminates, while the problem studied in [4] involved laminates that contain sharp through-the-thickness notches.

The problem of laminates with a through-hole has also attracted considerable attention. Effects of ply stacking sequence [5] and different material combinations [6] on global laminate strength reduction due to presence of a small hole were among the early interests. Subsequent analyses have focussed on the detailed stress distribution around the hole, especially the interlaminar stresses that cause local delamination [7-9]. In these works, delamination (interply cracking) is assumed to take place as the only matrix cracking mode. Experiments using graphite-epoxy laminates have shown, however, that the first matrix cracking form near the hole is usually not delamination.

In the present study, we use a $[0_2/90_2]_S$ graphite-epoxy laminate with a small central hole to examine the initiation and growth patterns of matrix cracks near the hole. In this case, a three dimensional stress analysis based on ply elasticity performed, which shows that severe stress concentrations along the hole boundary are present, and matrix cracks of different forms may initiate and propagate at these locations. At the same time, experiments performed on test specimens and inspected by x-radiography at different loading levels reveal the exact sequence of the various cracking events. Thus, the purpose of this study is to relate the experimental events with the analysis by means of a finite element simulation. A comparison is then made between the simulated and the experimental results.

2. EXPERIMENT AND RESULTS

In the experiment, test coupons were made from AS4-3501-06 graphite-epoxy prepreg tapes. The lamination stacking sequence was limited to $[0_2/90_2]_s$. The dimensions of the test coupons were 25.4 mm wide, 228.6 mm long and 1.016 mm thick. The radius of the central hole was 3.175 mm. Loading was applied axially on an Instron tester, with the cross-head speed set at 0.25 mm per minute. The loaded specimens were periodically inspected by DIB enhanced x-radiography.

Experimental results show three major forms of matrix cracking that emanate from the hole during loading. In their order of occurrence, these are (1) horizontal transverse cracks (intraply cracking) in the inner 90° -layer in the immediate region of the hole initially, and away from the hole region subsequently; (2) vertical splitting cracks (also a form of intraply cracking) in the outer 0° -layers emanating from the hole and propagating stably away from the hole; and (3) delamination (interply cracking) in the $0/90$ interface along the length of the 0° -layer splits, which displays a very stably growth behavior. Figure 1 is a schematic illustration of the cracking development patterns at five typical laminate stress levels. It can be seen that a few (two or three) 90° -layer transverse cracks and an initial sign of 0° -layer splits are present at about 276 MPa. At 379 MPa, four branches of 0° -splitting have already formed and propagated stably towards the top and bottom of the specimen. Note that propagation of the splits are accompanied by more 90° -layer transverse cracks, see illustration at 465 MPa. At 552 MPa, delamination in the $0/90$ interface has already occurred along each of the four 0° -layer splits. While the delamination grows stably with load, more transverse cracks have formed and the longer the 0° -splits have grown, see illustration at 724 MPa. At this load level, matrix-related damage around the hole is substantial; but no significant fiber breakage has yet occurred. In fact, the specimen can sustain a laminate stress of more than 1000 MPa before it breaks completely through the hole section.

To express quantitatively the observed matrix cracking, we choose to display two separate cracking quantities in terms of the applied laminate stress. The first is the linear length of the 0° -layer split and the second is the area of the $0/90$ interface delamination. Since for each specimen there are four branches of splits, which grow stably with load, a mean length is obtained from measuring all four splits at each load interval. Figure 2 shows the mean split length plotted against the laminate stress (in MPa), where the data are from a sample of six specimens. It is seen that the mean split grows almost linearly with the applied laminate stress; and by extrapolation of the data, we can deduce that the onset stress for 0° -layer split is at about 120 MPa. The solid line in the figure represents the simulated split growth. The adequacy of the simulated result will be discussed in the next section.

Similarly, Figure 3 shows the mean delamination area measured from the same six specimens (the areas were measured from the x-radiographs using an image analyzer). Here again, we see that the delamination growth rate is quite slow initially but becomes rapid as the laminate stress is increased. Data extrapolation yields the onset stress at about 220 MPa. The solid line in this figure is the

simulated results. However, we shall defer the discussion on simulation later in the next section.

3. SIMULATIONS AND RESULTS

The Finite Element Model

The problem of a laminate with a single central hole stems from the large composite structural panel with bolt or rivet holes. In these large structural laminates, the holes are often periodically placed. Assuming that the holes are located so far apart that they do not interact with each other, then a periodic element of the panel containing only one hole can be considered for analysis, see Figure 4a. For the problem considered here, the lay-up of the specimen is $[0_2/90_2]_s$ and the hole is placed at the laminate center. Thus, it is sufficient to model one-eighth the specimen due to symmetry as shown in Figure 4b. Since stress concentration is expected around the hole boundary, a finer finite element mesh is deployed in this region in order to capture the true nature of stress concentration.

It is noted that the basis of the finite element analysis is the assumption of ply elasticity, that is that the graphite-epoxy unidirectional ply is assumed as an elastic, homogeneous and orthotropic medium. The elastic and the thermal expansion constants of the AS-3501-06 ply system were characterized and given in [11]. The basic finite element is a 21-node solid brick and the computation is performed on a CRAY X/MP computer. Details of the computational procedures are contained in a separate user's manual [12].

Stresses Near the Hole Boundary

The laminate stress fields are calculated for two types of loading. The first is by prescribing a far-field laminate strain of $\epsilon_x = 10^{-6}$, and the other is by prescribing a uniform temperature change of $\Delta T = -1^\circ\text{C}$. Stress due to combined tension and temperature change can be obtained by superposition. The stress field near the hole boundary is in a complicated three dimensional state. It is not of interest here to examine all of the stress distributions in detail. Rather, we shall display only some typical ones that are thought to cause matrix cracking.

Figure 5 is a display of the xy-plane distribution for the six stress components which exist in the 0° -layer near the $0/90$ interface. Here, the largest stress is σ_x which is in the fiber direction. But when compared to the ply strength in the fiber direction this stress is rather insignificant in causing failure. Other stresses that may cause matrix failure are σ_y and τ_{xy} . These two combined can cause longitudinal splitting in this layer. The interlaminar shear stresses τ_{zx} and τ_{zy} are relatively small but could precipitate $0/90$ interface delamination.

Figure 6 is a similar display for the stresses in the 90° -layer near the mid-plane of the laminate. Here, we see the dominance of σ_x which is normal to the fibers in this layer. Concentration near the hole region will be certain to cause transverse cracks. All other stresses however have secondary influence.

The above stress distributions are computed for tension

loading only. We also need the stress distribution due to thermal cooling of the laminate in order to evaluate the combined stress state. For the laminate used, ΔT is set at -140°C . For simplicity however, we shall omit the display of the thermal stresses here.

From the stress analysis, it appears that 0° -layer splitting and 90° -layer transverse cracking are equally likely to occur first. However, because of the high stress gradient near the hole boundary it is not possible to make a prediction as to which of these two forms of matrix cracking will first occur. In what follows, we attempt to simulate the onset and propagation of some of these cracking forms from a fracture point of view.

Simulation of 0° -Layer Splitting

In the simulation of 0° -layer splitting, it is assumed that 30° -layer transverse cracks are absent while the split grows with loading. This assumption is necessary to reduce the geometric complexity of the cracked laminate. It is felt that omission of the transverse cracks will not adversely affect the accuracy of the simulation, at least not the onset of splitting. To simulate, a small split length s_0 is introduced in the 0° -layer as shown by the inserted sketch in Figure 7. This small split represents an effective material flaw which exists at the hole boundary and propagates to become a split whenever the critical condition is reached. Under the far-field constant strain loading, the split is assumed to propagate stably in the fiber direction. Thus, the finite element routine is to calculate the stresses and the strain energy release rate G at the split-tip as a function of split length s .

Figure 7 shows the variation of σ_y at the split-tip with the split length s . It is believed that this stress is responsible for the initiation and continuation of the split. From the figure, we see that σ_y is large when s is small; but it decreases sharply with increase of s . On the other hand, the associated split-tip shear stress τ_{xy} , which is not shown here, becomes relatively more dominant with increase in s . This indicates that once the split starts, it will propagate stably and in shearing mode.

The corresponding stresses due to thermal cooling are also calculated; their effect on splitting is included in the prediction, which is to be discussed below.

As mentioned, we first introduce a small split length s_0 and then calculate the split-tip strain energy release rate $G(s)$ as a function of $s \geq s_0$. $G(s)$ can be expressed in terms of the applied tension σ_x and thermal loading ΔT as [5]:

$$G(s)_i = \left\{ \left[\sqrt{C_\sigma} \sigma_x + \sqrt{C_T} \Delta T \right]^2 d \right\}_i \quad i = \text{I, II, III} \quad (1)$$

where the coefficients C_σ and C_T are functions of s and represent the strain energy release rates corresponding to $\sigma_x = 1$ and $\Delta T = -1^\circ\text{C}$, respectively. The parameter d is a length scale factor which is set at unity in this study. Finally, the subscript i refers to cracking modes of I, II and III (open, sliding and antiplane shear).

Now, Figures 8 and 9 show, respectively, the coefficients C_σ and C_T versus the split length s . Note that C_σ is predominantly of mode II, while C_T is predominantly of mode I. Thus, the combined crack growth is in mixed

mode.

The growth behavior of mixed mode matrix cracking is discussed in [11] and a criterion based on the total energy release rate is suggested:

$$\sum G(s)_i = G_c \quad (2)$$

where G_c is the total critical strain energy release rate for mixed mode cracking. For the material used here, G_c has a value estimated at 289 J/m^2 .

By using the coefficient curves in Figures 8 and 9, we can obtain from (1) and (2) the critical laminate strain ϵ_x as a function of split length s . The computed critical laminate strain can be readily converted in laminate stress; the stress versus s relation is shown in Figure 2 by the solid line. It is seen that the calculated result agrees initially with the experiment, predicting the onset of splitting. As the split grows longer, the splitting mechanisms are complicated by transverse cracking and also by delamination. Since these complicating mechanisms are not included in the splitting simulation model, a discrepancy between the experiment and the prediction results.

Simulation of $0/90$ Interface Delamination

In the simulation of the $0/90$ interface delamination, an idealization is also made. Namely, we assume that delamination occurs after the 0° -layer splitting has grown a sufficient length so that delamination is proceeding as an independent event. The simulation is carried out to mimic the delamination shape as observed in the experiment. The simulated shape is schematically shown in Figure 10. Here, we calculate the mean strain energy release rate coefficients at the delamination front as a function of the total delaminated area, see Figures 11 and 12. Then, by means of the criterion in (2) we obtain the delamination area versus laminate stress relation as shown in Figure 3 by the solid line. Again, the prediction for the onset of delamination is close, but discrepancy results once the delamination has grown larger. This is expected because the actual growth of delamination is concurrent with other forms of matrix cracking, as discussed earlier in the experimental study. This complex mechanics mechanisms was not included in the simulation model.

4. CONCLUSIONS

In this paper, we have shown that growth of matrix cracks in the vicinity of a small hole in a laminate can be reasonably simulated by a finite element routine based on a careful fracture mechanisms analysis. Still, the actual mechanisms are complicated and the simulation has to resort to some degree of idealization. This causes discrepancies between the simulated results and the experiment. It is conceivable that these discrepancies can be considerably removed if more is known about the physics of the phenomenon at the microscopic level and if a more powerful simulation technique is available.

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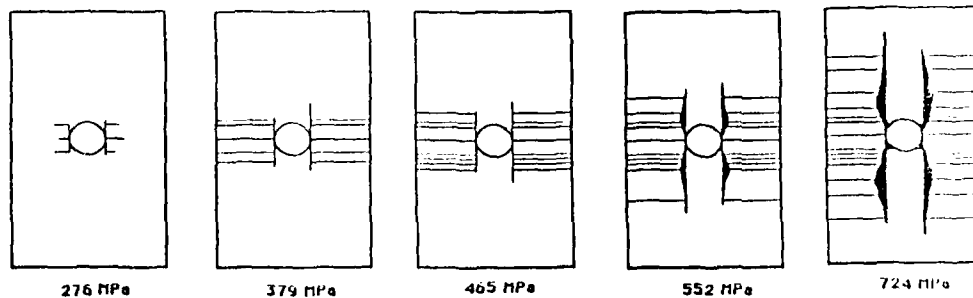


Figure 1. Progression of Matrix Cracking Under Ascending Laminate Stress.

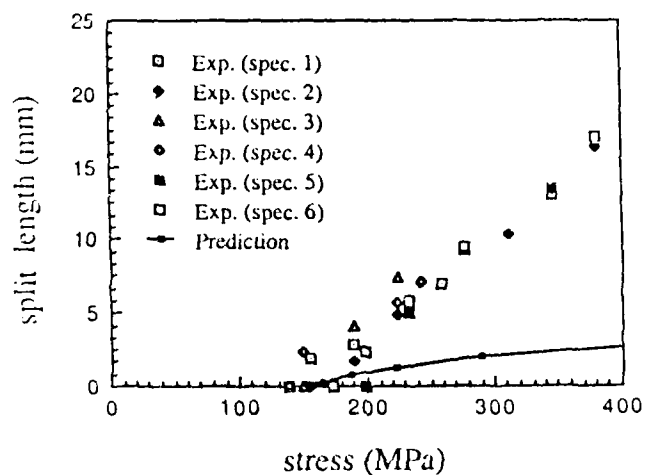


Figure 2. Splitting Length Versus Applied Laminate Stress.

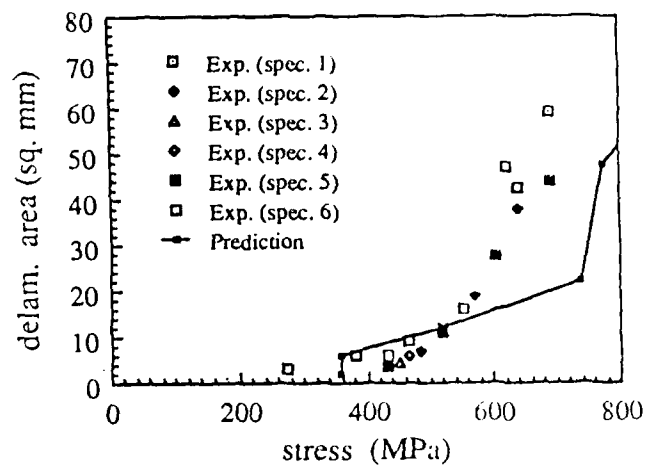


Figure 3. Delamination Area Versus Applied Laminate Stress

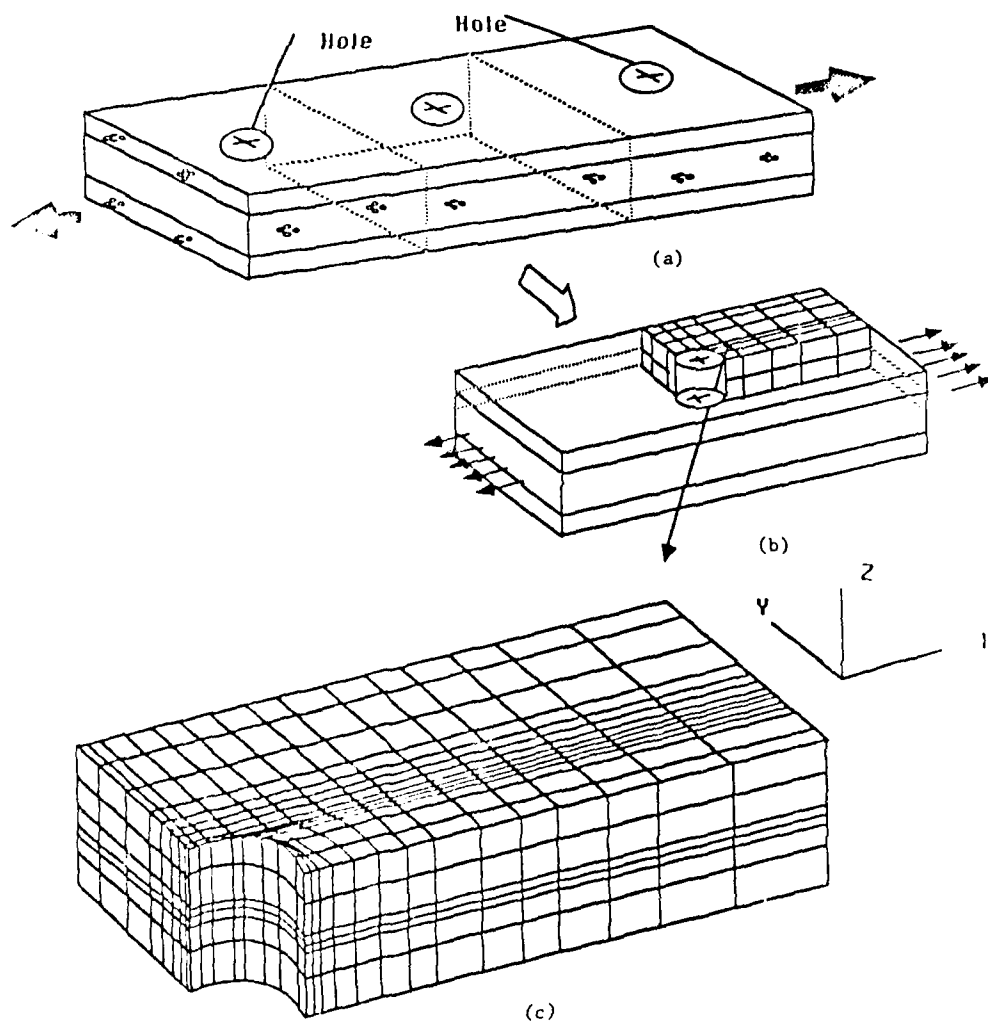


Figure 4. Finite Element Model for the Specimen Containing A Small Hole

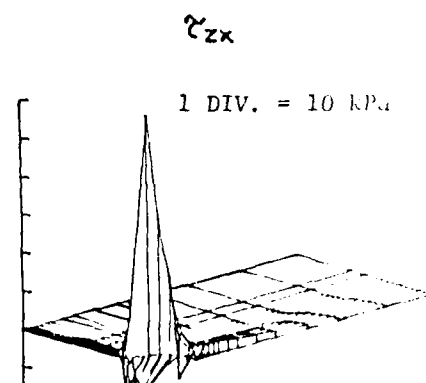
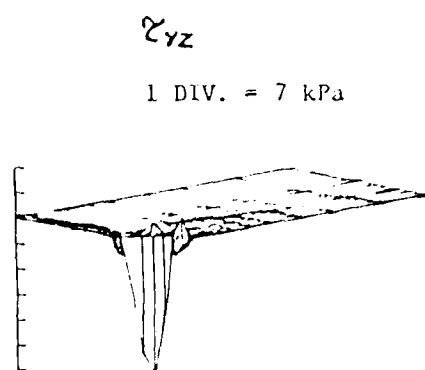
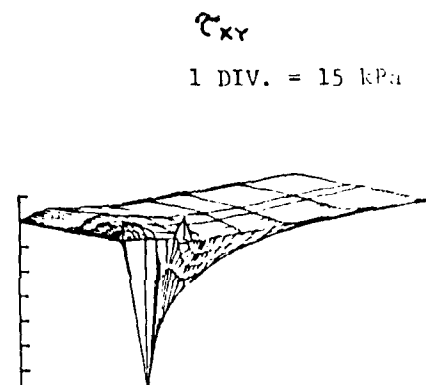
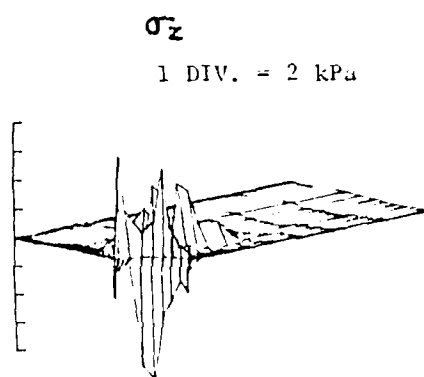
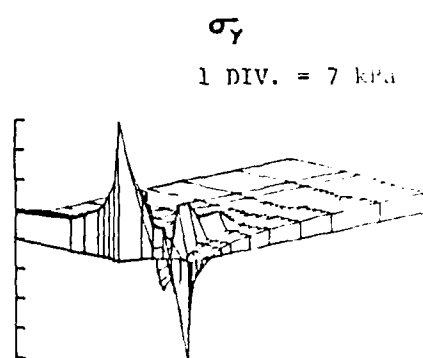
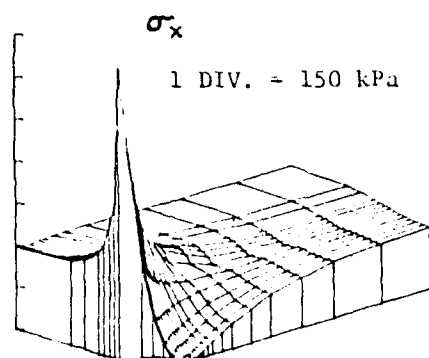


Figure 5. Distribution of Stresses in 0° -Layer Near $0/90$ Interface.

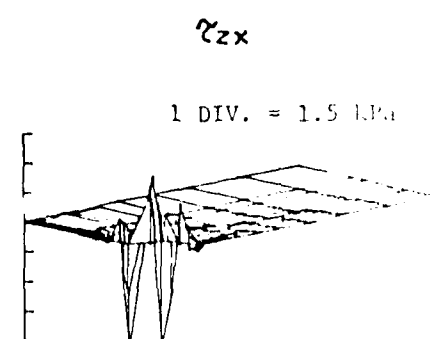
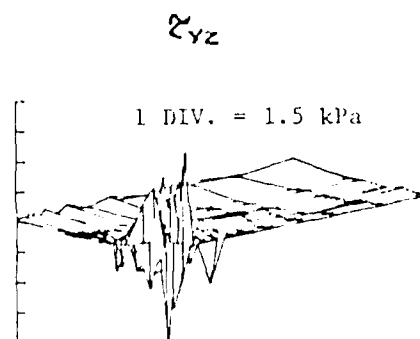
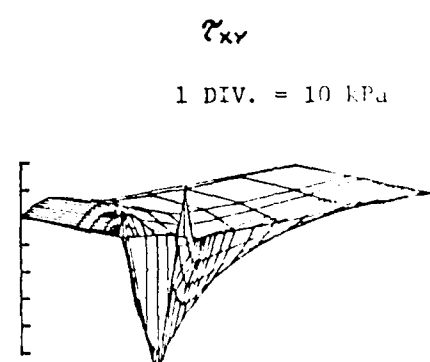
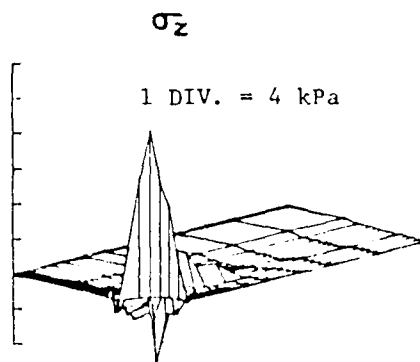
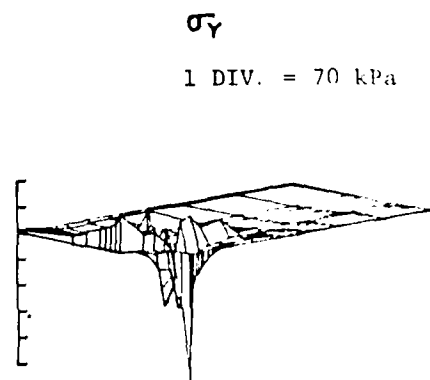
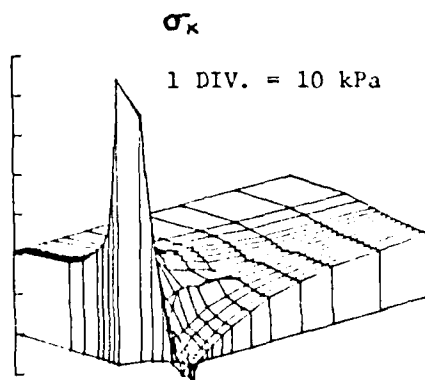


Figure 6. Distribution of Stresses in 90° -Layer Near $90/90$ Interface.

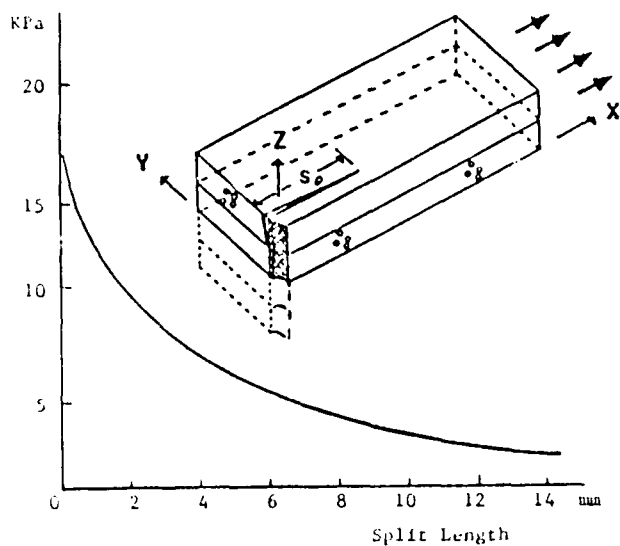


Figure 7. Split-Tip Normal (tensile) Stress Versus Split Length.

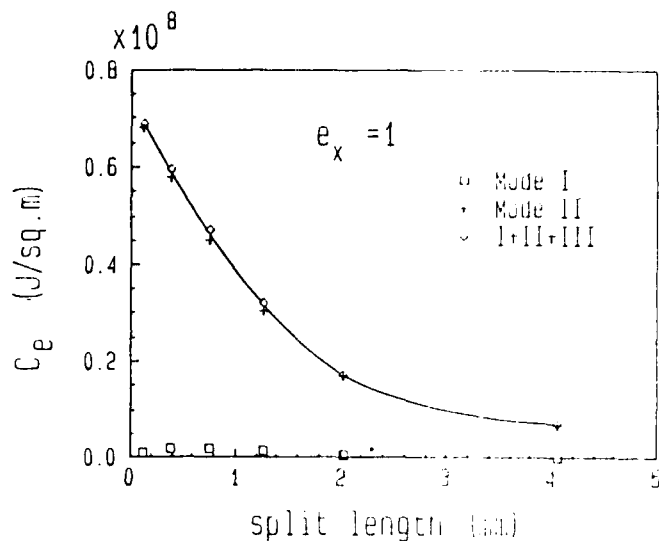


Figure 8. Energy Release Rate Coefficient C_E for 0° -Layer Splitting.

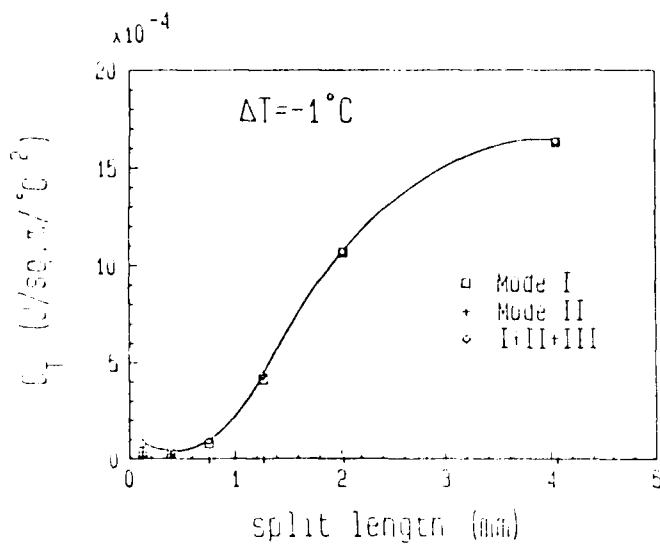


Figure 9. Energy Release Rate Coefficient C_T for 0° -Layer Splitting.

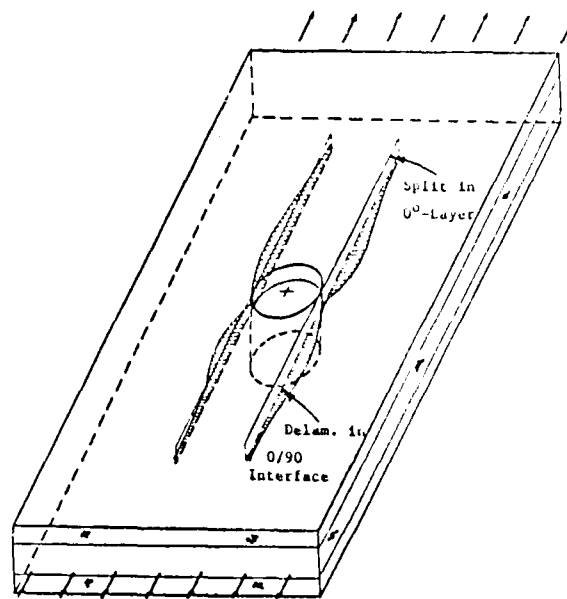


Figure 10. Simulation Model for 0/90 Interface Delamination.

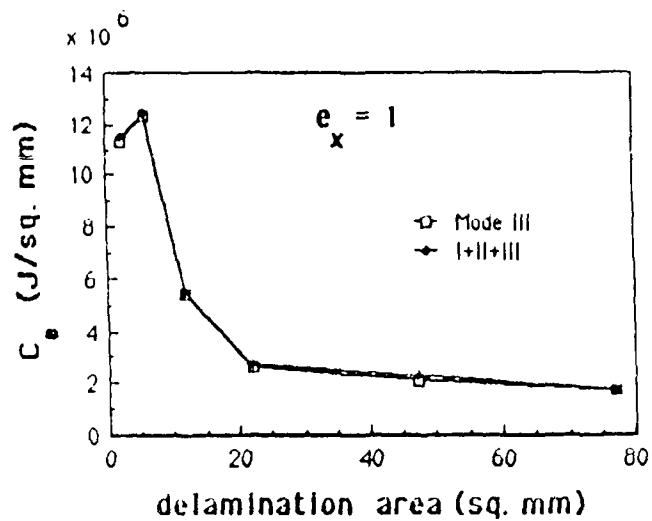


Figure 11. Energy Release Rate Coefficient C_e for 0/90 Interface Delamination.

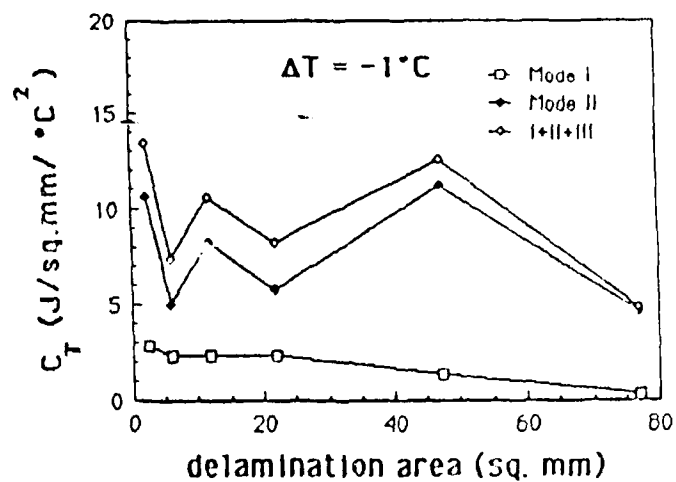


Figure 12. Energy Release Rate Coefficient C_T for 0/90 Interface Delamination.

**A COMPREHENSIVE STUDY
ON DAMAGE TOLERANCE PROPERTIES OF
NOTCHED COMPOSITE LAMINATES**

Appendix V

3-D Finite Element Crack Simulation Code

User's Guide and Source Code

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1 GENERAL PROGRAM CHARACTERISTICS

1.1 INTRODUCTION

This computer code has been developed for an independent and self contained operation. The program is written in FORTRAN 77 language, adoptable to any medium or large computer. The main function of the program is to simulate numerically the initiation and growth of a plane crack(s) in a 3-D solid, specifically, delamination or splitting or delamination with a split in composite plates. The plate may be subjected to either mechanical loading, thermal loading or both. In order to determine the layer interface which is likely to suffer delamination under the given loading, a search must be conducted by computing the interlaminar stresses. Once the site of delamination is determined, the program will then simulate the delamination growth under the applied loads.

The present computer code can handle (i) splitting along the fiber direction, (ii) delamination having a plane-contour of arbitrary shape and (iii) delamination in the presence of an opened split. The changes in the boundary conditions as the delamination grows are automatically adjusted in the program. There is no limitation to the number of layers or the stacking sequence. The layers may have different thicknesses and material properties. Each layer is assumed to be a homogeneous, orthotropic elastic medium with one of its principal axes aligned in the thickness directions of the plate (z-axis).

The code is divided into three independent programs: the preprocessor, the main code, and the post processor. The separation of the code in three stages allows modifications to be made in the data at the end

of each particular program so that certain parametric studies can be performed in one stage without repeating the calculations performed in the previous stage.

1.2 THE PREPROCESSOR PROGRAM

This is the first stage in the solution of the delamination problem. The input data necessary for this program consists of the specimen geometry, mesh plan, layer material properties, boundary conditions and the double nodes (double nodes are a pair of nodal points which occupy the same spatial position). The output of this program consists of the full details of the finite element mesh together with the numbered nodes, including the double nodes. Although this output data is sufficient to run the second stage, the data to be input into the main code, the data still needs to be supplemented with the crack opening sequence data set which can be formulated only following the output from the preprocessor.

1.3 THE MAIN CODE, KSAP II

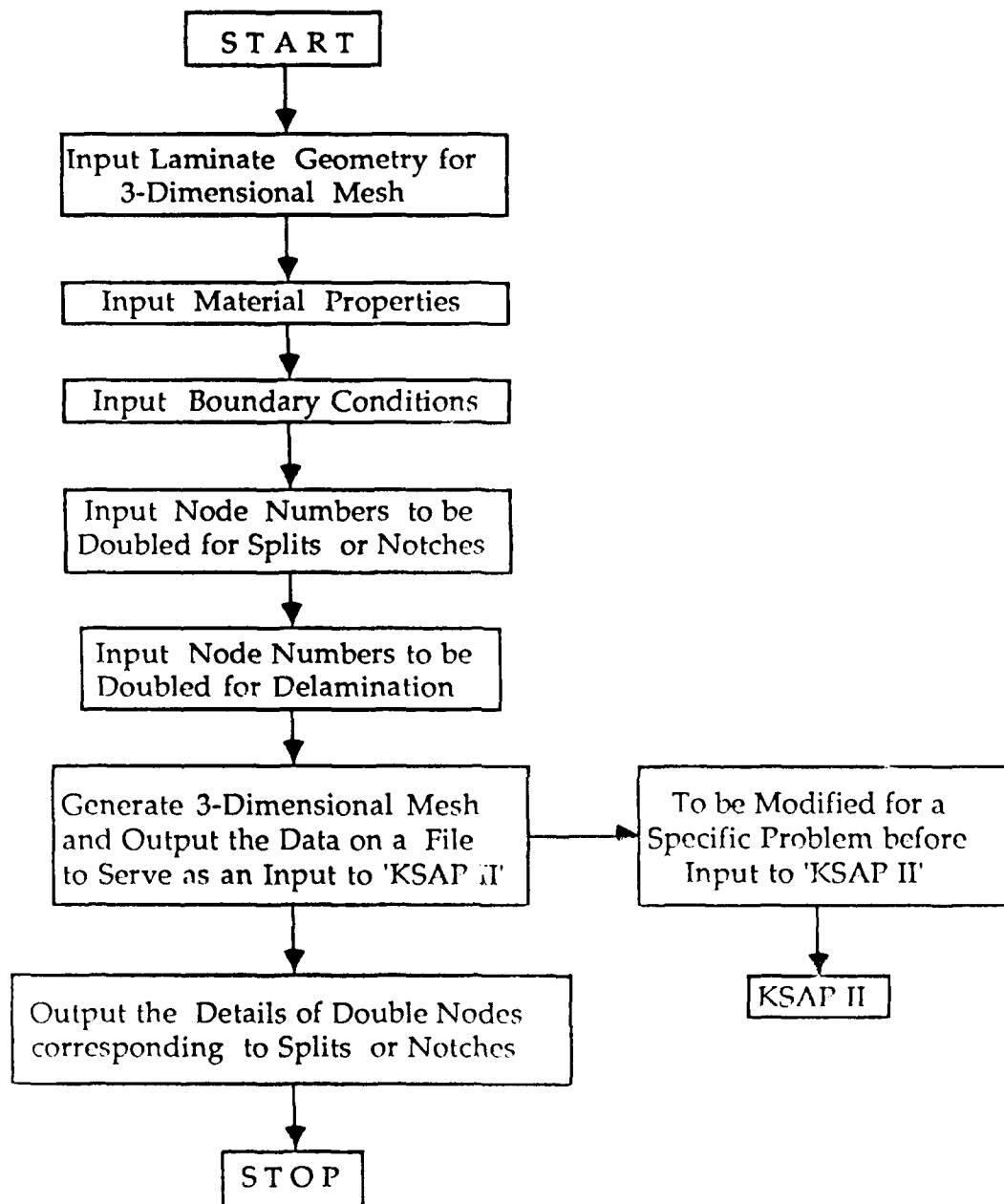
As the name implies, this is the main part in the solution procedure. The output data from the first stage, together with the crack opening sequence data serves as the input data for this program. The program solves the three dimensional problem using an 8 or 21 node solid element with three degrees of freedom (x,y,z) for each node.

1.4 THE POSTPROCESSOR PROGRAM

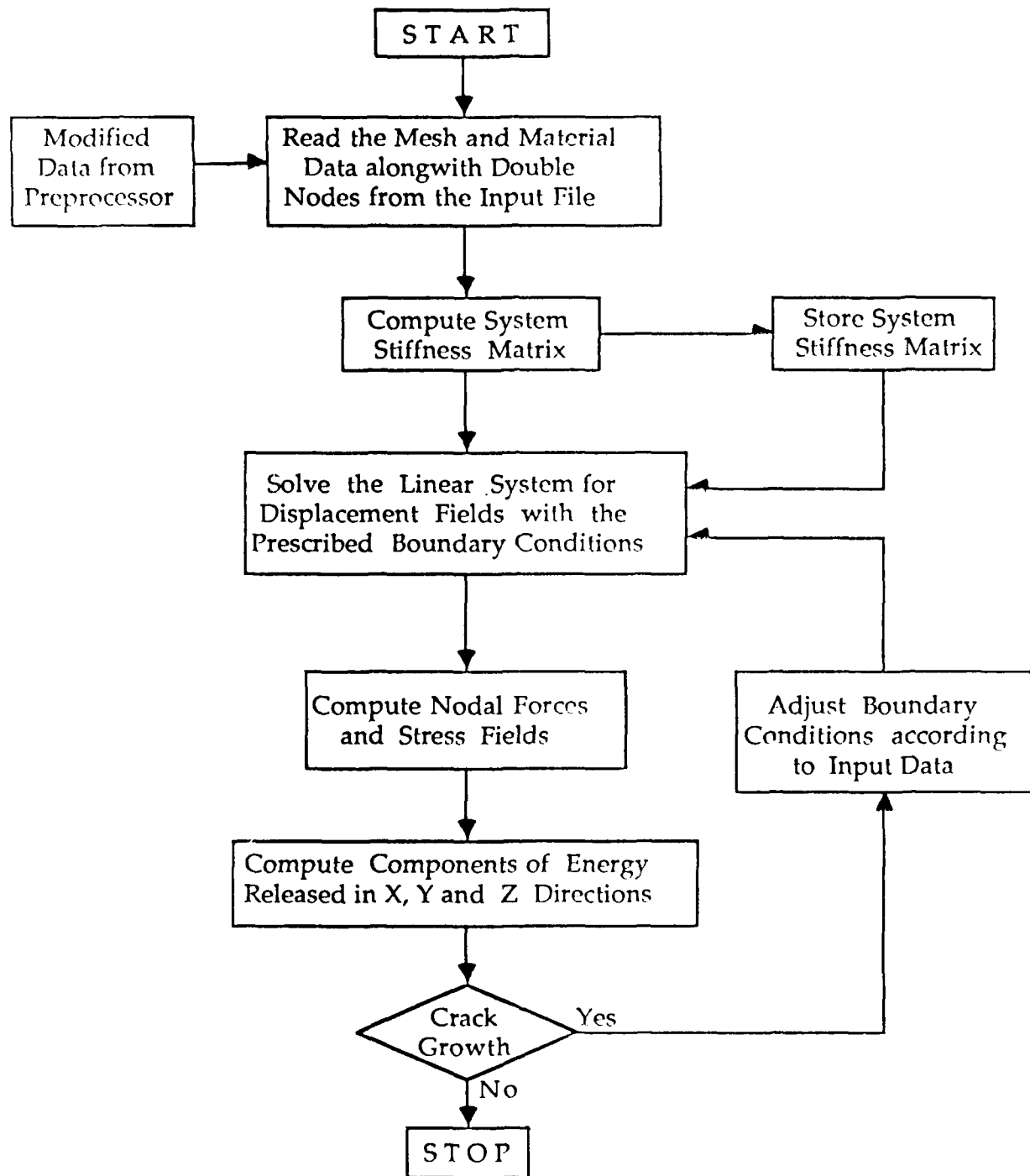
The post processor mainly produces 3-D plots of the stresses with hidden lines removed. The input data for this programs is a modified

output file from the KSAP II program. Various stress distribution plots can be output along any specified plane. The three-dimensional plots can be processed at any specified viewing direction.

The details of preprocessor program and the input can be found in Chapter 2. The modifications to the preprocessor output which are needed before it can be used further are found in Chapter 3. Chapter 4 describes the features of the KSAP II code and Chapter 5 describes the details of postprocessor program. Chapter 6 contains an illustrative example to explain the working of the total code. The FORTRAN source listings of the various parts of the code and their outputs are included in the Appendices A through D.



FLOW CHART FOR 'PREPROCESSOR' PROGRAM



FLOW CHART FOR 'KSAP II' PROGRAM

2 THE STRUCTURE OF THE PREPROCESSOR PROGRAM

2.1 INTRODUCTION

The preprocessor program generates the input data required for the main code, KSAP II. The input data required for the preprocessor program pertains to the dimensions of the plate, mesh plan, material properties of the layers, and the boundary conditions. In its present form, this program can generate data only for brick type elements with either 8 nodes representing the 8 corners of the element or 21 nodes as shown in Figure 2.1.

There are two options in generating the mesh, one is for rectangular mesh for laminates without any curved boundaries and the other is for generating mesh in a laminate with a central hole. The mesh pattern in the latter one is chosen to accommodate split (or split growth) tangential to the hole boundary along the loading direction. There is no limitation on the number of layers or the stacking sequence. Depending on the symmetry in geometry and/or loading, one-half, one-quarter or one-eighth of the plate may be analyzed. The displacement and force boundary conditions have to be appropriately specified in order to take the advantage of symmetry.

The program automatically assigns numbers to nodal points, and cartesian coordinates to each node according to input data. The nodes are numbered in an orderly fashion in x, y, z-directions and the 8 (or 21) nodes for each element can be generated arbitrarily from the set of coordinates given in x, y and z directions. The dimensions of elements in any direction can be controlled by changing the coordinates in that direction and thereby the density of the mesh in any region can be changed.

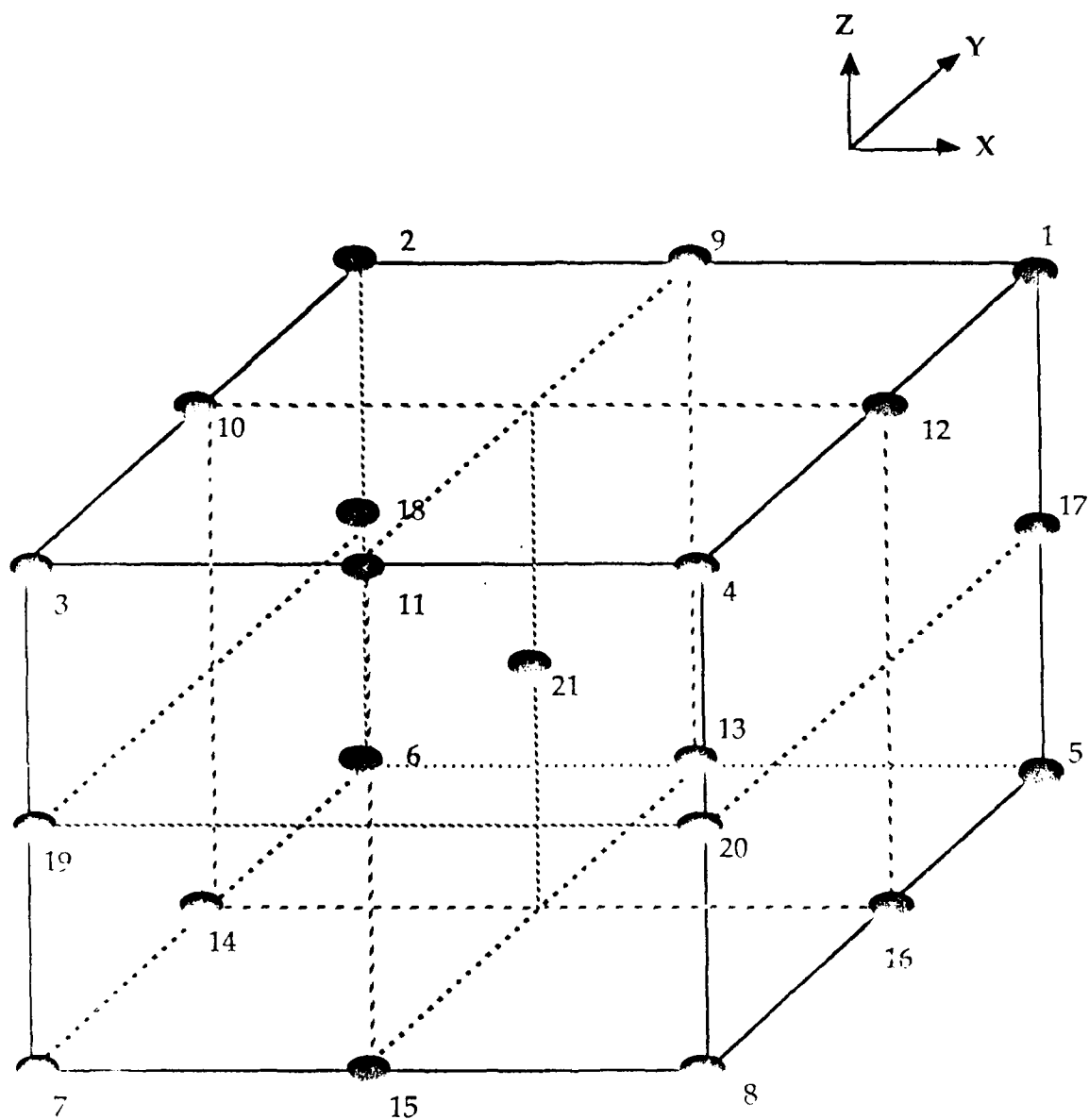


FIG. 2.1 THREE DIMENSIONAL 21 NODE ISOPARAMETRIC ELEMENT

The thermal loading simulation requires two data sets: assigning stress free temperature for each element and prescribing the temperature at which the plate is to be analyzed for delamination. The stress free temperature is assigned to each element while generating the elements. The temperature at which the plate is to be analyzed is provided while generating nodal points. The temperature distribution need not be uniform for the whole plate and each node can be assigned a different temperature. The details of this data input is explained in the next section 2.2.

Mechanical loading can be either a prescription of forces or a prescription of non-zero displacements at the nodes. The details of prescribing force boundary conditions are found in section 2.2. A plate subjected to uniform strain can be simulated by assigning non-zero displacements to the appropriate nodes. These non-zero displacements are changed to force boundary conditions by attaching a linear spring with a large stiffness value (k) to each node in the given displacement (d) direction and applying a force ($=k \times d$) at the other end of the spring. These boundary elements do not increase the total degrees of freedom of the stiffness matrix. The nodes having zero displacements, which are used to specify the symmetric planes, do not make use of these boundary elements and they essentially remove those degrees of freedom from the system of equations.

The main program, KSAP II can simulate a crack opening along a symmetric plane or along any plane given by $x=\text{constant}$ or $y=\text{constant}$ or $z=\text{constant}$. For example: the interlaminar boundary (layer interface) corresponds to $z=\text{constant}$. A crack along a symmetric plane (e.g. the mid-plane of a symmetric laminate) is simulated by suitably changing the boundary conditions at those nodes on that plane, which will be released to

simulate crack opening. The degrees of freedom of these nodes must be retained if a crack is to be simulated along the plane of symmetry. Hence they should not be removed by giving zero displacement in the direction of crack opening. The crack opening instruction along these nodes is explained in the next chapter. If a crack opening along a plane other than the symmetric plane is to be simulated then double nodes are to be assigned for each nodal point located on that plane. The double nodes need not be taken into account while generating the initial set of elements and nodes. Given the plane of crack (1 for yz-plane, 2 for zx-plane and 3 for xy-plane), the preprocessor program has the capability to renumber the mesh and update the node numbers for each element when the instruction pertaining to the double nodes is supplied.

A complete listing of the preprocessor program can be found in Appendix A. The following flow chart shown on the next page illustrates the general structure of the preprocessor program.

2.2 DATA INPUT TO PREPROCESSOR PROGRAM

The input data required for the preprocessor is made very simple and is kept to a minimum. For example, the element generation (assigning node numbers to the elements) can be done in only a few cards as explained in Section 2.2.2.

2.2.1 Details of the Data Input

The input data is arranged in the following nine groups of cards. Each group consists of one or more cards. Data in the groups I, VI, VII and VIII must be given in the specified format. Each entry must be made in the specified columns and a brief explanation of the entry can be found in entry description. The name listed under 'variable' is the name used for that entry in the program listing. The data in the groups II, III, IV and V may be given in free format. In these groups, if the data does not fit on one card, it may be continued on an immediately following card. Each of the groups II, VI and VIII may have several cards and the program recognizes the termination of that group only when it encounters a card with -1 as the first entry.

Group I Heading Card (Format A72):

HED(72) - heading information to be printed with the outputs

Group II Mesh Generation Cards (Free Format)

card 1:

NTYPE - Type of Element (8 node or 21 node)

card 2:

NONX, NONY, NONZ, RAD

- Number of Nodes in X, Y and Z directions. Radius of the hole (if RAD=0 given, rectangular mesh will be generated.

NOTE: If NTYPE=21, NONX, NONY, NONZ have to be odd numbers.

card 3:

XX(1), XX(2), ...XX(NONX)

- x- coordinates of the nodes in the x- direction.

card 4:

YY(1), YY(2), ... , YY(NONY)

- y- coordinates of the nodes in the y- direction.

card 5:

ZZ(1), ZZ(2), ... , ZZ(NONZ)

- z- coordinates of the nodes in the z - direction.

NOTE: For 21-node element generation, even numbered coordinates should be middle points of immediate neighboring points.

i.e., for $i=2,4,\dots$

$xx(i)=(xx(i-1)+xx(i+1))/2.$

$yy(i)=(yy(i-1)+yy(i+1))/2.$

$zz(i)=(zz(i-1)+zz(i+1))/2.$

Any mistake in the coordinates of even numbered coordinates will be corrected by the preprocessor.

For a laminate with a hole, x and y coordinates can be given with hole center as the origin. The coordinates of some of the nodes will be transformed and results in a mesh as shown in Figure 2.2.

Group III Element, Nodal Property Definition Cards (Free Format)

card 1: Nodal temperature at which analysis is to be carried out

N, TEMP, NEND, INC

- N - starting node number
- TEMP - magnitude of temperature of the node
- NEND - last node up to which nodes have same temperature
- INC - increment between N and NEND
- 1, 0.0, 0, 0 - data termination card

NOTE: This temperature will be different from stress free temperature of the element (see next card) for thermal loading only.

card 2: Element stress free temperature

N, TEMP, NEND, INC

- N - starting element number
- TEMP - stress free temperature of the element (curing temp.)
- NEND - last element number up to which elements have same temperature
- INC - increment between N and NEND
- 1, 0.0, 0, 0 - data termination card

card 3: Element material definition

N, MATER, NEND, INC

- N - starting element number
- MATER- material identification number (ex:1,2,3..)
- NEND - last element number up to which nodes have same temperature
- INC - increment between N and NEND
- 1, 0, 0, 0 - data termination card

NOTE: This card is to identify the elements as to which material they belong to. Material properties for different identification numbers are given in later cards.

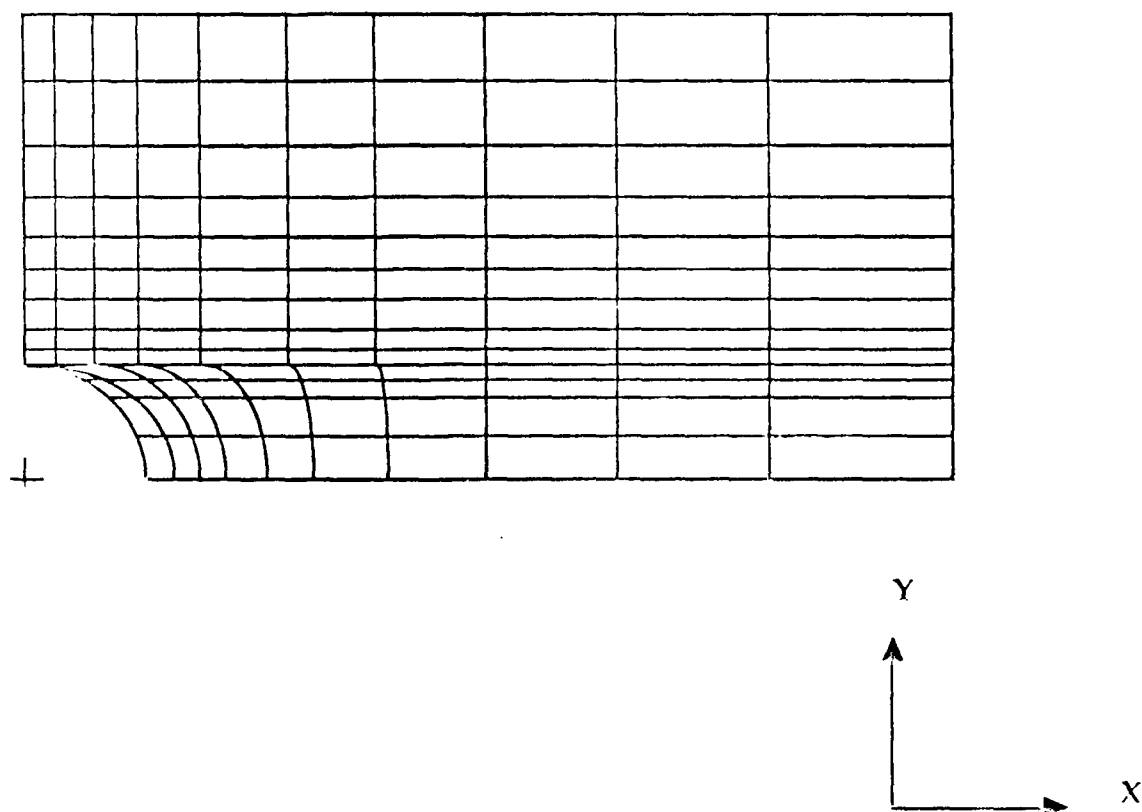


FIG. 2.2 FINITE ELEMENT MESH IN XY-PLANE IN A LAMINATE WITH A HOLE

card 4: Element material axis orientation definition
N, MORTT, NEND, INC

- N - starting element number
- MORTT- material axes orientation set number (ex:1,2,3..)
- NEND - last element number up to which nodes have same temperature
- INC - increment between N and NEND
- 1, 0, 0, 0 - data termination card

card 5: Element stiffness matrix reuse definition

M1, M2
M3, M4
- -
- -

- M1,M3 - starting element number
- M2,M4 - last element number upto which element stiffness is same
- 1, 0 - data termination card

NOTE: These cards to identify the elements with the same stiffness matrix and thereby saves computational time. A number of ranges (M1,M2; M3,M4;..etc.) can be given one after another.

Group IV Split or plane notch definition data (Free Format)

card 1:
NSD, IDIR

- NSD - number of nodes lying inside the split or plane notch region
- IDIR- direction number normal to the plane of the notch
 - if the normal is parallel to x- axis, IDIR=1
 - if the normal is parallel to y- axis, IDIR=2
 - if the normal is parallel to z- axis, IDIR=3

NOTE: If no split is required enter 0,1 and skip card 2.

card 2: node numbers defining split region
N, NEND, INC

- N - starting node number
- NEND - last node number
- INC - increment between N and NEND
- 1, 0, 0, - data termination card

NOTE: With this card, number of splits can be defined in parallel planes. Split defined by this card is simulated by doubling the nodes but these double nodes cannot be used to simulate crack growth. In order to read the displacement output of the nodes inside the split, refer to corresponding double nodes given in the output file 'EOP010.PAT'.

Group V Delamination definition data (Free Format)

card 1:

NTD

- NTD - total number of nodes defining delamination region
If there are no double nodes in the problem enter
0 and skip card 2

card 2: node numbers defining delamination region

NOND(1), NOND(2), ...NOND(NTD)

NOTE: The double nodes and the corresponding original nodes are not written in a separate output file. They are given in KSAPIN.DAT itself. They are arranged in the ascending order of the original nodes to facilitates easy modification of KSAPIN.DAT for crack growth simulation. So, it is advisable to give the nodes here in the order of their release.

For 21 node element, face center nodes are not used in KSAP II and hence they are eliminated from the double node list by the preprocessor.

card 3:

XL, XU, YL, YU, ZL, ZU

- XL, XU - lower and upper bounds of x- coordinate of the laminate boundary in which second set of double nodes are to be placed.
- YL, YU - lower and upper bounds of y- coordinate of the laminate boundary in which second set of double nodes are to be placed.
- ZL, ZU - lower and upper bounds of z- coordinate of the laminate boundary in which second set of double nodes are to be placed.

Group VI Material Property Data

Orthotropic, temperature dependent material properties may be prescribed. Here L,T,Z are the principal axes of the material. For each different material the following group (a) cards must be supplied.

(a) Material Properties (Format A4,I4,4X,G17.7)

card 1:

columns	entry description
1- 4	bbEL
5- 8	material identification number
13-29	value of Young's modulus in L-direction

card 2:

columns	entry description
1- 4	bbET

	5- 8	material identification number
	13-29	value of Young's modulus in T-direction
card 3:		
	columns	entry description
	-----	-----
	1- 4	bbEZ
	5- 8	material identification number
	13-29	value of Young's modulus in Z-direction
card 4:		
	columns	entry description
	-----	-----
	1- 4	NULT
	5- 8	material identification number
	13-29	value of the poisson's ratio, ν_{lt}
card 5:		
	columns	entry description
	-----	-----
	1- 4	NULZ
	5- 8	material identification number
	13-29	value of the poisson's ratio, ν_{lz}
card 6:		
	columns	entry description
	-----	-----
	1- 4	NUTZ
	5- 8	material identification number
	13-29	value of the poisson's ratio, ν_{tz}
card 7:		
	columns	entry description
	-----	-----
	1- 4	bGLT
	5- 8	material identification number
	13-29	value of the shear modulus, G_{lt}
card 8:		
	columns	entry description
	-----	-----
	1- 4	bGLZ
	5- 8	material identification number
	13-29	value of the shear modulus, G_{lz}
card 9:		
	columns	entry description
	-----	-----
	1- 4	bGTZ
	5- 8	material identification number
	13-29	value of the shear modulus, G_{tz}
card 10:		
	columns	entry description
	-----	-----
	1- 4	ALFL
	5- 8	material identification number
	13-29	value of the thermal expansion coeff., α_l
card 11:		
	columns	entry description
	-----	-----
	1- 4	ALFT
	5- 8	material identification number

card 12:	13-29	value of the thermal expansion coeff., α_t
	columns	entry description
	1- 4	ALFZ
	5- 8	material identification number
	13-29	value of the thermal expansion coeff., α_z

NOTE: If any of these 12 cards are not supplied then that particular value will be set equal to zero.

The 12 constants ($E_{11}, E_{tt}, \dots, z$) are defined with respect to a set of axes (L,T,Z) which are the principal material directions.

(b) Data termination Card (Format A4)

columns	entry description
1-2	-1 indicates the end of material property cards.

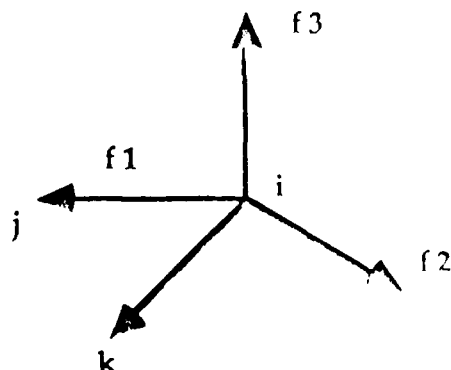
(c) Material Axes Orientation

In this set the data regarding the material principal axes (L,T,Z) relative to the global axes (x,y,z) is furnished. There can be several sets of orientations and one card should be input for each orientation as follows:

columns	variable	entry description
1 - 5	MORT	material axes orientation set number
6 - 10	NI	node number for point "i"
11- 15	NJ	node number for point "j"
16- 20	NK	node number for point "k"

NOTE: Orientation set numbers (MORT) must be input in increasing sequence beginning with "1".

Orthotropic material axes orientations are specified by means of the three node numbers, NI,NJ,NK. For the special case where orthotropic material axes coincide with the global axes (x,y,z), it is not necessary to input data in this section. Let f_1, f_2, f_3 be the three orthogonal vectors which define the axes of material orthotropy then their directions are as shown below:



Node numbers NI,NJ,NK are only used to locate points i,j,k respectively and any convenient nodes may be used.

End the material orientation definition cards with -1 card.

Group VII Force boundary conditions
(Format I6,1X,A4,1X,F10.0,12X,I6,I6)

columns	variable	entry description
1- 6	N	node at which force acts
8-11	LABEL	direction of force (in nodal coordinate system) FX, FY, or FZ
13-22	FORCE	value of the force
35-40	NEND	! If NEND is greater than N (for N positive) all nodes from N thru NEND in steps of INC has this specified force (if INC is left blank it is assumed to be 1)
41-46	INC	

NOTE: N=-1 signifies the end of this set of cards

Group VIII Displacement Boundary Conditions
(Format I6,1X,A4,1X,F10.0,12X,2I6)

This set of cards is used to constrain nodal displacements to specified values and to compute support reactions. Boundary elements are used to specify strain for the specimen. The boundary element is essentially a spring which has an axial displacement stiffness and it is defined by a single directed axis through the specified nodal point. If any nodal displacement (UX, UY or UZ) is specified to have 0.0 value then that degree of freedom is eliminated from the stiffness matrix.

columns	variable	entry description
1- 6	N	node number at which this displacement will be used
8-11	LABEL	type of displacement boundary condition
13-22	V	value of the displacement
35-40	NEND	! If NEND is greater than N (for N positive) then all nodes N thru NEND in steps of INC will have this specified displacement
41-46	INC	

NOTE: N=-1 signifies the end of this set of cards.

LABEL can be UX, UY or UZ (upper case) which means that the

specified displacement is in x, y, & z directions respectively.

Group IX Stress Output Locations (Free Format)

LOC1, LOC2, LOC3, LOC4, LOC5, LOC6, LOC7
- location numbers in ascending order

NOTE: In KSAP II, there is a provision to obtain stresses at a maximum of 7 locations in an element. Any 7 of the 27 locations shown in Figure 2.3 can be chosen.

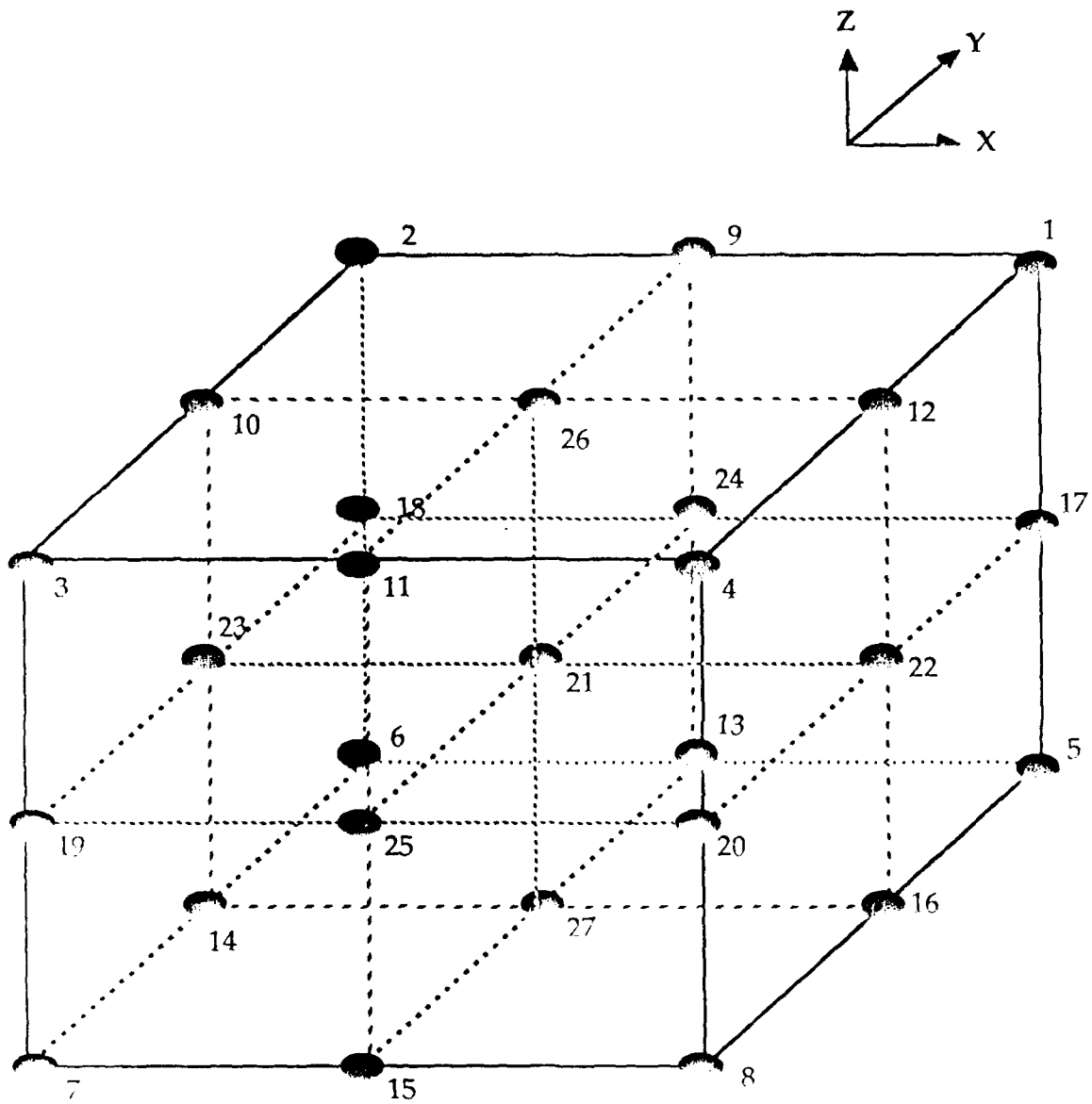


FIG. 2.3 STRESS OUTPUT LOCATION NUMBERS IN AN ELEMENT

3 MODIFICATION OF THE PREPROCESSOR OUTPUT DATA

3.1 INTRODUCTION

The output data of the preprocessor program is to be modified before it can serve as input data to the main code, KSAP II. The preprocessor program will output two files of data: one file will serve as input data file to the KSAP II code and the other file contains the renumbered double nodes for split or notch simulation. The first file is to be supplemented with information regarding the location of double nodes and the information regarding crack opening node sequence. In addition, it is also possible to give commands to selectively print the stress output.

3.2 DOUBLE NODES AND CRACK OPENING SEQUENCE

A double node is originally one node which has two node numbers. These are provided in the plane along which the crack propagation is to be simulated. The double nodes serve two purposes. If the displacements of both the nodes are specified to be same, then they behave as one single node. If, on the other hand, the displacements of the nodes are specified to be independent then they behave as two separate nodes thus simulating crack propagation through that node. Usually, a node has three degrees of freedom, in x, y, z -directions. In the case of double nodes each node has three degrees of freedom after they are separated. However, if the double node is on a symmetric plane, then each node will not have three degrees of freedom after they are separated. For example, Figure 3.1 shows the one-eighth part of a laminate subjected to a force in y -direction. $x=0$, $y=0$ and $z=0$ are symmetric planes. Let there be a transverse crack in the bottom ply as shown by the shaded area. The double nodes 50 and 51 are not

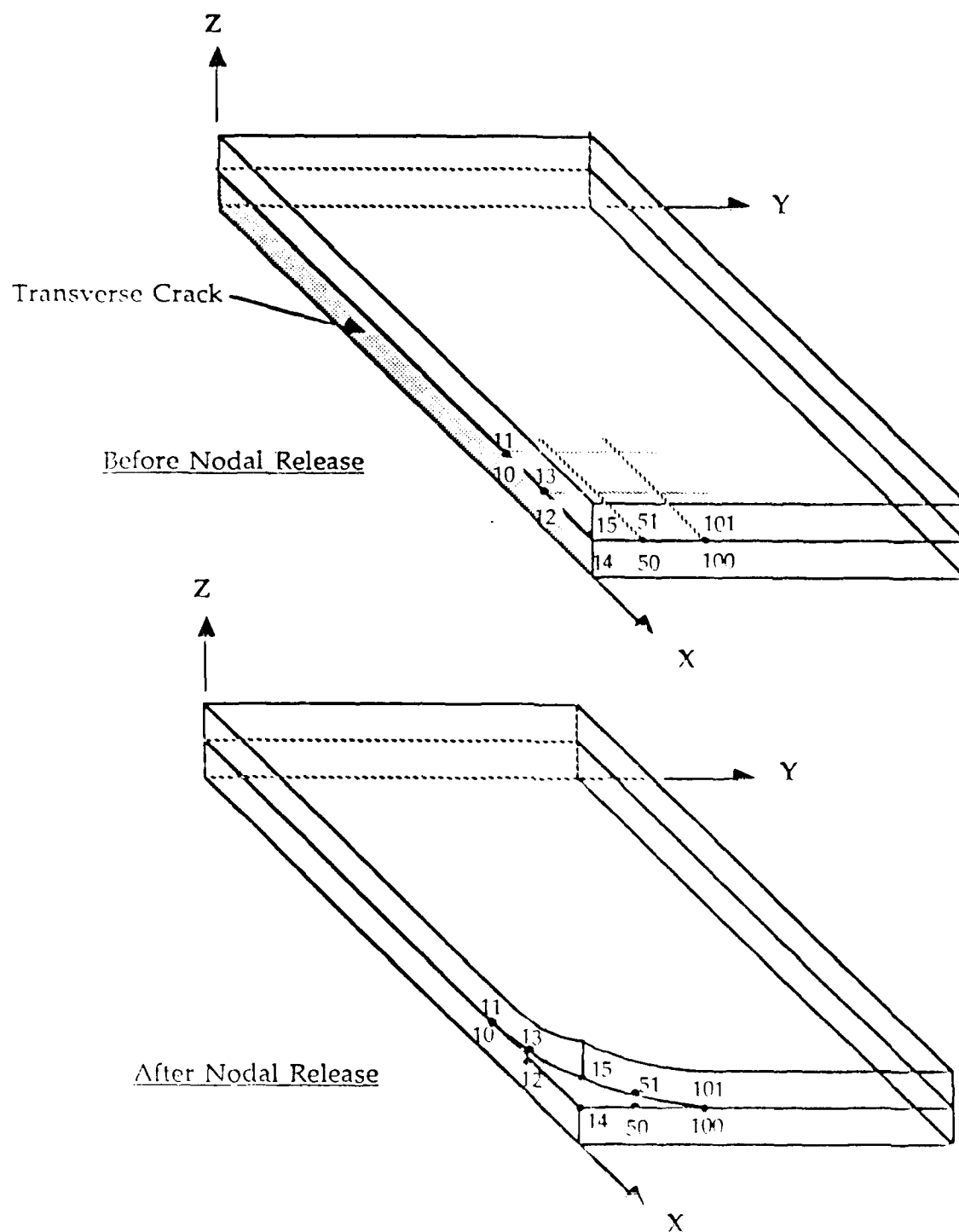


FIG. 3.1 DOUBLE NODE ARRANGEMENT FOR DELAMINATION SIMULATION

on any symmetric plane and hence, the three displacements (U_x, U_y , & U_z) of node 50 are respectively the same as those of node 51 before the nodes are separated. The nodes 50 and 51 each will have three independent degrees of freedom once they are separated. However, the double nodes 10 and 11 will behave differently. When they are together $U_y=0$ since they lie on the symmetric plane $y=0$; and U_x and U_z of node 10 are equal to U_x and U_z of node 11. When the two nodes are separated as the crack propagates, the upper node 11 will be still on the symmetric plane and it will have U_x and U_z degrees of freedom, whereas the bottom node 10 is no more constrained and will have all the three degrees of freedom, U_x, U_y and U_z , free.

3.3 DETAILS OF DATA MODIFICATION

At the end of the preprocessor output the following cards have to be added with regard to double nodes and the crack opening sequence:

I) Details of the constrained degrees of freedom (Free format)

- a) NB - total number of degrees of freedom of those double nodes which are constrained by the symmetric plane or constrained by specified displacements before they are released.
- b) Details of the constrained degrees of freedom. There should be NB following cards. Each card contains the following input:

NBC - node number
 IFIX - degree of freedom
 1 for x, 2 for y, 3 for z degree of freedom
 USAVE - value of the specified displacement
 (=0, for nodes on that particular symmetric plane)

II) Details of the other double nodes' degrees of freedom

- a) NPAIR - number of pairs of all double nodes including those constrained on the symmetric plane
- b) There should be NPAIR following cards. Each card will give details of one pair of double nodes and the possible degrees of freedom after the nodes are open.

NPC1 - node number 1 of the pair
NPC2 - node number 2 of the pair
NPX - 0 or 1

NPX=1 signifies that the two nodes are constrained to have the same displacement U_x , before the nodes are open and the nodes are completely free of each other in x- direction of freedom after they are open. NPX=0 signifies that their degrees of freedom are already specified as explained in Group I.

NPY - 0 or 1 ! In Y and Z directions similar to the X-
NPZ - 0 or 1 ! direction as explained above for NPX.

III) Data for each step of opening of nodes:

a) Opening of the double nodes to simulate crack propagation (Free format)

N1 - ! the node numbers of the paired double nodes
N2 - ! which are to be opened
IDF - 1,2 or 3 the degree of freedom which is to be freed.
IDF=1 denotes x-degree of freedom is freed from its double node's x-degree of freedom and the nodal force F_x becomes 0. Likewise, IDF=2 or 3 denote y or z-degree of freedom is freed.

For those nodes on the symmetric plane and constrained by by specified displacement which are to be freed the above card should be modified as

N1 - node number
N2 - 0
IDF - 1,2, or 3 depending on x, y, or z degree of freedom which is to be freed

There should be as many cards as there are degrees of freedom to be freed.

N1=N2=IDF= 0 signifies the end of this crack opening instruction and the stress and energy released associated for this opening will be calculated.

b) Selective stress print option (Free Format)

NBEG ! stresses will be output for the element from
NEND ! elements from NBEG thru NEND
This card should not be left blank. If 0,0 is entered stresses will not be printed.

If another step of opening is desired, the above (a) and (b) will be repeated. This may be continued until all the

nodes in Group-I and II are relaxed.

It may be noted that if the displacement and stress solution is desired before any crack is simulated a 0,0,0 card is necessary after Group I and Group II cards.

- IV) The crack propagation is terminated by a card containing '9999 9999 0' as input.

AD-A192 932

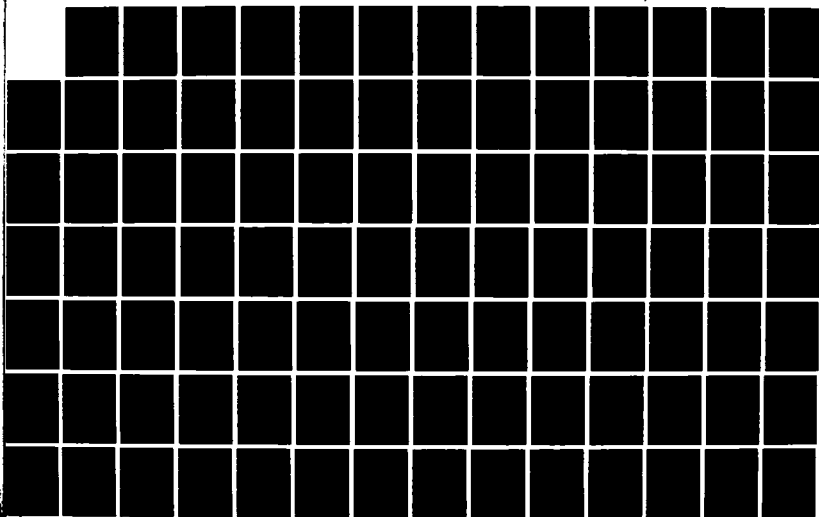
A COMPREHENSIVE STUDY ON DAMAGE TOLERANCE PROPERTIES OF
NOTCHED COMPOSITE... (U) DREXEL INST FF TECH PHILADELPHIA
PA DEPT OF MECHANICAL ENGINE... A S WANG ET AL. FEB 88
AFOSR-TR-88-0200 AFOSR-84-0334

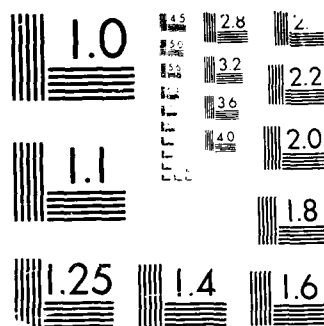
2/3

UNCLASSIFIED

F/G 11/4

NL





MICROCOPY RESOLUTION TEST CHART

1963-A NATIONAL BUREAU OF STANDARDS

4 THE STRUCTURE OF THE MAIN CODE, KSAP II

4.1 INTRODUCTION

KSAP II is the main program in the analysis of a delamination or splitting problem of a composite plate. The program simulates the crack opening using the data regarding the finite element mesh and the predetermined crack opening sequence. At each step, the program computes the energy released together with the stress and displacement fields.

4.2 GENERAL FEATURES OF THE CODE

The code uses an 8-node or 21-node solid brick element to calculate the stiffness matrix. Each node is assumed to have three degrees of freedom in x, y, z-directions. General orthotropic material properties can be assigned to the element. It is assumed that the whole element is at a uniform temperature given by the average of the temperatures at the 8 (or 21) nodes. The thermal loads are calculated using the difference between this average temperature and the stress free temperature of the element.

KSAP II code has the capability to simulate crack opening along the surface which passes through the points where double nodes are prescribed. Initially, the two nodes in each pair are assigned the same displacements in the three degrees of freedom. The system of linear equations are solved with the appropriate boundary conditions (mechanical loading or thermal loading or both) for nodal displacements and nodal forces. The stresses at the prescribed locations in each element are also calculated. The nodal forces of the double nodes are nothing but the internal forces holding these two nodes together. These forces are stored and will be used in the

next iteration as the crack opening is simulated through those nodes. The crack opening is simulated by changing the boundary conditions of the double nodes. This implies, obviously, that the displacements of the two nodes will not be the same. Then the system of linear equations are solved for nodal displacements and nodal forces under the changed boundary conditions. The difference in the displacements of the two nodes through which crack opening is simulated will be the crack opening displacement. Using the internal force which was necessary to hold them together (as found in the preceding iteration), the strain energy released can be computed as the crack opening is simulated through that node. This procedure can be continued until all the double nodes are opened.

Thus, strain energy released as the crack passes through successive double nodes can be calculated at each step. At each step the crack opening can be simulated through one or more pairs of double nodes and there is no limitation on the crack front shape.

If the crack is simulated along a symmetric plane, there is no need for double nodes in that plane. The crack opening can be done by simply changing the boundary conditions of the nodes on that plane from displacement boundary conditions to free force boundary conditions.

Once the strain energies released are calculated at each step then the energy release rates (energy released per unit area) may be obtained by dividing the energy released by the increment in crack area at that step.

A complete listing of the KSAP II code can be found in Appendix B. The following flow chart illustrates the general structure of the KSAP II program.

4.3 THE OUTPUT DATA FROM KSAP II

The output data from KSAP II consists of the details of the finite element mesh of the given problem as well as the solution of the laminated plate for the given crack simulation. For easy reference, the stresses and energies released are written in separate files, KSAPOUT.DAT and WORK.WOK respectively. The rest of the output (control information and displacements) is written in DISP.OUT.

The output file DISP.OUT contains the following information:

- i) control information
- ii) the nodal point data: cartesian coordinates of each node, the temperature at each node, and the boundary condition codes (1 means restrained, 0 means free to move in that degree of freedom)
- iii) the equation numbers assigned to each nodal degree of freedom
- iv) boundary elements data which are attached to nodes where non-zero displacement boundary conditions are prescribed
- v) the material property tables for different layers
- vi) element data which consists of corner nodes and the material table number to which the element belongs
- vii) data regarding equations, i.e., number of equations, bandwidth etc.
- viii) the solution data at each step consists of the nodal displacements (U, V, W) and forces (Fx, Fy, Fz)

(the first step is numbered as zero, the second step is numbered 1 and so on)

The output file KSAPOUT.DAT contains the following information:

- i) element stresses (SIG-XX, SIG-YY, SIG-ZZ, SIG-XY, SIG-YZ, SIG-ZX) at the prescribed locations in each element.
- ii) from the second step onwards, energy released in x, y, and z directions will also be output after the element stresses. This energy released output is the sum of the

energies released at all the double nodes relaxed at that iteration if more than one double node are relaxed.

4.4 LIMITATIONS OF KSAP II CODE

At present the program can handle upto 3000 nodes, and 100 pairs of double nodes. Should a problem involve finer mesh and more than 3000 nodes then the following dimension statements have to be suitably changed:

- 1) The degrees of freedom (3 times the total number of nodes) have to be changed in ICR(*), R(*) in the statement with serial numbers 3380, 3386, 3844 and 3849.
- 2) The number of nodes in ID(*,6) have to be changed in the statements with serial number 3379.
- 3) The double nodes' total degrees of freedom (6 times the pairs of double nodes) have to be changed in TNATN(*,*), TNAT(*,*), TCOL(*,1), TCOL(*), TCOLM(*), IST(*) in the statements with serial numbers 3384, 3385, 3386, 3848 and A(*,*), B(*,1), IPIVOT (*), INDEX(*,*), DT(*) in the statement with serial number 4366 to the same value.

5 POSTPROCESSOR PROGRAM

5.1 INTRODUCTION

The postprocessor program 'plot' is designed to present the stress output of the main code, KSAP II, in a graphical form. The stress distribution in any plane parallel to xy plane can be displayed on a graphics terminal or graphics output can be obtained on printronix printer or Hewlett Packard plotter. The code uses 3-D graphics routines from Template package. The program is written to run interactively and the interactive input consists of choice of device, stress number (1 for xx, 2 for yy, 3 for zz, 4 for xy, 5 for yz and 6 for zx stress), viewing position coordinates, scale factor to scale stress values. The stress values, coordinates and the related data are read from a prescribed data file.

5.2 DETAILS OF DATA FILE

For an 8 node element the following data precede stress data:

Heading - data file identification heading

NX, NY, NL

NX - number of coordinates in x direction

NY - number of coordinates in y direction

NL - number of layers of finite elements in the laminate

XX(I), I=1,NX - coordinate values in x direction

YY(I), I=1,NY - coordinate values in y direction

For 21 node element the following data should precede stress data:

Heading - data file identification heading

NNODES, NLOC

NNODES - number of nodes of finite element (8 or 21)

NLOC - number of stress output locations requested

NONX, NONY, NONZ

NONX - number of coordinates in x direction

NONY - number of coordinates in y direction

NONZ - number of coordinates in z direction

LOC(I), I=1, NLOC - stress output locations

X(I), I=1, NONX - coordinate values in x direction

Y(I), I=1, NONY - coordinate values in y direction

Z(I), I=1, NONZ - coordinate values in z direction

At the end of the above set of data, one set of stress output (for all the elements) should be copied from KSAFOUT.DAT.

The following plots (Figs.5.1 and 5.2) are obtained from stress output at step 0 in the example problem. Fig. 5.1 is the normal stress along y direction (stress no. 2 and finite element layer no. 3) in 0° layer. The interlaminar stress (stress no. 3 and finite element layer no. 3) in 0° layer is shown in Fig. 5.2. Both the plots are generated on Hewlett Packard plotter using eye coordinates (30, -30, 30).

EYEX=30; EYEV=-30; EYEZ=30
SCALE : 1 DIV =5000 psi

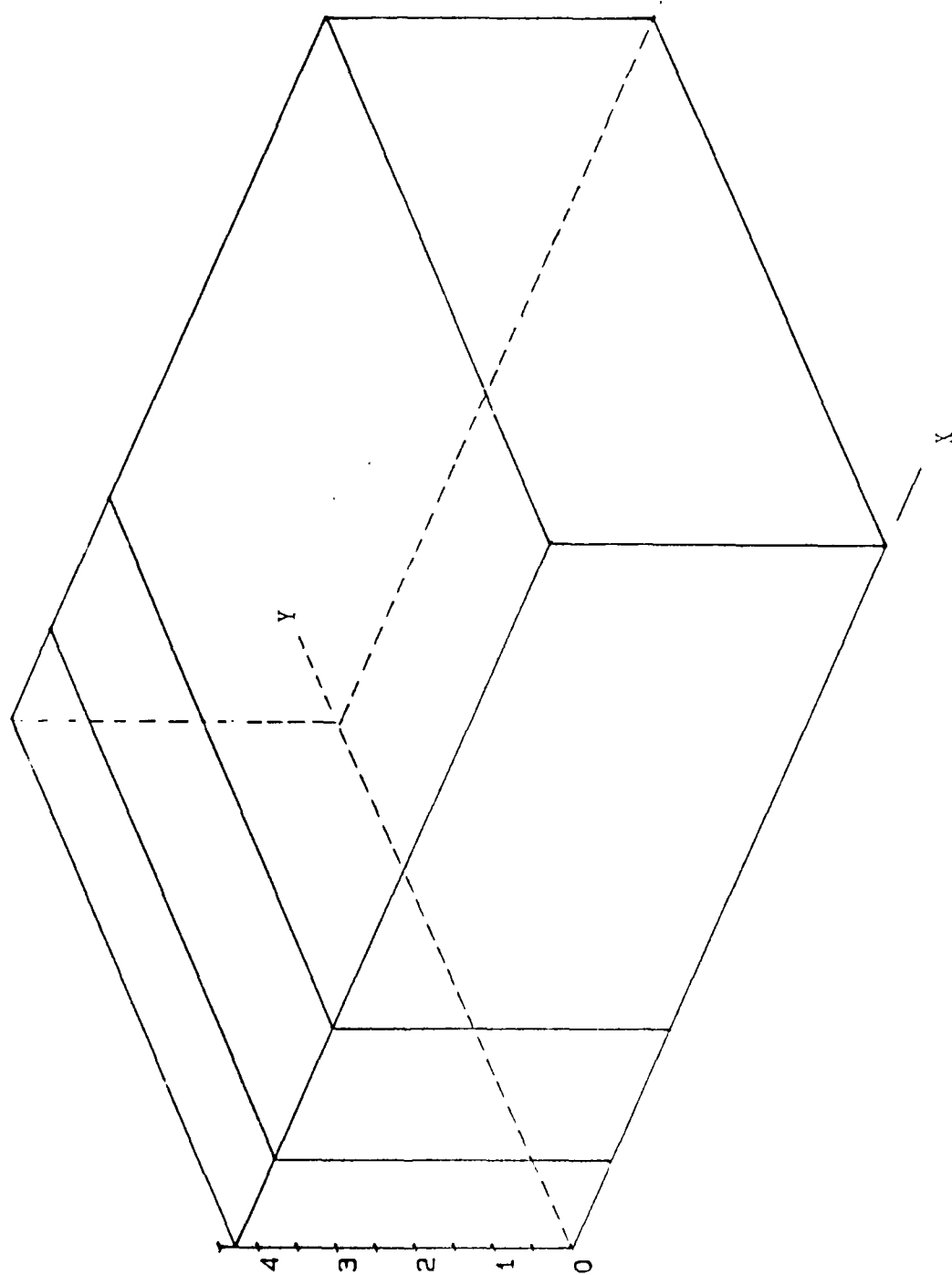


FIG. 5.1 NORMAL STRESS DISTRIBUTION ALONG Y DIRECTION IN 0° LAYER

EYEX=30; EYFY=-3-; EYEZ=30
 SCALE : 1 DIV. = 5 psi

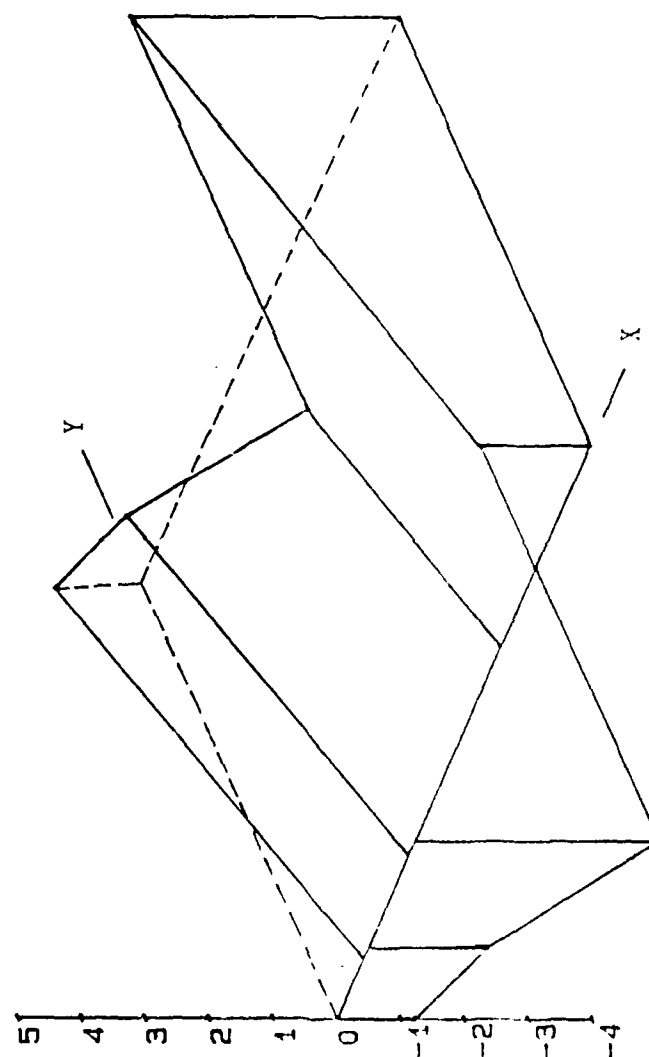


FIG. 5.2 INTERLAMINAR NORMAL STRESS DISTRIBUTION IN 0° LAYER

6 ILLUSTRATIVE EXAMPLE

6.1 INTRODUCTION

In this section we consider an example problem. The example is of a simple laminate construction and it does not represent a practical problem. The purpose here is to illustrate the procedure to operate the computer code. However, the code is developed for a more general use, subjected to the limitations discussed in the preceding sections.

The following paragraphs present the actual working steps in using the present code to generate the interlaminar stress distribution in a given interface plane. All input and output data for this example problem are found in Appendix D.

Laminate Geometry:

(Because of symmetry only one-eighth of the laminate is considered)

width of the laminate is 8.0"
length of the laminate is 6.0"
number of layers is 2
thickness of layer 1 is 1.0"
thickness of layer 2 is 1.0"

As shown in the Figure 6.1 $x=0$, $y=0$ and $z=0$ are symmetric surfaces and a uniform farfield strain is applied in x -direction.

The delamination cracking and mesh size are selected as follows:

4 equal divisions in x -direction
2 equal divisions in y -direction
4 equal divisions in z -direction

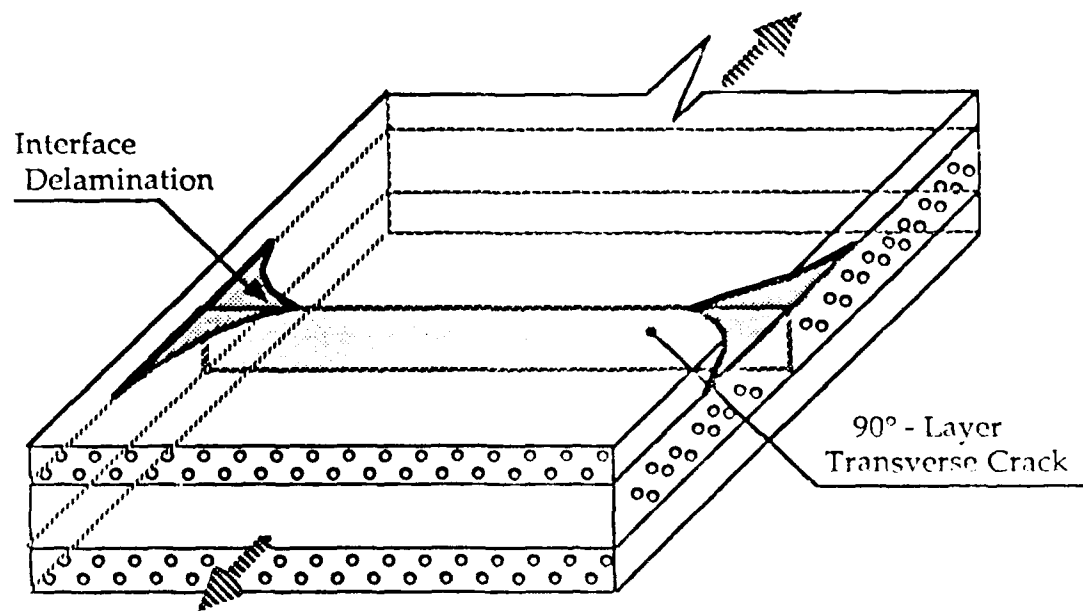
Material properties and loading information for each layer are furnished in the following manner:

Layer 1 (90° layer)

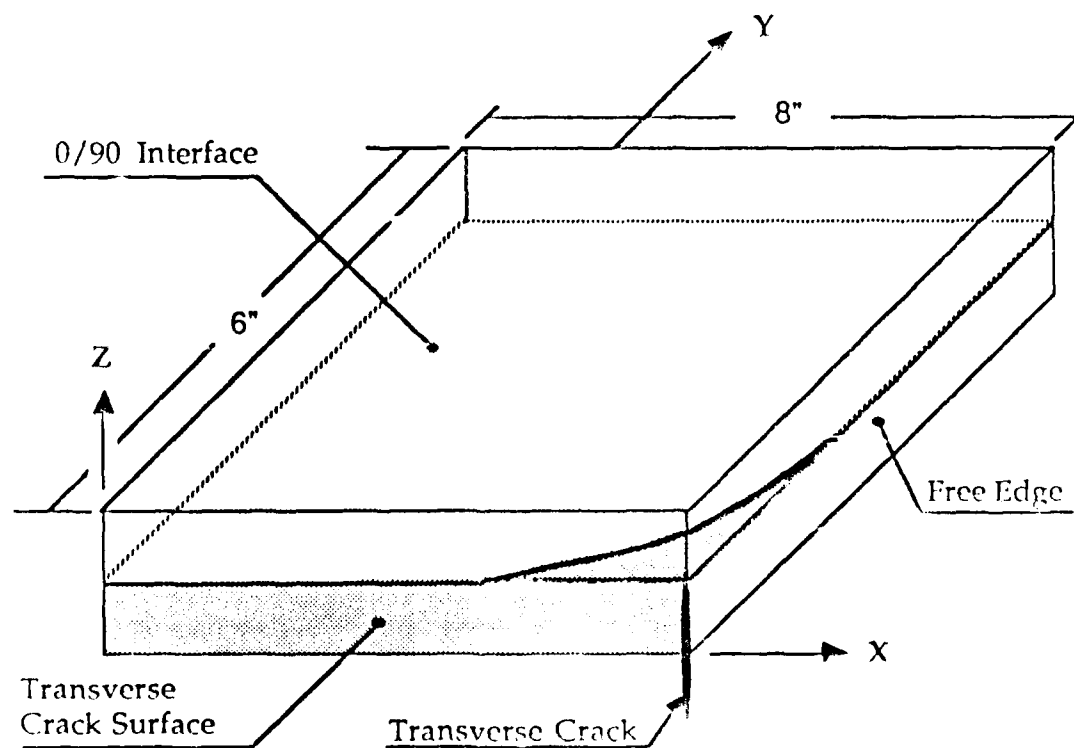
$$E_1 = 21.0 \times 10^6 \text{ psi}$$

$$E_t = 1.7 \times 10^6 \text{ psi}$$

$$E_z = 1.7 \times 10^6 \text{ psi}$$



(a) Transverse Crack/Free Edge Induced Delamination



(b) One-eighth part of the laminate simulated

FIG. 6.1 THE ISOMETRIC VIEWS OF THE EXAMPLE PROBLEM

$$\begin{aligned}
v_{1t} &= 0.30 \\
v_{1z} &= 0.30 \\
v_{tz} &= 0.54 \\
G_{1t} &= 0.94 \times 10^6 \text{ psi} \\
G_{1z} &= 0.94 \times 10^6 \text{ psi} \\
G_{tz} &= 0.50 \times 10^6 \text{ psi} \\
\alpha_1 &= 0.20 \times 10^{-6} / ^\circ\text{F} \\
\alpha_t &= 16.0 \times 10^{-4} / ^\circ\text{F} \\
\alpha_z &= 16.0 \times 10^{-4} / ^\circ\text{F}
\end{aligned}$$

Layer 2 (0° layer)

The same properties as above.

A uniform displacement of 0.001" is applied in y-direction simulating a constant strain loading and no thermal loading is applied (temp.=0). The delamination is assumed to take place between layer 1 and layer 2 starting at the outer edge at the intersection of free edge and transverse crack. Boundary conditions are provided to make x=0, y=0 and z=0 symmetric surfaces.

The initial finite element mesh without double nodes is as shown in the Figure 6.2.

6.2 PREPROCESSOR INPUT DATA

Group I

```
8 node E1.[02/902]s; delam- MECH load; 5x3x5 MESH-man.inp (10/30/87)
```

In this first group the heading to be printed is given on one card.

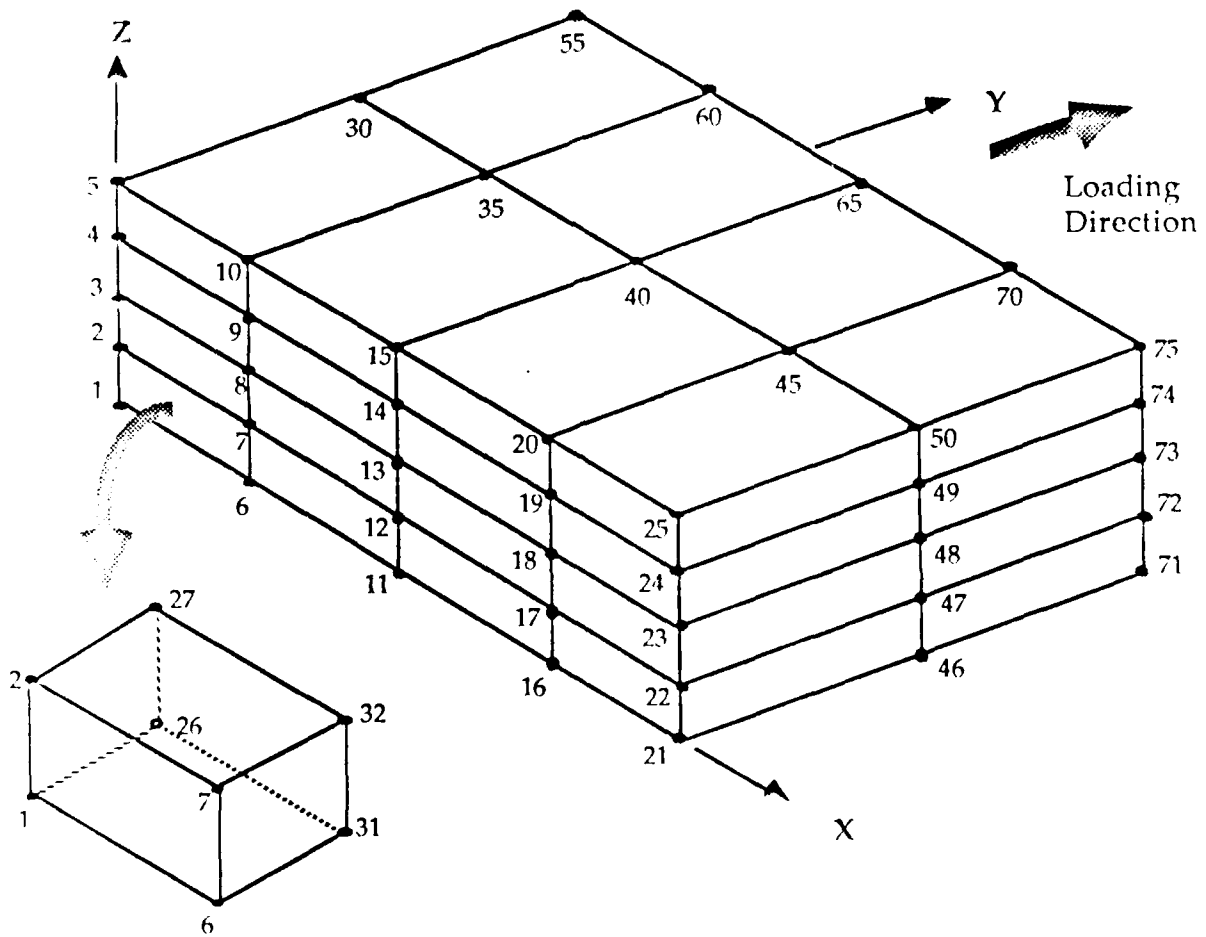


FIG. 6.2 INITIAL FINITE ELEMENT MESH WITHOUT DOUBLE NODES

Group II

8
5,3,5,0.0
0.0, 2.0, 4.0, 6.0, 8.0
0.0, 3.0, 6.0
0.0, 0.5, 1.0, 1.5, 2.0

The first card indicates that 8-node brick element is to be used. The number of coordinates in x, y and z directions are 5, 3 and 5 respectively. These are given in the second card. The fourth entry in this card is 0.0 and it indicates that there is no hole (hole radius = 0.0). The values of x, y and z coordinates are given in the subsequent three cards. No data termination card is required for this set of data.

Group III

1, 300.0, 75, 1
-1, 0.0, 0, 0
1, 300.0, 32, 1
-1, 0.0, 0, 0
1, 1, 32, 1
-1, 0, 0, 0
1, 1, 16, 1
17, 2, 32, 1
-1, 0, 0, 0
2, 16
18, 32
-1, 0

In this group, the cards 2, 4, 6, 9 and 12 are for data termination. The first card indicates that all the nodes from 1 to 75 in increments of 1 have a temperature of 300.0 °F. Similarly, the third card is for elements which signifies that stress free temperature for elements from 1 to 32 in increments of 1 is 300.0 °F. The material serial number to which each element belongs to is given in 5th card. In the present problem, two layers of laminate are made up of same material. So, in this card, it is given that elements 1 to 32 (in increments of 1) belong to material set 1.

However, the two layers have different orientations and they are indicated in the 7th and 8th cards. The elements 1 to 16 have the material axis orientation set 1 and 17 to 32 have set 2.

The 10th and 11th cards are to take advantage of the set of identical elements made of the same material. The first card denotes that elements 2 thru 16 are identical to element no. 1 and the same element stiffness matrix is used. Similarly, the 2nd card denotes that elements 18 thru 32 are the same as the preceding element no. 17.

Group IV

0,2

This card is for split or notch simulation. The first entry (0) indicates that there are 0 double nodes for split generation. The second entry 2 is for split plane parallel to xz-plane. Since the number of double nodes are zero, the value of second entry can be 1, 2 or 3 and no split will be generated.

Group V

15
3, 8, 13, 18, 23, 28, 33, 38, 43, 48, 53, 58, 63, 68, 73
0.0, 8.0, 0.0, 6.0, 1.0, 2.0

The information of nodes which are to be doubled is given in this set of cards. The first card says that there are 15 nodes to be doubled and the succeeding card gives the original numbers of the nodes which are to be doubled. The last card gives limits of the coordinates of the solid in which the second set of double nodes are to be placed.

Group VI

EL	1	21.0E6
ET	1	1.7E6
EZ	1	1.7E6
NULT	1	0.3
NULZ	1	0.3
NUTZ	1	0.54
GLT	1	0.94E06
GLZ	1	0.94E06
GTZ	1	0.50E06
ALFL	1	0.2E-6
ALFT	1	0.16E-4
ALFZ	1	0.16E-4

-1

1 5 25 55
 2 25 75 5
 -1

This set of cards will furnish data regarding the material properties. These properties can be given in any order. The last three cards are to define 2 sets of material principal axes orientations.

Group VII

-1

This set of cards is to specify force boundary conditions. In this particular problem a single -1 card signify that there are no force boundary conditions.

Group VIII

4	UY	0.0	24	5
5	UY	0.0	25	5
1	UX	0.0	51	25
2	UX	0.0	52	25
3	UX	0.0	53	25
4	UX	0.0	54	25
5	UX	0.0	55	25
1	UZ	0.0	21	5
26	UZ	0.0	46	5
51	UZ	0.0	71	5
51	UY	0.001	75	5

-1

The displacement boundary conditions are prescribed in this set of cards. For example, the first card specifies the y-component of displacement as 0.0 for the nodes from 4 thru 24 at 5 node intervals. That is, y-component displacement of nodes 4, 9, 14, 19, 24 have 0.0 value. The boundary conditions of other nodes are prescribed in the succeeding cards of this last set. In this set the original node numbers are to be given and they will be modified using the double nodes information given in Group V

6.3 MODIFICATIONS OF PREPROCESSOR OUTPUT DATA

The output of the preprocessor program will be two data files if NSD is not equal to zero. The file KSAPIN.DAT will consist most of the data necessary to run the main code, KSAP II. The other file FOR010.DAT will contain the information about the modified numbers of the double nodes and their original node numbers on the split plane. These are only for reference and do not appear in the modifications of KSAPIN.DAT.

The original node numbers related to delamination are listed towards the end of KSAPIN.DAT. Both the double nodes of each pair will have the same displacements before the crack passes through them, and both these will have free force boundary conditions once the crack passes through that pair and they are separated. However, some of these pairs lying on $y=0$ plane behave differently.

The double nodes (3,4),(9,10),(15,16),21,22), and (27,28) are located on the symmetric plane which also happens to be the plane of transverse crack. The y-component of the displacement (U_y) of the node in each pair before they are opened have the same value. When the crack passes through

that node, the two nodes will be separated. The y-component of the force (F_y) of the bottom node will be zero whereas the top node will be still on the symmetric plane ($y=0$) and hence will have a displacement boundary condition ($U_y=0$). It may also be observed that the other two components of the forces (F_x, F_z) will become zero for both the nodes of the pair once the crack passes through that pair. To facilitate these two types of degrees of freedom of the double nodes the input data of KSAP II has to be supplemented with the data as shown below.

```

10
 3  2 0.0
 4  2 0.0
 9  2 0.0
10  2 0.0
15  2 0.0
16  2 0.0
21  2 0.0
22  2 0.0
27  2 0.0
28  2 0.0
15
 3  4 1 0 1
 9 10 1 0 1
15 16 1 0 1
21 22 1 0 1
27 28 1 0 1
33 34 1 1 1
39 40 1 1 1
45 46 1 1 1
51 52 1 1 1
57 58 1 1 1
63 64 1 1 1
69 70 1 1 1
75 76 1 1 1
81 82 1 1 1
87 88 1 1 1

```

The number (10) in the first data card denotes the total number of y-degrees of freedom of double nodes ($5 \times 2 - 10$) on the symmetric plane ($y=0$). The following 10 data cards input the details of the node number, degree of freedom (1 for U_x , 2 for U_y and 3 for U_z) and the value of the displacement. All these data corresponds to that value before the nodes

are opened. Following this will be the comprehensive data input for the overall double nodes. The number 15 denotes that there are 15 pairs of double nodes whose details are given in the 15 succeeding cards. The first two numbers in each card (for example, 3 and 4) are two node numbers in each pair. The three following numbers (1's or 0's) will describe the behavior of the double nodes in x,y,z degrees of freedom respectively when the nodes are opened. The number 1 for any degree of freedom signifies that the nodes will have the same displacement before opening and will have zero nodal force in that degree of freedom after opening. Number 0 for any particular degree of freedom signifies that the nodes will not behave in the above manner with regard to that degree of freedom. Thus the degrees of freedom (y) for the double nodes on the symmetric plane, $y=0$ which are described in the previous set will have zeros in this set of data. For example, the nodes 3 and 4 will have $U_y=0$ corresponding to y-degree of freedom as they are specified to behave in another manner in the preceding set of data. The nodes 27 and 28 will have all 1's as they are located away from the plane $y=0$.

After furnishing the above data regarding the degrees of freedom of the double nodes, comes the data regarding the opening of the double nodes thus simulating a crack propagation as shown below:

```

0 0 0
 1 32      ! Free format
27 0 2
27 28 1
27 28 3
 0 0 0
  1 32
21 0 2
21 22 1
21 22 3
57 58 1
57 58 2
57 58 3

```

```
0 0 0
  1 32
9999 9999 0
```

At any step (iteration) the crack can be made to pass through one or more number of double nodes. Each data card consists of three numbers. The last number corresponds to the degree of freedom which is relaxed, that is, which will have free force boundary condition. If there is a zero as the second number then the first number should be a node number of the double nodes on the symmetric plane $y=0$ whose data is specified in the first set. For example, the '27 0 2' specifies that the y -degree of freedom (2) of node no. 27 (which is a node in 1st set of data) is relaxed ($F_y=0$). That is, this node is free to move in y -direction. If the second number is also non zero then the first two numbers correspond to the two nodes of a pair of double nodes and as explained above the third number specifies the degree of freedom in which these two nodes are free to move. Thus the data specifies that the crack passes through the pair of nodes 27 and 28 and these two nodes are free to move in x, z directions whereas in the y -direction only node no. 27 is free to move. That implies that node 28 will have the earlier specified displacement ($U_y=0$). All three zeros signify the end of the crack opening data for that step. Thus when it is desired to calculate the stresses and displacements before any crack is simulated it is necessary to place this card (0,0,0) as in line 1. Immediately following this card in each step a selective stress print option can be given. The two numbers signify the range of elements for which the stress print out is desired. If the first two numbers in the crack opening data are prescribed as 9999 and 9999 then that signals the termination of crack propagation sequence.

6.4 OUTPUT OF KSAP II PROGRAM

As can be seen the output of KSAP II is self explanatory. To start with, it consists of all the mesh details regarding the nodes, coordinates, the degrees of freedom, elements etc.. It also furnishes information of the material properties used and the material number to which each element belongs. The output also provides some details about total number of equations, bandwidth, number of blocks etc..

Then the results will be output as the crack is simulated. For each step the nodal displacements and element stresses (at the center of the element) are printed as desired in the input data. Usually, these results are output starting from no crack state (STEP 0) and crack can be simulated opening one or more nodes at each step. At each succeeding step (STEP 1,2,....) the energy released is calculated and is printed immediately after nodal displacements and forces. If the energy release rate is desired then it can be calculated by dividing these energies released by the incremental crack areas.

APPENDIX - A

LISTING OF THE PREPROCESSOR


```

55 !           the direction of displ. or rotation
56 !     VD()    --- Value of displ.
57 !     VR()    --- Value of rotation
58 !     LAX(2,3) -- 3 nodes (number) to define matl. princ. axis
59 !               (max. of 2 sets)
60 !     HED(18)  -- Heading information as first line of output
61 !     MORT()   -- material axes orientation set number
62 !     NI()    } --- definition of material principle axes
63 !     NJ()    }
64 !     NK()    }
65 ccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccccc
66
67     CHARACTER*30 FILNAM
68     CHARACTER*1 MESH
69     DIMENSION R(7),NR(15),NRR(21),NOS(3),NOE(3),NINC(3),
70 .   XX(20),YY(20),ZZ(15),XMESH(4000),YMESH(4000),
71
72 .   X(4000),Y(4000),Z(4000),IX(4000,6),T(4000),mip(4000,2),
73
74 .   IDM(3000),MAT(3000),NN(3000,21),MAXES(3000),TZ(3000),
75
76 .   E11(4),E22(4),E33(4),ANU12(4),ANU13(4),ANU23(4),
77 .   G12(4),G13(4),G23(4),NBD(200),ND(200,4),KD(200),
78 .   L(4),VD(200),VR(200),KR(200),HED(18),
79 .   NOND(200),XD(200),YD(200),ZD(200), nons(400),
80 .   ALP1(4),ALP2(4),
81 .   ALP3(4),MORT(10),NI(10),NJ(10),NK(10),
82 .   FX(200),FY(200),FZ(200),NBF(200),ID1(200)
83
84 c--     mip ( ) - the corresponding new nodal numbers
85 C
86
87     PI=3.1415926535897932
88     eps=1.0e-04
89     WRITE (5,*) '...ENTER INPUT FILE NAME.....'
90     READ (5,55) FILNAM
91     IRD=56      ! READ TAPE NO.
92     OPEN (UNIT=IRD,FILE=FILNAM,STATUS='OLD')
93     READ(IRD,25) (HED(I),I=1,18)
94     25 FORMAT(18A4)
95
96 ! Read whether the element is 8- or 21 noded
97 c     NTYPE=21
98 c     NTYPE=8
99     read (IRD,*) ntype
100     NELTYP=1
101     INTR=2      ! Integration order for r,s - coordinates
102     INIT=2      ! Integration order for t- direction
103     IF (NTYPE.EQ.21) THEN
104         INTR=4
105         INIT=2
106     END IF
107 C
108 C!!!! FOLLOWING STATEMENTS ARE FOR VARYING GRID GENERATION.

```

```

109 C
110 C      NODE GENERATION
111      READ (IRD,*),NONX,NONY,NONZ,RHOLE
112      READ (IRD,*)(XX(I),I=1,NONX)
113      READ (IRD,*)(YY(I),I=1,NONY)
114      READ (IRD,*)(ZZ(I),I=1,NONZ)
115
116 C--   correction for inside nodal coordinates for 21-node el.
117
118      if (ntype.eq.21) then
119      do 1444 i=1,nonx
120 1444   if (mod(i,2).eq.0) xx(i)=(xx(i-1)+xx(i+1))/2
121 C      type *,(xx(i),i=1,nonx)
122      do 1445 i=1,nony
123 1445   if (mod(i,2).eq.0) yy(i)=(yy(i-1)+yy(i+1))/2
124 C      type *,(yy(i),i=1,nony)
125      do 1446 i=1,nonz
126 1446   if (mod(i,2).eq.0) zz(i)=(zz(i-1)+zz(i+1))/2
127 C      type *,(zz(i),i=1,nonz)
128      end if
129
130      IF (RHOLE.GT.eps)CALL HOLE (RHOLE,NONX,NONY,NONZ,
131      .XX,YY,zz,X,Y,Z)
132      IF (RHOLE.GT.eps) GO TO 444
133
134      DO J=1,NONY
135      DO I=1,NONX
136      DO K=1,NONZ
137      N=(J-1)*NONX*NONZ+(I-1)*NONZ+K
138      X(N)=XX(I)
139      Y(N)=YY(J)
140      Z(N)=ZZ(K)
141      END DO
142      END I
143      END DO
144
145 C-   END OF POLAR MESH GENERATION
146
147 444      NTON=NONX*NONY*NONZ
148
149 C--   MESH PLOTTING OPTION ON HP PLOTTER-----
150      WRITE (5,*) 'Do you need MESH plot Original nodes?..(Y/N)'
151      READ (5,55) MESH
152 55      FORMAT (A)
153      IF (MESH.EQ.'Y'.OR.MESH.EQ.'y') CALL
154      . MESHPL (X,Y,NONX,NONY,NONZ,MIP,1,ntype)
155      DO I=1,NTON
156      XMESH(I)=X(I)
157      YMESH(I)=Y(I)
158      END DO
159      IF (NN(NTOE,7).GT.NTON) THEN
160      DO 666 I=1,NTOE
161      DO 666 J=1,NTYPE
162 666      IF (NN(I,J).GT.NTON) NN(I,J)=NN(I,J)-NTON

```

```

163         END IF
164
165 148     READ(IRD,*) N,TEMP,NEND,INC
166         IF(N.EQ.-1) GO TO 149
167         DO I=N,NEND,INC
168             T(I)=TEMP
169         END DO
170         GO TO 148
171
172 149     continue
173 300     CONTINUE
174 C-----
175 C     ELEMENT GENERATION
176 C
177         N1=NONX
178         N2=NONY
179         N3=NONZ
180         N13=N1*N3
181
182         IF(NTYPE.EQ.8) THEN
183
184             NOS(3) =N3-1           ! third level gener.code
185             NINC(3)=1
186             NOE(3) =(N2-1)*(N1-1)
187             NOS(2) =N2-1           ! second level gener. code
188             NINC(2)=N1*N3
189             NOE(2) =N1-1
190             NOS(1) =N1-1           ! first level gener. code
191             NINC(1)=N3
192             NOE(1) =1
193
194             NRR(1)=2+  N3+  N13
195             NRR(2)=2+           N13
196             NRR(3)=2
197             NRR(4)=2+  N3
198             NRR(5)=1+  N3+  N13
199             NRR(6)=1           +  N13
200             NRR(7) =1
201             NRR(8)=1+  N3
202
203         ELSE IF(NTYPE.EQ.21) THEN
204
205             NOS(3) =(N3-1)/2       ! third level gener.code
206             NINC(3)=2
207             NOE(3) =(N2-1)*(N1-1)/4
208             NOS(2) =(N2-1)/2       ! second level gener. code
209             NINC(2)=2*N1*N3
210             NOE(2) =(N1-1)/2
211             NOS(1) =(N1-1)/2       ! first level gener. code
212             NINC(1)=2*N3
213             NOE(1) =1
214
215             NRR(1) =3+2*N3+2*N13
216             NRR(2) =3           +2*N13

```

```

217      NRR(3) =3
218      NRR(4) =3+2*N3
219      NRR(5) =1+2*N3+2*N13
220      NRR(6) =1      +2*N13
221      NRR(7) =1
222      NRR(8) =1+2*N3
223
224      NRR(9 )=3+  N3+2*N13
225      NRR(10)=3      + N13
226      NRR(11)=3+  N3
227      NRR(12)=3+2*N3+  N13
228      NRR(13)=1+  N3+2*N13
229      NRR(14)=1      + N13
230      NRR(15)=1+  N3
231      NRR(16)=1+2*N3+  N13
232      NRR(17)=2+2*N3+2*N13
233      NRR(18)=2+2*N13
234      NRR(19)=2
235      NRR(20)=2+2*N3
236
237      NRR(21)=2+  N3+  N13
238      END IF
239      NO=0
240      DO 132 I=1,NTYPE
241 132  NN(NO+1,I)=NRR(I)
242
243 1   Perform element generation
244      DO 138 M=1,3
245      DO 138 K=2,NOS(M)
246      DO 138 I=1,NOE(M)
247      N=NO+I+NOE(M)*(K-1)
248      DO 1375 J=1,NTYPE
249 1375 NN(N,J)=NN(NO+I,J)+NINC(M)*(K-1)
250 138  continue
251      NO=N
252      NTOE=NO
253 1   DO 1415 IE=1,NTOE
254 1   TYPE *
255 1   TYPE *, ' ***** ELEMENT ' , IE
256 1415 TYPE 105, (NN(IE,J),J=1,NTYPE)
257 105  FORMAT(8I5)
258
259
260 1   Read element stress free temperature
261 158  READ(IRD,*) N,TEMP,NEND,INC
262      IF(N.EQ.-1) GO TO 159
263      DO 1585 IE=N,NEND,INC
264 1585 IZ(IE)=TEMP
265      GO TO 158
266 157  continue
267
268 1   Read element material identification number
269 1458 READ(IRD,*) N,MATRL,NEND,INC
270      IF(N.EQ.-1) GO TO 1459

```

```

271      DO 14585 IE=N,NEND,INC
272 14585 IDM(IE)=MATRL
273      GO TO 1458
274 1459 continue
275
276 C ---MAXES(I)-----MATL. AXIS ORIENT.:
277 !      Read element material axis orientation identification number
278 1658 READ(IRD,X) N,MORTI,NEND,INC
279      IF(N.EQ.-1) GO TO 1659
280      DO 16585 IE=N,NEND,INC
281 16585 MAXES(IE)=MORTI
282      GO TO 1658
283 1659 continue
284 !      Read if the stiffness matrix of an element (or a group of elements)
285 !      is same as the previous element
286
287      DO 145 I=1,NTOE
288 145  MAT(I)=0
289
290 176  READ(IRD,X) M1,M2
291 c   WRITE(111,X) 'M1,M2',M1,M2
292      IF(M1.EQ.-1)GO TO 1765
293      DO 1762 I=M1,M2
294 1762 MAT(I)=1
295      GO TO 176
296 1765 continue
297 C-----
298 C      BOUNDARY CONDITION GENERATION
299 c      Following statement are to fix all rotations when
300 c      only translational d.o.f. (eltype #8 is used)
301      DO 2010 I=1,NTON
302      DO 2010 J=1,6
303      IF(J.LE.3) IX(I,J)=0
304      IF(J.GE.4) IX(I,J)=1
305 2010 continue
306
307 !      While generating 21-node elements some nodes are at the center of faces
308 !      These degrees of freedom will be removed below:
309
310 !      Nodes not used in defining any element are found
311 !      and all its d.o.f. set equal to 1 (i.e., eliminated)
312      DO IE=1,NTOE
313      DO IN=1,NTYPE
314      II=NN(IE,IN)
315      IX(II,6)=10      !to identify which nodes are used
316      end do
317      end do
318
319      DO IND=1,NTON
320      IF(IX(IND,6).EQ.10)THEN
321      IX(IND,6)=1
322      ELSE
323      DO IDG=1,6
324      IX(IND,IDG)=1

```

```

325     END DO
326     END IF
327     end do
328 !!!!! FOR DOUBLING NODES !!!!!!!!!!!!!!!!!!!!!!!!!!!!!!!
329     do ip=1,nton
330         mip(ip,2)=0
331     end do
332     For Splitting.....
333     NSD=0
334     READ(IRD,*) NSD, IDIR      ! No. of nodes to be doubled, Direction Vector
335     IF(NSD.NE.0) THEN
336         NSD1=0
337 686     READ (IRD,*) N,NEND,INC
338         IF (N.EQ.-1) GO TO 688
339         DO 687 I=N,NEND,INC
340             NSD1=NSD1+1
341 687     NONS(NSD1)=I
342         GO TO 686
343     READ(IRD,*) (NONS(I),I=1,NSD)      !original node # to be doubled
344 688     IF (NSD1.NE.NSD) NSD=NSD1      !check on total no. of d.n's
345 c--    to delete face center no. from double node list.....
346         IF (NTYPE.EQ.21) CALL DELETE(NONX,NONY,NONZ,NSD,NONS)
347         do i=1,nsd
348             ia=nons(i)
349             mip(ia,2)=1
350         end do
351     end if
352     NTD=0
353     READ(IRD,*) NTD      ! Number of nodes to be doubled
354
355     Define the zone of delamination
356 c     store coord. of the nodes to be doubled
357     IF(NTD.NE.0) THEN
358         READ(IRD,*) (NOND(I),I=1,NTD)      !original node # to be doubled
359 c--    to delete face center no. from double node list.....
360         IF (NTYPE.EQ.21) call delete(nonx,nony,nonz,ntd,nond)
361         DO I=1,NTD
362             IA=NOND(I)
363             MIP(IA,2)=1
364         END DO
365         READ(IRD,*) XL,XU,YL,YU,ZL,ZU
366         DO I=NTD,1,-1
367             XD(I)=X(NOND(I))
368             YD(I)=Y(NOND(I))
369             ZD(I)=Z(NOND(I))
370         END DO
371     END IF
372 c     if (ntd+nsd.ne.0) then
373         IP=0
374         DO I=1,NTON
375             IP=IP+1
376             MIP(I,1)=IP
377             IF(MIP(I,2).EQ.1) THEN
378                 IP=IP+1

```

```

379      MIP(I,2)=IP
380      END IF
381      END DO
382 C      end if
383      IF (NSD.NE.0)
384      .call split (NTON,nsd,idir,nons,mip,x,y,z,I,ntype,nos,NN,ntoe)
385 !      For Delamination.....
386 C      DO I=1,NTON
387 C      TYPE *,I,(MIP(I,J),J=1,2)
388 C      END DO
389
390      IF(NTD.EQ.0) GO TO 899
391 C-  CORRECTIONS FOR DOUBLE NODES IN planes normal to x, y or z -directions
392 C  FIND KND (THE # OF THE NODE CURRENTLY TO BE DOUBLED)
393
394      DO 525 I=1,NTD
395      DO J=NOND(I),NTON
396
397      DIST=SQRT((X(J)-XD(I))**2+ (Y(J)-YD(I))**2+ (Z(J)-ZD(I))**2)
398      IF(DIST.LT.0.00001) THEN
399      KND=J
400
401 C      BEGIN CHANGING NODE NUMBERS & COORD.
402
403 C      Change element node numbers if it is > knd by adding 1 to it
404      DO 346 JE=1,NTOE
405      DO 345 K=1,NTYPE
406 345  IF (NN(JE,K).GT.KND) NN(JE,K)=NN(JE,K)+1
407 346  CONTINUE
408 C      Also move coords. downstream by one slot and also assign (knd+1)th
409 !      same as (knd)th
410      DO M=NTON,KND,-1
411      X(M+1)=X(M)
412      Y(M+1)=Y(M)
413      Z(M+1)=Z(M)
414      T(M+1)=T(M)
415      END DO
416
417 C--  Allocate nodes of the pair to the appropriate elements depending
418 !      on which side of the double nodes' plane they (element) lie
419      nface=NTYPE
420      DO 3455 K=1,NTOE
421
422      IF(NTYPE.EQ.8) THEN      ! Find the coordinates of the center of the
423      XC=0.                  ! element to determine if it belongs
424      YC=0.                  ! to the delamination zone
425      ZC=0.
426      DO IA=1,8
427      III=NN(K,IA)
428      XC=XC+X(III)
429      YC=YC+Y(III)
430      ZC=ZC+Z(III)
431      END DO
432      XC=XC/8.

```

```

433      YC=YC/8.
434      ZC=ZC/8.
435      ELSE
436      III=NN(K,21)
437      XC=X(III)
438      YC=Y(III)
439      ZC=Z(III)
440      END IF
441      IF(K.EQ.32) THEN
442      TYPE *, ' ... ELEMENT # ... ', K
443      TYPE *, ' - CENTER:', XC,YC,ZC
444      TYPE *, ' .. BEFORE ...'
445      TYPE *, (NN(K,M),M=1,NEACE)
446      END IF
447      DO 3453 M=1,nface
448      IF (NN(K,M).EQ.KND
449      .and.(XC.GT.XL.AND.XC.LT.XU)
450      .and.(YC.GT.YL.AND.YC.LT.YU)
451      .and.(ZC.GT.ZL.AND.ZC.LT.ZU))
452
453      . NN(K,M)=KND+1
454 3453      END DO
455 3455 END DO
456
457
458      NTON=NTON+1
459
460      go to 525      ! so that the modifications are done only once
461      ! for each double node
462      END IF      ! (DIST.LT.0.00001)
463      END DO      !J - LOOP
464 525      END DO      !I - loop
465
466      899 CONTINUE      ! SKIP IF NTD=0
467
468      write(5,*)'...Do You Need Mesh with Double Nodes (Y/N).. '
469      READ (5,55) MESH
470      IF (MESH.EQ.'Y'.OR.MESH.EQ.'y') CALL
471      . MESHPL (XMESH,YMESH,NONX,NONY,NONZ,MIP,2,otype)
472
473      IF (MESH.EQ.'Y'.OR.MESH.EQ.'y')
474      .write(5,*)'...Do You Want to Continue (Y/N).. '
475      IF (MESH.EQ.'Y'.OR.MESH.EQ.'y')
476      .READ (5,55) MESH
477      IF (MESH.EQ.'Y'.and.MESH.EQ.'y') stop
478
479      !!!!!!! ABOVE ARE FOR DOUBLING NODES !!!!!!!
480
481      Material properties
482      OPEN (UNIT=9,FILE='KSAPIN.DAT',STATUS='NEW')      !!!!!
483
484      nummat=0
485 500      read(IRD,550) A,I,U
486      if(i.gt.nummat) nummat=i

```

```

487 550  FORMAT(A4,I4,4X,G17.7)
488      IF(A.EQ.' EL') E11(I)=V
489      IF(A.EQ.' ET') E22(I)=V
490      IF(A.EQ.' EZ') E33(I)=V
491      IF(A.EQ.'NULT') ANU12(I)=V+E22(I)/E11(I)
492      IF(A.EQ.'NUTZ') ANU23(I)=V+E33(I)/E22(I)
493      IF(A.EQ.'NULZ') ANU13(I)=V+E33(I)/E11(I)
494      IF(A.EQ.' GLT') G12(I)=V
495      IF(A.EQ.' GLZ') G13(I)=V
496      IF(A.EQ.' GTZ') G23(I)=V
497      IF(A.EQ.'ALFL') ALP1(I)=V
498      IF(A.EQ.'ALFT') ALP2(I)=V
499      IF(A.EQ.'ALEZ') ALP3(I)=V
500      IF(A.EQ.'-1') GO TO 560
501      GO TO 500
502
503 560  DO I=1,10
504      READ(IRD,570) (NR(J),J=1,4)
505 570  FORMAT(4I5)
506      IF(NR(1).EQ.-1) GO TO 579
507      MORT(I)=NR(1)
508      NI(I)=NR(2)
509      NJ(I)=NR(3)
510      NK(I)=NR(4)
511      END DO
512 579  NORTH0=I-1
513
514
515
516 !THE BELOW PORTION IS MODIFICATION TO FIND DIRECTION VECTOR
517 !above one does not work for hole problems (polar mesh).--
518 C      TYPE *, 'NTON',NTON
519      DO 991 I=1,NTON
520      DO 992 J=1,NTON
521      IF (J.EQ.I) GO TO 992
522      IF (ABS(Y(J))-ABS(Y(I)).GT.1.E-06) GO TO 992
523      IF (ABS(Y(J)-Y(I)).GT.1.E-06) GO TO 992
524      IF (ABS(Z(J))-ABS(Z(I)).GT.1.E-06) GO TO 992
525      IF (ABS(Z(J)-Z(I)).GT.1.E-06) GO TO 992
526      IF (X(J)-X(I).LT.0.0) GO TO 992
527      DO 993 K=1,NTON
528      IF (K.EQ.I.OR.K.EQ.J) GO TO 993
529      IF ((ABS(X(K))-ABS(X(I))).GT.1.E-06) GO TO 993
530      IF (ABS(X(K)-X(I)).GT.1.E-06) GO TO 993
531      IF ((ABS(Z(K))-ABS(Z(I))).GT.1.E-06) GO TO 993
532      IF (ABS(Z(K)-Z(I)).GT.1.E-06) GO TO 993
533      IF ((Y(K)-Y(I)).LT.0.0) GO TO 993
534      DO 994 LLL=1,NTON
535      IF (LLL.EQ.I.OR.LLL.EQ.J.OR.LLL.EQ.K) GO TO 994
536      IF ((ABS(X(LLL))-ABS(X(I))).GT.1.E-06) GO TO 994
537      IF (ABS(X(LLL)-X(I)).GT.1.E-06) GO TO 994
538      IF ((ABS(Y(LLL))-ABS(Y(I))).GT.1.E-06) GO TO 994
539      IF (ABS(Y(LLL)-Y(I)).GT.1.E-06) GO TO 994
540      IF ((Z(LLL)-Z(I)).LT.0.0) GO TO 994

```

```

541      LQ=I      !y & z coord. same as J, xcoord. diff.
542      LX=J      !y & z coord. same as I, xcoord. diff.
543      LY=K      !.ne.I or J, x & z coord. same as I and y is diff.
544      LZ=LLL     !.ne. I,J or K, x & y coord. same as I and z diff.
545      GO TO 995
546 994      CONTINUE
547 993      CONTINUE
548 992      CONTINUE
549 991      CONTINUE
550 995      CONTINUE
551
552 C-- READ & GENERATE CONCENTRATED LOAD DATA:
553
554      NTBF=0
555      NINCC=1
556 640      READ(IRD,710) N1,A,V,N2,'INCC
557      IF(N1.EQ.-1) GO TO 699
558
559      DO I=N1,N2,NINCC
560      NTBF=NTBF+1
561      NBF(NTBF)=I
562      IF(A.EQ.' EX') EX(NTBF)=V
563      IF(A.EQ.' FY') FY(NTBF)=V
564      IF(A.EQ.' FZ') FZ(NTBF)=V
565      END DO
566      GO TO 640
567 699      CONTINUE
568 C
569 C      READ & GENERATE DISPL. B.C.E. DATA
570 C
571      NTBD=0
572      NINCC=1
573 700      READ(IRD,710) N1,A,V,N2,NINCC
574 710      FORMAT(16,1X,A4,1X,F10.0,12X,2I6)
575      IF(N1.EQ.-1) GO TO 799
576
577
578      IF(V.EQ.0) THEN
579      DO I=N1,N2,NINCC
580      IF(A.EQ.' UX') IX(I,1)=1
581      IF(A.EQ.' UY') IX(I,2)=1
582      IF(A.EQ.' UZ') IX(I,3)=1
583      END DO
584
585      ELSE
586      NELTYP=2
587      V1=V
588      V2=0
589      JD=1
590      JR=0
591      IF(A.EQ.' UX') then
592      L(1)=LQ
593      L(2)=LY !LQ -> LY ALONG Y DIR.
594      L(3)=LQ !LQ -> LZ ALONG Z DIR.

```

```

595      L(4)=LZ
596      else if(A.EQ.' UZ') then
597          L(1)=L0
598          L(2)=LX
599          L(3)=L0
600          L(4)=LY
601      else if(A.EQ.' UY') then
602          L(1)=L0
603          L(2)=LZ
604          L(3)=L0
605          L(4)=LX
606      END IF
607
608      DO I=N1,N2,NINCC
609          NTBD=NTBD+1
610          NBD(NTBD)=I
611          DO J=1,4
612              ND(NTBD,J)=L(J)
613          END DO
614          VD(NTBD)=V1
615          VR(NTBD)=V2
616          KD(NTBD)=JD
617          KR(NTBD)=JR
618      END DO
619      end if
620      GO TO 700
621 799 CONTINUE
622
623 C writing double nodes to for010.dat, and renumbering displacement
624 c boundary elements and force boundary node number
625
626      do i=nton,1,-1
627          do ic=1,2
628              ip=mip(i,ic)
629              if(ip.ne.0) then
630                  do id=1,6
631                      ix(ip,id)=ix(i,id)
632                  end do
633              end if
634          end do
635      end do
636
637      if(ntd+nsd.ge.1) then
638          DO K=1,NTBD
639
640              NBDK1=MIP(NBD(K),1)
641              NBDK2=MIP(NBD(K),2)
642              NBD(K)=NBDK1
643              KR(K)=MIP(KR(K),1)
644              KD(K)=MIP(KD(K),1)
645
646              IF(NBDK2.NE.0) THEN
647                  ntbd=ntbd+1
648                  NBD(NTBD)=NBDK2

```

```

649      KR(NTBD)=KR(K)
650      KD(NTBD)=KD(K)
651      VD(NTBD)=VD(K)
652      VR(NTBD)=VR(K)
653      DO M=1,4
654      ND(NTBD,M)=ND(K,M)
655      END DO
656      END IF
657      END DO
658
659 C      RENUMBER NI,NJ,NK (ORIENT. DEFINITION)
660      DO K=1,NORTHO
661      NI(K)=MIP(NI(K),1)
662      NJ(K)=MIP(NJ(K),1)
663      NK(K)=MIP(NK(K),1)
664      END DO
665
666      end if      ! ntd.ge.1
667
668
669 C      Renumbering the force boundary conditions and adding
670 C      new force boundary conditions of double nodes if necessary
671
672      DO K=1,NTBF
673      if(mip(nbf(k),2).ne.0) THEN
674      WRITE (5,*) ' ....D.N at force b.c; NODE no..', NBF(K)
675      STOP 'STOPPING due to double node at force boundary cond.'
676      END IF
677      NBF(K)=mip(NBF(K),1)
678      END DO
679 C
680 C      OUTPUT NODE DATA
681 C
682      WRITE(9,1100) (HED(I),I=1,18)
683 1100  FORMAT(18A4)
684      IADOF=NTD*6
685      IADOF==ADDITIONAL D.O.F. DUE TO DOUBLE NODES & DISP.BCS.
686      WRITE(9,1101) NTON,NELTYP,IADOF
687 1101  FORMAT(15,15,4X,'1',14X,'0',15X,15)
688
689
690      WRITE(9,11015) (IX(1,J),J=1,6),X(1),Y(1),Z(1),I(1)
691 11015  FORMAT(4X,'1C',14,5I5,3F10.4,5X,F10.0)
692      KN=0
693      WRITE(9,1102) 2,(IX(2,J),J=1,6),X(2),Y(2),Z(2),KN,I(2)
694      KNH=0
695      DO I=3,NTON-1
696      IXM1=ix(i,1)-ix(i-1,1)
697      IXM2=ix(i,2)-ix(i-1,2)
698      IXM3=ix(i,3)-ix(i-1,3)
699      IXM4=ix(i,4)-ix(i-1,4)
700      IXM5=ix(i,5)-ix(i-1,5)
701      IXM6=ix(i,6)-ix(i-1,6)
702      DXM=X(I)-X(I-1)

```

```

703      DYM=Y(I)-Y(I-1)
704      DZM=Z(I)-Z(I-1)
705      DTM=T(I)-T(I-1)
706      IXP1=ix(i+1,1)-ix(i,1)
707      IXP2=ix(i+1,2)-ix(i,2)
708      IXP3=ix(i+1,3)-ix(i,3)
709      IXP4=ix(i+1,4)-ix(i,4)
710      IXP5=ix(i+1,5)-ix(i,5)
711      IXP6=ix(i+1,6)-ix(i,6)
712      DXP=X(I+1)-X(I)
713      DYP=Y(I+1)-Y(I)
714      DZP=Z(I+1)-Z(I)
715      DTP=T(I+1)-T(I)
716      KN=0
717      IF (IXM1.EQ.IXP1.AND.IXM2.EQ.IXP2.AND.IXM3.EQ.IXP3.AND.
718      .   IXM4.EQ.IXP4.AND.IXM5.EQ.IXP5.AND.IXM6.EQ.IXP6.AND.
719      .   DXM.EQ.DXP.AND.DYM.EQ.DYP.AND.DZM.EQ.DZP.AND.
720      .   DTM.EQ.DTP) KN=1
721      IF (KN.EQ.0)
722      .WRITE(9,1102) I,(IX(I,J),J=1,6),X(I),Y(I),Z(I),KNM,T(I)
723      KNM=KN
724 1102  FORMAT(I5,6I5,3F10.4,I5,F10.0)
725      END DO
726      i=nton
727      WRITE(9,1102) I,(IX(I,J),J=1,6),X(I),Y(I),Z(I),KN,T(I)
728 C
729      if (neltyp.gt.1) then      !      OUTPUT FOR B.C.E. #7
730 C
731      WRITE(9,1201) NTBD
732 1201  FORMAT(4X,'7',I5/8X,'1.')
733      DO I=1,NTBD
734      WRITE(9,1202) NBD(I),(ND(I,J),J=1,4),KD(I),KR(I),VD(I),VR(I)
735 1202  FORMAT(7I5,5X,2F10.7,' 0.100E+21 ')
736      END DO
737      end if
738 C
739 C      OUTPUT ELEMENT DATA
740 C
741      MAXNOD=NTYPE
742      NOPSET=1      !Number of sets of data requesting stress output
743      WRITE(9,1300) NTOE,NUMMAT,NORTHO,MAXNOD,NOPSET,INTRS,INIT
744 1300  FORMAT(4X,'8',I5,I5,4X,'0',I5,5X,I5,3I5)
745
746 c      I=1
747      do i=1,nummat
748      WRITE(9,1301) I,TZ(I),E11(I),E22(I),E33(I),ANU12(I),ANU13(I),
749      .ANU23(I),G12(I),G13(I),G23(I),ALP1(I),ALP2(I),ALP3(I)
750 1301  FORMAT(I5,4X,'1',20X,'AXIS#1==0-LAYER; AXIS#2==90-LAYER.'/
751      *f10.0,3f10.0,3f10.4/3i10.0,3f10.7)
752 ccc      WRITE(9,13010) T(I),E11(I),E22(I),E33(I),ANU12(I),ANU13(I),
753 ccc      .ANU23(I),G12(I),G13(I),G23(I),ALP1(I),ALP2(I),ALP3(I)
754 13010  FORMAT(F10.0,3F10.0,3F10.4/3F10.0,3F10.7)
755      END DO
756

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```

757      DO I=1,NORTH0
758      WRITE(9,13011) MORT(I),NI(I),NJ(I),NK(I)
759 13011 FORMAT(4I5)
760      END DO
761
762 13005 FORMAT(4I5/4I5/3I5)
763
764      READ(IRD,*) LOC1,LOC2,LOC3,LOC4,LOC5,LOC6,LOC7
765      WRITE(9,13008) LOC1,LOC2,LOC3,LOC4,LOC5,LOC6,LOC7
766 13008 FORMAT(7I5)
767      TA=1.0
768      IF(T(1).EQ.TZ(1)) TA=0.0
769      WRITE (9,13012) TA
770 13012 FORMAT(///F10.0/)
771      IOP=1      ! I.D.# OF STRESS OUTPUT LOCATION SET
772      ISKIP0=0
773
774      DO I=1,NTDE
775      KGM=0
776      if(i.gt.1.and.
777      .IDM(I).EQ.IDM(i-1).AND.MAXES(I).EQ.MAXES(i-1).AND.IOP.EQ.IOP
778      .AND.TZ(I).EQ.TZ(i-1).AND.MAT(I).EQ.MAT(i-1)) then
779      KGM1=NN(I,7)-NN(I-1,7)
780      KGM2=NN(I,8)-NN(I-1,8)
781      KGM3=NN(I,5)-NN(I-1,5)
782      KGM4=NN(I,6)-NN(I-1,6)
783      KGM5=NN(I,3)-NN(I-1,3)
784      KGM6=NN(I,4)-NN(I-1,4)
785      KGM7=NN(I,1)-NN(I-1,1)
786      KGM8=NN(I,2)-NN(I-1,2)
787      KGMX=MAX0(KGM1,KGM2,KGM3,KGM4,KGM5,KGM6,KGM7,KGM8)
788      KGMMN=MIN0(KGM1,KGM2,KGM3,KGM4,KGM5,KGM6,KGM7,KGM8)
789      IF(KGMX.EQ.KGMMN) KGM=KGMX
790      end if
791
792      KGP=0
793      if(i.lt.ntoe.and.
794      .IDM(I).EQ.IDM(I+1).AND.MAXES(I).EQ.MAXES(I+1).AND.IOP.EQ.IOP
795      .AND.TZ(I).EQ.TZ(I+1).AND.MAT(I).EQ.MAT(I+1)) then
796      KGP1=NN(I+1,7)-NN(I,7)
797      KGP2=NN(I+1,8)-NN(I,8)
798      KGP3=NN(I+1,5)-NN(I,5)
799      KGP4=NN(I+1,6)-NN(I,6)
800      KGP5=NN(I+1,3)-NN(I,3)
801      KGP6=NN(I+1,4)-NN(I,4)
802      KGP7=NN(I+1,1)-NN(I,1)
803      KGP8=NN(I+1,2)-NN(I,2)
804      KGPMX=MAX0(KGP1,KGP2,KGP3,KGP4,KGP5,KGP6,KGP7,KGP8)
805      KGPMN=MIN0(KGP1,KGP2,KGP3,KGP4,KGP5,KGP6,KGP7,KGP8)
806      IF(KGPMX.EQ.KGPMN) KGP=KGPMX
807      end if
808      ISKIP=0
809 cc  ISKIP=1
810      kgz=0

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```

811      if (iskipo.eq.0.and.kgp.gt.0) kgz=kgp
812      il=i
813 c    WRITE(9,* ) 1,ISKIPO,ISKIP,KGM,KGP
814      IF (ISKIP.EQ.0) THEN
815      WRITE(9,13025) I1, IDM(I1), MAXES(I1), IOP, IZ(I1), kgz
816      , MAT(I1), (NN(I1, JX), JX=1, NTYPE)
817      END IF
818 13025 FORMAT(I5, I15, 2I5, F10.0, I5, 10X, I5/16 I5/8 I5)
819      ISKIP=ISKIP
820      KGM=KGM
821      END DO
822 1302  FORMAT(I5, I15, 2I5, F10.0, 15X, I5/8 I5)
823
824      NBMN=0
825      DO J=1, NTBF
826      NBM=99999
827      DO I=1, NTBF
828      IF (NBF(I).LT.NBM.AND.NBF(I).GT.NBMN) THEN
829      NBM=NBF(I)
830      IM=I
831      END IF
832      END DO
833      ID1(J)=IM
834      NBMN=NBM
835      END DO
836
837      DO J=1, NTBF
838      I=ID1(J)
839      WRITE(9,1303) NBF(I), EX(I), FY(I), FZ(I)
840      END DO
841 1303  FORMAT(I5, 4X, '1', 3F10.4)
842
843      WRITE(9,1305)
844 1305  FORMAT(/8X, '1.')
845
846      WRITE (9,1306)
847 1306  FORMAT(1X, '0'//1X, '0'//1X, '0 0 0'//1X, '9999 9999 0')
848      WRITE (10,* )
849      WRITE (10,* ) ' ....For Crack Simulation...'
850      WRITE (10,* ) ' ORIG. NODES  DOUBLE NODES '
851      WRITE (10,* ) ' -----'
852      ntono=nton-ntd-nsd
853      write (9,8128) ntd
854      do 850 i=1, ntono
855      if (mip(i,2).ne.0) then
856      do 860 j=1, nsd
857      if (i.eq.nons(j)) write(10,8126) I, mip(i,1), mip(i,2)
858      if (i.eq.nons(j)) go to 850
859 860  continue
860      write(9,8125) mip(i,1), mip(i,2), I
861      end if
862 850  continue
863 8125  format(2i5, 7h 1 1 1, ' ! ', i5)
864 8126  format(2X, I5, 3X, 2I5)

```

```

865 c      do i=1,ntono
866 c      if(mip(i,2).ne.0) then
867 c--      writing in the order they are given in the input file----
868          do i=1,ntd
869              ii=nond(i)
870              do j=1,3
871                  write(9,8128) mip(ii,1),mip(ii,2),j,11
872              end do
873 8128      format(2i5,i3,5X,'      !      ',15)
874 c          end if
875          end do
876
877          CLOSE (UNIT=9)
878          STOP
879      END
880 c
881 c=====
882      subroutine split (NTON,ntd,idir,nond,mip,x,y,z,I,
883      .ntype,nos,NN,ntoe)
884      dimension nond(1),mip(4000,2),iface(9),jface(9),nos(1),I(1)
885      .,xd(400),yd(400),zd(400),x(1),y(1),z(1),nn(3000,21)
886      NFF=4
887      IF (NTYPE.EQ.21) NFF=9
888 c      STORE COORD. OF THE NODES TO BE DOUBLED
889 c-  CORRECTIONS FOR DOUBLE NODES IN x or y or z -direction--
890
891      DO 819 I=1,4
892          IFACE(I)=I+4
893 819      JFACE(I)=I
894          IFACE(5)=13
895          IFACE(6)=14
896          IFACE(7)=27
897          IFACE(8)=16
898          IFACE(9)=15
899          JFACE(5)=9
900          JFACE(6)=10
901          JFACE(7)=26
902          JFACE(8)=12
903          JFACE(9)=11
904      NIELR=NOS(1)*NOS(2)  !!NIELR--TOTAL NUMBER OF ELEM. IN AN ELE-LAYER
905      IF (IDIR.LT.1.OR.IDIR.GT.3) IDIP=3
906      IF (IDIR=2) 901,902,903
907 901      IFACE(1)=2
908          IFACE(2)=3
909          IFACE(3)=6
910          IFACE(4)=7
911          JFACE(1)=1
912          JFACE(2)=4
913          JFACE(3)=5
914          JFACE(4)=8
915          IFACE(5)=10
916          IFACE(6)=18
917          IFACE(7)=23
918          IFACE(8)=19

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919      IFACE(9)=14
920      JFACE(5)=12
921      JFACE(6)=17
922      JFACE(7)=22
923      JFACE(8)=20
924      JFACE(9)=16
925      NTELR=1
926      GO TO 903
927 902   IFACE(1)=3
928       IFACE(2)=4
929       JFACE(1)=2
930       JFACE(2)=1
931       JFACE(3)=6
932       JFACE(4)=5
933       IFACE(5)=11
934       IFACE(6)=19
935       IFACE(7)=25
936       IFACE(8)=20
937 C     IFACE(9)=15
938 C     JFACE(5)=9
939       JFACE(6)=18
940       JFACE(7)=24
941       JFACE(8)=17
942       JFACE(9)=13
943       NTELR=NOS(1)
944 903   CONTINUE
945
946       DO 701 I=1,NTD-1
947         ICHANGE=0
948       DO 702 J=1,NTD-I
949         JJ=J+1
950         IF (NOND(J).LT.NOND(JJ)) GO TO 702
951         ICHANGE=1
952         AA=NOND(J)
953         NOND(J)=NOND(JJ)
954         NOND(JJ)=AA
955 702    CONTINUE
956         IF (ICHANGE.EQ.0) GO TO 703
957 701    CONTINUE
958
959 703    CONTINUE
960       DO I=1,NTD
961         XD(I)=X(NOND(I))
962         YD(I)=Y(NOND(I))
963         ZD(I)=Z(NOND(I))
964       END DO
965 C-- CORRECTIONS DOUB. NODES. ENDS.--
966
967 C     FIND KND (THE # OF THE NODE CURRENTLY TO BE DOUBLED)
968       DO 830 I=1,NTD
969         DO 820 J=NOND(I),NTON
970           DIF=SQRT((X(J)-XD(I))**2+(Y(J)-YD(I))**2+
971             .(Z(J)-ZD(I))**2)
972 C     IF(X(J).NE.XD(I)) GO TO 820

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973 C      IF(Y(J).NE.YD(I)) GO TO 820
974 C      IF(Z(J).NE.ZD(I)) GO TO 820
975      IF (DIF.GT.1.0E-13) GO TO 820
976      KND=J
977      GO TO 825      ! GET OUT OF J LOOP
978 820 CONTINUE
979 825 CONTINUE
980 C      BEGIN CHANGING NODE NUMBERS & COORD.
981 C      CHANGE NODE NUMBERS
982      DO J=1,NTOE
983      DO K=1,ntype
984      IF (NN(J,K).GT.KND) THEN
985      NN(J,K)=NN(J,K)+1
986      END IF
987      END DO
988      END DO
989
990 C-- substitution FOR DOUB. NODES x or y or z dir.--
991
992      DO K=1,NTOE
993      DO M=1,NFF
994      MM=JFACE(M)
995      IF (NN(K,MM).EQ.KND) THEN
996      MJ=IFACE(M)
997      NN(K+NTELR,MJ)=KND+1
998      END IF
999      END DO
1000     END DO
1001
1002 C-- corrections for doub. nodes ends.--
1003
1004 C      CHANGING COORD.
1005      DO J=NTON,KND,-1
1006      X(J+1)=X(J)
1007      Y(J+1)=Y(J)
1008      Z(J+1)=Z(J)
1009      T(J+1)=T(J)
1010      END DO
1011      NTON=NTON+1
1012 830 CONTINUE
1013      return
1014      end
1015 C=====
1016      subroutine delete(nonx,nony,nonz,nld,nond)
1017      dimension nond(1)
1018      ix=nonz
1019      iyy=nonx*nonz
1020      IFNO=0
1021      DO 422 I=1,NONZ
1022      DO 422 J=1,NONY
1023      DO 422 K=1,NONX
1024      IF (MOD(I,2).EQ.0) GO TO 411
1025      IF (MOD(J,2).NE.0.OR.MOD(K,2).NE.0) GO TO 422
1026 C      I-odd, j,k-even.....

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```

1027      go to 421
1028 411  if(mod((j+k),2).eq.0) go to 422
1029 421  is=ixy*(j-1)+ixx*(k-1)+1
1030      do 425 l=1,ntd
1031      if (is.ne.nond(l)) go to 425
1032      ifno=ifno+1
1033      do 426 m=1+1,ntd
1034 426  nond(m-1)=nond(m)
1035      go to 427
1036 425  continue
1037      go to 422
1038 427  ntd=ntd-1
1039 422  continue
1040 C      type *,ntd
1041 C      type *,(nond(1),i=1,ntd)
1042      return
1043      end
1044
1045 c=====
1046      SUBROUTINE MESHPL (XX,YY,NONX,NONY,NONZ,MIP,IPLOT,ntype)
1047 c-----
1048 c--      Program 'MESH.SOR' to be used with 'PREPRO214.FOR'
1049 c--      This routine plots the mesh in xy-direction..
1050 c--      This routine is based on HP-GL language. The
1051 c--      plot cannot be displayed on ITY. Before using
1052 c--      this routine ASSIGN a HP plotter to FOR090
1053 c--      IPLOT = 1 original nodes
1054 c--      IPLOT = 2 after double nodes
1055 c-----
1056      LOGICAL*1 ETX,ESC
1057      CHARACTER *1 JUNK,MESH
1058      DATA ETX/'3',ESC/'33'/
1059      DIMENSION X(4000),Y(4000),XX(1),YY(1)
1060      .,mip(4000,2)
1061      NTOT=NONX*NONY*NONZ
1062      DO 2 I=1,NTOT
1063      X(I)=XX(I)
1064 2      Y(I)=YY(I)
1065
1066      IX=NONZ
1067      IY=NONX*NONZ
1068
1069 C--      SCALING THE COORDINATES-----
1070
1071      XMIN=5000.
1072      XMAX=-5000.
1073      YMIN=5000.
1074      YMAX=-5000.
1075      DO 50 I=1,NTOT
1076      IF (X(I).GT.XMAX) XMAX=X(I)
1077      IF (X(I).LT.XMIN) XMIN=X(I)
1078      IF (Y(I).GT.YMAX) YMAX=Y(I)
1079 50      IF (Y(I).LT.YMIN) YMIN=Y(I)
1080      WRITE (5,*) 'TYPE LEVEL no. to plotted..'

```

```

1081      READ (5,*) LEVEL
1082      N=LEVEL
1083      XLL=XMIN
1084      YLL=YMIN
1085      XMM=XMAX
1086      YMM=YMAX
1087      XL=XMM-XLL
1088      YL=YMM-YLL
1089      WRITE (5,*) ' DO YOU WANT TO GIVE X,Y LIMITS..? (Y/N)'
1090      READ (5,33) MESH
1091      IF (MESH.EQ.'Y'.OR.MESH.EQ.'y') THEN
1092      WRITE (5,*) ' ENTER X- LIMITS'
1093      READ (5,*) XLL,XMM
1094      WRITE (5,*) ' ENTER Y- LIMITS'
1095      READ (5,*) YLL,YMM
1096      XL=XMM-XLL
1097      YL=YMM-YLL
1098      END IF
1099      XO=650.0
1100      YO=1246.0
1101      Xc= 8800.0/XL
1102      Yc=6000.0/YL
1103      SC=xc
1104      IF (XC*yl.gt.6000.0) SC=yc
1105
1106      IF (XL.LT.YL) THEN
1107      XO=1246.0
1108      YO=650.0
1109      Xc= 6000.0/XL
1110      Yc=8800.0/YL
1111      SC=xc
1112      IF (XC*yl.gt.8800.0) SC=yc
1113      END IF
1114 C      type *, 'x1,y1',x1,y1
1115      DO 1 I=1,NTOT
1116      X(I)=(X(I)-XLL)*SC+XO
1117 1      Y(I)=(Y(I)-YLL)*SC+YO
1118
1119 C      type *,x(1),x(ntot)
1120 C      type *,y(1),y(ntot)
1121 888      WRITE (90,9999) ESC,ESC,ESC,ESC
1122 9999      FORMAT (' ',A1,'.(',/, ' ',A1,'.0:0:',/, ' ',A1,'.140:;17:',/, ' ',
1123      . A1,'.N;19:',/, ' IN;DF;',/)
1124      WRITE (90,*) 'SP1;'
1125      WRITE (90,*) ' VS15.0;'
1126      IF (XL.LT.YL) THEN
1127      WRITE (90,*) ' R090;'
1128      WRITE (90,*) ' IP;IW;'
1129      END IF
1130
1131      XMM=(XMM-XLL)*SC+XO
1132      YMM=(YMM-YLL)*SC+YO
1133      XLL=XO
1134      YLL=YO

```

```

1135
1136      J1=level
1137      J2=(nony-1)*iy+j1
1138 C      DO 111 I=1,NONY
1139 C111      TYPE *,Y(I),YLL
1140 C      CALL LIMIT (Y,YLL,J1,J2,iy,J1)
1141 C      CALL LIMIT (Y,YMM,J1,J2,iy,J2)
1142 C      J2=IF2(Y,YMM,J1,NONY,1)
1143 C      TYPE *,'20 LOOP',J1,J2
1144      DO 20 J=J1,J2,iy
1145      NX1=J
1146      NX2=nx1+IY-IX
1147      CALL LIMIT (X,XLL,NX1,NX2,IX,NX1)
1148      CALL LIMIT (X,XMM,NX1,NX2,IX,NX2)
1149 C      NX1=IF1(X,XLL,NX1,NX2,IX)
1150 C      NX2=IF2(X,XMM,NX1,NX2,IX)
1151 C      TYPE *,'10 LOOP',NX1,NX2
1152 C      TYPE *,X(NX1),Y(NX1),X(NX2),Y(NX2)
1153      DO 10 I=NX1,NX2,IX
1154      IF (i.eq.nx1) WRITE (90,101), X(I),Y(I)
1155      WRITE (90,104), X(I),Y(I)
1156 104      FORMAT (' PD',2F11.3,','')
1157 10      continue
1158 20      WRITE (90,*) ' PU;'
1159
1160      I1=level
1161      I2=(NONX-1)*ix+i1
1162 C      CALL LIMIT (X,XLL,i1,i2,IX,i1)
1163 C      CALL LIMIT (X,XMM,i1,i2,IX,i2)
1164 C      I1=IF1(X,XLL,1,NONX,1)
1165 C      I2=IF2(X,XMM,I1,NONX,1)
1166 C      TYPE *,'30 LOOP',I1,I2
1167      DO 30 I=I1,I2,IX
1168      NY1=i
1169      NY2=NY1+(nony-1)*IY
1170 C      CALL LIMIT (Y,YLL,NY1,NY2,iy,NY1)
1171 C      CALL LIMIT (Y,YMM,NY1,NY2,iy,NY2)
1172 C      NY1=IF1(Y,YLL,NY1,NY2,iy)
1173 C      NY2=IF2(Y,YMM,NY1,NY2,iy)
1174 C      TYPE *,'40 LOOP',NY1,NY2
1175      DO 40 J=NY1,NY2,iy
1176      IF (J.EQ.NY1) WRITE (90,101), X(J),Y(J)
1177      WRITE (90,102), X(J),Y(J)
1178 40      CONTINUE
1179 101      FORMAT (' PU;PA',2F11.3,','')
1180 102      FORMAT (' PD',2F11.3,','')
1181 30      WRITE (90,*) ' PU;'
1182
1183      WRITE (5,*) 'DO YOU NEED NODE NOS. (Y/N).. '
1184      READ (5,33) JUNK
1185 33      FORMAT (A)
1186      IF (JUNK.EQ.'Y'.OR.JUNK.EQ.'y') GO TO 44
1187      GO TO 66
1188 !      WRITE (5,*) '..ENTER SSX,SSY...'

```

```

1189 !      READ (5,*) SSX,SSY
1190 44      SSX=0.1
1191      SSY=0.15
1192 !--    WRITING THE NODE NUMBERS-----
1193      WRITE (90,*) 'SIO.1,0.15;'
1194      J1=level
1195      J2=(nony-1)*iy+j1
1196 C      CALL LIMIT (Y,YLL,j1,j2,iy,J1)
1197 C      CALL LIMIT (Y,YMM,J1,j2,iy,J2)
1198      DO 100 J=J1,J2,IY
1199      NX1=j
1200      NX2=nx1+IY-ix
1201 C      CALL LIMIT (X,XLL,NX1,NX2,IX,NX1)
1202 C      CALL LIMIT (X,XMM,NX1,NX2,IX,NX2)
1203
1204 !      TYPE *,(KK, KK=NX1,NX2,IX)
1205      DO 100 IN=NX1,NX2,IX
1206      NXX=(IN-NX1)/IX+1      !skipping the face nos.
1207      IF (MOD(J,2).EQ.0.AND.MOD(NXX,2).EQ.0.and.
1208      . ntype.eq.21)GO TO 100
1209      I=IN
1210      IF (IPLOT.EQ.2) I=MIP(IN,1)
1211      SPL=-0.8
1212      XXX=X(IN)
1213      YYY=Y(IN)
1214      IF (XXX.LT.XLL.OR.YYY.LT.YLL) GO TO 100
1215      IF (XXX.GT.XMM.OR.YYY.GT.YMM) GO TO 100
1216      CALL SYMB (XXX,YYY,SPL,I)
1217      IF (IPLOT.EQ.1) GO TO 100
1218      I=MIP(IN,2)
1219      SPL=0.2
1220      IF (I.NE.0) CALL SYMB (XXX,YYY,SPL,I)
1221 100      WRITE (90,*) ' FU;'
1222 66      WRITE (90,*) 'SP0;'
1223      WRITE (5,*) 'TYPE another LEVEL no. to plotted..'
1224      READ (5,*) LEVEL
1225      if (level.ne.0) go to 888
1226      RETURN
1227      END
1228
1229      SUBROUTINE SYMB (X,Y,SPL,I)
1230      LOGICAL*1 ETX,ESC
1231      DATA ETX/'3/',ESC/'33/'
1232      NC=1
1233      IF (I.GT.9) NC=2
1234      IF (I.GT.99) NC=3
1235      IF (I.GT.999) NC=4
1236      IF (I.GT.9999) NC=5
1237      IF (I.GT.99999) NC=6
1238      SPC=-(0.33+0.5*(NC-1))+2.
1239      WRITE (90,101), X,Y
1240      IF (NC.EQ.1) WRITE (90,201) SPC,SPL,I,ETX
1241      IF (NC.EQ.2) WRITE (90,202) SPC,SPL,I,ETX
1242      IF (NC.EQ.3) WRITE (90,203) SPC,SPL,I,ETX

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```

1243      IF (NC.EQ.4) WRITE (90,204) SPC,SPL,I,ETX
1244      IF (NC.EQ.5) WRITE (90,205) SPC,SPL,I,ETX
1245      IF (NC.EQ.6) WRITE (90,206) SPC,SPL,I,ETX
1246 101   FORMAT (' PU;PA',2F11.3,';')
1247 201   FORMAT (' CP',2F6.2,';LB',I1,A2)
1248 202   FORMAT (' CP',2F6.2,';LB',I2,A2)
1249 203   FORMAT (' CP',2F6.2,';LB',I3,A2)
1250 204   FORMAT (' CP',2F6.2,';LB',I4,A2)
1251 205   FORMAT (' CP',2F6.2,';LB',I5,A2)
1252 206   FORMAT (' CP',2F6.2,';LB',I6,A2)
1253      RETURN
1254      END
1255
1256      SUBROUTINE LIMIT (A,AL,I1,I2,IN,IF1)
1257 c--    to find the lower limit of do loop...
1258      DIMENSION A(1)
1259      DO 10 I=I1,I2,IN
1260 C      TYPE A,A(I),AL
1261      IF (A(I).GE.(AL-1.0E-03)) THEN
1262      IF1=I
1263      RETURN
1264      END IF
1265 10     CONTINUE
1266      END
1267
1268      INTEGER FUNCTION IF2(A,AL,I1,I2,IN)
1269 c--    to find the lower limit of do loop...
1270      DIMENSION A(1)
1271      IF2=I2
1272      DO 10 I=I1,I2,IN
1273 C      type A,a(i),al
1274      IF (A(I).GE.(AL-1.0E-03)) THEN
1275      IF1=I
1276      RETURN
1277      END IF
1278 10     CONTINUE
1279      END
1280
1281 c&=====
1282      SUBROUTINE HOLE(RX,NONX,NONY,NONZ,XX,YY,ZZ,X,Y,Z)
1283 C      GENERATING POLAR COORDINATES AND POLAR MESH--
1284      DIMENSION XX(1),YY(1),ZZ(1),X(1),Y(1),R(100),NC(4)
1285      . ,Z(1)
1286 c      TYPE A,(XX(I),I=1,NONX)
1287 c      TYPE A,(YY(I),I=1,NONY)
1288 c      TYPE A,(ZZ(I),I=1,NONZ)
1289      pi=3.14159265
1290      EPS=1.0E-05
1291
1292      IX=NONZ
1293      IY=NONX*NONZ
1294      DO 31 I=1,NONX
1295      rad=rx
1296      if (xx(i).ne.0.0)rad=sqrt(xx(i)**2+rx**2)

```

```

1297      DO 31 J=1,NONY
1298
1299      xxx=xx(i)
1300      if (yy(j).le.(rx-eps)) then
1301      xxx=rad
1302      if (yy(j).ne.0.)xxx=sqrt(rad**2-yy(j)**2)
1303      nbk=(j-1)*iy+(i-2)*ix+1
1304      TYPE 4,NBK
1305      if ((xxx-x(nbk)).lt.(xx(2)-xx(1)))xxx=x(nbk)+xx(2)-xx(1)
1306      end if
1307
1308      nxy=(J-1)*IY+(I-1)*IX
1309      DO 31 K=1,NONZ
1310      N=nxy+K
1311      X(N)=xxx
1312      IF (I.EQ.NONX) X(N)=XX(NONX)
1313      Y(N)=YY(J)
1314      Z(N)=ZZ(K)
1315 31      continue
1316 C--      correcting for center nodes for 31 node element....
1317
1318      DO 20 K=1,NONZ
1319      DO 20 J=1,NONY
1320      DO 20 I=1,NONX
1321      IF (MOD(I,2).NE.0) GO TO 20
1322      N=(J-1)*IY+(I-1)*IX+K
1323      X(N)=(X(N-IX)+X(N+IX))/2.0
1324 20      CONTINUE
1325      do 30 j=1,nony
1326      do 30 i=1,nonx
1327      n=(j-1)*iy+(i-1)*ix+1
1328      type 4,n,x(n),y(n)
1329 30      CONTINUE
1330      RETURN
1331      END

```

APPENDIX - B
LISTING OF THE MAIN CODE 'KSAP II'

```

1 C  **  **  **  **  **  **  **  **  **  **  **  **  **  **  **  **  **
2 C
3 C          KSAP 11
4 C      SIMPLIFIED VERSION OF SAP4 FOR
5 C      USING ELEMENT TYPE 8 ONLY
6 C
7 C          September 1987
8 C
9 C
10 C
11 C  **  **  **  **  **  **  **  **  **  **  **  **  **  **  **  **  **
12 C
13      IMPLICIT REAL*8(A-H,O-Z)
14      REAL*4 T,IT
15      COMMON /JUNK/ HED(12),JUK(406)
16      COMMON /ELPAR/ NPAR(14),NUMNP,MBAND,NELTYP,N1,N2,N3,N4,N5,MTOT,NEQ
17      COMMON /EM/   QQQ(2846)
18      COMMON /DYN/   IDUS(11),NDYN
19      COMMON /TAPES/ NQQ(6)
20      COMMON /EXTRA/ MODEX,NT8,N10SV,NT10,KEQB,NUMEL,T(10)
21      COMMON /SOL/   NBLOCK,NEQB,LL,NE,IDUM,NEIG,NAD,NVV,ANORM,NEQ
22 C
23 C      PROGRAM CAPACITY CONTROLLED BY THE FOLLOWING TWO STATEMENTS ...
24 C
25      COMMON A(650001)      !CHANGE MTOT ALSO
26 C
27 C  -- OPEN SCRATCH FILES
28 C      OPEN (UNIT=1,STATUS='UNKNOWN',FORM='UNFORMATTED')
29 C  -----
30      open(unit=1,FILE='SCR:IASW.EMAN11',status='scratch',
31      .form='unformatted')
32 !      open(unit=2,status='scratch',form='unformatted')      !dmsa0:
33      open (unit=2,file='SCR:IASW.EMAN11',status='new',blocksize=4800,
34      .form='unformatted')      !dmsa0:
35 !      open(unit=3,status='scratch',form='unformatted')      !dmsa0:
36      open (unit=3,file='UTL:IASW.EMAN11',status='new',blocksize=4800,
37      .form='unformatted')      !dmsa0:
38 !      open (unit=3,file='MSA0:[ESK]',status='new',blocksize=4800,
39 !      .form='unformatted')      !dmsa0:
40      OPEN (UNIT=4,FILE='SCR:IASW.EMAN11',STATUS='NEW',BLOCKSIZE=4800,
41      .FORM='UNFORMATTED')      !dmsa0:
42 !      open(unit=55,status='UNKNOWN',form='unformatted')      !dmsa0:
43      OPEN (UNIT=55,FILE='SCR:IASW.EMAN11',STATUS='NEW',BLOCKSIZE=4800,
44      .FORM='UNFORMATTED')      !DMA0:
45      open(unit=8,FILE='SCR:IASW.EMAN11',status='scratch',
46      .FORM='UNFORMATTED')      !dmsa0:
47      open(unit=9,FILE='SCR:IASW.EMAN11',status='scratch',
48      .FORM='UNFORMATTED')      !dmsa0:
49 C      OPEN (UNIT=15,STATUS='SCRATCH',FORM='UNFORMATTED')      !DMA0:
50      open(unit=16,FILE='SCR:IASW.EMAN11',status='scratch',
51      .FORM='UNFORMATTED')      !dmsa0:
52      open(unit=18,FILE='SCR:IASW.EMAN11',status='scratch',
53      .FORM='UNFORMATTED')      !dmsa0:
54      open (unit=19,file='workdone.wok',status='new')      !dmsa0:

```

```

55      open (unit=33,file='SCR:LASW.EMANIIDISP.dat',status='new')
56      open (unit=34,file='ksapout.dat',status='new')
57      OPEN (UNIT=15,FILE='SCR:LASW.EMANIID',STATUS='NEW',BLOCKSIZE=4800,
58      .FORM='UNFORMATTED')
59
60 *-----
61
62 !      THE following should be 1 less than A() dimension
63      MTOT= 650000          ! 300000
64
65 C      USE THE IBM FORTRAN EXTENDED ERROR HANDLING FACILITY TO
66 C      ELIMINATE PRINTOUT OF UNDERFLOW ERROR MESSAGE (ERROR NUMBER 208)
67 C
68 C      CALL ERRSET (208,256,-1,1)
69 C
70 C
71      CALL STIME
72 C
73      NT8 = 8
74      REWIND NT8
75      NT10= 10
76      REWIND NT10
77      N1=1
78 C
79 C      P R O G R A M   C O N T R O L   D A T A
80 C
81      5 CALL TTIME(T(1))
82      READ (5,100,END=990) HED,NUMNP,NELTYP,LL,NF,NDYN,MODEX,NAD,
83      1      KEQB,N10SV,NDOF
84      IF(MODEX.GT.0) MODEX = 1
85      IF (NUMNP.EQ.0) STOP
86      WRITE (33,200) HED,NUMNP,NELTYP,LL,NF,NDYN,MODEX,NAD,KEQB,N10SV
87      WRITE (19,299) HED !WORKDONE.WOK FILE TILTE.....
88      WRITE (34,299) HED !KSAPOUT.DAT (STRESSES) FILE TILTE.....
89      IF(KEQB.LT.2) KEQB = 99999
90      IF (NDYN.NE.0) LL=1
91      IF(LL.GE.1) GO TO 10
92      WRITE (33,300)
93      STOP
94 C*** DATA PORTHOLE SAVE
95      10 IF(MODEX.EQ.1)
96      *WRITE (NT8)      HED,NUMNP,NELTYP,LL,NF,NDYN
97 C
98      KDYN = IABS(NDYN) +1
99      IF(KDYN.LE.5) GO TO 14
100     WRITE (33,310) NDYN
101     STOP
102 C
103 C      RE-START MODE ACTIVATED IF NDYN.EQ.-2 OR NDYN.EQ.-3
104 C
105     14 IF(NDYN.LT.0) GO TO 20
106 C
107 C      I N P U T   J O I N T   D A T A
108 C

```

```

109      N2=N1+6*NUMNP
110      N3=N2+NUMNP
111      N4=N3+NUMNP
112      N5=N4+NUMNP
113      N6=N5+NUMNP
114      IF(N6.GT.MTOT) CALL ERROR(N6-MTOT)
115 C
116      CALL INPUTJ(A(N1),A(N2),A(N3),A(N4),A(N5).NUMNP,NEQ)
117 C
118 C      F O R M      E L E M E N T      S T I F F N E S S E S
119 C
120      CALL TTIME(T(2))
121 C
122      MBAND=0
123      NUMEL=0
124      REWIND 1
125      REWIND 2
126 C
127      DO 900 M=1,NELTYP
128      READ  (5,1001) NPAR
129 C***  DATA PORTHOLE SAVE
130      IF(MODEX.EQ.1) WRITE (NT8) NPAR
131      WRITE (1) NPAR
132      NUMEL=NUMEL+NPAR(2)
133      MTYPE=NPAR(1)
134 C
135      CALL ELTYPE(MTYPE)
136 C
137      900 CONTINUE
138 C
139 C      D E T E R M I N E      B L O C K S I Z E
140 C
141 C      ADDSTF
142 C
143      LL1=LL+NDOF      ! IN the following LL is replaced with LL1
144      NEQB=(MTOT - 4*LL)/(MBAND + LL1 + 1)/2      !modified with ndof
145 C
146 C      OVER-RIDE THE SYSTEM MATRIX BLOCKSIZE WITH THE INPUT (NON-ZERO)
147 C      VALUE, KEQB.
148 C      THIS OVER-RIDE ENTRY IS TO ALLOW PROGRAM CHECKING OF MULTI-
149 C      BLOCK ALGORITHMS WITH WHAT WOULD NORMALLY BE ONE BLOCK DATA.
150 C
151      IF(KEQB.LT.NEQB) NEQB = KEQB
152 C
153      GO TO  (690,700,700,700,730), KDYN
154 C
155 C      STATIC SOLUTION
156 C
157      690 CONTINUE
158      NEQB1=(MTOT - MBAND)/(2*(MBAND+LL1) + 1)
159      NEQB2=(MTOT - MBAND - LL1*(MBAND-2))/(3*LL1 + MBAND + 1)
160      IF (NEQB1.LT.NEQB) NEQB=NEQB1
161      IF (NEQB2.LT.NEQB) NEQB=NEQB2
162      NBLOCK = (NEQ-1)/NEQB + 1

```

```

163      IF(NEQB.GT.NEQ) NEQB=NEQ
164      GO TO 790
165 C
166 C      EIGENSOLUTION
167 C
168 C          1. DETERMINANT SEARCH ALGORITHM
169 C
170      700 IF (NEQB.LT.NEQ) GO TO 710
171          NIM=3
172          NC=NE + NIM
173          NVN=6
174          NCA=NEQ*MAX0(MBAND,NC)
175          NTOT=NCA + 4*NEQ + 3*NVN*NEQ + 5*NC
176          NEIG=0
177          IF(NTOT.LE.NTOT) GO TO 720
178 C
179 C          2. SUBSPACE ITERATION ALGORITHM
180 C
181      710 NV=MIN0(2*NE,NE+6)
182          IF (NAD.NE.0) NV=NAD
183          NEQB1=(NTOT - MBAND)/(2*MBAND + 1)
184          NEQB2=(NTOT - MBAND - 2*NV - NV*(MBAND-2))/(3*NV + MBAND + 1)
185          NEQB3=(NTOT - 3*NV*NV - 3*NV)/(2*NV + 1)
186          NEQB4=(NTOT - 6*NV)/(1 + MBAND)
187          IF (NEQB1.LT.NEQB) NEQB=NEQB1
188          IF (NEQB2.LT.NEQB) NEQB=NEQB2
189          IF (NEQB3.LT.NEQB) NEQB=NEQB3
190          IF (NEQB4.LT.NEQB) NEQB=NEQB4
191          NEIG=1
192 C
193      720 CONTINUE
194          NBLOCK = (NEQ-1)/NEQB + 1
195          IF (NEQB.GE.NEQ) NEQB=NEQ
196 C
197 C      HISTORY OR SPECTRUM ANALYSIS
198 C
199          KREM = 1000
200          NTOT = NBLOCK*NEQB*NE + KREM
201          IF(NTOT.LT.NTOT)
202      *WRITE (33,320)
203          GO TO 790
204 C
205 C      STEP-BY-STEP DIRECT INTEGRATION
206 C
207      730 CONTINUE
208 C      DISPLACEMENT COMPONENTS FOR DIRECT OUTPUT (*NSD*)
209          NN2 = NEQ
210 C      DISPLACEMENT COMPONENTS REQUIRED FOR RECOVERY OF ALL OF THE
211 C      REQUESTED ELEMENT STRESS COMPONENTS (*NSS*)
212          NN3 = NEQ
213 C
214 C          1. DECOMPOSITION
215 C
216          NEQB1 = (NTOT-NN2-NN3-NEQ-MBAND)/(2*MBAND+1)

```

```

217 C
218 C      2. TIME INTEGRATION PHASE
219 C
220      mcal= MBAND+2*(NN2+NN3)+5*NEQ +(2*MBAND+1)
221
222
223      write (33,555) mcal
224 555      format(/5x,' Minimum dimension, MTOT, required for array A( )
225      = ',18/5X,50(1H+))//)
226      if(mtot.le.mcal) STOP      ! Abnormal stop as dim. of A is insufficient
227
228      NEQB2 = (MTOT-MBAND-2*(NN2+NN3)-5*NEQ)/(MBAND+1)
229 C
230      IF(NEQB1.LT.NEQB) NEQB = NEQB1
231      IF(NEQB2.LT.NEQB) NEQB = NEQB2
232      IF(NEQB.GT.NEQ) NEQB = NEQ
233      NBLOCK = (NEQ-1)/NEQB +1
234 C
235 C      3. INPUT PHASE
236 C
237 C      NUMBER OF TIME FUNCTIONS (*NEN*)
238      NN2 = 10
239 C      MAXIMUM NUMBER OF FUNCTION DEFINITION POINTS (*MXLP*)
240      NN3 = 40
241 C
242      NN4 = 6*NUMNP + 2*NN2*NEQ
243      IF(NN4.GT.MTOT)
244      *WRITE (33,320)
245      NN4 = NEQ*2*(NN2+1) + NN2*(1+2*NN3)
246      IF(NN4.GT.MTOT)
247      *WRITE (33,320)
248 C
249      790 CONTINUE
250 C
251 C      INPUT NODAL LOADS
252 C
253      N3=N2+NEQB*ALL
254      N4=N3+6*LL
255      WRITE (33,201) NEQ,MBAND,NEQB,NBLOCK
256 C
257      CALL TTIME(T(3))
258 C
259      CALL INL(A(N1),A(N2),A(N3),A(N4),NUMNP,NEQB,LL)
260 C
261      CALL TTIME(T(4))
262 C
263 C      FORM TOTAL STIFFNESS
264 C
265      NE2B=2*NEQB
266      N2=N1+NEQB*MBAND
267      N3=N2+NEQB*ALL
268      N4=N3+4*LL
269      NN2=N1+NE2B*MBAND
270      NN3=NN2+NE2B*ALL

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271      NN4=NN3+4ALL
272 C
273      CALL ADDSTF (A(N1),A(NN2),A(NN3),A(NN4),NUMEL,NBLOCK,NE2B,LL,MBAND
274      1,ANORM,NUV)
275 C
276      CALL TTIME(T(5))
277 C
278 C      S O L U T I O N   P H A S E
279 C
280      20 GO TO (30,40,50,60,70), KDYN
281 C
282 C      STATIC SOLUTION
283 C
284      30 IF(MODEX.EQ.0) GO TO 32
285      DO 31 I=6,10
286      31 T(I) = T(5)
287      GO TO 90
288 C
289      32 CALL SOLEQ
290      CALL TTIME(T(6))
291      DO 33 I=7,10
292      33 T(I) = T(6)
293      GO TO 90
294 C
295 C      EIGENVALUE EXTRACTION
296 C
297      40 T(6) = T(5)
298 C      CALL SOLEIG
299      CALL TTIME(T(7))
300      T(8) = T(7)
301      T(9) = T(7)
302      T(10)= T(7)
303      GO TO 90
304 C
305 C      FORCED DYNAMIC RESPONSE ANALYSIS
306 C
307      50 T(6) = T(5)
308      IF(NDYN.LT.0) GO TO 52
309 C      CALL SOLEIG
310      CALL TTIME (T(7))
311      GO TO 54
312      52 DO 53 I=1,6
313      53 T(I+1)=T(I)
314      REWIND 2
315      READ (2) NEQ,NBLOCK,NEQB,MBAND,N1,NE,(QQQ(I),I=1,NE)
316      REWIND 55
317      IMAX=NEQBANE
318      READ (55) (A(I),I=1,NE)
319      DO 56 L=1,NBLOCK
320      56 READ (55) (A(I),I=1,IMAX)
321      54 CONTINUE
322 C      54 CALL HISTRY
323      CALL TTIME (T(8))
324      T(9) = T(8)

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```

325      T(10)= T(8)
326      GO TO 90
327 C
328 C      RESPONSE SPECTRUM ANALYSIS
329 C
330      60 T(6) = T(5)
331      IF(NDYN.LT.0) GO TO 62
332 C      CALL SOLEIG
333      CALL TTIME (T(7))
334      T(8) = T(7)
335      GO TO 64
336      62 DO 63 I=1,7
337      63 T(I+1)=T(I)
338      REWIND 2
339      READ (2) NEQ,NBLOCK,NEQB,MBAND,N1,NF
340      REWIND 55
341      IMAX=NEQB*NF
342      READ (55) (A(I),I=1,NF)
343      DO 66 L=1,NBLOCK
344      66 READ (55) (A(I),I=1,IMAX)
345      64 CONTINUE
346 C      64 CALL RESPEC
347      CALL TTIME (T(9))
348      T(10)= T(9)
349      GO TO 90
350 C
351 C      STEP-BY-STEP (DIRECT INTEGRATION) ANALYSIS
352 C
353      70 DO 71 I=6,9
354      71 T(I) = T(5)
355 C      CALL STEP
356      CALL TTIME(T(10))
357 C
358 C      COMPUTE AND PRINT OVERALL TIME LOG
359 C
360      90 TT = 0.0
361      DO 95 I=1,9
362      T(I) = T(I+1)-T(I)
363      TT = TT + T(I)
364      95 CONTINUE
365 C
366      WRITE (33,203) (T(K),K=1,9),TT
367 C
368      GO TO 5
369      990 CONTINUE
370 C -- CLOSE ALL SCRATCH FILES
371      CLOSE (UNIT=1)
372      CLOSE (UNIT=2)
373      CLOSE (UNIT=3)
374      CLOSE (UNIT=4)
375      CLOSE (UNIT=55)
376      CLOSE (UNIT=8)
377      CLOSE (UNIT=9)
378      close (unit=15)

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379      close (unit=16)
380      close (unit=18)
381      close (unit=19)
382 C
383      STOP
384 C
385      100 FORMAT (12A6/10I5)
386 299      FORMAT(/2X,12A6/)
387 200      FORMAT(1H1,12A6///
388      1 38H C O N T R O L   I N F O R M A T I O N, // 4X,
389      2 27H NUMBER OF NODAL POINTS   =, 15 / 4X,
390      3 27H NUMBER OF ELEMENT TYPES  =, 15 / 4X,
391      4 27H NUMBER OF LOAD CASES     =, 15 / 4X,
392      5 27H NUMBER OF FREQUENCIES    =, 15 / 4X,
393      6 27H ANALYSIS CODE (NDYN)     =, 15 / 4X,
394      7 16H   EQ.0,  STATIC,          / 4X,
395      8 26H   EQ.1,  MODAL EXTRACTION, / 4X,
396      9 25H   EQ.2,  FORCED RESPONSE,  / 4X,
397      A 27H   EQ.3,  RESPONSE SPECTRUM, / 4X,
398      * 28H   EQ.4,  DIRECT INTEGRATION, / 4X,
399      B 27H SOLUTION MODE (MODEX)     =, 15 / 4X,
400      C 19H   EQ.0,  EXECUTION,        / 4X,
401      D 20H   EQ.1,  DATA CHECK,      / 4X,
402      E 19H NUMBER OF SUBSPACE,        / 4X,
403      F 27H ITERATION VECTORS (NAD)   =, 15 / 4X,
404      G 27H EQUATIONS PER BLOCK       =, 15 / 4X,
405      H 27H TAPE10 SAVE FLAG (N10SV) =, 15 / 4X)
406 201 FORMAT (38H1E Q U A T I O N   P A R A M E T E R S, //
407      *      34H TOTAL NUMBER OF EQUATIONS   =,15,
408      1      /34H BANDWIDTH                  =,15,
409      2      /34H NUMBER OF EQUATIONS IN A BLOCK =,15,
410      3      /34H NUMBER OF BLOCKS            =,15)
411 203 FORMAT (1H1,31H O V E R A L L   T I M E   L O G, //
412      1 5X,30H NODAL POINT INPUT           =, F8.2 /
413      2 5X,30H ELEMENT STIFFNESS FORMATION  =, F8.2 /
414      3 5X,30H NODAL LOAD INPUT             =, F8.2 /
415      4 5X,30H TOTAL STIFFNESS FORMATION     =, F8.2 /
416      5 5X,30H STATIC ANALYSIS              =, F8.2 /
417      6 5X,30H EIGENVALUE EXTRACTION        =, F8.2 /
418      7 5X,30H FORCED RESPONSE ANALYSIS     =, F8.2 /
419      8 5X,30H RESPONSE SPECTRUM ANALYSIS   =, F8.2 /
420      * 5X,30H STEP-BY-STEP INTEGRATION     =, F8.2 //
421      9 5X,30H TOTAL SOLUTION TIME          =, F8.2 /)
422 C
423 300 FORMAT (// 48H ** ERROR. (AT LEAST ONE LOAD CASE IS REQUIRED) )
424 310 FORMAT (// 33H ** ERROR. ANALYSIS CODE (NDYN =,13,9H) IS BAD. )
425 320 FORMAT (// 47H ** WARNING. ESTIMATE OF STORAGE FOR A DYNAMIC,
426      1      32H ANALYSIS EXCEEDS AVAILABLE CORE, // 1X)
427 C
428 1001 FORMAT (14I5)
429      END
430 C=====
431      SUBROUTINE ADDSTF (A,B,STR,THASS,NUMEL,NBLOCK,NE2B,LL,MBAND,ANORM,
432      INVV)

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433 C
434     IMPLICIT REAL*8(A-H,O-Z)
435 C
436 C     CALLED BY:  MAIN
437 C
438 C     FORMS GLOBAL EQUILIBRIUM EQUATIONS IN BLOCKS
439 C
440     DIMENSION A(NE2B,MBAND),B(NE2B,LL),STR(4,LL),TMASS(NE2B)
441 C
442     COMMON /DYN/  NT,NOT,ALFA,DT,BETA,NFN,NGM,NAT,NDYN
443     COMMON /EM/  LRD,ND,LR(63),IPAD,SS(2331)
444     COMMON /EXTRA/  MDEX,NTS,IFILL(14)
445 C
446     NEQB=NE2B/2
447     K=NEQB+1
448     X=NBLOCK
449     MB=DSQRT(X)
450     MB=MB/2+1
451     NEBB=MB*NE2B
452     MM=1
453     NDEG=0
454     NVV=0
455     ANORM=0.
456     NSHIFT=0
457     KEWIND 3
458     KEWIND 4
459     KEWIND 9
460 C
461 C     READ ELEMENT LOAD MULTIPLIERS
462 C
463     WRITE (33,2000)
464     DO 50 L=1,LL
465     READ  (5,1002)  (STR(I,L),I=1,4)
466 50 WRITE (33,2002) L,(STR(I,L),I=1,4)
467     IF(MDEX.EQ.0) WRITE (8) STR
468 C
469 C     FOR A STEP-BY-STEP ANALYSIS (NDYN.EQ.4) READ THE SOLUTION
470 C     CONTROL CARD.  THE TIME STEP (DT) AND THE DAMPING COEFFICIENTS
471 C     (ALFA/BETA) ARE REQUIRED FOR THE ASSEMBLY OF THE EFFECTIVE
472 C     SYSTEM STIFFNESS MATRIX IN THIS ROUTINE.
473 C
474     IF(NDYN.NE.4) GO TO 65
475 C
476     READ  (5,1004)  NFN,NGM,NAT,NT,NOT,DT,ALFA,BETA
477     WRITE (33,2004)  NFN,NGM,NAT,NT,NOT,DT,ALFA,BETA
478     IF(NAT.EQ.0) NAT = 1
479     IF(NOT.EQ.0) NOT = 1
480     IF(DT.GT.1.0E-12) GO TO 55
481     WRITE (33,3000)
482     STOP
483 C
484 C     COMPUTE INTEGRATION COEFFICIENTS FOR ASSEMBLY OF EFFECTIVE
485 C     SYSTEM STIFFNESS (STEP-BY-STEP ANALYSIS ONLY)
486 C

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487 55 TETA = 1.4
488 DT1 = TETA*DT
489 DT2 = DT1**2
490 A0 = (6.+3.*ALEFA*DT1)/(DT2+3.*BETA*DT1)
491 C
492 65 IF(MODEX.EQ.1) RETURN
493 C
494 C FORM EQUATIONS IN BLOCKS ( 2 BLOCKS AT A TIME)
495 C
496 DO 1000 M=1,NBLOCK ,2
497 DO 100 I=1,NE2B
498 DO 100 J=1,MBAND
499 100 A(I,J)=0.
500 READ (3) ((B(I,L),I=1,NEQB),L=1,LL),(TMASS(I),I=1,NEQB)
501 IF (M.EQ.NBLOCK) GO TO 200
502 READ (3) ((B(I,L),I=K,NE2B),L=1,LL),(TMASS(I),I=K,NE2B)
503 200 CONTINUE
504 C
505 REWIND 55
506 REWIND 2
507 NA=55
508 NUME=NUM7
509 IF (MM.NE.1) GO TO 75
510 NA=2
511 NUME=NUMEL
512 NUM7 =0
513 C
514 75 DO 700 N=1,NUME
515 READ (NA) LRD,ND, (LM(I),I=1,ND), (SS(I),I=1,LRD)
516 MSHFT = ND * (ND+1)/2 + 4 *ND
517 DO 600 I=1,ND
518 LMN=1-LM(I)
519 II=LM(I)-NSHIFT
520 IF (II.LE.0.OR.II.GT.NE2B) GO TO 600
521 IMS=I+MSHFT
522 TMASS(II)=TMASS(II)+ SS(IMS)
523 DO 300 L=1,LL
524 DO 300 J=1,4
525 KK = ND *(ND+1)/2 + ND*(J-1)
526 300 B(II,L)=B(II,L)+SS(I+KK)*STR(J,L)
527 DO 500 J=1,ND
528 JJ=LM(J)+LMN
529 IF(JJ) 500,500,390
530 390 IF(J-I) 396,394,394
531 394 KK = ND*I-(I-1)*I/2 + J-ND
532 GO TO 400
533 396 KK =ND*IJ -(J-1)*J/2+I-ND
534 400 A(II,JJ)=A(II,JJ)+SS( KK)
535 500 CONTINUE
536 600 CONTINUE
537 C
538 C DETERMINE IF STIFFNESS IS TO BE PLACED ON TAPE 55
539 C
540 IF (MM.GT.1) GO TO 700

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```

541      DO 650 I=1,ND
542      II=LM(I) -NSHIFT
543      IF(II.GT.NE2B.AND.II.LE.NE8B) GO TO 660
544 650 CONTINUE
545      GO TO 700
546 660 WRITE (55) LRD,ND,(LM(I),I=1,ND),(SS(I),I=1,LRD)
547      NUM7=NUM7+1
548 C
549 700 CONTINUE
550      DO 710 L=1,NEQB
551      ANORM=ANORM + A(L,1)
552      IF (A(L,1).NE.0.) NDEG=NDEG + 1
553      IF (A(L,1).EQ.0.) A(L,1)=1.E+20
554      IF (TMASS(L).NE.0.) NVV=NVV + 1
555 710 CONTINUE
556 C
557 C      FOR STEP-BY-STEP ANALYSIS ADD THE MASS CONTRIBUTIONS TO
558 C      THE EQUATION DIAGONAL COEFFICIENTS
559 C
560      IF(NDYN.NE.4) GO TO 716
561      DO 714 I=1,NEQB
562 714 A(I,1) = A(I,1) + A0* TMASS(I)
563      WRITE (4) ((A(I,J),I=1,NEQB),J=1,MBAND)
564      GO TO 718
565 716 WRITE (4) ((A(I,J),I=1,NEQB),J=1,MBAND),((B(I,L),I=1,NEQB),L=1,LL)
566 718 WRITE (9) (TMASS(I),I=1,NEQB)
567 C
568      IF(M.EQ.NBLOCK) GO TO 1000
569      DO 720 L=K,NE2B
570      ANORM=ANORM + A(L,1)
571      IF (A(L,1).NE.0.) NDEG=NDEG + 1
572      IF (A(L,1).EQ.0.) A(L,1)=1.E+20
573      IF (TMASS(L).NE.0.) NVV=NVV + 1
574 720 CONTINUE
575 C
576      IF(NDYN.NE.4) GO TO 726
577      DO 724 I=K,NE2B
578 724 A(I,1) = A(I,1) + A0* TMASS(I)
579      WRITE (4) ((A(I,J),I=K,NE2B),J=1,MBAND)
580      GO TO 728
581 726 WRITE (4) ((A(I,J),I=K,NE2B),J=1,MBAND),((B(I,L),I=K,NE2B),L=1,LL)
582 728 WRITE (9) (TMASS(I),I=K,NE2B)
583 C
584      IF (MM.EQ.MB) MM=0
585      MM=MM+1
586 1000 NSHIFT=NSHIFT+NE2B
587      IF (NDEG.GT.0) GO TO 730
588      WRITE (33,1010)
589      STOP
590 730 ANORM=(ANORM/NDEG)*1.E-8
591 C
592      RETURN
593 1002 FORMAT (4F10.0)
594 1004 FORMAT (5I5,3F10.0)

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595 1010 FORMAT (51H0STRUCTURE WITH NO DEGREES OF FREEDOM CHECK DATA )
596 2000 FORMAT (/// 10H STRUCTURE,13X,7HELEMENT,4X,4HLOAD,4X,
597 1 11HMULTIPLIERS,/ 10H LOAD CASE,12X,1HA,9X,1HB,9X,1HC,9X,1HD,/ 1X)
598 2002 FORMAT (1G,7X,4F10.3)
599 2004 FORMAT (45H1S T E P - B Y - S T E P S O L U T I O N ,
600 1 37H C O N T R O L I N F O R M A T I O N , ///
601 2 5X, 35HNUMBER OF TIME VARYING FUNCTIONS =, 15 //
602 3 5X, 35HGROUND MOTION INDICATOR =, 15 /
603 4 8X, 10HEQ.0, NONE, /
604 5 8X, 29HGT.0, READ ACCELERATION INPUT, //
605 6 5X, 35HNUMBER OF ARRIVAL TIMES =, 15 /
606 7 8X, 26HEQ.0, ALL FUNCTIONS ARRIVE, /
607 8 8X, 18H AT TIME ZERO, //
608 9 5X, 35HNUMBER OF SOLUTION TIME STEPS =, 15 //
609 A 5X, 35HOUTPUT (PRINT) INTERVAL =, 15 //
610 B 5X, 35HSOLUTION TIME INCREMENT =, E14.4 //
611 C 5X, 30HMASS- PROPORTIONAL DAMPING, /
612 D 5X, 35HCOEFFICIENT (ALPHA) =, E14.4 //
613 E 5X, 30HSTIFFNESS-PROPORTIONAL DAMPING, /
614 F 5X, 35HCOEFFICIENT (BETA) =, E14.4 /// 1X)
615 3000 FORMAT (27H0*** ERROR ZERO TIME STEP, / 1X)
616 END
617 C
618 C
619 C=====
620 SUBROUTINE BOUND
621 IMPLICIT REAL*8(A-H,O-Z)
622 COMMON A(1)
623 COMMON /ELPAR/ NPAR(14),NUMNP,MBAND,NELTYP,N1,N2,N3,N4,N5,MTOT,NEQ
624 COMMON /JUNK/ LT,LH,L,IPAD,SIG(20),IFILL(386)
625 COMMON /EXTRA/ MODEX,NT8,N10SV,NT10,IFILL2(12)
626 IF (NPAR(1).EQ.0) GO TO 500
627 CALL CLAMP (NPAR(2),A(N1),A(N2),A(N3),A(N4),NUMNP,MBAND)
628 RETURN
629 500 continue
630 c-- WRITE (33,2002)
631 Nume=NPAR(2)
632 DO 800 MM=1,NUME
633 CALL STRSC (A(N1),A(N3),NEQ,0)
634 c-- WRITE (33,2001)
635 DO 800 L=LT,LH
636 CALL STRSC (A(N1),A(N3),NEQ,1)
637 c-- WRITE (33,3002) MM,L,(SIG(1),I=1,2) !printing suppressed
638 IF(N10SV.EQ.1)
639 *WRITE (NT10) MM,L,SIG(1),SIG(2)
640 800 CONTINUE
641 RETURN
642 2001 FORMAT (//)
643 2002 FORMAT (48H1B O U N D A R Y E L E M E N T F O R C E S /,
644 1 14H M O M E N T S, // 8H ELEMENT,3X,4HLOAD,14X,5HFORCE,
645 2 9X,6HMOment, / 8H NUMBER,3X,4HCASE, // 1X)
646 3002 FORMAT (18,17,4X,2E15.5)
647 END
648 C

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649 C
650 C&=====
651     SUBROUTINE CALBAN (MBAND,NDIF,LM,XM,S,P,ND,NDM,NS)
652     IMPLICIT REAL*8(A-H,O-Z)
653 C
654 C     CALLED BY:  RUSS,TEAM,PLNAX,BRICK8,IPLATE,CLAMP,ELST3D,PIPER
655 C
656 C-----CALCULATES BAND WIDTH AND WRITES STIFFNESS MATRIX ON TAPE 2
657     DIMENSION LM(1),XM(1),S(NDM,NDM),P(NDM,4)
658     COMMON /EXTRA/ MODEX,NT8,IFILL(14)
659     MIN=100000
660     MAX=0
661     DO 800 L=1,ND
662     IF (LM(L).EQ.0) GO TO 800
663     IF (LM(L).GT.MAX) MAX=LM(L)
664     IF (LM(L).LT.MIN) MIN=LM(L)
665 800 CONTINUE
666     NDIF=MAX-MIN+1
667     IF (NDIF.GT.MBAND) MBAND=NDIF
668     IF(MODEX.EQ.1) GO TO 810
669 C
670     LRD=ND*(ND+1)/2+5*ND
671     WRITE(2) LRD,ND,(LM(I),I=1,ND),((S(I,J),J=1,ND),I=1,ND),
672     1 ((P(I,J),I=1,ND),J=1,4),(XM(I),I=1,ND)
673     RETURN
674 C
675 810 WRITE (1) ND,NS,(LM(I),I=1,ND)
676     RETURN
677 C
678     END
679 C
680 C
681 C&=====
682     SUBROUTINE CLAMP (NUMEL,ID,X,Y,Z,NUMNP,MBAND)
683     IMPLICIT REAL*8(A-H,O-Z)
684     COMMON/EM/LM(24),ND,NS,S(24,24),P(24,4),XM(24),SA(12,24),TT(12,4),
685     1 IFILL1(3048)
686     DIMENSION X(1),Y(1),Z(1),ID(NUMNP,1)
687     COMMON / JUNK / R(6),RM(4),IFILL2(410)
688     COMMON /EXTRA/ MODEX,NT8,IFILL3(14)
689     WRITE (33,2000) NUMEL
690     NS=2
691     ND=6
692     READ(5,1005) RM
693     WRITE (33,2005) RM
694     IF(MODEX.EQ.1)
695     *WRITE (NT8) RM
696     DO 30 NI=1,ND
697     XM(NI) = 0.0
698     DO 20 NJ=1,ND
699     20 S(NI,NJ)=0.0
700     30 CONTINUE
701     DO 50 NK=1,NS
702     DO 40 NL=1,ND

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595 1010 FORMAT (51H0STRUCTURE WITH NO DEGREES OF FREEDOM CHECK DATA )
596 2000 FORMAT (/// 10H STRUCTURE,13X,7HELEMENT,4X,4HLOAD,4X,
597 1 11HMULTIPLIERS,/ 10H LOAD CASE,12X,1HA,9X,1HB,9X,1HC,9X,1HD,/ 1X)
598 2002 FORMAT (16,7X,4F10.3)
599 2004 FORMAT (45H1S T E P - B Y - S T E P S O L U T I O N ,
600 1 37HC O N T R O L I N F O R M A T I O N , ///
601 2 5X, 35HNUMBER OF TIME VARYING FUNCTIONS =, 15 //
602 3 5X, 35HCONTINUING MOTION INDICATOR =, 15 /

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```

703 40 SA(NK,NL) = 0.0
704 DO 50 NI=1,4
705 TI(NK,NI) = 0.0
706 50 CONTINUE
707 NE=0
708 WRITE (33,2007)
709 210 KG=0
710 MARK=0
711 200 READ (5,1000) NF,NI,NJ,NK,NL,KD,KR,KN,SD,SR,TRACE
712 IF (TRACE.EQ.0.) TRACE=1.0E+10
713 IF (KG.GT.0) GO TO 550
714 KG=KN
715 IF (MODEX.EQ.1) GO TO 530
716 IF (NJ.EQ.0) GO TO 250
717 X1=X(NJ)-X(NI)
718 Y1=Y(NJ)-Y(NI)
719 Z1=Z(NJ)-Z(NI)
720 X2=X(NL)-X(NK)
721 Y2=Y(NL)-Y(NK)
722 Z2=Z(NL)-Z(NK)
723 T1=Y1*Z2-Y2*Z1
724 T2=Z1*X2-Z2*X1
725 T3=X1*Y2-X2*Y1
726 GO TO 260
727 250 T1=X(NI)-X(NP)
728 T2=Y(NI)-Y(NP)
729 T3=Z(NI)-Z(NP)
730 260 XL=T1*T1+T2*T2+T3*T3
731 XL=DSQRT(XL)
732 IF (XL.GT.1.0E-6) GO TO 270
733 WRITE (33,3000)
734 3000 FORMAT (32H0*** ERROR ZERO ELEMENT LENGTH, / 1X)
735 STOP
736 270 CONTINUE
737 T1=T1/XL
738 T2=T2/XL
739 T3=T3/XL
740 IF (KD.EQ.0) GO TO 300
741 SA(1,1)=T1*TRACE
742 SA(1,2)=T2*TRACE
743 SA(1,3)=T3*TRACE
744 S(1,1)=T1*T1*TRACE
745 S(1,2)=T1*T2*TRACE
746 S(1,3)=T1*T3*TRACE
747 S(2,2)=T2*T2*TRACE
748 S(2,3)=T2*T3*TRACE
749 S(3,3)=T3*T3*TRACE
750 PP=TRACE*SD
751 R(1)=T1*PP
752 R(2)=T2*PP
753 R(3)=T3*PP
754 GO TO 350
755 300 DO 310 I=1,3
756 R(I)=0.0

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```

757      SA(1,I)=0.0
758      DO 310 J=1,3
759 310   S(I,J)=0.0
760 350   IF (KR.EQ.0) GO TO 400
761      SA(2,5)=T2*TRACE
762      SA(2,4)=T1*TRACE
763      SA(2,6)=T3*TRACE
764      S(4,4)=T1*T1*TRACE
765      S(4,5)=T1*T2*TRACE
766      S(4,6)=T1*T3*TRACE
767      S(5,5)=T2*T2*TRACE
768      S(5,6)=T2*T3*TRACE
769      S(6,6)=T3*T3*TRACE
770      PP=TRACE*SR
771      R(4)=T1*PP
772      R(5)=T2*PP
773      R(6)=T3*PP
774      GO TO 450
775 400   DO 410 I=4,6
776      R(I)=0.0
777      SA(2,I)=0.0
778      DO 410 J=1,6
779 410   S(I,J)=0.0
780 450   DO 500 I=1,ND
781      DO 500 J=1,ND
782 500   S(I,I)=S(I,J)
783      DO 520 I=1,ND
784      DO 520 J=1,4
785 520   P(I,J)=R(I)*RM(J)
786 530   NN=NP
787      NNI=NI
788      NNJ=NJ
789      NNK=NK
790     >NNL=NL
791     >NKD=KD
792     >NKR=KR
793     >SSD=SD
794     >SSK=SR
795     >TTR=TRACE
796      GO TO 560
797 550   MARK=1
798 555  >NN=NN+KG
799     >NNI>NNI+KG
800 560  >KEL=NE+1
801     >WRITE (33,2010) KEL,NN,NNI,NNJ,NNK>NNL,>NKD,>NKR,KN,SSD,SSK,TTR
802     >NE=NE+1
803     >IF (MODEX.EQ.1)
804     >WRITE (NTB) NE,NN,NNI,NNJ,NNK>NNL,>NKD,>NKR,SSD,SSK,TTR
805     >DO 600 I=1,ND
806 600  >LH(I)=ID(NN,I)
807     >NDM=24
808     >CALL CALBAN (MBAND,NDIF,LH,XM,S,P,ND,NDM,NS)
809     >IF (MODEX.EQ.1) GO TO 650
810     >WRITE (1) ND,NS,(LH(L),L=1,ND),((SA(L,K),L=1,NS),K=1,ND),

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811      1 ((TT(L,K),L=1,NS),K=1,4)
812      650 CONTINUE
813      IF (NE.EQ.NUMEL) RETURN
814      IF (NN.LT.NP) GO TO 555
815      IF (MARK.EQ.1) GO TO 210
816      GO TO 200
817      1000 FORMAT (8I5,3F10.0)
818      1005 FORMAT (4F10.0)
819      2000 FORMAT (34H1B O U N D A R Y E L E M E N T S, ///
820      1          27H ELEMENT TYPE          =      7, /
821      2          21H NUMBER OF ELEMENTS =,I6    ///1X)
822      2005 FORMAT (30H ELEMENT LOAD CASE MULTIPLIERS, // 8X,7HCASE(A),8X,
823      1          7HCASE(B),8X,7HCASE(C),8X,7HCASE(D),/ 4F15.4 /// 1X)
824      2007 FORMAT (53H ELEMENT NODE NODES DEFINING CONSTRAINT DIRECTION,
825      1          3X,38HCODE CODE GENERATION SPECIFIED,6X,
826      2          22HSPECIFIED SPRING, /
827      3          53H NUMBER (N) (NI) (NJ) (NK) (NL),
828      4          3X,38H KD KR CODE (KN) DISPLACEMENT,6X,
829      5          22H ROTATION RATE, / 1X)
830      2010 FORMAT (1X,2(2X,I5),2X,4(4X,I5),2(2X,I5),7X,I5,2E15.4,E13.4)
831      END
832 C
833 C&=====
834      SUBROUTINE CROSS2 (A,B,C,IERR)
835 C
836 C      CALLED BY : INP21
837 C
838      IMPLICIT REAL*8(A-H,O-Z)
839 C
840 C      THIS ROUTINE FORMS THE VECTOR PRODUCT C = A*B WHERE *C*
841 C      IS NORMALIZED TO UNIT LENGTH
842 C
843      DIMENSION A(3),B(3),C(3)
844 C
845      X = A(2) * B(3) - A(3) * B(2)
846      Y = A(3) * B(1) - A(1) * B(3)
847      Z = A(1) * B(2) - A(2) * B(1)
848      XLN =DSQRT(X*X+Y*Y+Z*Z)
849      IERR = 1
850      IF(XLN.LE.1.0E-08) RETURN
851      XLN = 1.0 /XLN
852      C(3) = Z * XLN
853      C(2) = Y * XLN
854      C(1) = X * XLN
855      IERR = 0
856      RETURN
857      END
858 C&=====
859      SUBROUTINE DER3DS (NEL,XX,B,DET,R,S,T,NOD9,H,P,IELD,IELX)
860 C
861 C      CALLED BY : THDFE
862 C
863      IMPLICIT REAL*8(A-H,O-Z)
864 C

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865 C
866 C
867 C . . . . .
868 C .
869 C .   P R O G R A M   .
870 C .
871 C .   EVALUATES STRAIN-DISPLACEMENT MATRIX B AT POINT (R,S,T) .
872 C .
873 C .   CURVILINEAR HEXAHEDRON   8 TO 21 NODES   .
874 C .
875 C . . . . .
876 C
877 C
878 C
879   DIMENSION   XX(3,1),B(6,1),NOD9(1),H(1),P(3,1)
880   DIMENSION   XJ(3,3),XJI(3,3)
881 C
882 C
883 C   FIND INTERPOLATION FUNCTIONS AND THEIR DERIVATIVES
884 C   EVALUATE JACOBIAN MATRIX AT POINT (R,S,T)
885 C   COMPUTE DETERMINANT OF JACOBIAN MATRIX AT POINT (R,S,T)
886 C
887 C
888   CALL FNCT (R,S,T,H,P,NOD9,XJ,DET,XX,IELD,IELX,NEL)
889 C
890 C
891 C   COMPUTE INVERSE OF JACOBIAN MATRIX
892 C
893 C
894   DUM=1.0/DET
895   XJI(1,1)=DUM*( XJ(2,2)*XJ(3,3) - XJ(2,3)*XJ(3,2))
896   XJI(2,1)=DUM*(-XJ(2,1)*XJ(3,3) + XJ(2,3)*XJ(3,1))
897   XJI(3,1)=DUM*( XJ(2,1)*XJ(3,2) - XJ(2,2)*XJ(3,1))
898   XJI(1,2)=DUM*(-XJ(1,2)*XJ(3,3) + XJ(1,3)*XJ(3,2))
899   XJI(2,2)=DUM*( XJ(1,1)*XJ(3,3) - XJ(1,3)*XJ(3,1))
900   XJI(3,2)=DUM*(-XJ(1,1)*XJ(3,2) + XJ(1,2)*XJ(3,1))
901   XJI(1,3)=DUM*( XJ(1,2)*XJ(2,3) - XJ(1,3)*XJ(2,2))
902   XJI(2,3)=DUM*(-XJ(1,1)*XJ(2,3) + XJ(1,3)*XJ(2,1))
903   XJI(3,3)=DUM*( XJ(1,1)*XJ(2,2) - XJ(1,2)*XJ(2,1))
904 C
905 C
906 C   EVALUATE B MATRIX IN GLOBAL (X,Y,Z) COORDINATES
907 C
908 C
909   DO 130 K=1, IELD
910     K2=K*3
911     DO 115 L=1,3
912       B(L,K2-2) = 0.0
913       B(L,K2-1) = 0.0
914 115   B(L,K2 ) = 0.0
915 C
916 C   DIRECT STRAINS (1=EXX, 2=EYY, 3=EZZ)
917 C
918   DO 120 I=1,3

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919      B(1,K2-2) = B(1,K2-2) + XJI(1,I)* P(I,K)
920      B(2,K2-1) = B(2,K2-1) + XJI(2,I)* P(I,K)
921      120 B(3,K2 ) = B(3,K2 ) + XJI(3,I)* P(I,K)
922 C
923 C      SHEAR STRAINS (4=EXY, 5=EYZ, 6=EZX)
924 C
925      B(4,K2-2) = B(2,K2-1)
926      B(4,K2-1) = B(1,K2-2)
927      B(5,K2-1) = B(3,K2 )
928      B(5,K2 ) = B(2,K2-1)
929      B(6,K2-2) = B(3,K2 )
930      130 B(6,K2 ) = B(1,K2-2)
931 C
932 C
933      RETURN
934 C
935      END
936 C=====
937      SUBROUTINE ELTYPE(MTYPE)
938 C
939      IMPLICIT REAL8(A-H,O-Z)
940 C
941 C      CALLED BY:  MAIN,STRESS
942 C
943      GO TO (1,2,3,4,5,6,7,8,9,10,11,12),MTYPE
944 C
945 C      THREE DIMENSIONAL TRUSS ELEMENTS
946 C
947      1 CONTINUE
948 C      1 CALL TRUSS
949      GO TO 900
950 C
951 C      THREE DIMENSIONAL BEAM ELEMENTS
952 C
953      2 CONTINUE
954 C      2 CALL BEAM
955      GO TO 900
956 C
957 C      PLANE STRESS ELEMENTS
958 C
959      3 CONTINUE
960 C      3 CALL PLANE
961      GO TO 900
962 C
963 C      AXISYMMETRIC SOLID ELEMENTS
964 C
965      4 CONTINUE
966 C      4 CALL PLANE
967      GO TO 900
968 C
969 C      THREE DIMENSIONAL SOLID ELEMENTS
970 C
971      5 CONTINUE
972 C      5 CALL THREEED

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973      GO TO 900
974 C
975 C      PLATE BENDING ELEMENTS
976 C
977      6 CONTINUE
978 C      6 CALL SHELL
979      GO TO 900
980 C
981 C
982      7 CALL BOUND
983      GO TO 900
984 C
985 C      THICK SHELL ELEMENTS
986 C
987      8 CALL SOL21
988      GO TO 900
989 C
990      9 WRITE (33,100) MTYPE
991      GO TO 900
992 C
993      10 WRITE (33,100) MTYPE
994      GO TO 900
995 C
996      11 WRITE (33,100) MTYPE
997      GO TO 900
998 C
999 C      STRAIGHT OR CURVED PIPE ELEMENTS
1000 C
1001      12 CONTINUE
1002 C      12 CALL PIPE
1003 C
1004      900 RETURN
1005 C
1006      100 FORMAT ('ELEMENT',I4,' IS NOT IMPLEMENTED YET')
1007      END
1008 C=====
1009      SUBROUTINE ERROR(N)
1010      WRITE (33,2000) N
1011      2000 FORMAT ('// 20M STORAGE EXCEEDED BY  I6)
1012      STOP
1013      END
1014 C=====
1015      SUBROUTINE FACEPR (NEL,KDIS,KXYZ,XX,NOD9,H,P,PL,NEACE,LT,PWA,KLS)
1016 C
1017 C      CALLED BY : THDFE
1018 C      CALLS : FNCT
1019 C
1020      IMPLICIT REAL*8(A-H,O-Z)
1021 C
1022 C
1023 C      THIS ROUTINE COMPUTES NODE FORCES DUE TO APPLIED ELEMENT FACE
1024 C      PRESSURE DISTRIBUTIONS
1025 C
1026 C

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1027      DIMENSION      XX(3,1),NOD9(1),H(1),P(3,1),PL(1),PWA(1)
1028      DIMENSION      XJ(3,3),ETA(3),KEACE(6,8),KCRD(6),EVAL(6),IPRM(3),
1029      1                PR(8),NODES(8),IPR4(4)
1030      COMMON /GAUSS/ XK(4,4),WGT(4,4)
1031 C
1032      DATA KEACE / 1, 2, 1, 4, 1, 5,
1033      1                4, 3, 5, 8, 2, 6,
1034      2                8, 7, 6, 7, 3, 7,
1035      3                5, 6, 2, 3, 4, 8,
1036      4                12, 10, 17, 20, 9, 13,
1037      5                20, 19, 13, 15, 10, 14,
1038      6                16, 14, 18, 19, 11, 15,
1039      7                17, 18, 9, 11, 12, 16/
1040 C
1041      DATA KCRD / 1, 1, 2, 2, 3, 3/
1042      DATA EVAL / 1.,-1., 1.,-1., 1.,-1./
1043      DATA IPRM / 2, 3, 1/
1044      DATA IPR4 / 2, 3, 4, 1/
1045 C
1046 C      DETERMINE THE ELEMENT NODES CONTRIBUTING TO FORCE CALCULATIONS
1047 C      ON THIS FACE
1048 C
1049      DO 2 I=1,4
1050      NODES(I) = KEACE(NFACE,I)
1051      NODES(I+4) = 0
1052 2 CONTINUE
1053 C
1054      IF(KDIS.LT.9) GO TO 9
1055 C
1056      NN9 = KDIS-8
1057 C
1058      DO 8 K=5,8
1059      DO 4 I=1,NN9
1060 C
1061      J = I
1062      IF(KEACE(NFACE,K).EQ.NOD9(I)) GO TO 6
1063 C
1064 4 CONTINUE
1065      GO TO 8
1066 C
1067 6 NODES(K) = J
1068 8 CONTINUE
1069 C
1070 9 CONTINUE
1071 C
1072 C      SET UP THE PRESSURE VECTOR FOR THE FOUR FACE CORNER NODES
1073 C
1074 C      1. ADJUST THE SIGN OF THE PRESSURES SO THAT POSITIVE
1075 C      PRESSURE ALWAYS COMPRESSES THE ELEMENT
1076 C
1077      FACT = -EVAL(NFACE)
1078 C
1079      GO TO (10,30), LT
1080 C

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1081 C          2. DISTRIBUTED PRESSURE GIVEN AT THE CORNER NODES
1082 C
1083      10 DO 25 K=1,8
1084 C
1085          IF(NODES(K).EQ.0) GO TO 25
1086 C
1087          IF(K.GT.4) GO TO 15
1088 C
1089          PR(K) = PWA(K) * FACT
1090          GO TO 25
1091 C
1092      15 J = K-4
1093          L = IPR4(J)
1094          PR(K) = (PWA(J) + PWA(L)) * 0.5 * FACT
1095 C
1096      25 CONTINUE
1097          GO TO 75
1098 C
1099 C          3. ELEMENT FACE EXPOSED TO HYDROSTATIC PRESSURE
1100 C
1101      30 GAMMA = PWA(1) * FACT
1102 C
1103          XLN = 0.0
1104          DO 35 K=1,3
1105              ETA(K) = PWA(K+4) - PWA(K+1)
1106      35 XLN = XLN + ETA(K)**2
1107          XLN = DSQRT(XLN)
1108 C
1109          IF(XLN.GT.1.0E-6) GO TO 40
1110 C
1111          WRITE (33,3000) KLS,NEL
1112      3000 FORMAT (31H***** PRESSURE LOAD SET (,I3,15H) FOR ELEMENT (,
1113      1          15,43H) HAS UNDEFINED HYDROSTATIC SURFACE NORMAL., / 1X)
1114          STOP
1115 C
1116      40 DO 45 K=1,3
1117          45 ETA(K) = ETA(K) / XLN
1118 C
1119          DO 70 N=1,8
1120 C
1121          IF(NODES(N).EQ.0) GO TO 70
1122 C
1123          XLN = 0.0
1124          NOD = NODES(N)
1125          IF(N.GT.4) NOD = NOD + 8
1126 C
1127          DO 50 I=1,3
1128      50 XLN = XLN + (XX(I,NOD) - PWA(I+1)) * ETA(I)
1129 C
1130          PR(N) = XLN * GAMMA
1131 C
1132          IF(XLN.LT.0.0) PR(N) = 0.0
1133 C
1134      70 CONTINUE

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1135      75 CONTINUE
1136 C
1137 C      SET UP VARIABLES FOR THE SURFACE INTEGRATION
1138 C
1139      ML = KCRD(NEACE)
1140      MM = IPKM(ML)
1141      MN = IPKM(MM)
1142 C
1143 C      SURFACE INTEGRATION LOOP
1144 C
1145      ETA(ML) = EVAL(NEACE)
1146 C
1147      DO 300 LX=1,3
1148 C
1149      ETA(MM) = AK(LX,3)
1150 C
1151      DO 300 LY=1,3
1152 C
1153      ETA(MN) = AK(LY,3)
1154 C
1155      WT = WGT(LX,3)*WGT(LY,3)
1156 C
1157 C      EVALUATE THE INTERPOLATION FUNCTIONS AND JACOBIAN MATRIX
1158 C
1159      CALL FNCT (ETA(1),ETA(2),ETA(3),H,P,MOD9,XJ,DET,XX,KDIS,KX12,NEL)
1160 C
1161 C      COMPUTE THE DIRECTION COSINES OF THE UNIT SURFACE NORMAL VECTOR
1162 C      AT THIS SAMPLE POINT
1163 C
1164      A1 = XJ(MM,2)*XJ(MN,3)-XJ(MM,3)*XJ(MN,2)
1165      A2 = XJ(MM,3)*XJ(MN,1)-XJ(MM,1)*XJ(MN,3)
1166      A3 = XJ(MM,1)*XJ(MN,2)-XJ(MM,2)*XJ(MN,1)
1167 C
1168      AA=DSQRT(A1**2 + A2**2 + A3**2)
1169      IF(AA.GT.1.0E-8) GO TO 100
1170 C
1171      WRITE (33,3010) NEACE,NEL
1172 3010 FORMAT (38H***** UNDEFINED NORMAL IN FACE (,I1,5H) FOR,
1173      1      10H ELEMENT (,I5,2H)., / 1X)
1174      STOP
1175 C
1176 100 FACT = 1.0/AA
1177      A1 = A1* FACT
1178      A2 = A2* FACT
1179      A3 = A3* FACT
1180 C
1181 C      COMPUTE THE FIRST FUNDAMENTAL FORM (AREA DIFFERENTIAL)
1182 C
1183      AA = 0.0
1184      BB = 0.0
1185      CC = 0.0
1186      DO 120 I=1,3
1187      AA = AA + XJ(MM,I)**2
1188      CC = CC + XJ(MN,I)**2

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1189 120 BB = BB + XJ(MN,1)*XJ(MN,1)
1190 C =DSQRT(AA*CC - BB**2)
1191 C
1192 C INTERPOLATE FOR THE PRESSURE AT THIS SAMPLE POINT
1193 C
1194 PRESS = 0.0
1195 C
1196 DO 130 K=1,8
1197 C
1198 IF(NODES(K).EQ.0) GO TO 130
1199 C
1200 NOD = NODES(K)
1201 IF(K.GT.4) NOD = NOD + 8
1202 C
1203 PRESS = PRESS + H(NOD)*PR(K)
1204 130 CONTINUE
1205 C
1206 FACT = WTA*CA*PRESS
1207 C
1208 C ASSEMBLE THE NODE FORCE CONTRIBUTION
1209 C
1210 DO 160 L=1,8
1211 C
1212 IF(NODES(L).EQ.0) GO TO 160
1213 C
1214 IF(L.GT.4) GO TO 140
1215 C
1216 C 1. CORNER NODES
1217 C
1218 N = NODES(L)
1219 K = 3*N
1220 GO TO 150
1221 C
1222 C 2. SIDE NODES
1223 C
1224 140 J = NODES(L)
1225 N = J+8
1226 K = 3*NOD9(J)
1227 C
1228 150 QU = FACT*H(N)
1229 C
1230 PL(K-2) = PL(K-2) + QU*A1
1231 PL(K-1) = PL(K-1) + QU*A2
1232 PL(K ) = PL(K ) + QU*A3
1233 160 CONTINUE
1234 C
1235 300 CONTINUE
1236 C
1237 RETURN
1238 END
1239 C=====
1240 SUBROUTINE FNCT (R,S,T,H,P,NOD9,XJ,DET,XX,IELD,IELX,NEL)
1241 C
1242 C CALLED BY : FAGEPR

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1243 C
1244     IMPLICIT REAL*8(A-H,O-Z)
1245 C
1246 C
1247 C . . . . .
1248 C .
1249 C .   P R O G R A M
1250 C .
1251 C .       TO FIND INTERPOLATION FUNCTIONS ( H )
1252 C .       AND DERIVATIVES ( P ) CORRESPONDING TO THE NODAL
1253 C .       POINTS OF A CURVILINEAR ISOPARAMETRIC HEXAHEDRON
1254 C .       OR SUBPARAMETRIC HEXAHEDRON (8 TO 21 NODES)
1255 C .
1256 C .       TO FIND JACOBIAN ( XJ ) AND ITS DETERMINANT ( DET )
1257 C .
1258 C . . . . .
1259 C
1260 C
1261     DIMENSION H(1),P(3,1),NOD9(1),IPERM(8),XJ(3,3),XX(3,1)
1262 C
1263     DATA IPERM / 2,3,4,1,6,7,8,5 /
1264 C
1265     IEL = IELD
1266     NND9= IELD-8
1267 C
1268     RP=1.0 + R
1269     SP=1.0 + S
1270     TP=1.0 + T
1271     RM=1.0 - R
1272     SM=1.0 - S
1273     TM=1.0 - T
1274     RK=1.0 - R*R
1275     SS=1.0 - S*S
1276     TT=1.0 - T*T
1277 C
1278 C
1279 C     INTERPOLATION FUNCTIONS AND THEIR DERIVATIVES
1280 C
1281 C
1282 C     8-NODE BRICK
1283 C
1284     H(1)=0.125*RP*SP*TP
1285     H(2)=0.125*RM*SP*TP
1286     H(3)=0.125*RM*SM*TP
1287     H(4)=0.125*RP*SM*TP
1288     H(5)=0.125*RP*SP*TM
1289     H(6)=0.125*RM*SP*TM
1290     H(7)=0.125*RM*SM*TM
1291     H(8)=0.125*RP*SM*TM
1292 C
1293     P(1,1)=0.125*SP*TP
1294     P(1,2)=-P(1,1)
1295     P(1,3)=-0.125*SM*TP
1296     P(1,4)=-P(1,3)

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1297      P(1,5)=0.125*SP*TM
1298      P(1,6)=-P(1,5)
1299      P(1,7)=-0.125*SM*TM
1300      P(1,8)=-P(1,7)
1301 C
1302      P(2,1)=0.125*RP*TP
1303      P(2,2)=0.125*RM*TP
1304      P(2,3)=-P(2,2)
1305      P(2,4)=-P(2,1)
1306      P(2,5)=0.125*RP*TM
1307      P(2,6)=0.125*RM*TM
1308      P(2,7)=-P(2,6)
1309      P(2,8)=-P(2,5)
1310 C
1311      P(3,1)=0.125*RP*SP
1312      P(3,2)=0.125*RM*SP
1313      P(3,3)=0.125*RM*SM
1314      P(3,4)=0.125*RP*SM
1315      P(3,5)=-P(3,1)
1316      P(3,6)=-P(3,2)
1317      P(3,7)=-P(3,3)
1318      P(3,8)=-P(3,4)
1319 C
1320      IF(IEL.EQ.8) GO TO 50
1321 C
1322 C
1323 C      ADD DEGREES OF FREEDOM IN EXCESS OF 8
1324 C
1325      I=0
1326      2 I=I + 1
1327      IF(I.GT.NND9) GO TO 40
1328      NN=NDD9(I) - 8
1329      GO TO (9,10,11,12,13,14,15,16,17,18,19,20,21) ,NN
1330 C
1331      9 H(9) = 0.25*RR*SP*TP
1332      P(1,9) = -0.50*RR*SP*TP
1333      P(2,9) = 0.25*RR*TP
1334      P(3,9) = 0.25*RR*SP
1335      GO TO 2
1336      10 H(10)=0.25*RM*SS*TP
1337      P(1,10)=-0.25*SS*TP
1338      P(2,10)=-0.50*RM*SS*TP
1339      P(3,10)= 0.25*RM*SS
1340      GO TO 2
1341      11 H(11)=0.25*RR*SM*TP
1342      P(1,11)=-0.50*RR*SM*TP
1343      P(2,11)=-0.25*RR*TP
1344      P(3,11)= 0.25*RR*SM
1345      GO TO 2
1346      12 H(12)=0.25*RP*SS*TP
1347      P(1,12)= 0.25*SS*TP
1348      P(2,12)=-0.50*RP*SS*TP
1349      P(3,12)= 0.25*RP*SS
1350      GO TO 2

```

```

1351      13 H(13)=0.25*RR*SP*TM
1352      P(1,13)=-0.50*RR*SP*TM
1353      P(2,13)= 0.25*RR*TM
1354      P(3,13)=-0.25*RR*SP
1355      GO TO 2
1356      14 H(14)=0.25*RM*SS*TM
1357      P(1,14)=-0.25*SS*TM
1358      P(2,14)=-0.50*RM*SS*TM
1359      P(3,14)=-0.25*RM*SS
1360      GO TO 2
1361      15 H(15)=0.25*RR*SM*TM
1362      P(1,15)=-0.50*RR*SM*TM
1363      P(2,15)=-0.25*RR*TM
1364      P(3,15)=-0.25*RR*SM
1365      GO TO 2
1366      16 H(16)=0.25*RP*SS*TM
1367      P(1,16)= 0.25*SS*TM
1368      P(2,16)=-0.50*RP*SS*TM
1369      P(3,16)=-0.25*RP*SS
1370      GO TO 2
1371      17 H(17)=0.25*RP*SP*TT
1372      P(1,17)=0.25*SP*TT
1373      P(2,17)=0.25*RP*TT
1374      P(3,17)=-0.50*RP*SP*TT
1375      GO TO 2
1376      18 H(18)=0.25*RM*SP*TT
1377      P(1,18)=-0.25*SP*TT
1378      P(2,18)= 0.25*RM*TT
1379      P(3,18)=-0.50*RM*SP*TT
1380      GO TO 2
1381      19 H(19)=0.25*RM*SM*TT
1382      P(1,19)=-0.25*SM*TT
1383      P(2,19)=-0.25*RM*TT
1384      P(3,19)=-0.50*RM*SM*TT
1385      GO TO 2
1386      20 H(20)=0.25*RP*SM*TT
1387      P(1,20)= 0.25*SM*TT
1388      P(2,20)=-0.25*RP*TT
1389      P(3,20)=-0.50*RP*SM*TT
1390      GO TO 2
1391      21 H(21)=RR*SS*TT
1392      P(1,21)=-2.0*RR*SS*TT
1393      P(2,21)=-2.0*SS*RR*TT
1394      P(3,21)=-2.0*RR*SS
1395      GO TO 2
1396 C
1397 C      MODIFT FIRST 8 FUNCTIONS IF 9 OR MORE NODES IN ELEMENT
1398 C
1399      40 IH=0
1400      41 IH=IH + 1
1401      IF (IH.GT.NND9) GO TO 50
1402      II=IH + 7
1403      IF (II.EQ.IELX) GO TO 51
1404      42 IN=NOD9(IH)

```

```

1405     IF (IN.GT.16) GO TO 46
1406     I1=IN -8
1407     I2=IPERM(I1)
1408     H(I1)=H(I1) - 0.5*H(IN)
1409     H(I2)=H(I2) - 0.5*H(IN)
1410     H(IH+8)=H(IN)
1411     DO 45 J=1,3
1412     P(J,I1)=P(J,I1) - 0.5*P(J,IN)
1413     P(J,I2)=P(J,I2) - 0.5*P(J,IN)
1414     45 P(J,IH+8)=P(J,IN)
1415     GO TO 41
1416     46 IF (IN.EQ.21) GO TO 30
1417     I1=IN -16
1418     I2=I1 + 4
1419     H(I1)=H(I1) - 0.5*H(IN)
1420     H(I2)=H(I2) - 0.5*H(IN)
1421     H(IH+8)=H(IN)
1422     DO 47 J=1,3
1423     P(J,I1)=P(J,I1) - 0.5*P(J,IN)
1424     P(J,I2)=P(J,I2) - 0.5*P(J,IN)
1425     47 P(J,IH+8)=P(J,IN)
1426     GO TO 41
1427 C
1428 C     MODIFY FIRST 20 FUNCTIONS IF NODE 21 IS PRESENT
1429 C
1430     30 IH=0
1431     31 IH=IH + 1
1432     IN=NOD9(IH)
1433     IF (IN.EQ.21) GO TO 35
1434     IF (IN.GT.16) GO TO 33
1435     I1=IN -8
1436     I2=IPERM(I1)
1437     H(I1)=H(I1) + 0.125*H(21)
1438     H(I2)=H(I2) + 0.125*H(21)
1439     DO 32 J=1,3
1440     P(J,I1)=P(J,I1) + 0.125*P(J,21)
1441     32 P(J,I2)=P(J,I2) + 0.125*P(J,21)
1442     GO TO 31
1443     33 I1=IN - 16
1444     I2=I1 + 4
1445     H(I1)=H(I1) + 0.125*H(21)
1446     H(I2)=H(I2) + 0.125*H(21)
1447     DO 34 J=1,3
1448     P(J,I1)=P(J,I1) + 0.125*P(J,21)
1449     34 P(J,I2)=P(J,I2) + 0.125*P(J,21)
1450     GO TO 31
1451     35 DO 36 I=1,8
1452     H(I)=H(I) - 0.125*H(21)
1453     DO 36 J=1,3
1454     36 P(J,I)=P(J,I) - 0.125*P(J,21)
1455     NN=RND9 + 7
1456     IF (NN.EQ.8) GO TO 50
1457     DO 38 I=9,NN
1458     H(I)=H(I) - 0.25*H(21)

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1459      DO 38 J=1,3
1460      38 P(J,I)=P(J,I) - 0.25*P(J,21)
1461      H(NND9+8)=H(21)
1462      DO 39 J =1,3
1463      39 P(J,NND9+8)=P(J,21)
1464 C
1465 C
1466 C      EVALUATE JACOBIAN MATRIX AT POINT (R,S,T)
1467 C
1468 C
1469      50 IF (IELX.LT.IELD) RETURN
1470      51 DO 100 I=1,3
1471      DO 100 J=1,3
1472      DUM=0.0
1473      DO 90 K=1,IELX
1474      90 DUM=DUM + P(I,K)*XX(J,K)
1475      100 XJ(I,J)=DUM
1476 C
1477 C
1478 C      COMPUTE DETERMINANT OF JACOBIAN MATRIX AT POINT (R,S,T)
1479 C
1480 C
1481      DET = XJ(1,1)*XJ(2,2)*XJ(3,3)
1482      1 + XJ(1,2)*XJ(2,3)*XJ(3,1)
1483      2 + XJ(1,3)*XJ(2,1)*XJ(3,2)
1484      3 - XJ(1,3)*XJ(2,2)*XJ(3,1)
1485      4 - XJ(1,2)*XJ(2,1)*XJ(3,3)
1486      5 - XJ(1,1)*XJ(2,3)*XJ(3,2)
1487      IF(DET.GT.1.0E-8) GO TO 110
1488      WRITE (33,2000) NEL,R,S,T
1489      STOP
1490      110 IF (IELX.LT.IELD) GO TO 42
1491 C
1492 C
1493      RETURN
1494 C
1495 C
1496 C
1497      2000 FORMAT (49H0ERR0R***  NEGATIVE OR ZERO JACOBIAN DETERMINANT,
1498      1          23H COMPUTED FOR ELEMENT (,15,1H), /
1499      2          12X, 3HR =, F10.5 /
1500      3          12X, 3HS =, F10.5 /
1501      4          12X, 3HT =, F10.5 / 1X)
1502 C
1503 C
1504      END
1505 C=====
1506      SUBROUTINE INL(ID,B,TR,TMASS,NUMNP,NEQB,LL)
1507 C
1508      IMPLICIT REAL*8(A-H,O-Z)
1509 C
1510 C      CALLED BY:  MAIN
1511 C
1512 C      INPUT NODAL LOADS AND MASSES

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1513 C
1514     DIMENSION ID(NUMNP,6),B(NEQB,LL),TR(6,LL),TMASS(NEQB)
1515     COMMON / JUNK / R(6),TXM(6),IFILL1(406)
1516     COMMON /EXTRA/ MODEX,NT8,IFILL2(14)
1517 C
1518     NT=3
1519     REWIND NT
1520     KSHE=0
1521     WRITE (33,2002)
1522     IF(MODEX.EQ.1) GO TO 50
1523     DO 750 I=1,NEQB
1524     TMASS(I)=0.
1525     DO 750 K=1,LL
1526 750 B(I,K)=0.0
1527 C
1528     50 DO 900 NN=1,NUMNP
1529 C
1530     DO 100 I=1,6
1531     TXM(I)=0.
1532     DO 100 J=1,LL
1533 100 TR(I,J)=0.0
1534 C
1535     IF(NN.EQ.1) GO TO 300
1536 150 IF(N.NE.NN) GO TO 400
1537     DO 200 I=1,6
1538     IF (L) 180,180,190
1539 180 TXM(I)=R(I)
1540     GO TO 200
1541 190 TR(I,L)=R(I)
1542 200 CONTINUE
1543 300 READ (5,1001) N,L,R
1544     IF (N.EQ.0) GO TO 150
1545     WRITE(33,2001) N,L,R
1546     GO TO 150
1547 C
1548 400 IF(MODEX.EQ.1) GO TO 900
1549     DO 300 J=1,6
1550     II=ID(NN,J)-KSHE
1551     IF (II) 800,800,500
1552 500 DO 600 K=1,LL
1553 600 B(II,K)=TR(J,K)
1554     TMASS(II)=TXM(J)
1555 610 IF(II.NE.NEQB) GO TO 800
1556     WRITE (NT) B,TMASS
1557     KSHE=KSHE+NEQB
1558     DO 700 I=1,NEQB
1559     TMASS(I)=0.
1560     DO 700 K=1,LL
1561 700 B(I,K)=0.0
1562 800 CONTINUE
1563 900 CONTINUE
1564 C
1565     IF(MODEX.EQ.1) RETURN
1566 C

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1567      WRITE (NT) B,TMASS
1568 C
1569      RETURN
1570 1001 FORMAT (2I5,7F10.4)
1571 2001 FORMAT (2(3X,I4),6E15.5)
1572 2002 FORMAT (47H1N O D A L   L O A D S   (S T A T I C)   O R   ,
1573      A      29H A S S E S   (D Y N A M I C), ///
1574      B      3X,4HNODE,2X,4HLOAD,
1575      1 2(9X,6HX-AXIS,9X,6HY-AXIS,9X,6HZ-AXIS), / 7H NUMBER,3X,4HCASE,
1576      2 3(10X,5HFORCE), 3(9X,6HMOMENT), / 1X)
1577      END
1578 C=====
1579      SUBROUTINE INPUTJ(ID,X,Y,Z,T,NUMNP,NEQ)
1580 C
1581      IMPLICIT REAL*8(A-H,O-Z)
1582 C
1583 C      CALLED BY:  MAIN
1584 C
1585      DIMENSION X(1),Y(1),Z(1),ID(NUMNP,6),T(1)
1586 C
1587      COMMON /EXTRA/ MODEX,NT8,IFILL(14)
1588 C
1589 C---- SPECIAL NODE CARD FLAGS
1590 C
1591 C      IT      =   COORDINATE SYSTEM TYPE   (CC 1, ANY NODE CARD)
1592 C               EQ.C, CYLINDRICAL
1593 C      IPR      =   PRINT SUPPRESSION FLAG   (CC 6, CARD FOR NODE 1 ONLY)
1594 C               EQ. , NORMAL PRINTING
1595 C               EQ.A, SUPPRESS SECOND PRINTING OF NODAL ARRAY DATA
1596 C               EQ.B, SUPPRESS PRINTING OF ID-ARRAY
1597 C               EQ.C, BOTH *** AND *B*
1598 C
1599      DIMENSION IPRC(4)
1600 C
1601      DATA IPRC/1H ,1HA,1HB,1HC/
1602 C
1603      IPR = IPRC(1)
1604      RAD = DATAN(1.0D0)/45.0D0
1605 C
1606 C
1607 C---- READ OR GENERATE NODAL POINT DATA-----
1608      WRITE (33,2000)
1609      WRITE (33,2001)
1610      NOLD=0
1611      10 READ  (5,1000) IT,N,JPR,(ID(N,I),I=1,6),X(N),Y(N),Z(N),KN,T(N)
1612 C
1613 C
1614 C** ** NEXT LINE IS DELETED FOR NOT PRINTING NODAL INPUT DATA
1615 C      WRITE (33,2002) IT,N,JPR,(ID(N,I),I=1,6),X(N),Y(N),Z(N),KN,T(N)
1616 C** ** **
1617 C
1618      IF(N.EQ.1) IPR = JPR
1619      IF(IT.NE.IPRC(4)) GO TO 15
1620      DUM = Z(N)* RAD

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1621      Z(N) = X(N)*DCOS(DUM)
1622      X(N) = X(N)*DSIN(DUM)
1623      15 CONTINUE
1624      IF(NOLD.EQ.0) GO TO 50
1625 C-----CHECK IF GENERATION IS REQUIRED-----
1626      DO 20 I=1,6
1627      IF(ID(N,I).EQ.0.AND.ID(NOLD,I).LT.0) ID(N,I)=ID(NOLD,I)
1628      20 CONTINUE
1629      IF(KN.EQ.0) GO TO 50
1630      NUM=(N-NOLD)/KN
1631      NUMN=NUM-1
1632      IF(NUMN.LT.1) GO TO 50
1633      XNUM=NUM
1634      DX=(X(N)-X(NOLD))/XNUM
1635      DY=(Y(N)-Y(NOLD))/XNUM
1636      DZ=(Z(N)-Z(NOLD))/XNUM
1637      DT=(T(N)-T(NOLD))/XNUM
1638      K=NOLD
1639      DO 30 I=1,NUMN
1640      KK=K
1641      K=K+KN
1642      X(K)=X(KK)+DX
1643      Y(K)=Y(KK)+DY
1644      Z(K)=Z(KK)+DZ
1645      T(K)=T(KK)+DT
1646      DO 30 I=1,6
1647      ID(K,I)=ID(KK,I)
1648      IF (ID(K,I).GT.1) ID(K,I)=ID(KK,I)+KN
1649      30 CONTINUE
1650 C
1651      50 NOLD=N
1652      IF(N.NE.NUMNP) GO TO 10
1653 C
1654 C---- PRINT ALL NODAL POINT DATA-----
1655 C
1656      IF(IPR.EQ.IPRC(2) .OR. IPR.EQ.IPRC(4)) GO TO 52
1657      WRITE (33,2003)
1658      WRITE (33,2001)
1659      WRITE (33,2005) (N,(ID(N,I),I=1,6),X(N),Y(N),Z(N),T(N),N=1,NUMNP)
1660      52 CONTINUE
1661 C
1662 C-----NUMBER UNKNOWN AND SET MASTER NODES NEGATIVE-----
1663 C
1664      NEQ=0
1665      DO 60 N=1,NUMNP
1666      DO 60 I=1,6
1667      ID(N,I)=IABS(ID(N,I))
1668      IF(ID(N,I)-1) 57,58,59
1669      57 NEQ=NEQ+1
1670      ID(N,I)=NEQ
1671      GO TO 60
1672      58 ID(N,I)=0
1673      GO TO 60
1674      59 ID(N,I)=-ID(N,I)

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1675      60 CONTINUE
1676 C
1677 C---- PRINT MASTER INDEX ARRAY
1678 C
1679      IF(IPR.EQ.IPRC(3) .OR. IPR.EQ.IPRC(4)) GO TO 62
1680      WRITE (33,2004) (N,(ID(N,I),I=1,6),N=1,NUMNP)
1681      62 CONTINUE
1682      IF(MODEX.EQ.0) GO TO 70
1683 CAAA DATA PORTHOLE SAVE
1684      WRITE (NT8) ((ID(N,I),I=1,6),N=1,NUMNP)
1685      WRITE (NT8) (X(N),N=1,NUMNP)
1686      WRITE (NT8) (Y(N),N=1,NUMNP)
1687      WRITE (NT8) (Z(N),N=1,NUMNP)
1688      WRITE (NT8) (T(N),N=1,NUMNP)
1689      ENDFILE NT8
1690 C
1691      REWIND 2
1692      WRITE (2) ID
1693 C
1694      RETURN
1695 C
1696      70 CONTINUE
1697      REWIND 8
1698      WRITE (8) ID
1699 C
1700      RETURN
1701 C
1702 1000 FORMAT (2(A1,I4),5I5,3F10.0,I5,F10.0)
1703 2000 FORMAT (//23H NODAL POINT INPUT DATA )
1704 2001 FORMAT (5HONODE 3X 24HBOUNDARY CONDITION CODES 11X
1705      . 23HNODAL POINT COORDINATES / 7H NUMBER 2X 1HX 4X 1HY 4X 1HZ 3X
1706      . 2HXX 3X 2HYY 3X 2HZZ12X 1HX 12X 1HY 12X 1HZ 12X 1HT )
1707 C
1708 C
1709 C** ** ** NEXT LINE IS IGNORED WITH LINE #31600
1710 C2002 FORMAT (1X,A1,I4,A1,I3,5I5,3F13.3,I5,F13.3)
1711 C** ** **
1712 C
1713 C
1714 2003 FORMAT (//21H1GENERATED NODAL DATA)
1715 2004 FORMAT (//17H1EQUATION NUMBERS/
1716      1 35H      N      X      Y      Z      XX      YY      ZZ /(7I5))
1717 2005 FORMAT (I5,6I5,4F13.3)
1718      END
1719 C&=====
1720      SUBROUTINE INP21 (NUMMAT,MAXTP,NORTH0,NDLS,NOPSET,NT8SV,NUMNP,X,
1721      1      Y,Z,DEN,RHO,NTP,EE,DCA,NEACE,LT,PWA,LOC,MAXPTS)
1722 C
1723 C      CALLED BY : THDFE
1724 C      CALLS : VECTR2,CROSS2
1725 C
1726      IMPLICIT REAL*8(A-H,O-Z)
1727 C
1728 C

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1729 C      THIS ROUTINE READS AND PRINTS ALL 21-NODE SOLID ELEMENT DATA
1730 C      BETWEEN THE CONTROL CARD AND THE ELEMENT DATA CARDS
1731 C
1732 C
1733 C      COMMON / JUNK/  XLF(4),YLF(4),ZLF(4),TLF(4),PLF(4),FILL1(22),V2(3)
1734 C      COMMON /EXTRA/  MODEX,NT8
1735 C
1736 C      DIMENSION  X(1),Y(1),Z(1),DEN(1),RHO(1),NTP(1),EE(MAXTP,13,1),
1737 1          DCA(3,3,1),NEACE(1),LT(1),PWA(7,1),LOC(7,1),
1738 2          MAXPTS(1)
1739 C      DIMENSION  HED(6)
1740 C
1741 C      READ AND PRINT OF MATERIAL PROPERTIES
1742 C
1743 C      WRITE (33,3000)
1744 C
1745 C      DO 10 I=1,NUMMAT
1746 C
1747 C      READ (5,1001) M,NTP(I),DEN(I),RHO(I),(HED(N),N=1,6)
1748 C
1749 C      SET DEFAULT VALUES IF REQUIRED AND CHECK FOR INPUT ERRORS
1750 C
1751 C      IF(RHO(I).EQ.0.0) RHO(I) = DEN(I) / 386.4
1752 C      IF(NTP(I).EQ.0) NTP(I) = 1
1753 C
1754 C      WRITE (33,3002) M,NTP(I),DEN(I),RHO(I),(HED(N),N=1,6)
1755 C
1756 C      IF(I.EQ.M) GO TO 2
1757 C      WRITE (33,4001)
1758 C      STOP
1759 C
1760 C      2 IF(NTP(M).LE.MAXTP) GO TO 4
1761 C      WRITE (33,4002) MAXTP
1762 C      STOP
1763 C      4 NT = NTP(M)
1764 C
1765 C      READ PROPERTIES FOR EACH TEMPERATURE
1766 C
1767 C      DO 6 K=1,NT
1768 C      READ (5,1002) (EE(K,L,M),L=1,13)
1769 C      WRITE (33,3003) (EE(K,L,M),L=1,13)
1770 C      6 CONTINUE
1771 C
1772 C      TEMPERATURE CARDS MUST BE ASCENDING ORDER
1773 C
1774 C      IF(NT.EQ.1) GO TO 10
1775 C      DO 8 J=2,NT
1776 C      IF(EE(J,1,M).GT.EE(J-1,1,M)) GO TO 8
1777 C      WRITE (33,4003)
1778 C      STOP
1779 C      8 CONTINUE
1780 C      10 CONTINUE
1781 C*** DATA PORTHOLE SAVE
1782 C      IF(NT8SV.EQ.0) GO TO 12

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1783      DO 11 M=1,NUMMAT
1784      WRITE (NT8) M,NIP(M),DEN(M),RHO(M)
1785      NT = NIP(M)
1786      WRITE (NT8) ((EE(K,L,M),L=1,13),K=1,NT)
1787      11 CONTINUE
1788 C***
1789 C
1790 C      MATERIAL AXIS ORIENTATION SETS
1791 C
1792      12 IF(NORTH0.EQ.0) GO TO 21
1793 C
1794      WRITE (33,3004)
1795 C
1796      DO 20 M=1,NORTH0
1797      READ (5,1003) N,NI,NJ,NK
1798      WRITE (33,3005) N,NI,NJ,NK
1799 C
1800 C*** DATA PORTHOLE SAVE
1801      IF(NT8SV.EQ.1)
1802      *WRITE (NT8)      N,NI,NJ,NK
1803 C***
1804 C      CHECK FOR ADMISSABILITY OF DATA
1805 C
1806      IF(N.EQ.M) GO TO 13
1807      WRITE (33,4004)
1808      STOP
1809 C
1810      13 IF(NI.GT.0 .AND. NI.LE.NUMNP) GO TO 5015
1811      L = NI
1812      5014 WRITE (33,4005) L
1813      STOP
1814      5015 IF(NJ.GT.0 .AND. NJ.LE.NUMNP) GO TO 5016
1815      L = NJ
1816      GO TO 5014
1817      5016 IF(NK.GT.0 .AND. NK.LE.NUMNP) GO TO 14
1818      L = NK
1819      GO TO 5014
1820      14 CONTINUE
1821 C
1822 C      GENERATE DIRECTION COSINE ARRAY FOR THIS DATA SET
1823 C
1824      CALL VECTR2 (DCA(1,1,M),X(NI),Y(NI),Z(NI),X(NJ),Y(NJ),Z(NJ),IERR)
1825      IF(IERR.EQ.0) GO TO 16
1826      WRITE (33,4006)
1827      STOP
1828      16 CALL VECTR2 (V2,X(NI),Y(NI),Z(NI),X(NK),Y(NK),Z(NK),IERR)
1829      IF(IERR.EQ.0) GO TO 17
1830      WRITE (33,4007)
1831      STOP
1832      17 CALL CROSS2 (DCA(1,1,M),V2,DCA(1,3,M),IERR)
1833      IF(IERR.EQ.0) GO TO 18
1834      WRITE (33,4008)
1835      STOP
1836      18 CALL CROSS2 (DCA(1,3,M),DCA(1,1,M),DCA(1,2,M),IERR)

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1837      IF(IERR.EQ.0) GO TO 20
1838      WRITE (33,4009)
1839      STOP
1840 20 CONTINUE
1841 C
1842 C      READ AND PRINT DISTRIBUTED SURFACE LOAD DATA
1843 C
1844 21 IF(NDLS.EQ.0) GO TO 31
1845 C
1846      WRITE (33,3006)
1847 C
1848      DO 30 M=1,NDLS
1849 C
1850          READ (5,1004) N,NFACE(M),LT(M)
1851          WRITE (33,3007) N,NFACE(M),LT(M)
1852 C
1853 C      CHECK FOR DATA ADMISSABILITY
1854 C
1855      IF(N.EQ.M) GO TO 22
1856      WRITE (33,4010)
1857      STOP
1858 22 IF(NFACE(M).GE.1 .AND. NFACE(M).LE.6) GO TO 23
1859      WRITE (33,4011)
1860      STOP
1861 23 IF(LT(M).EQ.0) LT(M) = 1
1862      IF(LT(M).GE.1 .AND. LT(M).LE.2) GO TO 24
1863      WRITE (33,4012)
1864      STOP
1865 24 IF(LT(M).EQ.2) GO TO 26
1866      READ (5,1005) (PWA(I,M),I=1,4)
1867      DO 25 I=2,4
1868 25 IF(PWA(I,M).EQ.0.0) PWA(I,M) = PWA(1,M)
1869      WRITE (33,3008) (PWA(I,M),I=1,4)
1870      GO TO 30
1871 26 READ (5,1005) (PWA(I,M),I=1,7)
1872      WRITE (33,3009) (PWA(I,M),I=1,7)
1873 30 CONTINUE
1874 C
1875 CAAA DATA PORTHOLE SAVE
1876      IF(NTBSV.EQ.0) GO TO 5031
1877      DO 5030 M=1,NDLS
1878          WRITE (NTS) NFACE(M),LT(M),(PWA(I,M),I=1,7)
1879 5030 CONTINUE
1880 5031 CONTINUE
1881 CAAA
1882 C
1883 C      READ AND PRINT OF STRESS OUTPUT REQUEST LOCATION SETS
1884 C
1885 31 IF(NOPSET.EQ.0) GO TO 40
1886 C
1887      WRITE (33,3010) (I,I=1,7)
1888      WRITE (34,*) '---STRESS OUTPUT LOCATIONS---'
1889 C
1890      DO 40 M=1,NOPSET

```

```

1184      BB = 0.0
1185      CC = 0.0
1186      DO 120 I=1,3
1187      AA = AA + XJ(MM,I)**2
1188      CC = CC + XJ(MN,I)**2

```

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```

1891      READ (5,1006) (LOC(I,M),I=1,7)
1892      WRITE (33,3011) M,(LOC(I,M),I=1,7)
1893      WRITE (34,3011) M,(LOC(I,M),I=1,7)
1894 C
1895      L = 0
1896      DO 35 J=1,7
1897      IF(LOC(J,M).EQ.0) GO TO 36
1898      L = L + 1
1899      IF(LOC(J,M).GE.1 .AND. LOC(J,M).LE.27) GO TO 35
1900      WRITE (33,4013) J
1901      MDEX = 1
1902      GO TO 36
1903      35 CONTINUE
1904 C
1905      36 IF(L.GT.0) GO TO 37
1906      L = 1
1907      LOC(1,M) = 21
1908      37 MAXPTS(M) = L
1909 C
1910      40 CONTINUE
1911 C*** DATA PORTHOLE SAVE
1912      IF(NTBSV.EQ.1)
1913      *WRITE (NT8) ((LOC(I,J),I=1,7),J=1,NOPSET)
1914 C***
1915 C
1916 C      ELEMENT LOAD CASE MULTIPLIERS
1917 C
1918      49 WRITE (33,2012)
1919 C
1920      READ (5,1007) XLF,YLF,ZLF,TLF,PLF
1921      WRITE (33,3013) XLF,YLF,ZLF,TLF,PLF
1922 C*** DATA PORTHOLE SAVE
1923      IF(NTBSV.EQ.1)
1924      *WRITE (NT8) XLF,YLF,ZLF,TLF,PLF
1925 C***
1926 C
1927      RETURN
1928 C
1929 C      FORMATS
1930 C
1931      1001 FORMAT (2I5,2F10.0,6A6)
1932      1002 FORMAT (2F10.0/6F10.0)
1933      1003 FORMAT (4I5)
1934      1004 FORMAT(3I5)
1935      1005 FORMAT (2F10.0)
1936      1006 FORMAT (2I5)
1937      1007 FORMAT (4F10.0)
1938 C
1939      3000 FORMAT (7,35H MATERIAL PROPERTY TABLES
1940      3002 FORMAT (7,25H MATERIAL NUMBER = (,I3,1H),/
1941      1      10H NUMBER OF, /
1942      2      25H TEMPERATURE POINTS = (,I3,1H),/
1943      3      25H WEIGHT DENSITY = (,E12.4,1H),/
1944      4      25H MASS DENSITY = (,E12.4,1H),/

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1945      5      23H IDENTIFICATION      = (,6A6,1H),//
1946      6 1X,11HTEMPERATURE,9X,3HE11,9X,3HE22,9X,3HE33,4X,3HV12,4X,3HV13,
1947      7 4X,3HV23,6X,3HG12,8X,3HG13,8X,3HG23,3X,7HALPHA-1,3X,7HALPHA-2,
1948      8 3X,7HALPHA-3,/1X)
1949 3003 FORMAT (F12.2,3F12.1,3F7.3,3F11.1,3E10.3)
1950 3004 FORMAT (//50H M A T E R I A L A X I S O R I E N T A T I O N      ,
1951      1 3X,9HT A B L E      ,//
1952      2 28H      SET      NODE      NODE      NODE      ,/
1953      3 23H NUMBER      NI      NJ      NK, / 1X)
1954 3005 FORMAT (4I7)
1955 3006 FORMAT(//51H D I S T R I B U T E D S U R F A C E L O A D
1956      1      11HT A B L E      ,//,1X)
1957 3007 FORMAT (//7X,27HLOAD SET NUMBER      = ,16 /
1958      1      7X,27HLOAD SURFACE ELEMENT FACE = ,16 /
1959      1      7X,27HLOAD TYPE CODE      = ,16/1X)
1960 3008 FORMAT (12H DISTRIBUTED, 11X,4HP(1),11X,4HP(2),11X,4HP(3),11X,
1961      1      4HP(4), / 4X,8HPRESSURE,4F15.3)
1962 3009 FORMAT (12H HYDROSTATIC,10X,5HGAMMA,11X,4HX(S),11X,4HY(S),11X,
1963      1      4HZ(S),11X,4HX(N),11X,4HY(N),11X,4HZ(N), /
1964      2      4X,8HPRESSURE, 7F15.3)
1965 3010 FORMAT (//51H S T R E S S O U T P U T R E Q U E S T T A B L E ,
1966      * //
1967      48H      SET ,7(2X,5HPPOINT), / 8H NUMBER ,7(4X,1H(,11,1H)),, 1X)
1968 3011 FORMAT (18,7I7)
1969 3012 FORMAT (//34H E L E M E N T L O A D C A S E      ,3X,
1970      1 21HM U L T I P L I E R S      ,//
1971      *      31X,6HCASE A,4X,6HCASE B,4X,6HCASE C,
1972      2 4X,6HCASE D,/1X)
1973 3013 FORMAT (
1974      1 27H X-DIRECTION GRAVITY =      ,4F10.2/
1975      2 27H Y-DIRECTION GRAVITY =      ,4F10.2/
1976      3 27H Z-DIRECTION GRAVITY =      ,4F10.2/
1977      4 27H THERMAL LOADING      =      ,4F10.2/
1978      5 27H PRESSURE LOADING      =      ,4F10.2 //1X)
1979 C
1980 4001 FORMAT (40HOERROR*** MATERIAL CARDS OUT OF ORDER.,/1X)
1981 4002 FORMAT (52HOERROR*** NUMBER OF TEMPERATURE CARDS EXCEEDS USER,
1982      1 10H MAXIMUM (,14,2H),, / 1X)
1983 4003 FORMAT (51HOERROR*** TEMPERATURES MUST BE INPUT IN ASCENDING
1984      1 7H ORDER., / 1X)
1985 4004 FORMAT (47HOERROR*** AXIS ORIENTATION CARD OUT OF ORDER.,1X)
1986 4005 FORMAT (30HOERROR*** UNDEFINED NODE NUMBER = ,15 / 1X)
1987 4006 FORMAT (38HOERROR*** VECTOR IJ HAS ZERO LENGTH.,/1X)
1988 4007 FORMAT (38HOERROR*** VECTOR IK HAS ZERO LENGTH.,/1X)
1989 4008 FORMAT (43HOERROR*** IJ AND IK VECTORS ARE PARALLEL.,1X)
1990 4009 FORMAT (43HOERROR*** IJ AND IJ VECTORS ARE PARALLEL.,1X)
1991 4010 FORMAT (50HOERROR*** SET NUMBERS MUST BE IN ASCENDING ORDER,1X)
1992 4011 FORMAT (40HOERROR*** INVALID SURFACE FACE NUMBER.,/1X)
1993 4012 FORMAT (30HOERROR*** INVALID LOAD TYPE.,/1X)
1994 4013 FORMAT (42HOERROR*** INVALID OUTPUT POINT NUMBER = ,15 / 1X)
1995 C
1996 C
1997      END
1998 C3=====

```

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1999      SUBROUTINE PRINTD (ID,D,B,NEQB,NUMNP,LL,NBLOCK,NEQ,NT,MQ)
2000      IMPLICIT REAL*8(A-H,O-Z)
2001 C
2002 C      CALLED BY: SOLEQ,SOLEIG,RESPEC
2003 C
2004      DIMENSION ID(NUMNP,6),B(NEQB,LL),D(6,LL)
2005      DATA Q11,Q21,Q12,Q22,Q13,Q23/' LOAD',' CASE',' EIGEN-',' VECTOR',
2006 1      ' MODE ',' NUMBER'/
2007 C
2008      REWIND 8
2009      READ (8) ID
2010      M=NEQ
2011      NN=NEQB*NBLOCK
2012 C
2013      IF (MQ.EQ.2) GO TO 50
2014      IF (MQ.EQ.3) GO TO 55
2015      REWIND NT
2016      Q1=Q11
2017      Q2=Q21
2018      GO TO 60
2019 50 Q1=Q12
2020      Q2=Q22
2021      GO TO 60
2022 55 Q1=Q13
2023      Q2=Q23
2024      REWIND NT
2025      READ (NT)
2026 60 continue
2027 c-- WRITE (33,2003) Q1,Q2      !removed as there is a print in SOLEQ
2028 C
2029      N=NUMNP
2030      rewind nt      !*****
2031 C
2032      DO 500 KK=1,NUMNP
2033 C
2034          I=6
2035          DO 250 II=1,6
2036              DO 100 L=1,LL
2037 100 D(I,L)=0.
2038              IF(M.GT.NN) GO TO 150
2039              IF (M.EQ.0) GO TO 150
2040              READ (NT) B
2041              NN=NN-NEQB
2042 150 IF(ID(N,I).LT.1) GO TO 250
2043              K=M-NN
2044              M=M-1
2045 C
2046              DO 200 L=1,LL
2047 200 D(I,L)=B(K,L)
2048 250 I=I-1
2049 C
2050 cc- WRITE (33,2004) N,(L,(D(I,L),I=1,6),L=1,LL)
2051 C
2052      500 N=N-1

```

```

2053 C
2054     RETURN
2055 C
2056 2003 FORMAT (1H1,38HNODE DISPLACEMENTS / ,
2057     1      17HROTATION S, // 3X,4HNODE,2X,A6,2(12X,2HX-,1X,
2058     2      2HY-,12X,2HZ-), / 7H NUMBER,2X,A6,3(3X,11HTRANSLATION),
2059     3      3(6X,9HROTATION), / 1X)
2060 2004 FORMAT (1G,18,6E14.5, : / (7X,18,6E14.5) )
2061 C-- 2004 FORMAT (1H0,16,18,6E14.5 / (7X,18,6E14.5) )
2062 C
2063     END
2064 C=====
2065     SUBROUTINE SOL31
2066 C
2067 C     CALLED BY : ELTYPE
2068 C     CALLS : STRES
2069 C
2070     IMPLICIT REAL*8(A-H,O-Z)
2071 C
2072 C     3 / D 8 TO 21 NODE SOLID ELEMENTS
2073 C
2074     COMMON /ELPAR/ NPAR(14),NUMNP,MBAND,NELTYP,N1,N2,N3,N4,N5,MTOT,REQ
2075     COMMON /EM/ NS,ND,LM(63)
2076     COMMON/JUNK/ LT,LH,L,N6,SIG(42),N7,N8,N9,N10,N11,N12,N13,N14,
2077     1      N15,N16,N17
2078     COMMON /EXTRA/ MODEX,NT8,N10SV,NT10
2079 C
2080     COMMON      A(1)
2081 C
2082 C
2083     IF(NPAR(1).EQ.0) GO TO 500
2084 C
2085 C     ERROR CHECKS AND SET DEFAULT VALUES IF REQUIRED
2086 C
2087     WRITE (33,1000)
2088     IF(NPAR(2).GT.0) GO TO 10
2089     WRITE (33,1001) (NPAR(K),K=1,10)
2090     WRITE (33,1002)
2091     STOP
2092 10 IF(NPAR(3).GT.0) GO TO 20
2093     WRITE (33,1001) (NPAR(K),K=1,10)
2094     WRITE (33,1003)
2095     STOP
2096 20 IF(NPAR(4).EQ.0) NPAR(4) = 1
2097     IF(NPAR(7).EQ.0) NPAR(7) = 21
2098     IF(NPAR(7).GE.8 .AND. NPAR(7).LE.21) GO TO 30
2099     WRITE (33,1001) (NPAR(K),K=1,10)
2100     WRITE (33,1004)
2101     STOP
2102 30 IF(NPAR(9).EQ.0) NPAR(9) = 2
2103     IF(NPAR(9).GE.2 .AND. NPAR(9).LE.4) GO TO 40
2104     WRITE (33,1001) (NPAR(K),K=1,10)
2105     WRITE (33,1005)
2106     STOP

```

```

2107 40 IF(NPAR(10).EQ.0) NPAR(10) = 2
2108 IF(NPAR(10).GE.2 .AND. NPAR(10).LE.4) GO TO 50
2109 WRITE (33,1001) (NPAR(K),K=1,10)
2110 WRITE (33,1005)
2111 STOP
2112 C
2113 C STORAGE ALLOCATION
2114 C
2115 C A(N6) = STARTING LOCATION OF WEIGHT DENSITY
2116 C A(N7) = STARTING LOCATION OF MASS DENSITY
2117 C A(N8) = STARTING LOCATION OF VECTOR CONTAINING THE ACTUAL
2118 C NUMBER OF TEMPERATURE POINTS FOR EACH MATERIAL TABLE
2119 C A(N9) = STARTING LOCATION OF MATERIAL PROPERTY TABLE
2120 C A(N10) = STARTING LOCATION OF DIRECTION COSINE ARRAYS FOR
2121 C MATERIAL ORIENTATION AXIS
2122 C A(N11) = STARTING LOCATION OF SURFACE LOAD FACE NUMBERS
2123 C A(N12) = STARTING LOCATION OF SURFACE LOAD CODE TYPES
2124 C A(N13) = STARTING LOCATION OF PRESSURE WORKING ARRAY
2125 C A(N14) = STARTING LOCATION OF OUTPUT REQUEST LOCATION SETS
2126 C A(N15) = STARTING LOCATION OF VECTOR CONTAINING THE ACTUAL
2127 C NUMBER OF REQUESTED OUTPUT LOCATION IN EACH OUTPUT SET
2128 C A(N16) = STARTING LOCATION OF ELEMENT STIFFNESS MATRIX
2129 C
2130 50 N6 = N5 + NUMNP
2131 N7 = N6 + NPAR(3)
2132 N8 = N7 + NPAR(3)
2133 N9 = N8 + NPAR(3)
2134 N10 = N9 + NPAR(3) * NPAR(4) * 13
2135 N11 = N10 + NPAR(5) * 9
2136 N12 = N11 + NPAR(6)
2137 N13 = N12 + NPAR(6)
2138 N14 = N13 + NPAR(6) * 7
2139 N15 = N14 + NPAR(8) * 7
2140 N16 = N15 + NPAR(8)
2141 N17 = N16 + NPAR(7) * 189
2142 C
2143 IF(N17.GT.MTOT) CALL ERROR(N17-MTOT)
2144 C
2145 C PROCESS ELEMENT DATA, AND GENERATE ELEMENT MATRICES
2146 C
2147 CALL THDFE (A(N1),A(N2),A(N3),A(N4),A(N5),A(N6),A(N7),A(N8),A(N9),
2148 1 A(N10),A(N11),A(N12),A(N13),A(N14),A(N15),A(N16),
2149 2 NPAR(2),NPAR(3),NPAR(4),NPAR(5),NPAR(6),NPAR(7),
2150 3 NPAR(8),NPAR(9),NPAR(10),NUMNP)
2151 C
2152 RETURN
2153 C
2154 C RECOVER ELEMENT STRESSES (STATIC CASES ONLY)
2155 C
2156 500 WRITE (34,2001)
2157 NUME = NPAR(2)
2158 C
2159 read (5,*) nll,neu
2160 501 format(3i5)

```

```

2161      if(n11.le.0) n11=1
2162      if(nuu.le.0) nuu=nuue
2163      DO 800 mm=1,NUME
2164 C
2165 C
2166 C*** STRESS PORTHOLE
2167      CALL STRSC (A(N1),A(N3),REQ,0)
2168      IF(N10SV.EQ.1)
2169      *WRITE (NT10) NS
2170 C***
2171 C
2172      IF(NS.EQ.1) GO TO 800
2173 C
2174 C-   WRITE (34,5000)
2175 C
2176      DO 700 L=LT,LH
2177 C
2178 C
2179      CALL STRSC (A(N1),A(N3),REQ,1)
2180      LOC = NS/6
2181      K1 = -5
2182 C
2183      DO 600 M=1,LOC
2184      K1 = K1 + 6
2185      K2 = K1 + 5
2186 C
2187      if(.and.(n11.and.mm.le.nuu)) then
2188      IF(N.EQ.1) WRITE (34,3001) mm,L,N,(SIG(I),I=K1,K2)
2189      IF(N.GT.1) WRITE (34,4001) N,(SIG(I),I=K1,K2)
2190      end if
2191 C
2192 C*** STRESS PORTHOLE
2193      IF(N10SV.EQ.1)
2194      *WRITE (NT10) mm,L,N,(SIG(I),I=K1,K2)
2195 C***
2196      600 CONTINUE
2197 C
2198 C-   WRITE (34,5000)
2199 C
2200      700 CONTINUE
2201      800 CONTINUE
2202      RETURN
2203 C
2204 C   FORMATS
2205 C
2206 1000 FORMAT (53H121 - N O D E S O L I D E L E M E N T I N P U T ,
2207      1 10HD A T A ,//38HCONTROL INFORMATION ,/1X)
2208 1001 FORMAT (48HOKKOR DETECTED WHILE PROCESSING MASTER ELEMENT ,
2209      1 12HCONTROL CARD, //16X,1H(,10IS,1H),/1X)
2210 1002 FORMAT (32H NO 3-D SOLID ELEMENTS SPECIFIED,/1X)
2211 1003 FORMAT (23H NO MATERIALS REQUESTED, / 1X)
2212 1004 FORMAT (49H MAXIMUM NUMBER OF NODES MUST BE GE.8 .AND. LE.21, /1X)
2213 1005 FORMAT (42H INTEGRATION ORDER MUST BE GE.2 .AND. LE.4,/1X)
2214 2001 FORMAT (54H121 - N O D E S O L I D E L E M E N T S T R E S S

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2215      * //
2216      *23H ELEMENT LOAD LOCATION,9X,6HSIG-XX,9X,6HSIG-YY,9X,6HSIG-ZZ,
2217      3 9X,6HSIG-XY,9X,6HSIG-YZ,9X,6HSIG-ZX, //1X)
2218      3001 FORMAT (I8,I6,I9,6E15.6)
2219      4001 FORMAT ( 14X,I9,6E15.6)
2220      5000 FORMAT ( / )
2221 C
2222      END
2223 C=====
2224      SUBROUTINE SSLAW (D,E,TEMP,DCA,KAXES,KMAT,NEL,DUM,ALPHA)
2225 C
2226 C      CALLED BY : THDFE
2227 C
2228      IMPLICIT REAL*8(A-H,O-Z)
2229 C
2230 C      THIS ROUTINE FORMS THE STRESS-STRAIN LAW IN MATERIAL COORDINATES
2231 C      (X1,X2,X3) AND TRANSFORMS THE MATERIAL SYSTEM LAW TO GLOBAL
2232 C      COORDINATES (X,Y,Z).
2233 C
2234      DIMENSION D(6,6),E(12),TEMP(6,6),DCA(3,3),IPRM(3),DUM(6,6),
2235      1          ALPHA(6)
2236 C
2237      DATA IPRM / 2,3,1 /
2238 C
2239 C      FORM THE DIRECT STRAIN PARTITION OF THE STRAIN-STRESS LAW IN
2240 C      MATERIAL COORDINATES (X1,X2,X3)
2241 C
2242      DO 20 I=1,3
2243      ALPHA(I) = E(I+9)
2244      ALPHA(I+3) = 0.0
2245      IF(E(I).GT.1.0E-08) GO TO 15
2246      WRITE (33,3000) I,I,KMAT,NEL
2247      STOP
2248      3000 FORMAT (23H0ERROR*** MODULUS E(,2I1,16H) FOR MATERIAL (,I3,
2249      1          14H) IN ELEMENT (,I5,10H) IS ZERO., / 1X)
2250      15 TEMP(I,I) = 1.0/E(I)
2251      20 CONTINUE
2252 C
2253      TEMP(1,2) = -E(4)* TEMP(2,2)
2254      TEMP(2,1) =      TEMP(1,2)
2255      TEMP(1,3) = -E(5)* TEMP(3,3)
2256      TEMP(3,1) =      TEMP(1,3)
2257      TEMP(2,3) = -E(6)* TEMP(3,3)
2258      TEMP(3,2) =      TEMP(2,3)
2259 C
2260 C      INVERT THE DIRECT STRAIN PARTITION
2261 C
2262      DO 60 N=1,3
2263      X = 1.0/TEMP(N,N)
2264      DO 30 J=1,3
2265      30 TEMP(N,J) = - TEMP(N,J)* X
2266 C
2267      DO 50 I=1,3
2268      IF(N.EQ.I) GO TO 50

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```

2269      DO 40 J=1,3
2270      IF(N.EQ.J) GO TO 40
2271      TEMP(I,J) = TEMP(I,J) + TEMP(I,N) * TEMP(N,J)
2272      40 CONTINUE
2273      50 TEMP(I,N) = TEMP(I,N) * X
2274      C
2275      TEMP(N,N) = X
2276      60 CONTINUE
2277      C
2278      C      FORM THE COMPLETE STRESS-STRAIN LAW IN MATERIAL COORDINATES
2279      C
2280      DO 70 I=1,6
2281      DO 70 J=1,6
2282      70 D(I,J) = 0.0
2283      C
2284      DO 80 I=1,3
2285      DO 80 J=1,3
2286      80 D(I,J) = TEMP(I,J)
2287      C
2288      D(4,4) = E(7)
2289      D(5,5) = E(9)
2290      D(6,6) = E(8)
2291      C
2292      C      TRANSFORM THE MATERIAL LAW TO GLOBAL COORDINATES (X,Y,Z)
2293      C
2294      IF(KAXES.LT.1) RETURN
2295      C
2296      C      TRANSFORMATION BETWEEN MATERIAL STRAINS AND GLOBAL STRAINS
2297      C
2298      DO 100 I1=1,3
2299      I2 = IPRM(I1)
2300      DO 90 J1 = 1,3
2301      J2 = IPRM(J1)
2302      TEMP(I1 ,J1 ) = DCA(J1,I1)*DCA(J1,I1)
2303      TEMP(I1+3,J1 ) = DCA(J1,I1)*DCA(J1,I2) * 2.0
2304      TEMP(I1 ,J1+3) = DCA(J1,I1)*DCA(J2,I1)
2305      TEMP(I1+3,J1+3) = DCA(J1,I1)*DCA(J2,I2) +
2306      1      DCA(J2,I1)*DCA(J1,I2)
2307      90 CONTINUE
2308      100 CONTINUE
2309      C
2310      C      ROTATE THE MATERIAL LAW TO THE GLOBAL SYSTEM
2311      C
2312      DO 130 I=1,6
2313      DO 120 J=1,6
2314      X = 0.0
2315      DO 110 K=1,6
2316      110 X = X + D(I,K)*TEMP(K,J)
2317      120 DUM(I,J) = X
2318      130 CONTINUE
2319      C
2320      DO 160 I=1,6
2321      DO 150 J=1,6
2322      X = 0.0

```

```

2323      DO 140 K=1,6
2324 140 X = X + TEMP(K,I)*ADUM(K,J)
2325      D(I,J) = X
2326      D(J,I) = X
2327 150 CONTINUE
2328 160 CONTINUE
2329 C
2330 C      TRANSFORM THE EXPANSION COEFFICIENTS FROM MATERIAL COORDINATES
2331 C      TO GLOBAL COORDINATES
2332 C
2333 C
2334      DO 200 I=1,6
2335      X = 0.0
2336      DO 190 K=1,3
2337 190 X = X + TEMP(K,I)*AE(K+9)
2338      IF(I.GT.3) X =X*2.0
2339 200 ALPHA(I) = X
2340 C
2341      RETURN
2342      END
2343 C=====
2344      SUBROUTINE STRESS(STR,B,D,NEQB,LB,LL,NEQ,NBLOCK)
2345      IMPLICIT REAL*8(A-H,O-Z)
2346 C
2347 C      CALLS: ELTYPE
2348 C      CALLED BY: SOLEQ
2349 C
2350      DIMENSION D(NEQ,LB),B(NEQB,LL),STR(4,LL)
2351      COMMON /ELPAR/ NPAR(14),NUMNP,MBAND,NELTYP,N1,N2,N3,N4,N5,MTOT,MEQ
2352      COMMON /JUNK/ LT,LH,IFILL(429)
2353      COMMON /EXTRA/ MODEX,NT8,NIOSV,NT10,IFILL2(12)
2354 C
2355      READ (8) STR
2356      NT=(LL-1)/LB +1
2357      LH=0
2358 C*** STRESS PORTHOLE
2359      IF(NIOSV.EQ.1)
2360      *WRITE (NT10) NELTYP,NT
2361 C
2362      DO 1000 I1=1,NT
2363 C
2364      LT =LH+1
2365      LLI=1-LT
2366      LH=LT+LB-1
2367      IF(LH.GT.LL) LH=LL
2368 C
2369 C      MOVE DISPLACEMENTS INTO CORE FOR LB LOAD CONDITIONS
2370 C
2371      REWIND 2
2372 C*** STRESS PORTHOLE
2373      IF(NIOSV.EQ.1)
2374      *WRITE (NT10) LT,LH
2375      NQ=NEQB*NBLOCK
2376      DO 200 NN=1,NBLOCK

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```

2377      READ (2) B
2378      N=NEQB
2379      IF (NN.EQ.1) N=NEQ-NQ+NEQB
2380      NQ=NQ-NEQB
2381      DO 200 J=1,N
2382      I=NQ+J
2383      DO 200 L=LT,LH
2384      K=L+LLT
2385      200 D(I,K)=B(J,L)
2386      LK=LH-LT+1
2387 C
2388 C      CALCULATE STRESSES FOR ALL ELEMENTS FOR LB LOAD CONDITIONS
2389 C
2390      REWIND 1
2391      DO 1000 M=1,NELTIP
2392      READ (1) NPAR
2393 C*** STRESS PORTHOLE
2394      IF(NIOSV.EQ.1)
2395      *WRITE (NTIO) NPAR
2396      MTYPE=NPAR*1
2397      NPAR(1)=0
2398      CALL ELTYPE(MTYPE)
2399      1000 CONTINUE
2400 C
2401      RETURN
2402      END
2403 C&=====
2404      SUBROUTINE STRSC(STR,D,NEQ,NTAG)
2405      IMPLICIT REAL*8(A-H,O-Z)
2406 C
2407 C      CALLED BY: TRUSS,BEAM,PLANE,THREED,SHELL,BOUND,PIPE
2408 C
2409      DIMENSION STR(4,1),D(NEQ,1)
2410      COMMON /JUNK/ LT,LH,L,IPAD,SG(20),SIG(7),EXTRA(186)
2411      COMMON /EM/ NS,ND,B(42,63),TI(42,4),LM(63)
2412 C
2413      IF (NTAG.EQ.0) GO TO 800
2414      LL=L-LT+1
2415      DO 300 I=1,NS
2416      SG(I)=0.0
2417      DO 300 J=1,4
2418      300 SG(I)=SG(I)+TI(I,J)*STR(J,L)
2419      DO 500 J=1,ND
2420      JJ=LM(J)
2421      IF(JJ.EQ.0) GO TO 500
2422      DO 400 I=1,NS
2423      400 SG(I)=SG(I)+B(I,J)*D(JJ,LL)
2424 C
2425      500 CONTINUE
2426      GO TO 900
2427      800 READ (1) ND,NS,(LM(I),I=1,ND),(( B(I,J),I=1,NS),J=1,ND),
2428      1 ((TI(I,J),I=1,NS),J=1,4)
2429      900 RETURN
2430      END

```

```

2431 C=====
2432 SUBROUTINE ST8R31 (E,B,S,XX,NOD9,H,P,SIGDT,DELT,FT,DL,XM,NEL,ND,
2433 1 IELD,IELX,KTL,KGL,KMS,NINT,NINTZ,WIDEN,MSDEN)
2434 C
2435 C CALLED BY : TH3EE
2436 C CALLS : DER3DS
2437 C
2438 IMPLICIT REAL*8(A-H,O-Z)
2439 C
2440 C
2441 C . . . . .
2442 C .
2443 C .
2444 C . HEXAHEDRAL CURVILINEAR THREE-DIMENSIONAL ELEMENTS
2445 C .
2446 C . ISOPARAMETRIC OR SUBPARAMETRIC
2447 C .
2448 C .
2449 C . . . . .
2450 C
2451 C
2452 C
2453 DIMENSION E(6,1),B(6,1),S(63,1),XX(3,1),NOD9(1),H(1),P(3,1),
2454 1 SIGDT(1),DELT(1),FT(1),DL(1),XM(1),D(9),SDT(6),BV(63),
2455 2 W(3,3),IPERM(3,3),KDX(3),LDX(3)
2456 C
2457 COMMON /GAUSS/ XG(4,4),WGT(4,4)
2458 C REAL MSDEN
2459 REAL*8 MSDEN
2460 C
2461 DATA IPERM / 1,4,6, 4,2,5, 6,5,3 /
2462 C
2463 VOL = 0.0
2464 C
2465 C DETERMINE IF THE MATERIAL IS ORTHOTROPIC (ISO.EQ.1, ISOTROPIC)
2466 C
2467 DUM = 0.0
2468 DO 20 I=4,6
2469 I = I-1
2470 DO 20 K=1,J
2471 20 DUM = DUM +DABS(E(K,I))
2472 ISO = 1
2473 IF(DUM.GT.1.0E-6) ISO = 0
2474 IF(ISO.EQ.0) GO TO 24
2475 DO 22 I=2,3
2476 DUM = DUM +DABS(E(I ,1 ) -E(I-1,I-1))
2477 22 DUM = DUM +DABS(E(I+3,I+3) -E(I+2,I+2))
2478 DUM = DUM +DABS(E(1 ,2 ) - E(2 ,3 ))
2479 DUM = DUM +DABS(E(2 ,3 ) - E(3 ,1 ))
2480 IF ( DUM.GT.1.0E-6 ) ISO=0
2481 24 CONTINUE
2482 C
2483 C
2484 C VOLUME INTEGRATION LOOP

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2485 C
2486 C
2487      DO 10 LX=1,NINT
2488      DO 10 LY=1,NINT
2489      E1=XG(LX,NINT)
2490      E2=XG(LY,NINT)
2491      DO 10 LZ=1,NINTZ
2492      E3=XG(LZ,NINTZ)
2493 C
2494      WT=WGT(LX,NINT)*WGT(LY,NINT)*WGT(LZ,NINTZ)
2495 C
2496 C      EVALUATE STRAIN-DISPLACEMENT MATRIX B AND JACOBIAN DETERMINANT
2497 C
2498      CALL DERBDS (NEL,XX,B,DET,E1,E2,E3,NOD9,H,P,IELD,IELX)
2499 C
2500      ADD CONTRIBUTION TO ELEMENT STIFFNESS
2501 C
2502      FACT = WT* DET
2503      FACT2 =DSQRT(FACT)
2504 C
2505      DO 25 I=1,IELD
2506      K3 = 3*I
2507      K2 = K3-1
2508      K1 = K2-1
2509      BV(K1) = B(1,K1)* FACT2
2510      BV(K2) = B(2,K2)* FACT2
2511      BV(K3) = B(3,K3)* FACT2
2512      25 CONTINUE
2513 C
2514      DO 30 I=1,ND
2515      DO 30 J=1,ND
2516      30 S(I,J) = S(I,J) + BV(I)* BV(J)
2517 C
2518 C      ACCUMULATE ELEMENT VOLUME
2519 C
2520      VOL = VOL + FACT
2521 C
2522 C      COMPUTE GRAVITY LOADS
2523 C
2524      IF(KGL.EQ.0) GO TO 150
2525      DO 130 K=1,IELD
2526      130 BL(K) = BL(K) + H(K)*FACT* WT*DEN
2527 C
2528 C      COMPUTE THERMAL LOADING NODE FORCE VECTOR
2529 C
2530      150 IF(KTL.EQ.0) GO TO 190
2531 C
2532 C      1. ELEMENT TEMPERATURE DIFFERENCE AT THIS INTEGRATION POINT
2533 C      (R,S,T)
2534 C
2535      DT = 0.0
2536      DO 160 R=1,IELD
2537      160 DT = DT + H(R)* DELT R)
2538      DT = DT* FACT

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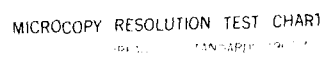
2539 C
2540 C      2. INITIAL STRESSES AT (R,S,T)
2541 C
2542      DO 170 K=1,6
2543 170 SDT(K) = SIGDT(K)*DT
2544 C
2545 C      3. NODE FORCES
2546 C
2547      DO 180 K=1,ND
2548      DO 175 I=1,6
2549 175 FT(K) = FT(K) + B(I,K)* SDT(I)
2550 180 CONTINUE
2551 C
2552 C      WRITE(28,*) ' DT,DELT',DT,(DELT(K),K=1,IELD)
2553 C      WRITE(28,*) ' SIGDT ',(SIGDT(K),K=1,6)
2554 C      WRITE(28,*) ' FT -----'
2555 C      WRITE(28,*) '(FT(K),K=1,6)
2556 190 CONTINUE
2557 10 CONTINUE
2558 C
2559      DO 35 I=1,2
2560      IC = ND-I
2561      DO 35 J=1,IC
2562      M=J+I
2563 35 S(M,J) = S(J,M)
2564 C
2565 C      COMPLETE THE K-MATRIX WITH APPROPRIATE MATERIAL CONSTANT MULTI-
2566 C      PPLICATIONS OF THE INTEGRATED B(I)*B(J) ARRAY.
2567 C
2568 C      1. TEST FOR MATERIAL TYPE
2569 C
2570      IF(ISO.EQ.0) GO TO 75
2571 C
2572 C      A. ISOTROPIC MATERIAL
2573 C
2574      D1 = E(1,1)
2575      D2 = E(1,2)
2576      D3 = E(4,4)
2577 C
2578      DO 60 I=1,IELD
2579      K3 = 3*I
2580      K2 = K3-1
2581      K1 = K2-1
2582      K0 = K1-1
2583      DO 60 J=1,IELD
2584      L3 = 3*J
2585      L2 = L3-1
2586      L1 = L2-1
2587      L0 = L1-1
2588 C
2589      IC = 0
2590      DO 40 II=1,3
2591      M = II+ K0
2592      DO 40 JJ=1,3

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2593      N = JJ+ L0
2594      IC = IC+ 1
2595      D(IC) = S(m,N)
2596      40 CONTINUE
2597 C
2598      S(K1,L1) = D(1)* D1 + (D(5) + D(9))* D3
2599      S(K2,L2) = D(5)* D1 + (D(1) + D(9))* D3
2600      S(K3,L3) = D(9)* D1 + (D(5) + D(1))* D3
2601      S(K1,L2) = D(2)* D2 + D(4)* D3
2602      S(K2,L1) = D(4)* D2 + D(2)* D3
2603      S(K2,L3) = D(6)* D2 + D(8)* D3
2604      S(K3,L2) = D(8)* D2 + D(6)* D3
2605      S(K3,L1) = D(7)* D2 + D(3)* D3
2606      S(K1,L3) = D(3)* D2 + D(7)* D3
2607 C
2608      60 CONTINUE
2609 C
2610      GO TO 110
2611 C
2612 C      B. ANISOTROPIC MATERIAL
2613 C
2614      75 DO 100 I=1, IELE
2615      K0 = 3*I-3
2616      DO 100 J=1, IELE
2617      L0 = 3*J-3
2618 C
2619      DO 80 II=1, 3
2620      M = II+K0
2621      DO 80 JJ=1, 3
2622      N = JJ+L0
2623      W(II,JJ) = S(M,N)
2624      80 CONTINUE
2625 C
2626      DO 100 K=1, 3
2627      I1 = K0+K
2628      DO 82 II=1, 3
2629      82 KDX(II)=IPEKM+II,K)
2630      DO 95 L=1, 3
2631      I2 = L0+L
2632      DO 83 II=1, 3
2633      83 LDX(II)=IPEKM+II,L)
2634 C
2635      SUM=0.0
2636 C
2637      DO 90 II=1, 3
2638      K1 = KDX(II)
2639      DO 85 JJ=1, 3
2640      K2 = LDX(JJ)
2641 C
2642      85 SUM = SUM + W(II,JJ)*E(K1,K2)
2643      90 CONTINUE
2644 C
2645      S(I1,I2) = SUM
2646 C

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2647     95 CONTINUE
2648    100 CONTINUE
2649    110 CONTINUE
2650 C
2651 C
2652 C     REFLECT FOR SYMMETRY
2653 C
2654     DO 200 I=1,ND
2655     DO 200 J=I,ND
2656     200 S(J,I) = S(I,J)
2657 C
2658 C     CONSTRUCT THE LUMPED MASS MATRIX
2659 C
2660     IF(KMS.EQ.0) RETURN
2661 C
2662     FACT = VOL* MSDEN/ IELD
2663     DO 220 K=1,ND
2664     220 XM(K) = FACT
2665 C
2666 C
2667     RETURN
2668     END
2669 C3=====
2670     SUBROUTINE THDFE (ID,X,Y,Z,T,DEN,RHO,NTP,EE,
2671     1     DCA,NFACE,LT,PWA,LOC,MAXPTS,SS,
2672     2     NUME,NUMMAT,MAXTP,NORTH0,NDLS,MAXNOD,
2673     3     NOPSET,INTRS,INTT,NUMNP)
2674 C
2675 C     CALLED BY : SOL21
2676 C     CALLS : INF21,CALBAN,SSLAW,DER3DS,STGR21,FACEPR
2677 C
2678     IMPLICIT REAL*8(A-H,O-Z)
2679 C
2680 C     ROUTINE FOR THE STIFFNESS, MASS AND STRESS MATRIX GENERATION
2681 C     FOR THE 8-TO-21 NODE ISO-(OR SUB)-PARAMETRIC ORTHOTROPIC
2682 C     HEXAHEDRON.
2683 C
2684     COMMON /JUNK/ XLF(4),YLF(4),ZLF(4),TLF(4),PLF(4),FILL1(22),V2(3),
2685     1     FILL2(12),LS(4),KLS(4),NOD(21),NOD9M(13),KOD(21),
2686     2     NREAD,TAG,E(12)
2687     COMMON /ELPAK/ IFILL3(15),MBAND
2688     COMMON /EM/ SDT(42,63),SE(42,4),NS,ND,LM(63)
2689     DIMENSION RE(63,4),XM(63),D(6,6),TEMP(6,6),DUM(6,6),
2690     *     ALPHA(6),XX(3,21),B(6,63),H(21),P(3,21),SIGDT(6),
2691     *     DELT(21),FT(63),BL(21),PL(63),LOCOP(7),VIS(6)
2692 C
2693     COMMON /GAUSS/ XG(4,4),WGT(4,4),STPTS(27,3)
2694     COMMON /DYN / IFILL4(11),NDYN
2695     COMMON /EXTRA/ MODEX,NT8
2696 C
2697     DIMENSION ID(NUMNP,1),X(1),Y(1),Z(1),T(1),DEN(1),RHO(1),
2698     1     NTP(1),EE(MAXTP,13,1),DCA(3,3,1),NFACE(1),LT(1),
2699     2     PWA(7,1),LOC(7,1),MAXPTS(1),SS(63,1)
2700 C

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2701 C      DATA T61, T62  /'A', 'A'
2702          STPTS(1,1)=1.
2703          STPTS(2,1)=-1.
2704          STPTS(3,1)=-1.
2705          STPTS(4,1)=1.
2706          STPTS(5,1)=1.
2707          STPTS(6,1)=-1.
2708          STPTS(7,1)=-1.
2709          STPTS(8,1)=1.
2710          STPTS(9,1)=0.
2711          STPTS(10,1)=-1.
2712          STPTS(11,1)=0.
2713          STPTS(12,1)=1.
2714          STPTS(13,1)=0.
2715          STPTS(14,1)=-1.
2716          STPTS(15,1)=0.
2717          STPTS(16,1)=1.
2718          STPTS(17,1)=1.
2719          STPTS(18,1)=-1.
2720          STPTS(19,1)=-1.
2721          STPTS(20,1)=1.
2722          STPTS(21,1)=0.
2723          STPTS(22,1)=1.
2724          STPTS(23,1)=-1.
2725          STPTS(24,1)=0.
2726          STPTS(25,1)=0.
2727          STPTS(26,1)=0.
2728          STPTS(27,1)=0.
2729          STPTS(1,2)=1.
2730          STPTS(2,2)=1.
2731          STPTS(3,2)=-1.
2732          STPTS(4,2)=-1.
2733          STPTS(5,2)=1.
2734          STPTS(6,2)=1.
2735          STPTS(7,2)=-1.
2736          STPTS(8,2)=-1.
2737          STPTS(9,2)=1.
2738          STPTS(10,2)=0.
2739          STPTS(11,2)=-1.
2740          STPTS(12,2)=0.
2741          STPTS(13,2)=1.
2742          STPTS(14,2)=0.
2743          STPTS(15,2)=-1.
2744          STPTS(16,2)=0.
2745          STPTS(17,2)=1.
2746          STPTS(18,2)=1.
2747          STPTS(19,2)=-1.
2748          STPTS(20,2)=-1.
2749          STPTS(21,2)=0.
2750          STPTS(22,2)=0.
2751          STPTS(23,2)=0.
2752          STPTS(24,2)=1.
2753          STPTS(25,2)=-1

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2755 STPTS(26,3)=0.
 2756 STPTS(27,3)=0.
 2757 STPTS(1,3)=1.
 2758 STPTS(2,3)=1.
 2759 STPTS(3,3)=1.
 2760 STPTS(4,3)=1.
 2761 STPTS(5,3)=-1.
 2762 STPTS(6,3)=-1.
 2763 STPTS(7,3)=-1.
 2764 STPTS(8,3)=-1.
 2765 STPTS(9,3)= 1.
 2766 STPTS(10,3)= 1.
 2767 STPTS(11,3)= 1.
 2768 STPTS(12,3)= 1.
 2769 STPTS(13,3)=-1.
 2770 STPTS(14,3)=-1.
 2771 STPTS(15,3)=-1.
 2772 STPTS(16,3)=-1.
 2773 STPTS(17,3)=0.
 2774 STPTS(18,3)=0.
 2775 STPTS(19,3)=0.
 2776 STPTS(20,3)=0.
 2777 STPTS(21,3)=0.
 2778 STPTS(22,3)=0.
 2779 STPTS(23,3)=0.
 2780 STPTS(24,3)=0.
 2781 STPTS(25,3)=0.
 2782 STPTS(26,3)=1.
 2783 STPTS(27,3)=-1.
 2784 XG(1,1) = 0.
 2785 XG(2,1) = 0.
 2786 XG(3,1) = 0.
 2787 XG(4,1) = 0.
 2788 XG(1,2) = -.5773502691896D0
 2789 XG(2,2) = .5773502691896D0
 2790 XG(3,2) = 0.
 2791 XG(4,2) = 0.
 2792 XG(1,3) = -.7745966692415D0
 2793 XG(2,3) = 0.
 2794 XG(3,3) = .7745966692415D0
 2795 XG(4,3) = 0.
 2796 XG(1,4) = -.8611363115941D0
 2797 XG(2,4) = -.3399810435849D0
 2798 XG(3,4) = .3399810435849D0
 2799 XG(4,4) = .8611363115941D0
 2800 WGT(1,1) = 2.0
 2801 WGT(2,1) = 0.0
 2802 WGT(3,1) = 0.0
 2803 WGT(4,1) = 0.0
 2804 WGT(1,2) = 1.0
 2805 WGT(2,2) = 1.0
 2806 WGT(3,2) = 0.0
 2807 WGT(4,2) = 0.0
 2808 WGT(1,3) = .55555555555556 D0

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2809      WGT(2,3) = .38888888888889      DO
2810      WGT(3,3) = .55555555555556      DO
2811      WGT(4,3) = 0.0
2812      WGT(1,4) = .3478548451375      DO
2813      WGT(2,4) = .6521451548625      DO
2814      WGT(3,4) = .6521451548625      DO
2815      WGT(4,4) = .3478548451375      DO
2816 C
2817      NTBSV = MODER
2818      DO 10 I=4,6
2819      DO 10 J=1,6
2820 10 B(I,J) = 0.0
2821      DO 14 I=1,42
2822      DO 14 J=1,4
2823 14 SF(I,J)=0.0
2824 C
2825 C      PRINT ELEMENT CONTROL VARIABLES
2826 C
2827      WRITE (33,3001) NOME,NUMMAT,MAXTP,NORTH0,NDLS,MAXNOD,NOPSET,INTRS,
2828 1      INTT
2829 C
2830 C      READ AND CHECK INPUT UP TO THE ELEMENT DATA CARDS
2831 C
2832      CALL INP21      (NUMMAT,MAXTP,NORTH0,NDLS,NOPSET,NTBSV,NUMNP.X,
2833 1      Y,2,BEN,RHO,NTP,EE,DCA,NFACE,LT,PWA,LOC,MAXPTS)
2834 C
2835 C      READ ELEMENT DATA CARDS
2836 C
2837      NREAD = 8
2838      IF(MAXNOD.GT.8) NREAD = 21
2839 C
2840      WRITE (33,3014) (I,I=1,8)
2841      IF(MAXNOD.GT.8)
2842 *WRITE (33,3016) (I,I=9,21)
2843 C
2844      NEL = 0
2845 C
2846 C      CARD FOR ELEMENT NUMBER ONE ONLY
2847 C
2848      READ (5,1008) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,RTINT
2849 1,IREUSE,(LS(I),I=1,4)
2850      READ (5,1009) (NOD(I),I=1,NREAD)
2851      IREUSE = 0
2852      IF(INEL.EQ.1) GO TO 51
2853      WRITE (33,4014) INEL
2854      WRITE (33,4014)
2855      STOP
2856 C
2857 C      CARDS FOR ALL OTHER ELEMENTS
2858 C
2859 50 READ (5,1008) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,RTINT
2860 1,IREUSE,(LS(I),I=1,4)
2861      READ (5,1009) (NOD(I),I=1,NREAD)
2862 C

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2863 C      DATA ADMISSIBILITY CHECK
2864 C
2865      51 IF(NDIS.EQ.0) NDIS = MAXNOD
2866          IF(NDIS.LE.MAXNOD) GO TO 5051
2867          WRITE (33,3015) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,NTINT
2868      1,IREUSE,(LS(I),I=1,4)
2869          WRITE (33,4015) NDIS,MAXNOD
2870          STOP
2871      5051 IF(NDIS.GE.8) GO TO 52
2872          WRITE (33,4023) NDIS
2873          STOP
2874      52 IF(NXYZ.EQ.0) NXYZ = NDIS
2875          IF(NXYZ.LE.NDIS) GO TO 5052
2876          WRITE (33,4016) NXYZ,NDIS
2877          WRITE (33,4099)
2878          MODEX = 1
2879          GO TO 53
2880      5052 IF(NXYZ.GE.8) GO TO 53
2881          WRITE (33,4024) NXYZ
2882          WRITE (33,4099)
2883          MODEX = 1
2884      53 IF(NMAT.GE.1 .AND. NMAT.LE.NUMMAT) GO TO 54
2885          WRITE (33,3015) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,NTINT
2886      1,IREUSE,(LS(I),I=1,4)
2887          WRITE (33,4017)
2888          WRITE (33,4099)
2889          MODEX = 1
2890      54 IF(MAXES.LE.NORTHO) GO TO 55
2891          WRITE (33,3015) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,NTINT
2892      1,IREUSE,(LS(I),I=1,4)
2893          WRITE (33,4018)
2894          WRITE (33,4099)
2895          MODEX = 1
2896      55 IF(IOP.GE.0 .AND. IOP.LE.NOPSET) GO TO 56
2897          WRITE (33,3015) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,NTINT
2898      1,IREUSE,(LS(I),I=1,4)
2899          WRITE (33,4019)
2900          WRITE (33,4099)
2901          MODEX = 1
2902      56 DO 57 I=1,4
2903          IF(LS(I).GE.0 .AND. LS(I).LE.NDLS) GO TO 57
2904          WRITE (33,3015) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,NTINT
2905      1,IREUSE,(LS(J),J=1,4)
2906          WRITE (33,4020) LS(I)
2907          WRITE (33,4099)
2908          MODEX = 1
2909      57 CONTINUE
2910 C
2911 C      DEFAULT VALUES IF REQUIRED
2912 C
2913      IF(KG.EQ.0) KG = 1
2914      IF(NRSINT.EQ.0) NRSINT = INTR5
2915      IF(NTINT.EQ.0) NTINT = INTT
2916 C

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2917      DO 58 I=1,8
2918      IF(NOD(I).GE.1 .AND. NOD(I).LE.NUMNP) GO TO 58
2919      WRITE (33,3015) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,NTINT
2920      1,IREUSE,(LS(J),J=1,4)
2921      WRITE (33,4021) I,NOD(I)
2922      STOP
2923 53 CONTINUE
2924      IF(MAXNOD.LT.9) GO TO 60
2925      II = 0
2926      DO 59 I=9,21
2927      IF(NOD(I).EQ.0) GO TO 59
2928      II = II + 1
2929      NOD9M(II) = I
2930      IF(NOD(I).LE.NUMNP) GO TO 59
2931      WRITE (33,3015) INEL,NDIS,NXYZ,NMAT,MAXES,IOP,TZ,KG,NRSINT,NTINT
2932      1,IREUSE,(LS(J),J=1,4)
2933      WRITE (33,4021) I,NOD(I)
2934      STOP
2935 59 CONTINUE
2936 C
2937      I = II + 8
2938      IF(I.EQ.NDIS) GO TO 60
2939      WRITE (33,4025) I,NDIS
2940      STOP
2941 C
2942 60 NEL = NEL + 1
2943      ML = INEL - NEL
2944      IF(ML) 65,70,80
2945 65 WRITE (33,4022) INEL
2946      STOP
2947 C
2948 C      SAVE THE DATA FOR ELEMENT NUMBER *INEL* FOR POSSIBLE USE IN
2949 C      DATA GENERATION
2950 C
2951 C
2952 70 KDIS = NDIS
2953      KXYZ = NXYZ
2954      KMAT = NMAT
2955      KAXES = MAXES
2956      KIOP = IOP
2957      TTZ = TZ
2958      KKG = KG
2959      KRSINT = NRSINT
2960      KTINT = NTINT
2961      KREUSE = IREUSE
2962      DO 72 I=1,4
2963 72 KLS(I) = LS(I)
2964      DO 74 I=1,NREAD
2965 74 KOD(I) = NOD(I)
2966      TAG = TGI
2967 C
2968      GO TO 90
2969 C
2970 C      INCREMENT THE NON-ZERO NODE NUMBERS FROM THE PRECEDING ELEMENT

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2863 C      DATA ADMISSIBILITY CHECK
2864 C
2865      51 IF (NDIS.EQ.0) NDIS = MAXNOD

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2971 C
2972      80 DO 85 I=1,NKRD
2973          IF(KOD(I).LT.1) GO TO 85
2974          KOD(I) = KOD(I) + KRK
2975      85 CONTINUE
2976          TAG = TG2
2977 C
2978      90 ND = 3 * NDIS
2979 C
2980 C      COMPUTE THE AVERAGE ELEMENT TEMPERATURE USING COORDINATE NODES
2981 C
2982          TAV = 0.0
2983          DO 95 K=1,KXYZ
2984              I = KOD(K)
2985      95 TAV = TAV + T(I)
2986          TAV = TAV / KXYZ
2987 C
2988 C      PERFORM TEMPERATURE INTERPOLATION FOR THE PROPERTY SET
2989 C
2990          NT = NIP(KMAT)
2991          IF(NT.GT.1) GO TO 100
2992      97 DO 98 I=1,12
2993          98 E(I) = EE(1,I+1,KMAT)
2994              GO TO 112
2995      100 IF(TAV.GE.EE(1,1,KMAT)) GO TO 104
2996      102 WRITE (33,4030) TAV,NEL,KMAT
2997          STOP
2998      104 IF(TAV.GT.EE(NT,1,KMAT)) GO TO 103
2999          IF(TAV.EQ.EE(1,1,KMAT)) GO TO 97
3000 C
3001          IF(MODEX.EQ.1) GO TO 112
3002 C
3003          DO 106 K=2,NT
3004              K2 = K
3005              K1 = K-1
3006              IF(TAV.GT.EE(K1,1,KMAT) .AND. TAV.LE.EE(K2,1,KMAT)) GO TO 108
3007      106 CONTINUE
3008      108 DT = EE(K2,1,KMAT) - EE(K1,1,KMAT)
3009          RATIO = (TAV - EE(K1,1,KMAT)) / DT
3010          DO 110 I=1,12
3011      110 E(I) = EE(K1,I+1,KMAT) + RATIO * (EE(K2,I+1,KMAT) - EE(K1,I+1,KMAT))
3012 C
3013      112 CONTINUE
3014 C
3015 C      FORM THE STRESS-STRAIN LAW IN MATERIAL COORDINATES AND TRANSFORM
3016 C      TO GLOBAL (X,Y,Z) COORDINATES
3017 C
3018          IF(MODEX.EQ.0)
3019      *CALL SSLAW (D,E,TEMP,DCA(1,1,KAXES),KAXES,KMAT,NEL,DUM,ALPHA)
3020 C
3021 C      STORE THE NODE COORDINATES FOR THIS ELEMENT
3022 C
3023          IF(MODEX.EQ.1) GO TO 410
3024 C

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3025      DO 130 I=1,KDIS
3026      II = KOD(I)
3027      IF(I.LT.9) GO TO 125
3028      JJ = NOD9M(I-8)
3029      II = KOD(JJ)
3030      125 XX(1,I) = X(II)
3031      XX(2,I) = Y(II)
3032      XX(3,I) = Z(II)
3033      130 CONTINUE
3034 C
3035 C      COMPUTE THE ELEMENT STIFFNESS, MASS, THERMAL AND GRAVITY LOAD
3036 C      MATRICES
3037 C
3038      DO 170 I=1,63
3039      DO 170 J=1,4
3040      170 RF(I,J)=0.0
3041 C
3042      IF(KREUSE.EQ.1) GO TO 300
3043 C
3044      DO 180 I=1,KDIS
3045      180 DL(I)=0.0
3046      DO 190 I=1,ND
3047 C
3048 C
3049 C      1. THERMAL LOADS
3050 C
3051      190 FT(I)=0.0
3052      KTL = 0
3053      DUX = 0.0
3054      DO 200 I=1,4
3055      200 DUX = DUX + DABS(TLF(I))
3056      IF(DUX.GT.1.0E-06) KTL = 1
3057      IF(KTL.EQ.1) THEN      !!!!!
3058      WRITE(99,*) ' $$$$ $$$$ ktl==1'      !!!!!
3059      END IF      !!!!!
3060      IF (NDYN.GT.0) KTL=0
3061      IF(KTL.EQ.0 .OR. NDYN.GT.0) GO TO 235
3062 C
3063 C      A. INITIAL STRESS CONSTANTS
3064 C
3065      DO 210 I=1,6
3066      SIGDT(I) = 0.0
3067      DO 205 K=1,6
3068      205 SIGDT(I) = SIGDT(I) + D(I,K)* ALPHA(K) ! 1 changed to I (first)
3069      210 CONTINUE
3070 C      WRITE(28,*) ' sigdt in THDFE'
3071 C      WRITE(28,*) (sigdt(k),k=1,6)
3072 C
3073 C      B. VECTOR OF NODE TEMPERATURE DIFFERENCES
3074 C
3075      DO 230 I=1,KDIS
3076      II = KOD(I)
3077      IF(I.LT.9) GO TO 220
3078      J = NOD9M(I-8)

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3079      II = KOD(J)
3080 220 DELT(I) = T(II) - TTZ
3081 230 CONTINUE
3082 C
3083 C      C. CLEAR THE THERMAL LOAD NODE FORCE VECTOR
3084 C
3085 C      2. GRAVITY LOADS
3086 C
3087 235 DUX=0.0
3088      DO 250 I=1,4
3089 250 DUX = DUX +DABS(XLF(I)) +DABS(YLF(I)) +DABS(ZLF(I))
3090      KGL = 0
3091      IF(DUX.GT.1.0E-6) KGL = 1
3092      IF (NDYN.GT.0) KGL=0
3093 C
3094 C
3095 C      3. MASS MATRIX
3096      KMS = 0
3097      IF(NDYN.GT.0) KMS = 1
3098 C
3099      DO 270 K=1,ND
3100 C
3101 C      4. STIFFNESS MATRIX
3102 C
3103 270 XM(K) = 0.0
3104      DO 280 I=1,ND
3105      DO 280 K=1,ND
3106 280 SS(I,K) = 0.0
3107 C
3108 C
3109      CALL ST8K21 (D,B,SS,XX,NOD9M,H,P,SIGDT,DELT,FT,DL,XM,NEL,ND,KDIS,
3110 1      KXYZ,KTL,KGL,KMS,KRSINT,KTINT,DEN(KMAT),RHO(KMAT))
3111 C
3112 C
3113 C      NODE FORCES DUE TO THERMAL DISTORTION
3114 C
3115 300 IF (KTL.EQ.0) GO TO 325
3116      DO 320 I=1,ND
3117      DO 310 K=1,4
3118 310 RF(I,K) = FT(I)* TLF(K)
3119 320 CONTINUE
3120 C
3121 C      NODE FORCES DUE TO STATIC ACCELERATIONS
3122 C
3123 C
3124 325 IF (KGL.EQ.0) GO TO 350
3125      DO 340 I=1,KDIS
3126      K3 = 3*I
3127      K2 = K3-1
3128      K1 = K2-1
3129      DO 330 L=1,4
3130      RF(K1,L) = RF(K1,L) + XLF(L)*DL(I)
3131      RF(K2,L) = RF(K2,L) + YLF(L)* DL(I)
3132 330 RF(K3,L) = RF(K3,L) + ZLF(L)* DL(I)

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3133 340 CONTINUE
3134 C
3135 C COMPUTE NODE FORCES DUE TO ELEMENT SURFACE LOADINGS
3136 C
3137 350 IF(NDLS.LT.1.OR.NDYN.LT.0) GO TO 405
3138 C
3139 DO 400 L=1,4
3140 IF(PLS(L).EQ.0.0) GO TO 400
3141 M = KLS(L)
3142 IF(M.LT.1) GO TO 400
3143 DO 360 K=1,ND
3144 C
3145 360 PL(K) = 0.0
3146 CALL FACEPR (NEL,NDIS,KX12,XX,NOD9M,H,P,PL,NFACE(M),LT,M)
3147 1 PWA(1,M),M
3148 C
3149 DO 370 I=1,ND
3150 C
3151 370 RE(I,L) = RE(I,L) + PL(I)* PLE(L)
3152 400 CONTINUE
3153 405 CONTINUE
3154 C
3155 C ASSIGN EQUATION NUMBERS TO THE ELEMENT DEGREES OF FREEDOM
3156 C
3157 410 K = -3
3158 DO 420 I=1,KDIS
3159 II = KOD(I)
3160 IF(I.LT.9) GO TO 415
3161 JJ = NOD9M(I-8)
3162 II = KOD(JJ)
3163 415 K = K+3
3164 DO 420 L=1,3
3165 M = K+L
3166 420 LMM(M) = ID(II,L)
3167 C
3168 IF(KIOP.GT.0) NS = GMAXPTS(KIOP)
3169 IF(KIOP.EQ.0) NS = 6
3170 IF (NDYN.GT.0) NS=42
3171 C
3172 C SAVE STIFFNESS AND LOAD MATRICES
3173 C
3174 CALL CALBAN (MBAND,NDIE,Lm,XM,SS,RE,ND,63,NS)
3175 C
3176 C COMPUTE STRESS RECOVERY MATRICES
3177 C
3178 IF (NDYN.LT.1) GO TO 425
3179 NOP=7
3180 DO 422 I=1,7
3181 422 LOCOP(I)=I + 20
3182 GO TO 450
3183 425 IF (KIOP.EQ.0) GO TO 440
3184 NOP = MAXPTS(KIOP)
3185 DO 430 I=1,NOP
3186 430 LOCOP(I) = LOC(I,KIOP)

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3187      GO TO 450
3188      440 NOP = 1
3189      LOCOP(1) = 21
3190 C
3191      450 IF(MODEX.EQ.1) GO TO 510
3192 C
3193 C      CONSIDER EACH OUTPUT LOCATION
3194 C
3195      DO 500 L=1,NOP
3196 C
3197      M= LOCOP(L)
3198      E1= STPTS(M,1)
3199      E2= STPTS(M,2)
3200      E3= STPTS(M,3)
3201 C
3202 C      COMPUTE THE STRAIN-DISPLACEMENT MATRIX AT THIS LOCATION
3203 C
3204      CALL DER3DS (NEL,XX,L,DET,E1,E2,E3,NOD9M,H,P,KDIS,KXYZ)
3205 C
3206      DO 470 I=1,6
3207      N= 6*(L-1)+I
3208      DO 465 J=1,ND
3209      Q = 0.0
3210      DO 460 K=1,6
3211      460 Q = Q + D(I,K)* B(K,J)
3212      465 SDT(N,J) = Q
3213      470 CONTINUE
3214 C
3215 C      FORM THE INITIAL STRESS CORRECTIONS DUE TO THERMAL LOADS
3216 C
3217      IF(KTL.EQ.0 .OR. NDYN.GT.0) GO TO 500
3218 C
3219 C
3220 C      1. TEMPERATURE DIFFERENCE AT THIS LOCATION
3221 C
3222      Q = 0.0
3223      DO 480 K=1,KDIS
3224 C
3225 C      2. VECTOR OF INITIAL STRESSES
3226 C
3227      480 Q = Q + H(K)* DELT(K)
3228      DO 485 K=1,6
3229      485 VIS(K) = -Q * SIGDT(K)
3230 C
3231      DO 490 I=1,6
3232      N = 6*(L-1)+I
3233 C
3234      DO 490 K=1,4
3235      490 SF(N,K) = VIS(I)* TLF(K)
3236 C
3237      500 CONTINUE
3238 C
3239 C      SAVE THE STRESS RECOVERY ARRAYS
3240 C

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3241 C
3242 510 CONTINUE
3243 C
3244 IF(MODEX.EQ.0)
3245 1WRITE (1) ND,NS,(LM(I),I=1,ND),((SDT(I,J),I=1,NS),J=1,ND),
3246 2 ((SF(I,J),I=1,NS),J=1,4)
3247 C
3248 C PRINT DATA FOR THE CURRENT ELEMENT
3249 C
3250 WRITE (33,3015) NEL,KDIS,KXYZ,KMAT,KAXES,KIOP,TTZ,KRS,KRSINT,KRTINT,
3251 1 KREUSE,KLS
3252 WRITE (33,3017) (KOD(I),I=1,NREAD)
3253 C
3254 C*** DATA PORTHOLE SAVE
3255 IF(NTSSV.EQ.1)
3256 1WRITE (NTS) NEL,KDIS,KXYZ,KMAT,KAXES,KIOP,TTZ, KRSINT,KRTINT,
3257 2 KREUSE,KLS,NREAD,
3258 3 (KOD(I),I=1,NREAD)
3259 C***
3260 C
3261 C CHECK FOR THE LAST ELEMENT
3262 C
3263 IF(NUMB-NEL) 65,600,530
3264 530 IF(ML) 50,50,60
3265 C
3266 600 RETURN
3267 C
3268 C FORMATS
3269 C
3270 1008 FORMAT (G15,F10.0,4I5,4I2)
3271 1009 FORMAT (16I5)
3272 C
3273 3001 FORMAT ( 7X,34HNUMBER OF 21-NODE ELEMENTS = 16//
3274 1 7X,34HNUMBER OF MATERIAL SETS = 16//
3275 2 7X,26HMAXIMUM NUMBER OF MATERIAL, /
3276 3 7X,34HTEMPERATURE INPUT POINTS = 16 //
3277 4 7X,19HNUMBER OF MATERIAL, /
3278 5 7X,34HAXIS ORIENTATION SETS = 16//
3279 * 7X,34HNUMBER OF DISTRIBUTED LOAD SETS = 16//
3280 6 7X,34HMAXIMUM NUMBER OF ELEMENT NODES = 16 //
3281 7 7X,34HNUMBER OF STRESS OUTPUT SETS = 16 //
3282 8 7X,34HR,S COORDINATE INTEGRATION ORDER = 16 //
3283 9 7X,34HT COORDINATE INTEGRATION ORDER =16 // 16
3284 3014 FORMAT (52H13 / D 3 T O 2 1 N O D E S O L I D E L E ,
3285 1 18H M E N T D A T A , // 8H ELEMENT 2(2X,5HNODES),2(2X,
3286 2 5HMAIL.),2X,6HSTRESS,4X,6HSTRESS,2X,4HNODE,2(2X,5HGAUSS-,2X,
3287 3 2HK-,5X,3HLSA,3X,3HLSB,3X,3HLSC,3X,3HLSO, /
3288 4 8H NUMBER,7H -NDIS-,7H -NXYZ-,2X,5HTABLE,3X,4HAXES,2X,6HOUTPUT,
3289 5 6X,4HFREE,2X,4HINC.,2(3X,4HPTS.),2X,6HMATRIX,2X,4(2X,4H-OR-), /
3290 6 26X,3HNO.,4X,3HSET,5X,3HSET,5X,5HTEMP.,2X,4H-KG-,2X,5H-K-,5-,4X,
3291 7 3H-T-,2X,6HRE-USE,2X, 8(2X,2HN-,I2) )
3292 3015 FORMAT (18,4I7,18,F10.1,10,2I7,18,2X,4I6)
3293 3016 FORMAT (84X,8(2X,2HN-,I2),: / 84X,5(2X,2HN-,I2) )
3294 3017 FORMAT (84X,8I6,: / 84X,8I6,: / 84X,5I6)

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3295 C
3296 4014 FORMAT (33HOERROR*** ENCOUNTED ELEMENT (,15,13H), BUT EXPECT,
3297 1 21H TO READ ELEMENT ONE., / 1X)
3298 4015 FORMAT (42HOERROR*** NUMBER OF DISPLACEMENT NODES (,15,4H) IS,
3299 1 30H LARGER THAN MAXIMUM ALLOWED (,15,2H)., / 1X)
3300 4016 FORMAT (40HOERROR*** NUMBER OF COORDINATE NODES (,15,6H) MUST,
3301 1 39H BE .LE. NUMBER OF DISPLACEMENT NODES (,15,2H).
3302 4017 FORMAT (36HOERROR*** ILLEGAL MATERIAL NUMBER. )
3303 4018 FORMAT (44HOERROR*** ILLEGAL MATERIAL AXIS REFERENCE. )
3304 4019 FORMAT (41HOERROR*** ILLEGAL OUTPUT SET REFERENCE. )
3305 4020 FORMAT (41HOERROR*** PRESSURE LOAD SET REFERENCE (,15,4H) IS,
3306 1 9H ILLEGAL. )
3307 4021 FORMAT (16HOERROR*** THE ,12,18H-TH ELEMENT NODE (,15,4H) IS,
3308 1 9H ILLEGAL., / 1X)
3309 4022 FORMAT (28HOERROR*** ELEMENT NUMBER (,15,11H) IS OUT OF,
3310 1 10H SEQUENCE., / 1X)
3311 4023 FORMAT (42HOERROR*** NUMBER OF DISPLACEMENT NODES (,15,
3312 1 25H) MUST BE AT LEAST EIGHT. )
3313 4024 FORMAT (40HOERROR*** NUMBER OF COORDINATE NODES (,15,
3314 1 25H) MUST BE AT LEAST EIGHT. )
3315 4025 FORMAT (38HOERROR*** NUMBER OF NON-ZERO NODES (,13,6H) READ,
3316 1 50H DOES NOT EQUAL THE NUMBER OF DISPLACEMENT NODES (,
3317 2 13,2H)., / 1X)
3318 4030 FORMAT (33HOERROR*** AVERAGE TEMPERATURE (,F10.2,5H) FOR,
3319 1 10H ELEMENT (,15,29H) OUT OF RANGE FOR MATERIAL (,13,
3320 2 2H)., / 1X)
3321 4099 FORMAT (12X,31HPROCEED IN DATA CHECK ONLY MODE, / 1X)
3322 C
3323 END
3324 C=====
3325 SUBROUTINE VECTR2 (V,XI,YI,ZI,XJ,YJ,ZJ,IERR)
3326 C
3327 C CALLED BY : INP21
3328 C
3329 C IMPLICIT REAL*8(A-H,O-Z)
3330 C
3331 C THIS ROUTINE FORMS A UNIT LENGTH VECTOR AVA FROM POINT AIA
3332 C TO POINT AJA IN X,Y,Z SPACE
3333 C
3334 C DIMENSION V(3)
3335 C
3336 C IERR = 1
3337 C X = XJ - XI
3338 C Y = YJ - YI
3339 C Z = ZJ - ZI
3340 C XLN =DSQRT(X*X+Y*Y+Z*Z)
3341 C IF(XLN.LE.1.0E-08) RETURN
3342 C XLN = 1.0 / XLN
3343 C IERR = 0
3344 C V(3) = Z * XLN
3345 C V(2) = Y * XLN
3346 C V(1) = X * XLN
3347 C RETURN
3348 C END

```

```

3349 C1=====
3350      SUBROUTINE STIME
3351      TS=0.0
3352      RETURN
3353      END
3354 C
3355 C
3356 C1=====
3357 C      SUBROUTINE TTIME
3358 C          T - CUMULATIVE TASK TIME, RETURNED IN UNITS OF SECONDS
3359 C      SUBROUTINE TTIME(T)
3360      INTEGER*4 get_time, time
3361      DATA get_time /2/
3362      CALL LIB$STAT TIMER,get_time,time/
3363      T = time / 100.0
3364      RETURN
3365      END
3366 C1=====
3367
3368      SUBROUTINE SOLEQ
3369      IMPLICIT REAL*8(A-H,O-Z)
3370 C
3371 C      CALLS:  SESOL,PRINTD,STRESS
3372 C      CALLED BY:  MAIN
3373 C
3374 C      STATIC SOLUTION PHASE
3375 C
3376      COMMON A(1)          I      ?????????
3377      COMMON /ELPAR/ NP(14),NUMNP,MBAND,NELTYP,N1,N2,N3,N4,N5,MTOT,NEQ
3378      COMMON /SOL  / NBLOCK,NEQB,LL,NE,IFILL(7)
3379      DIMENSION ID(3000,6),B(40000),NX(3,200)
3380      COMMON/CRK,NCKRD,ICR(9000)
3381      DATA NCKRD=4/          I      ?????????
3382
3383      !xxxx MIN. DIMENSION = ( nb + 6*mpair) : also check in SESOL routine
3384      DIMENSION TMAT(600,600)
3385      COMMON/TMT/TMAT(600,600),TCOL(600),TCOL2(600),TCOLM(600),
3386      .IST(600),K(9000)
3387
3388      dimension nbc(200),ifix(200),dsave(200),npe(2,200),hcol,
3389      . FORCNB(3,200),FORC1(3,200),FORC2(3,200),ENERG(3)
3390
3391 C
3392      REAL*4 TTS4,ttsub(10)
3393      INTR= 100          ! INTERMEDIATE PRINTING
3394
3395      rewind 5
3396      read (8) ((idn,i,n=1,numnp),i=1,6)      ! degrees of freedom
3397 c      WRITE(23,*)      *** id = array in soleq *****
3398 c      do n=1,numnp
3399 c          WRITE(23,1028) (idn,i),i=1,6)
3400 c      end do
3401 1028      format (21,2015)
3402

```

```

3403 *** TO read data regarding
3404 c nodes where forces to be found whose disp. are specified (NB)
3405 c & double nodes along the crack propagation (NPAIR)
3406 read (5,*), nb ! no of boundary nodes where forces are to found
3407 IF(INTPR.LE.2) WRITE(33,*) ' nb = ',nb
3408 c -- input node, degree of freedom, displacement
3409 do ij=1,nb
3410 read(5,*) nbc(ij),ifix(ij),dsave(ij)
3411 ! if(INTPR.LE.2)
3412 WRITE(33,*) nbc(ij),ifix(ij),dsave(ij)
3413 end do
3414 ncrkd=0
3415 if(INTPR.LE.2) WRITE(33,*) 'node,ix,ndof,ncrkd,icr(ndof),ist(ncrkd)'
3416 if(INTPR.LE.2) WRITE(33,*) 'ij', (id(ij,ix), ix=1,6), ij=1,169
3417
3418 do ib=1,nb
3419 node=nbc(ib)
3420 ix=ifix(ib)
3421 ndof=id(node,ix)
3422 ncrkd=ncrkd+1
3423 if(INTPR.LE.2)
3424 . WRITE(33,1029),node,ix,ndof,ncrkd
3425 icr(ndof)=ncrkd
3426 ist(ncrkd)=-ncrkd
3427 if(INTPR.LE.2)
3428 . WRITE(33,1029),node,ix,ndof,ncrkd,icr(ndof),ist(ncrkd)
3429 end do
3430 1029 format (2x,5i8,5i8)
3431 if(INTPR.LE.1) WRITE(33,1028) (ICR(IJ), IJ=1,NEQ)
3432 if (INTPR.LE.2) WRITE(33,1028) IST
3433 1030 FORMAT (2x,20F6.3)
3434
3435
3436 read (5,*), npair ! no of double nodes & the double nodes
3437 ! if(INTPR.LE.2)
3438 WRITE(33,*) ' npair = ',npair
3439 do ipair=1,npair
3440 read(5,*) (npc(1,ipair), i=1,2), (NX(IJ,ipair), IJ=1,3)
3441 ! if(INTPR.LE.2)
3442 WRITE(33,*) (npc(1,ipair), i=1,2), (nx(ij,ipair), ij=1,3)
3443 do ix=1,2
3444 node=npc(ix,ipair)
3445 do iy=1,3 ! d.o.f. at each node
3446 ndof=id(node,iy)
3447 if(ndof.ne.0.and.icr(ndof).eq.0.and.nx(iy,ipair).eq.1) then
3448 ncrkd=ncrkd+1
3449 icr(ndof)=ncrkd
3450 ist(ncrkd)=ncrkd
3451 end if
3452 end do
3453 end do
3454 end do
3455
3456

```

```

3457 C
3458 C      SOLVE FOR THE DISPLACEMENT VECTORS
3459 C
3460      CALL TTIME(TT(1))
3461
3462 C-      N1=1
3463 C-      NCRKD - TOTAL NO. OF ADDITIONAL COLUMNS
3464      LL=1+NCRKD
3465
3466      NSB0=(MBAND+1)*ANEQB
3467      NSB=(MBAND+LL)*ANEQB
3468 C      N3=NSB+1
3469      N3=N3+LL*ANEQB
3470      NSBB=NEQB*LL*(2+MBAND-1)/NEQB
3471      IF(NSBB.LT.NSB) NSBB=NSB
3472      N4=N3+NSBB
3473      MI = MBAND + NEQB -1
3474      IF(INTPR.LE.2)
3475      .WRITE(33,*) LL, MBAND, NSB, N3, NSBB, N4, MI, N1, N2, N3
3476      IF(INTPR.LE.2)
3477      .WRITE(33,10301) LL,MBAND,NSB,N3,NSBB,N4,MI ,N1,N2,N3
3478 10301      format(2x,12i8)
3479      rewind 15
3480      rewind 4
3481      do ij=1,nblock
3482      read (4) (A(IK),IK=1,NSB0)
3483      WRITE(15) (A(IJK),IJK=1,NSB0)
3484      end do
3485      CALL SESOL (A(N1),A(N3),A(N4),LL,NBLOCK,NEQB,NSB,MI,4,3,2,55)
3486      CALL TTIME(TT(2))
3487      NL=2
3488      NL1=18
3489      NWV=LL*ANEQB
3490      REWIND NL
3491      REWIND NL1
3492      DO NJ=1,NBLOCK
3493      READ (NL) (A(IJ),IJ=1,NWV)
3494      WRITE(NL1) (A(IJ),IJ=1,NWV)
3495      END DO
3496
3497      WRITE(16) TMAT,tool
3498      do ipair=1,npair
3499      node1=npoc(1,ipair)
3500      node2=npoc(2,ipair)
3501      do idf=1,6
3502      ndof1=id(node1,idf)
3503      ndof2=id(node2,idf)
3504      if(ndof1.ne.0.and.ndof2.ne.0) then
3505      icr1=icr(ndof1)
3506      icr2=icr(ndof2)
3507      if(ist(icr1).gt.0.and.ist(icr2).gt.0) ist(icr2)=ist(icr1)
3508      end if
3509      end do
3510      end do

```

```

3511
3512
3513       write (33,*), ' ---          NODES          ----          D.O.F. RELEASED ----'
3514       do itr=1,100
3515       call ttime(ttsub(1))
3516       read (5,*) ip1,ip2,ix
3517       write (33,*) ip1,ip2,ix
3518       if(ip1.eq.9999.and.ip2.eq.9999) go to 1995
3519       do while (ip1.ne.0)
3520       if (ip2.eq.0) then
3521         node=ip1
3522         ndof=id(node,ix)
3523         if(ndof.ne.0) then
3524           icr1=icr(ndof)
3525           ist(icr1)=icr1
3526         end if
3527       else
3528         node1=ip1
3529         node2=ip2
3530         idf=ix
3531         ndof1=id(node1,idf)
3532         ndof2=id(node2,idf)
3533         if(ndof1.ne.0.and.ndof2.ne.0) then
3534           icr1=icr(ndof1)
3535           icr2=icr(ndof2)
3536           ist(icr2)=icr2
3537         end if
3538       end if
3539       read (5,*) ip1,ip2,ix
3540       write (33,*) ip1,ip2,ix
3541       end do
3542
3543       if(INTPR.LE.2) then
3544       WRITE(33,*) ' ----- IST -----'
3545       WRITE(33,1028) (ist(ij),ij=1,nrkd)
3546       end if
3547
3548       REWIND 16
3549       READ (16) TMAT,tcol
3550
3551       do i=1,neg
3552       icri=icr(i)
3553       isti=ist(icri)
3554       if(icri.le.nb.and.isti.gt.0) tcol(icri)=tcol(icri)+tcol2(icri)
3555       end do
3556
3557       do i=1,nrkd
3558       isti=ist(i)
3559       do j=1,nrkd
3560       istj=ist(j)
3561       if(isti.gt.0.and.istj.le.0) tcol(i)=tcol(i)-TMAT(i,j)*dsave(j) 'disp
3562       if(isti.le.0.or.istj.le.0) TMAT(i,j)=0.
3563       if(isti.le.0.and.istj.le.0.AND.isti.eq.istj) TMAT(i,j)=1.
3564       end do

```

```

3565      end do
3566      IF (INTPR.LE.1) WRITE(33,*)
3567      ' ---- TMAT,TCOL after IST manipulation-----'
3568      DO I=1,NCRKD
3569      IF (INTPR.LE.1) WRITE(33,1091) (TMAT(I,J),J=1,NCRKD),TCOL(I)
3570      END DO
3571      do i=1,norkd
3572      tcolm(i)=0.
3573      do j=1,norkd
3574      tmatm(i,j)=0.
3575      end do
3576      end do
3577      do i=1,norkd
3578      ist=iabs(ist(i))
3579      tcolm(ist)=tcolm(ist)+tcol(i)
3580      do j=1,norkd
3581      istj=iabs(ist(j))
3582      tmatm(ist,istj)=tmatm(ist,istj)+TMAT(i,j)
3583      end do
3584      end do
3585      do i=1,norkd
3586      if(tmatm(i,i).eq.0) tmatm(i,i)=1.0
3587      end do
3588      IF (INTPR.LE.1) WRITE(33,*) ' ---- TMATm,TCOLm Before matin-----'
3589      DO I=1,NCRKD
3590      IF (INTPR.LE.1) WRITE(33,1091) (TMATm(I,J),J=1,NCRKD),TCOLm(I)
3591      end do
3592
3593      call ttime(ttsub(2))
3594
3595      call MATIN(tmatm,Norkd,tcolm,1,DETERM)
3596
3597      call ttime(ttsub(3))
3598      IF (INTPR.LE.1) WRITE(33,*) ' ---- TMATm,TCOLm after matin-----'
3599      DO I=1,NCRKD
3600      IF (INTPR.LE.1) WRITE(33,1091) (TMATm(I,J),J=1,NCRKD),TCOLm(I)
3601      end do
3602      do i=1,norkd
3603      ist=iabs(ist(i))
3604      tcol(i)=tcolm(ist)
3605      if(ist(i).le.0) tcol(i)=dsave(i)      ! disp
3606      end do
3607      NWV=LL*NEQB
3608      Rewind nll
3609      do nj=1,nblock-1
3610      read (nll) (c(ij),ij=1,nwv)
3611      end do
3612      do nj=1,nblock
3613      nconst=(nj-1)*neqb
3614      read (nll) (c(ij),ij=1,nwv)
3615      backspace nll
3616      backspace nll
3617      do i=1,neqb
3618      b(i)=nconst+c(i)

```

```

3619      do k=1,norkd
3620      nk=neqb+(k-1)*neqb+1
3621      b(1+nconst)=b(1+nconst)-a(nk)*tcol(k)
3622      end do
3623      end do
3624
3625      IF(INTPR.LE.1) WRITE(33,*) ' --- intermediate solution ---'
3626      IF(INTPR.LE.1) WRITE(33,1091) (b(1j),1j=1,neq)
3627
3628      end do 1nj
3629      do i=1,neq
3630      1ci=1cr(i)
3631      if(1ci.ne.0) b(1)=tcol(1ci)
3632      end do
3633 c      WRITE(33,*) ' --- final displacement solution ---'
3634 c      WRITE(33,1091) (b(1j),1j=1,neq)
3635      REWIND NL
3636      DO NJ=NBLOCK,1,-1
3637      NCONST=(NJ-1)*NEQB+1
3638      NU=NCONST+NEQB-1
3639      WRITE(NL) (B(IJ),IJ=NCONST,NU)
3640      END DO
3641
3642      call ttime(ttsub(4))
3643
3644      do i=1,neq
3645      r(i)=0.
3646      end do
3647      rewind 15
3648      do nj=1,nblock
3649      DO IJK=NSBO,NSB
3650      A(IJK)=0.0
3651      END DO
3652      read (15) (a(IJK),IJK=1,NSBO)
3653      nconst=(nj-1)*neqb
3654      1j=0
3655      J1=1+NCONST
3656      do i=1,neqb
3657      1j=1j+1
3658      1n=1+nconst
3659      r(1n)=r(1n)+a(1j)*b(1n)
3660      end do
3661      do j=2,M8AND
3662      do i=1,neqb
3663      1n=1+nconst
3664      jn=j+nconst+1-1
3665      1j=1j+1
3666      r(1n)=r(1n)+a(1j)*b(jn)
3667      r(jn)=r(jn)+a(1j)*b(1n)
3668      end do
3669      end do
3670      end do
3671 c      WRITE(33,*) ' ----- r vector -----'
3672 c      WRITE(33,1091) (r(1j),1j=1,neq)

```

```

3673
3674 C
3675 C      Correction for thermal case  --
3676 c      To find the mechanical loads subtract the thermal loads
3677 c      from R(x)
3678 c      ( It is assumed no external loads are applied at double nodes)
3679      Rewind 15
3680      nsbl=neqb*mband
3681      do nj=1,nblock
3682      read (15) (a(ij),ij=1,nb6)
3683      nconst=(nj-1)*neqb
3684      do ij=1,neqb
3685      r(ij+nconst)=r(ij+nconst)-a(nsbl+ij)
3686      end do
3687      end do
3688      N=NUMNP
3689      ITR1=ITR-1
3690      WRITE(33,582) ITR1
3691 582  FORMAT (1H1, ' ##### STEP # ',I4, ' #####'/1X,40(1H_))
3692
3693      WRITE(33,X) ' #####'
3694      WRITE(33,X) ' ---- NODAL DISPLACEMENTS AND FORCES IN SOLID ----'
3695      WRITE(33,X) ' #####'
3696 CC  WRITE(33,X) ' (no mech. loads at the double nodes)'
3697      WRITE(33,20034)
3698      WRITE(33,20035)
3699 20034 FORMAT(/2X,75(1H-))
3700
3701      NAUX=1
3702      DO 500 N=1,NUMNP
3703      IFLAG=0
3704      DO 250 I=1,3
3705      D(I)=0.
3706      D(I+3)=0.0
3707 150 IF (ID(N,I).LT.1) GO TO 250
3708      IDNI=ID(N,I)
3709      IF (1cr(IDNI).NE.0) IFLAG=1
3710 200 D(I)=R(NAUX)
3711      D(I+3)=R(NAUX)
3712      NAUX=NAUX+1
3713 250 continue
3714 C
3715 C      IF (INTFR.LE.3.AND.IFLAG.EQ.0) GO TO 500
3716      WRITE (33,2004) N,(D(I),I=1,6)
3717 C
3718 500 CONTINUE
3719
3720 1091 format (2X,6G12.5)
3721 2004 FORMAT(2X,15,6G12.5)
3722      WRITE(33,20034)
3723 20035 FORMAT(2X, ' NODE',3X,'U',11X,'V',11X,'W',10X,'F1',10X,'F2',
3724      . 10X,'F3'/2X,75(1H-))
3725
3726      IF(ITR.NE.1) THEN

```

```

3727      ENERG(1)=0.0
3728      ENERG(2)=0.0
3729      ENERG(3)=0.0
3730      DO IPAIR=1,NPAIR
3731      NP1=NPC(1,IPAIR)
3732      NP2=NPC(2,IPAIR)
3733      DO IDG=1,3
3734      ND1=ID(NP1,IDG)
3735      ND2=ID(NP2,IDG)
3736 C-    WRITE(33,*) 'NP1,NP2,ND1,ND2',NP1,NP2,ND1,ND2
3737 C-    WRITE(33,*) 'FORC1,B s',FORC1(IDG,IPAIR),B(ND1),B(ND2)
3738      naux=nx(idg,ipair)
3739      IF(ND1.GT.0.AND.ND2.GT.0.and.naux.ne.0)
3740      . ENERG(IDG)=ENERG(IDG)-(FORC2(IDG,IPAIR)*B(ND2)+
3741      . FORC1(IDG,IPAIR)*B(ND1))*0.50
3742      END DO
3743      END DO
3744
3745
3746      DO IB=1,NB
3747      NPO=NBC(IB)
3748 C-    DO IDG=1,3
3749      idg=ifix(ib)
3750      NDO=ID(NPO,IDG)
3751      IF(NDO.GT.0) ENERG(IDG)=ENERG(IDG)-FORCNB(IDG,IB)*B(NDO)*0.50
3752 C-    END DO
3753      END DO
3754
3755      WRITE(34,*)
3756      . ' ----- ENERGY RELEASED in ( x, y, z ) directions -----'
3757      WRITE(34,1048) ENERG
3758      IF (ITR.EQ.2) WRITE(19,1049)
3759      WRITE(19,1048) ENERG
3760 1048  format (5X,3(g15.8,3X))
3761 1049  format (5X,7X,'X',18X,'Y',18X,'Z')
3762      WRITE(34,*) ' ====='
3763      END IF
3764      DO IPAIR=1,NPAIR
3765      NP1=NPC(1,IPAIR)
3766      NP2=NPC(2,IPAIR)
3767      DO IDG=1,3
3768      ND1=ID(NP1,IDG)
3769      ND2=ID(NP2,IDG)
3770      FORC1(IDG,IPAIR)=R(ND1)
3771      FORC2(IDG,IPAIR)=R(ND2)
3772 C-    WRITE(33,*) 'NP1,NP2,ND1,ND2', NP1,NP2,ND1,ND2
3773 C-    WRITE(33,*) ' IDG,IPAIR, FORC1,FORC2',IDG,IPAIR,FORC1(IDG,IPAIR),
3774 C-    . FORC2(IDG,IPAIR)
3775      END DO
3776      END DO
3777
3778      DO IB=1,NB
3779      NPO=NBC(IB)
3780      DO IDG=1,3

```

```

3781      NDO=ID(NPO, IDG)
3782      FORCNB(IDG, IB)=K(NDO)
3783      END DO
3784      END DO
3785
3786      call ttime(ttsub(5))
3787
3788 C      PRINT DISPLACEMENTS
3789 C
3790      N2=N1+NUMNPA6
3791      N3=N2+6*LL
3792
3793      LL1=1          !***** REASSIGNED *****
3794
3795      CALL PRINTD (A(N1), A(N2), A(N3), NEQB, NUMNP, LL1, NBLOCK, NEQ, 2.1)
3796      CALL TTIME=TT(3)
3797 C
3798 C      COMPUTE AND PRINT ELEMENT STRESSES
3799 C
3800      N2=N1+4*LL1
3801      N3=N2+NEQB*LL1
3802      LB=(MTGT-N3)/(NEQ+12)
3803      CALL STRESS(A(N1), A(N2), A(N3), NEQB, LB, LL1, NEQ, NBLOCK)
3804
3805 C      COMPUTE TIME LOG FOR THE DOUBLE NODES SOLUTION PHASE
3806 C
3807      DO K=1, 4
3808          ttsub(K) = ttsub(K+1)-ttsub(K)
3809      end do
3810      IF (INTPR.LE.3) WRITE (35,1985) (ttsub(L), L=1,4)
3811 1985      format(5X, ' time for tmatn formation          =', f8.2, /
3812      .          5X, ' time for matin                    =', f8.2, /
3813      .          5X, ' time to find global disp.         =', f8.2, /
3814      .          5X, ' time to find global nodal forces =', f8.2//)
3815 C
3816      end do      !itr
3817 1995      continue
3818      CALL TTIME=TT(4)
3819 C
3820 C      COMPUTE TIME LOG FOR THE STATIC SOLUTION PHASE
3821 C
3822      DO 50 K=1, 3
3823 50      TT(K) = TT(K+1)-TT(K)
3824      WRITE (34,2000) (TT(L), L=1,3)
3825 C
3826 2000 FORMAT (//, 48H S T A T I C      S O L U T I O N      T I M E      L O G,
3827      1          5X, 31HEQUATION SOLUTION          =, f8.2 /
3828      2          5X, 31HDISPLACEMENT OUTPUT =, f8.2 /
3829      3          5X, 31HSTRESS RECOVERY          =, f8.2 //)
3830 C
3831 C      RETURN
3832      RETURN
3833      END
3834 C!=====

```

```

3835
3836      SUBROUTINE SESOL
3837      .(A,B,MAXA,NV,NBLOCK,NEQB,NAV,MI,NSTIF,NRED,NL,NR)
3838      IMPLICIT REAL*8(A-H,O-Z)
3839
3840      real*4 tt(10)
3841 C      CALLED BY: SOLEQ
3842
3843      COMMON /ELPAK/ NF(14),NUMNP,MA,NELTYP,NZ1,NZ2,NZ3,NZ4,N5,NTOT,NEQ
3844      COMMON/CRK/NCRKD,ICK(9000)          ! NEQ
3845 c-      DATA ICR/0,1,2,0,0,3,0,4/      !change CVT,CVT2 line also
3846 c-      DATA IST/-1,-2,-3,-4/          ! IST(NCRKD)
3847 c-      DATA DISP/2.0,3.0,2.0,2.0,6*0.0/ ! DISP(NEQ)
3848      COMMON/TMT/TMT(600,600),TCOL(600),TCOL2(600),TCOLM(600),
3849      .IST(600),R(9000)
3850      DIMENSION A(NAV),B(NAV),MAXA(MI)
3851      call ttime(tt(1))
3852      INTPR=100
3853      if (INTPR.LE.2) WRITE(33,*) 'NV,NBLOCK,NEQB,NAV,MI,NSTIF,NRED,NL,NR'
3854      if (INTPR.LE.2) WRITE(33,1029),NV,NBLOCK,NEQB,NAV,MI,NSTIF,NRED,NL,NR
3855 1029      format(2X,12I8)
3856
3857      if (INTPR.LE.2) WRITE(33,1028) (ICK(IJ),IJ=1,NEQ)
3858      if (INTPR.LE.2) WRITE(33,1028) (IST(IJK),IJK=1,NCRKD)
3859 1028      FORMAT( 2X,20I4)
3860      if (INTPR.LE.2) WRITE(33,1030) DISP
3861 1030      FORMAT (2X,20F6.3)
3862      MM=1
3863      MA2=MA - 2
3864      IF(MA2.EQ.0) MA2=1
3865      INC=NEQB - 1
3866      NWA=NEQB*MA
3867      NTB=(MA-2)/NEQB + 1
3868      NEB=NTB*NEQB
3869      NEBT=NEB + NEQB
3870      NWV=NEQB*NV
3871      NWVV=NEBT*NV
3872
3873      N1=NL
3874      N2=NR
3875      if (INTPR.LE.2)
3876      .WRITE(33,*) ' mm,ma2,inc,neqb,nwa,ntb,neb,nebt,nwv,nwvv'
3877      if (INTPR.LE.2)
3878      .WRITE(33, 1029), mm,ma2,inc,neqb,nwa,ntb,neb,nebt,nwv,nwvv
3879      REWIND NSTIF
3880      REWIND NRED
3881      REWIND N1
3882      REWIND N2
3883
3884      if (INTPR.LE.2) WRITE(33,*) ' NAV =',NAV          !***
3885      DO IJ=1,NAV          !***
3886      A(IJ)=0.          !***
3887      B(IJ)=0.          !***
3888      END DO          !***

```

```

3889
3890
3891 *** Taking the npar coeffs. out and placing in a vertical matrix
3892 C- NCRKD - TOTAL NO. OF ADDITIONAL COLUMNS
3893 IF (INTPR.LE.1) WRITE(33,*) ' IB,JB,I,J,ICI,ICJ,IJ,IICI,JICI,A(IJ)'
3894 NO=NAV-NCRKD*NEQB
3895 DO I=1,MI
3896 DO J=1,NCRKD
3897 IJ=(J-1)*MI
3898 B(IJ)=0.
3899 END DO
3900 END DO
3901 DO NJ=1,NBLOCK
3902
3903 DO J=1,NCRKD
3904 IJ=(J-1)*MI
3905 INJ=NEQB+(J-1)*MI
3906 DO I=1,MA-1
3907 IJ=IJ+1
3908 INJ=INJ+1
3909 B(IJ)=B(INJ)
3910 END DO
3911 END DO
3912
3913 do j=1,ncrkd
3914 ij=ma+(j-1)*mi
3915 do i=ma,mi
3916 b(ij)=0.
3917 ij=ij+1
3918 end do
3919 end do
3920
3921
3922 READ (UNIT1) (A(IJ),IJ=1,NO)
3923 NCONST=NEQB*(NJ-1)
3924 DO IB=1,NEQB
3925 DO JB=1,MA
3926 IJ=(JB-1)*NEQB+IB
3927 I=IB+NCONST
3928 J=JB+NCONST+IB-1
3929 IF (I.LE.NEQ.AND.J.LE.NEQ) THEN
3930 ICI=ICR(I)
3931 ICJ=ICR(J)
3932 J2=JB+IB-1
3933 J2ICI=J2+(ICI-1)*MI
3934 IBICJ=IB+(ICJ-1)*MI
3935 IF (ICI.NE.0) B(J2ICI)=A(IJ)
3936 IF (ICJ.NE.0) B(IBICJ)=A(IJ)
3937
3938 IF (ICI.NE.0.AND.ICJ.NE.0) THEN
3939 IMAT(ICI,ICJ)=A(IJ)
3940 IMAT(ICJ,ICI)=A(IJ)
3941 J2ICI=J2+(ICI-1)*MI
3942 B(J2ICI)=0.

```

```

3943      IBICJ=IB+(ICJ-1)*MI
3944      B(IBICJ)=0.
3945      END IF
3946
3947      IF (INTPR.LE.1) WRITE(33,1018) IB,JB,I,J,ICI,ICJ,IJ,IICJ,JICI,A(IJ)
3948 1018    FORMAT(2X,9I5,F10.3)
3949      IF (ICJ.NE.0.OR.ICI.NE.0) THEN
3950          A(IJ)=0.
3951          IF (I.EQ.J) A(IJ)=1.
3952      END IF
3953      END IF
3954      END DO
3955      END DO
3956
3957      NIJ=NEQB*MI
3958      DO I=1,NEQB
3959          NIJ=NIJ+1
3960          ICI=ICR(1+NEQB*I)
3961          IF (ICI.NE.0) THEN
3962              ICI=ICI+1
3963              A(NIJ)=0.
3964          END IF
3965      END DO
3966
3967      NIJ=NO
3968      DO J=1,NCRKD
3969          DO I=1,NEQB
3970              NIJ=NIJ+1
3971              IJ=I+(J-1)*MI
3972              A(NIJ)=B(IJ)
3973          END DO
3974      END DO
3975
3976 c-      WRITE(33,*) 'NJ=',NJ, ' Reordered A -MATRIX'
3977      IF (INTPR.LE.1) THEN
3978          DO I=1,NEQB
3979              IA=(NJ-1)*NEQB+I
3980              WRITE(33,1019) (A(IJ),IJ=I,NAV,NEQB)
3981 1019    FORMAT(2X,1F9.2)
3982          END DO
3983      END IF
3984
3985      WRITE(N1) A
3986
3987      END DO    NJ = 1 to p
3988
3989      REWIND N1
3990      REWIND NSTIF
3991      DO NJ=1,NBLOCK
3992          READ (N1) A
3993          WRITE(NSTIF, A)
3994      END DO
3995
3996

```

```

3997       IF (INTPR.LE.1) then
3998       WRITE(33,X)      ---- Tmat -----
3999       DO I=1,NCRKD
4000       WRITE(33,1019) (Tmat(I,J),J=1,NCRKD),TCOL:I)
4001       END DO
4002       end if
4003
4004
4005 C--      STOP
4006
4007 call time(0.02)
4008 **** main loop over all blocks
4009 99      REWIND NTIF
4010
4011      DO 600 NI=1,NBLOCK
4012
4013 C--      WRITE(33,X)      ----- MAIN LOOP ----- NJ = ,NJ      ....
4014
4015       IF (NI.NE.1) GO TO 10
4016       READ (NTIF) (A(IJ),IJ=1,NAV)
4017       IF (INTPR.LE.1) WRITE(33,X) 'NJ=',NJ,'      A - MATRIX'
4018       IF (INTPR.LE.1) WRITE(33,1020) (A(IJ),IJ=1,NAV)
4019 1020  FORMAT(2X,11G11.4)
4020       IF (NEQ.GT.1) GO TO 100
4021       MAXA(1)=1
4022       WRITE(NRCD) A,MAXA
4023       IF (A(1) 1,174,3)
4024       1 KK=1
4025 C--       IF (INTPR.LE.1) WRITE(33,1010) KK,A(1)
4026       3 DO 5 L=1,NV
4027       5 A(1+L)=A(1+L),A(1)
4028       KK=1+NV
4029       WRITE(NL) (A(KK),KK=2,KN)
4030       RETURN
4031 10      IF (NTB.EQ.1) GO TO 100
4032       REWIND N1
4033       REWIND N2
4034       READ (N1) (A(IJ),IJ=1,NAV)
4035       IF (INTPR.LE.1) WRITE(33,X) 'NJ=',NJ,'      A - MATR'
4036       IF (INTPR.LE.1) WRITE(33,1020) (A(IJ),IJ=1,NAV)
4037
4038 100      continue
4039       IF (INTPR.LE.1) WRITE(33,X) ' BEFORE FINDING COLUMN HEIGHTS
4040
4041       IMX=NEQB*(MA+NV)
4042       IF (INTPR.LE.1) WRITE(33,X) 'NEQB,MA,NV,IMX',NEQB,MA,NV,Im-
4043       DO I=1,NEQB
4044       IF (INTPR.LE.1) WRITE(33,1020) (A(IJ),IJ=1,IMX,NEQB)
4045       Imx=Imx+1
4046       END DO
4047 **** Find column height
4048       KU=1
4049       Im=MIN0(MA,NEQB)
4050       MAXA(1)=1

```

```

4051      DO 110 N=2,M1
4052      IF (N.LE.MA) THEN          ****
4053      KU=KU + NEQB
4054      KK=KU
4055      MM=MINO(N,KM)
4056      ELSE                          ****
4057      KU=KU + 1
4058      KK=KU
4059      IF (N.LE.NEQB) GO TO 140
4060      MM=MM - 1
4061      END IF                        ****
4062 140      DO 160 K=1,MM
4063      IF (A(KK)) 110,160,110
4064 160      KK=KK - INC
4065 110      MAXA(N)=KK
4066
4067      IF (A(1)) 172,174,176
4068 174      KK=(NJ-1)*NEQB + 1
4069      IF (KK.GT.NEQ) GO TO 590
4070      IF (INTPR.LE.1) WRITE (33,1000) KK
4071      STOP
4072 172      KK=(NJ-1)*NEQB + 1
4073 C--      IF (INTPR.LE.1) WRITE (33,1010) KK,A(1)
4074
4075
4076 ****      Factorize leading block
4077
4078 176      DO 200 N=2,NEQB
4079      NH=MAXA(N)
4080      IF (NH-N) 200,200,210
4081 210      KL=N + INC
4082      K=N
4083      D=0.
4084      DO 220 KK=KL,NH,INC
4085      K=K - 1
4086      AKK=A(KK)
4087      C=AKK/A(K)
4088      D=D + C*AKK
4089 220      A(KK)=C
4090      A(N)=A(N) - D
4091
4092      IF (A(N)) 222,224,230
4093 224      KK=(NJ-1)*NEQB + N
4094      IF (KK.GT.NEQ) GO TO 590
4095      IF (INTPR.LE.1) WRITE (33,1000) KK
4096      STOP
4097 222      KK=(NJ-1)*NEQB + N
4098 C--      IF (INTPR.LE.1) WRITE (33,1010) KK,A(N)
4099
4100 230      IC=NEQB
4101      DO 240 J=1,MA2
4102      MJ=MAXA(N+J) - IC
4103      IF (MJ.LE.N) GO TO 240          ****
4104      KU=MINO(MJ,NH)                ****

```

```

4105      KN=N + IC
4106      C=0.
4107      DO 300 KK=KL,KU,INC
4108 300    C=C + A(KK)*A(KK+IC)
4109      A(KN)=A(KN) - C
4110 240    IC=IC + NEQB
4111
4112      K=N + NWA
4113      DO 450 L=1,NV
4114      KJ=K
4115      C=0.
4116      DO 440 KK=KL,NH,INC
4117      KJ=KJ - 1
4118 440    C=C + A(KK)*A(KJ)
4119      A(K)=A(K) - C
4120 450    K=K + NEQB
4121
4122 200    CONTINUE
4123      IF (INTPR.LE.1) WRITE(33,A) ' -- AFTER FACTORIZING LEADING BLOCK
4124      IMX=NEQB*(MA+NV)
4125      DO I=1,NEQB
4126 C-    IF (INTPR.LE.1) WRITE(33,1020) (A(IJ),IJ=I,IMX,NEQB)
4127      IMX=IMX+1
4128      END DO
4129 1205    FORMAT(2X,10G9.2)
4130
4131
4132 ***** Carry over into trailing blocks
4133
4134      DO 400 NK=1,NTB
4135      IF (INTPR.LE.1) WRITE(33,X) 'NJ,NK ',NJ,NK,'      B- MAT'
4136      IF ((NK+NJ).GT.NBLOCK) GO TO 400
4137      NI=NI
4138      IF ((NJ.EQ.1).OR.(NK.EQ.NTB)) NI=NSTIF
4139      READ (NI) (B(IJ),IJ=1,NAV)
4140
4141
4142 C-    IF (INTPR.LE.1) WRITE(33,1020) (B(IJ),IJ=1,NAV)
4143      ML=NK*NEQB + 1
4144      MR=MIN0((NK+1)*NEQB,MI)
4145      IF (MA.EQ.1) ML=MR
4146      MD=MI - ML
4147      KL=NEQB + (NK-1)*NEQB*NEQB
4148      N=1
4149
4150      DO 500 M=ML,MR
4151      NH=MAXA(M)
4152      KL=KL + NEQB
4153      IF (NH.LT.KL) GO TO 505      !AAA
4154      K=NEQB
4155      D=0.
4156      DO 520 KK=KL,NH,INC
4157      C=A(KK)*A(K)
4158      D=D + C*A(KK)

```

```

4159      A(KK)=C
4160  520    K=K - 1
4161      B(N)=B(N) - B
4162      IF (MD.LE.0) GO TO 530      (***
4163      IC=NEQB
4164      DO 540 J=1,MD
4165      MJ=MAXA(M+J) - IC
4166      IF (MJ.LT.KL) GO TO 540      (***
4167      KU=MINO(MJ,NH)
4168      KN=N + IC
4169      C=0.
4170      DO 575 KK=KL,KU,INC
4171  575    C=C + A(KK)*A(KK+IC)
4172      B(KN)=B(KN) - C
4173  540    IC=IC + NEQB
4174
4175  580    KN=N + NWA
4176      K=NEQB + NWA
4177      DO 610 L=1,NV
4178      KJ=K
4179      C=0.
4180      DO 620 KK=KL,NH,INC
4181      C=C + A(KK)*A(KJ)
4182  620    KJ=KJ - 1
4183      B(KN)=B(KN) - C
4184      KN=KN + NEQB
4185  610    K=K + NEQB
4186
4187  505    MD=MD - 1
4188  500    N=N + 1
4189
4190      IF (NTB.NE.1) GO TO 560
4191      WRITE (NRED) A,MAXA
4192      DO 570 I=1,NAN
4193  570    A(I)=B(I)
4194      GO TO 600
4195  560    WRITE (N2) J
4196
4197  400    CONTINUE
4198
4199      M=N1
4200      N1=N2
4201      N2=M
4202  590    WRITE (NRED) A,MAXA
4203
4204  600    CONTINUE
4205
4206      call ttime(tt(5))
4207
4208  ****    Vector back substitution
4209
4210      DO 700 K=1,NWVV
4211  700    B(K)=0.
4212      REWIND NL

```

```

4213
4214      DO 800 NJ=1,NBLOCK
4215      BACKSPACE NRCD
4216
4217      READ (NRCD) (A(IJ),IJ=1,NAV),(MAXA(IJ),IJ=1,MI)
4218 c--    WRITE(33,X) ' Vector back sub. NJ=',NJ,'      A= MAT'
4219
4220 c-    IF (INTPR.LE.1) WRITE(33,1020) (A(IJ),IJ=1,NAV)
4221      BACKSPACE NRCD
4222      K=NEBT
4223      DO 810 L=1,NV
4224      DO 820 I=1,NEB
4225      B(K)=B(K)-NEQB
4226 820    K=K - 1
4227 310    K=K + NEBT + NEB
4228      KN=0
4229      KK=NWA
4230      NDIF=NEQB
4231      IF (NJ.EQ.1) NDIF=NEQB - (NBLOCK*NEQB - NEQ)
4232      DO 855 L=1,NV
4233      DO 850 K=1,NDIF
4234 850    B(KN+K)=A(KK+K).A(K)
4235      KK=KK + NEQB
4236 855    KN=KN + NEBT
4237      IF (MA.EQ.1) GO TO 915
4238      ML=NEQB + 1
4239      KL=NEQB
4240      DO 860 M=ML,MI
4241      KL=KL + NEQB
4242      KU=MAXA(M)
4243      IF (KU-KL) 860,870,870
4244 870    K=NEQB
4245      KM=M
4246      DO 880 L=1,NV
4247      KJ=K
4248      DO 890 KK=KL,KU,INC
4249      B(KJ)=B(KJ) - A(KK)*B(KM)
4250 890    KJ=KJ - 1
4251      KM=KM + NEBT
4252 880    K=K + NEBT
4253 860    CONTINUE
4254      N=NEQB
4255      DO 910 I=2,NEQB
4256      KL=N + INC
4257      KU=MAXA(N)
4258      IF (KU-KL) 910,920,920
4259 920    K=N
4260      DO 930 L=1,NV
4261      KJ=K
4262      DO 940 KK=KL,KU,INC
4263      KJ=KJ - 1
4264 940    B(KJ)=B(KJ) - A(KK)*B(K)
4265 930    K=K + NEBT
4266 910    N=N + 1

```

```

4267
4268      915 KK=0
4269          KN=0
4270          DO 950 L=1,NV
4271              DO 960 K=1,NEUB
4272                  KK=KK + 1
4273      960      A(KK)=B(KN+K)
4274      950      KN=KN + NEBT
4275
4276          WRITE (NL) (A(K),K=1,NWV)
4277          IF (INTPR.LE.1) WRITE(33,*) ' Solution --'
4278          IF (INTPR.LE.1) WRITE (33,1020) (A(K),K=1,NWV)
4279      300      CONTINUE
4280
4281          call ttime:tt(4))
4282
4283      ****      To find y - vector
4284          do i=1,nckrd
4285              tcol2(i)=tcol(i)
4286              if(ist(i).le.0) tcol(i)=0.
4287          end do
4288          rewind nstif
4289          backspace nl
4290          nc2=neqb*ma
4291          do nj=1,nblock
4292
4293              read (nstif) a
4294              nij=no
4295
4296
4297
4298              read (nl) (B(ij),ij=1,nwv)
4299              backspace nl
4300              backspace nl
4301              nij=neqb*(nv-nckrd)
4302
4303
4304              NIJ10=NO
4305              NIJ20=NEQB*(NV-NCKRD)
4306
4307              do J=1,nckrd
4308                  do I=1,j
4309                      tauX=0.0
4310                      NIJO=NIJ10+(I-1)*NEQB
4311                      NIJ=NIJ20+(J-1)*NEQB
4312                      do k=1,neqb
4313                          NIJO=NIJO+1
4314                          NIJ=NIJ+1
4315                          tauX=tauX-A(NIJO)*B(NIJ)
4316                      end do
4317                      tmat(i,j)=tmat(i,j)+tauX
4318                  end do
4319              end do
4320              do j=1,nckrd

```

```

4321      do i=1,j-1
4322      tmat(j,i)=tmat(i,j)
4323      end do
4324      end do
4325
4326      do i=1,nckrd
4327      tau=0.0
4328      NIJO=NIJ10+(i-1)*NEQB
4329      do k=1,nega
4330      NIJO=NIJO+1
4331      tau=tau+A(NIJO)*B(k)
4332      end do
4333      tcold(i)=tcold(i)+tau
4334      end do
4335      IF(INTR.LE.1) then
4336      WRITE(33,*) '---- TMAT,TCOL towards end ----'
4337      DO I=1,NCKRD
4338      WRITE(33,1091) (Tmat(I,J),J=1,NCKRD),TCOL(I)
4339 1091      format (3x,8gl2.5)
4340      END DO
4341      end if
4342
4343      end do i=1,nj
4344      call ttime(tt(5))
4345      if(INTR.LE.2) WRITE(33,*) ' TIME LOG IN SESOL -----'
4346      do k=1,4
4347      tt(k)=tt(k+1)-tt(k)
4348      end do
4349      if(INTR.LE.2) WRITE(33,995) (tt(i),i=1,4)
4350 995      format(10x, Time to form matrix for double nodes etc.=,f8.2,
4351      .      /10x, Time to decompose A - matrix =,f8.2,
4352      .      /10x, Time for vector back substitution =,f8.2,
4353      .      /10x, Time to form TMAT =,f8.2)
4354 1000 FORMAT (/ 40H STOP *** ZERO DIAGONAL ENCOUNTERED DURING,
4355      1      18H EQUATION SOLUTION, /
4356      2      13x,18H EQUATION NUMBER =, I6 )
4357 1010 FORMAT (/ 50H WARNING *** NEGATIVE DIAGONAL ENCOUNTERED DURING,
4358      1      18H EQUATION SOLUTION, /
4359      2      13x,18H EQUATION NUMBER =, I6, 5x, THVALUE =, E20.5 )
4360
4361      RETURN
4362      END
4363 C3=====
4364      SUBROUTINE MATIN(A,N,S,m,DETERM)
4365      IMPLICIT REAL*8 (A-H,O-Z)
4366      DIMENSION A(600,600),B(600,1),IPIVOT(600),INDEX(600,2),BT(600)
4367      EQUIVALENCE (IROW,JROW),(ICOL,JCOL),(AMAX,T,SWAP)
4368      DETERM=1.0
4369      do 20 I=1,N
4370 20      IPIVOT(I)=0.0
4371      do 550 I=1,N
4372      AMAX=0.0
4373
4374      do 105 J=1,N

```

```

4375      IF(IPIVOT(J)-1) 60,105,60
4376 60    do 100 K=1,N
4377      IF(IPIVOT(K)-1) 30,100,740
4378 30    IF(AMAX -DABS(A(J,K))) 25,100,100
4379 35    IROW=J
4380      ICOLUM=K
4381      AMAX=DABS(A(J,K))
4382 100    CONTINUE
4383 105    CONTINUE
4384
4385      IPIVOT(ICOLUM)=IPIVOT(ICOLUM)+1
4386      IF(IROW-ICOLUM) 140,260,140
4387 140    DETERM=-DETERM
4388      do 200 L=1,N
4389      SWAP=A(IROW,L)
4390      A(IROW,L)=A(ICOLUM,L)
4391 200    A(ICOLUM,L)=SWAP
4392      IF(M) 260,260,210
4393 210    do 250 L=1,m
4394      SWAP=B(IROW,L)
4395      B(IROW,L)=B(ICOLUM,L)
4396 250    B(ICOLUM,L)=SWAP
4397 260    INDEX(1,1)=IROW
4398      INDEX(1,2)=ICOLUM
4399      PIVOT=A(ICOLUM,ICOLUM)
4400      DT(1)=PIVOT
4401      A(ICOLUM,ICOLUM)=1.0
4402      do 350 L=1,N
4403 350    A(ICOLUM,L)=A(ICOLUM,L)/PIVOT
4404      IF(M) 380,380,360
4405 360    do 370 L=1,m
4406 370    B(ICOLUM,L)=B(ICOLUM,L)/PIVOT
4407 380    do 550 L1=1,N
4408      IF(L1-ICOLUM) 400,550,400
4409 400    T=A(L1,ICOLUM)
4410      A(L1,ICOLUM)=0.0
4411      do 450 L=1,N
4412 450    A(L1,L)=A(L1,L)-A(ICOLUM,L)*T
4413      IF(M) 550,550,460
4414 460    do 500 L=1,M
4415 500    B(L1,L)=B(L1,L)-B(ICOLUM,L)*T
4416 550    CONTINUE
4417      do 710 I=1,N
4418      L=N+1-I
4419 C--    DETERM=DETERM*DT(L)
4420      IF(INDEX(L,1)-INDEX(L,2)) 630,710,630
4421 630    JROW=INDEX(L,1)
4422      JCOLUM=INDEX(L,2)
4423      do 705 K=1,N
4424      SWAP=A(K,JROW)
4425      A(K,JROW)=A(K,JCOLUM)
4426      A(K,JCOLUM)=SWAP
4427 705    CONTINUE
4428 710    CONTINUE

```

```
4429      DO 11 K=1,N
4430      IF(PIVOT(K).NE.1) GO TO 12
4431 11     CONTINUE
4432      RETURN
4433 12     WRITE(33,991)
4434 991    FORMAT(10X,'MATRIX IS SINGULAR')
4435 740    RETURN
4436      END
```

APPENDIX - C

LISTING OF THE POSTPROCESSOR, 'PLOT'

```

1
2
3 1-- THIS IS PROGRAM FOR PLOTTING 3-D GRAPHS USING TEMPLATE
4 1-- ROUTINES. FROM THE OUT OF KSAP II, THIS PROGRAM CAN
5 1-- SORT OUT STRESSES AND CORRESPONDING COORDINATE LOCATIONS.
6 1-- THE STRESSES MAY BE SCALED CONVENIENTLY AND EYE
7 1-- COORDINATES CAN BE CHOSEN TO OBTAIN DIFFERENT SIZES OF
8 1-- THE SAME 3-D PLOT.
9
10 DIMENSION XX(40),YY(40),ZZ(40,40),STRESS(15,15,6,10),
11 .WORK(3200),STRE(1000),PENS(6),WIV(8)
12 DIMENSION HEAD(7)
13 CHARACTER*12 FILNAM
14 INTEGER SIZ
15
16 DATA IO/5/,IOD/7/,CORE/3.0/,OUTFIL/8.0/,FONTFIL/11.0/
17 DATA PENS/1,2,3,4,5,6/
18 DATA IORTHO,PPD,EX,SY/1,20.0,8.0,8.0/
19
20 WRITE (5,X) 'ENTER FILENAME'
21 READ (5,SSS).FILNAM
22 SSS FORMAT (A)
23 OPEN(UNIT=IOD,FILE=FILNAM,STATUS='OLD')
24 WRITE(10,X) '0:SCREEN,1:PRINTER,2:PLOTTER'
25 READ(10,X) IDEV
26 READ (IOD,22), (HEAD(I), I=1,6)
27 22 FORMAT(1X,6A4)
28
29 READ (IOD,X) NNODES,NLOC
30 IF (NLOC.NE.1) CALL SORT21(RAD,IOD,XX,YY,NX,NY,STRE,nnodes,nloc)
31 IF (NNODES.EQ.21) GO TO 899
32 IF (nloc.ne.1) GO TO 899
33 READ(10,X) NX,NY,NL
34 ! WRITE(5,X) NX,NY,NL
35 READ(10,X) (XX(I), I=1,NX)
36 ! WRITE(5,X) (XX(I), I=1,NX)
37 READ(10,X) (YY(I), I=1,NY)
38 ! WRITE(5,X) (YY(I), I=1,NY)
39 NX=NX-1
40 NY=NY-1
41 READ(10,7000) ((STRESS(I,J,K,L),K=1,6), I=1,NX), J=1,NY), L=1,NL)
42 7000 FORMAT(25X,6E15.6)
43 ! WRITE(5,7000) ((STRESS(I,J,K,L),K=1,6), I=1,NX),
44 ! .J=1,NY), L=1,NL)
45 20 WRITE(10,X) 'ENTER STRESS X LAYER #'
46 READ(10,X) S11, LN
47 ! WRITE(5,X) S11, LN
48 K=0
49 DO J=1,NY
50 DO I=1,NX
51 K=K+1
52 STRE(K)=STRESS(I,J,S11,LN)
53 END DO
54 END DO

```

```

55
56 c-- finding stress location coordinates..
57 c-- from nodal coordinates for 8 node element.
58      do i=1,nx
59      if (i.gt.1) xx(i)=(xx(i)+xx(i-1))/2.
60      end do
61      xx(nx)=xx(nx+1)
62      do i=1,ny
63      if (i.gt.1) yy(i)=(yy(i)+yy(i-1))/2.
64      end do
65      yy(ny)=yy(ny+1)
66
67 899  CONTINUE
68      NXNY=NX*NY
69      SMAX=-1.0E+30
70      SMIN=1.0E+30
71      DO I=1,NXNY
72      IF (STRE(I).GT.SMAX) SMAX=STRE(I)
73      IF (STRE(I).LE.SMIN) SMIN=STRE(I)
74      END DO
75      WRITE(5,*) 'SMIN= ',SMIN,' SMAX= ',SMAX
76 10  WRITE(10,*) 'EYEX,EYFY,EYEZ'
77      READ(10,*) EX,EY,EZ
78      WRITE(10,*) 'TYPE SCALE FACTOR'
79      READ(10,*) FACT
80      DO I=1,NXNY
81      STRE(I)=STRE(I)/FACT
82      END DO
83      DO J=1,NY
84      I1=1+(J-1)*NX
85      I2=I1+NX-1
86      II=0
87      DO I=I1,I2
88      II=II+1
89      Z(II,J)=STRE(I)
90      END DO
91      WRITE (85,*) (STRE(I),I=11,I2)
92      WRITE (35,*) '-----'
93      END DO
94      DO J=1,NY
95      WRITE(5,*) J,':',(Z(I,J),I=1,NX)
96      END DO
97      WRITE(5,7000) (STRE(I),I=1,NXNY)
98      WRITE (5,*) 'BEFORE SCALING..'
99 c-- SCALING THE COORDINATES----
100 C      CMAX=XX(NX)
101 C      IF (CMAX.LT.YY(NY)) CMAX=YY(NY)
102 C      WRITE (5,*) 'CMAX',CMAX
103 C      CMAX=9.0/CMAX
104 C      DO 444 I=1,NX
105 444  XX(I)=XX(I)*CMAX
106 C      DO 445 J=1,NY
107 445  YY(I)=YY(I)*CMAX
108      FX=NZ

```

```

109      FY=NY
110      IF (IDEV.EQ.2) CALL UCGNFG(3.0)
111
112      CALL USTART
113      IF(IDEV.EQ.1) CALL UPSET('OUTP',OUTFIL)
114      IF (IDEV.EQ.0) CALL UERASE
115      CALL UPSET ('ENTFILE',ENTFILE)
116 C      CALL USET ('NLSAX15')
117      CALL USET ('ZAX15')
118      CALL USET ('SK15')
119      CALL USET ('PERC')
120      CALL USET ('ORTH')
121 C      CALL USTUD ('W15')
122      CALL UVWPRT (0.0,100.0, 0.0,100.0)
123      CALL UPVSRF (S16,PA,PI,WCR,EX,EI,EX,EY,EZ,PENS)
124      WRITE (5,X) '1.1500 finished..'
125 666   IF (IDEV.EQ.0) CALL UHMOVE
126 7     FORMAT (A1)
127      CALL UEND
128 999   STOP
129      END
130
131      subroutine sort21 (KAD,LOC,XP,YP,NX,NY,ZP,nnodes,nloc)
132 C-----
133 C-- THE FOLLOWING ARE THE STRESS LOCATIONS USED IN KSAP II
134 C-- PROGRAM. DEPENDING ON THE REQUIRED STRESS PLANE LOCATION
135 C-- SORTING WILL BE DONE. AVERAGING IS ALSO CARRIED OUT
136 C-- BETWEEN ADJACENT ELEMENTS.
137 C-- LOC - STRESS OUTPUT LOCATIONS IN SAP....
138 C--
139 C--      6--13-- 5      13--14--12      3-- 9-- 1      v
140 C--      ! ! !      ! ! !      ! ! !      |
141 C--      14--17--10     13--21--22     10--26--12     |
142 C--      ! ! !      ! ! !      ! ! !      |
143 C--      7--15-- 8      19--25--20     3--11-- 4      ----> x
144 C--      bottom      middle      top
145 C-----
146      DIMENSION XP(1),YP(1),ZP(1),LOC(7)
147      ,X(50),Y(50),Z(50),IEMT(2,5,5),ID(3),JD(2),STRES(300,7,6)
148      DATA IEMT/2,19,3,14,25,16,6,18,3, 15,25,11,27,21,26,
149      , 13,24,9, 3,20,4,16,22,12,5,17,17/
150      WRITE (5,X) 'ENTER STRESS NO.(1,2,3,4,5,6)'
151      READ (5,X) NLT
152      READ (10,X) NONX,NONY,NONZ
153      READ (10,*) NLLOC(1,1-1,NLLOC)
154      READ (10,X) X(1),1-1,NONX)
155      READ (10,X) Y(1),1-1,NONY)
156      READ (10,X) Z(1),1-1,NONZ)
157      IF (NNODES.EQ.3) THEN
158      NONX=2*NONX-1
159      NONY=2*NONY-1
160      NONZ=2*NONZ-1
161      DO 111 1-NONX,1,-1
162      J=I/2

```

```

163      IF (MOD(I,2).NE.0) X(I)=X(J+1)
164      IF (MOD(I,2).EQ.0) X(I)=(X(I+1)+X(J))/2.
165 111    CONTINUE
166      DO 222 I=NONY,1,-1
167      J=I/2
168      IF (MOD(I,2).NE.0) Y(I)=Y(J+1)
169 222    IF (MOD(I,2).EQ.0) Y(I)=(Y(I+1)+Y(J))/2.
170      DO 333 I=NONZ,1,-1
171      J=I/2
172      IF (MOD(I,2).NE.0) Z(I)=Z(J+1)
173 333    IF (MOD(I,2).EQ.0) Z(I)=(Z(I+1)+Z(J))/2.
174      END IF
175
176      WRITE (5,*) ' SELECT LEVEL (z-coord. No.) of xy-plane'
177      DO 41 I=1,NONZ
178 41      write (5,51) I,Z(I)
179 51      FORMAT (2X,I2,2X,F8.4)
180      READ (5,*) nlev
181      JS=0
182      IF (MOD(NLEV,2).NE.0) GO TO 310
183 C--    MIDDLE.....
184      JS=2
185      DO 20 J=1,3
186      DO 20 I=1,3
187      IF (LOC(I).EQ.IBMT(2,J,1)) GO TO 21
188 20      CONTINUE
189      GO TO 26
190 210    IF (NLEV.EQ.1) GO TO 23
191      JS=3
192      DO 10 J=1,3
193      DO 10 I=1,3
194      IF (LOC(I).EQ.IBMT(3,J,1)) GO TO 21
195 10      CONTINUE
196      IF (NLEV.EQ.NON2) GO TO 26
197 23      JS=1
198      DO 11 J=1,3
199      DO 11 I=1,3
200      IF (LOC(I).EQ.IBMT(1,J,1)) GO TO 21
201 11      CONTINUE
202 25      WRITE (5,*) '...LEVEL NO. DOESN T MATCH WITH LOC.
203      NOS.
204      STOP
205 21      CONTINUE
206      DO 40 J=1,3
207      ID(J)=0
208      DO 50 I=1,3
209      DO 50 K=1,NLOC
210      IF (LOC(K).EQ.IBMT(3,I,J)) THEN
211      ID(J)=1
212      GO TO 40
213      ENDIF
214 50      CONTINUE
215 40      CONTINUE
216      DO 45 I=1,3

```

```

217      JD(I)=0
218      DO 55 J=1,3
219      DO 55 K=1,NLOC
220      IF (LOC(K).EQ.1) THEN
221      JD(I)=1
222      GO TO 45
223      ENDIF
224 55    CONTINUE
225 45    CONTINUE
226      NEX=(NOMX-1)/2
227      NX=0
228      DO 70 J=1,NOMX
229      IF (JD(1).EQ.0.AND.J.EQ.1) GO TO 70
230      IF (JD(2).EQ.0.AND.J.EQ.NOMX) GO TO 70
231      IF (JD(2).EQ.0.AND.MOD(J,2).EQ.0) GO TO 70
232      NX=NX+1
233      XP(NX)=X(J)
234 70    CONTINUE
235      XP(NX)=X(NOMX)      !TO KEEP SIZE....
236      XP(1)=X(1)         !TO KEEP SIZE....
237      NEY=(NOMY-1)/2
238      NY=0
239      DO 80 J=1,NOMY
240      IF (JD(1).EQ.0.AND.J.EQ.1) GO TO 80
241      IF (JD(2).EQ.0.AND.J.EQ.NOMY) GO TO 80
242      IF (JD(2).EQ.0.AND.MOD(J,2).EQ.0) GO TO 80
243      NY=NY+1
244      YP(NY)=Y(J)
245 80    CONTINUE
246      YP(NY)=Y(NOMY)      !TO KEEP SIZE....
247      YP(1)=Y(1)         !TO KEEP SIZE....
248      NEZ=(NOMZ-1)/2
249      IF (NNOBES.EQ.8) THEN
250      NEX=(NOMX+1)/2-1
251      NEY=(NOMY+1)/2-1
252      NEZ=(NOMZ+1)/2-1
253      END IF
254      NEXY=NEX*NEZ
255      NE=NEX*NEZ
256 c-- Reading stresses 1 - 21. no.; J-loc. no.
257      DO 91 I=1,NE
258      DO 91 J=1,NLOC
259      READ (100,11) (X(I,J),I=1,3),R=1,8)
260 91    CONTINUE
261 92    FORMAT (13A10,10E10)
262
263      NN=(NLOC-1)/2
264      IF (J3.EQ.1) NN=NN+1
265      NEE=NN*NEZ
266 c-- stresses on elements nnn1 onwards are required..
267      K=0
268      DO 99 J=1,NEZ
269      NI=NEE+NEZ*(J-1)
270      NL=NI+NEZ-1

```

```

271      DO 38 JJ=1,3
272      IF (JD(JJ).EQ.0) GO TO 38
273      DO 90 I=N1,N2
274      DO 91 II=1,3
275      IF (ID(II).EQ.0) GO TO 91
276      IL=IBMT(JJ,JJ,II)
277      LL=0
278      DO 92 L=1,NLOC
279  90    IF (IL.EQ.LOC(L)) LL=L
280      K=K+1
281      ZP(K)=STRES(1,LL,NST)
282      IF (II.NE.1.OR.1.EQ.N1) GO TO 91
283      IF (ID(3).EQ.0.OR.ID(1).EQ.0) GO TO 91
284      K=K+1
285      ZP(K)=(ZP(K)+ZP(K+1))/2.
286  91    CONTINUE
287  90    CONTINUE
288      IJ=K-NX+1
289      IF (JJ.NE.1.OR.J.EQ.1) GO TO 88
290      IF (JD(3).EQ.0) GO TO 88
291      K=K-2*NX
292      DO 93 KK=1,NX
293      K=K+1
294  93    ZP(K)=(ZP(K)+ZP(K+NX))/2.
295  89    continue
296  89    continue
297      do i=1,nv
298      j1=1+(i-1)*nx
299      ju=j1+nx-1
300  88    format (2x,10ell.4)
301      end do
302      RETURN
303      end

```

APPENDIX - D

LISTING OF THE EXAMPLE RESULTS

INPUT DATA for 'PTEPROCESSOR' : man.inp

```

1 1 node E1.102/9021s; delam= mesh load: Sx3x5 MESH-man.inp (10/30/8
2 4      !type of the element (8 OR 21)
3 5,3,5,0      ! NO. of coord. in x,y & z dir.. Rad. of hole at
4 0.0, 2.0,4.0,6.0,8.0      !x - coordinates      !Rad. =0 means :
5 0.0, 3.0,6.0      !y - coordinates
6 0.0, 0.5,1.0,1.5,2.0      !z - coordinates
7 1. 300.0, 75, 1      !from node no.,model temp.,to node no., incre
8 -1, 0.0, 0, 0      !data termination indicator....
9 1. 300.0, 32, 1      !from el. no., stress free temp., to el. no., incre
10 -1, 0.0, 0, 0      !data termination indicator....
11 1. 1. 32, 1      !from element no.,mat. no.,to node no.,incre
12 -1, 0, 0, 0      !data termination indicator....
13 1. 1. 16, 1      !from el. no.,mat.axis orient set no.,to node no.,incre
14 17, 2, 32, 1      !from el. no.,mat.axis orient set no.,to node no.,incre
15 -1, 0, 0, 0      !data termination indicator....
16 2,16      !from el. no., to el. no., for same stiffness
17 18,32      !from el. no., to el. no., for same stiffness
18 -1,0      !data termination indicator.. for same stiffness elements
19 0,2      !no. of nodes to simulate split, dir. normal to the plane
20 15      !no. of double nodes for Delamination Region
21 3,8,13,18,23,28,33,38,43,48,53,58,63,68,73      !double nodes
22 0.0,3.0, 0.0,6.0, 1.0,2.0      !x,y,z limits of the solid that has d
23 EL 1 21.0E06      !Elastic constant...
24 ET 1 1.7E06      !Elastic constant...
25 EZ 1 1.7E06      !Elastic constant...
26 GULT 1 0.3      !Elastic constant... as per elasticity notation
27 GULZ 1 0.3      !Elastic constant... as per elasticity notation
28 GUTZ 1 0.54      !Elastic constant...
29 GLT 1 0.94E06      !Elastic constant...
30 GLZ 1 0.94E06      !Elastic constant...
31 GTZ 1 0.50E06      !Elastic constant...
32 ALFL 1 0.2E-06      !Thermal expansion coefficient in L- dir.
33 ALLT 1 0.16E-04      !Thermal expansion coefficient in T- dir.
34 ALFZ 1 0.16E-04      !Thermal expansion coefficient in Z- dir.
35 -1      !data termination indicator for Mat. constant
36 1 5 25 55      !mat. axis orientation for element set =1
37 2 25 25 5      !mat. axis orientation for element set =2
38 -1      !data termination indicator....
39 -1      !data termination for force b.c.
40 4 UY 0.0 24 5 !from,type,value,to,element
41 5 UY 0. 25 5 ! of
42 1 UX 0. 51 25 ! disp. boundary condition
43 2 UX 0. 52 25
44 3 UX 0. 53 25
45 4 UX 0. 54 25
46 5 UX 0. 55 25
47 1 UZ 0.0 71 5
48 51 UY 0.006 75 1 !nonzero disp. b.c.
49 -1      !data termination for disp. b.c.
50 21,0,0,0,0,0,0      !stress loc. nos.

```

OUTPUT FROM 'PREPROCESSOR' : 12.00 10.00

1 0 Node E1.(02/902)s; delat- Allen 1011: 00000 delatman.inp (10/30/87)

[illegible]

55	57	0	0	0	1	1	1	8.0000	3.0000	1.0000	0	300.
56	58	0	0	0	1	1	1	8.0000	3.0000	1.0000	0	300.
57	60	0	0	0	1	1	1	8.0000	3.0000	2.0000	1	300.
58	61	1	0	1	1	1	1	0.0000	6.0000	0.0000	0	300.
59	62	1	0	0	1	1	1	0.0000	6.0000	0.5000	0	300.
60	63	1	0	0	1	1	1	0.0000	6.0000	1.0000	0	300.
61	64	1	0	0	1	1	1	0.0000	6.0000	1.0000	0	300.
62	66	1	0	0	1	1	1	0.0000	6.0000	2.0000	1	300.
63	67	0	0	1	1	1	1	2.0000	6.0000	0.0000	0	300.
64	68	0	0	0	1	1	1	2.0000	6.0000	0.5000	0	300.
65	69	0	0	0	1	1	1	2.0000	6.0000	1.0000	0	300.
66	70	0	0	0	1	1	1	2.0000	6.0000	1.0000	0	300.
67	72	0	0	0	1	1	1	2.0000	6.0000	2.0000	1	300.
68	73	0	0	1	1	1	1	4.0000	6.0000	0.0000	0	300.
69	74	0	0	0	1	1	1	4.0000	6.0000	0.5000	0	300.
70	75	0	0	0	1	1	1	4.0000	6.0000	1.0000	0	300.
71	76	0	0	0	1	1	1	4.0000	6.0000	1.0000	0	300.
72	78	0	0	0	1	1	1	4.0000	6.0000	2.0000	1	300.
73	79	0	0	1	1	1	1	6.0000	6.0000	0.0000	0	300.
74	80	0	0	0	1	1	1	6.0000	6.0000	0.5000	0	300.
75	81	0	0	0	1	1	1	6.0000	6.0000	1.0000	0	300.
76	82	0	0	0	1	1	1	6.0000	6.0000	1.0000	0	300.
77	84	0	0	0	1	1	1	6.0000	6.0000	2.0000	1	300.
78	85	0	0	1	1	1	1	8.0000	6.0000	0.0000	0	300.
79	86	0	0	0	1	1	1	8.0000	6.0000	0.5000	0	300.
80	87	0	0	0	1	1	1	8.0000	6.0000	1.0000	0	300.
81	88	0	0	0	1	1	1	8.0000	6.0000	1.0000	0	300.
82	90	0	0	0	1	1	1	8.0000	6.0000	2.0000	1	300.
83	91	0	0	0	1	1	1	8.0000	6.0000	2.0000	1	300.
84	92	1										
85	93	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
86	94	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
87	95	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
88	96	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
89	97	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
90	98	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
91	99	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
92	100	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
93	101	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
94	102	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
95	103	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
96	104	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
97	105	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
98	106	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
99	107	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
100	108	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
101	109	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
102	110	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
103	111	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
104	112	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
105	113	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
106	114	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
107	115	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
108	116	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
109	117	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		
110	118	1	2	1	7	1	0	0.000000	0.000000	0.100E+21		

105	90	1	2	1	7	1	0	0.0000000	0.0000000	0.100E+21
110	64	1	2	1	7	1	0	0.0000000	0.0000000	0.100E+21
111	70	1	2	1	7	1	0	0.0000000	0.0000000	0.100E+21
112	76	1	2	1	7	1	0	0.0000000	0.0000000	0.100E+21
113	82	1	2	1	7	1	0	0.0000000	0.0000000	0.100E+21
114	88	1	2	1	7	1	0	0.0000000	0.0000000	0.100E+21
115	8	32	1	0	2	0	0	0.0000000	0.0000000	0.100E+21
116	1	1								
LAYER1==90-LAYER; LAYER2==90-LAYER.										
117	300.	21000000.	1700000.	1700000.	0.0243	0.0243	0.5400			
118	940000.	940000.	560000.	11000000	0.0000000	0.0000000	0.0000160			
119	1	6	30	66						
120	2	30	90	6						
121	31	0	0	0	0	0	0			
122										
123										
124										
125	0.									
126										
127	1			1	1	1	300.	0		0
128	38	32	2	8	37	31	1	0		
129	2			1	1	1	100.	0		1
130	44	32	8	14	43	31	0	13		
131	3			1	1	1	100.	0		1
132	50	44	14	20	49	48	13	19		
133	4			1	1	1	100.	12		1
134	56	50	20	26	55	49	19	12		
135	5			1	1	1	100.	0		1
136	68	62	32	38	67	61	31	31		
137	6			1	1	1	100.	0		1
138	74	68	38	44	73	67	37	43		
139	7			1	1	1	100.	0		1
140	80	74	44	50	79	73	43	49		
141	8			1	1	1	100.	0		1
142	86	80	50	56	85	79	49	55		
143	9			1	1	1	100.	0		1
144	99	93	3	9	98	92	2	10		
145	10			1	1	1	100.	0		1
146	45	39	9	15	44	38	0	14		
147	11			1	1	1	100.	0		1
148	51	45	15	21	50	44	14	20		
149	12			1	1	1	100.	12		1
150	57	51	21	27	56	50	20	26		
151	13			1	1	1	100.	0		1
152	69	63	33	39	68	62	31	37		
153	14			1	1	1	100.	0		1
154	75	69	39	45	74	68	37	44		
155	15			1	1	1	100.	0		1
156	31	75	45	51	80	74	43	50		
157	16			1	1	1	100.	0		1
158	87	81	51	57	86	80	50	56		
159	17			1	1	1	100.	0		0
160	41	35	5	11	40	34	4	10		
161	18			1	1	1	100.	0		1
162	47	41	11	17	46	40	11	16		

163	19			1	2	1	300.	0		1
164	53	47	17	23	52	46	16	22		
165	20			1	2	1	300.	10		1
166	59	53	23	29	58	52	22	28		
167	21			1	2	1	300.	0		1
168	71	65	35	41	70	64	34	40		
169	22			1	2	1	300.	0		1
170	77	71	41	47	76	70	40	46		
171	23			1	2	1	300.	0		1
172	83	77	47	53	82	76	48	52		
173	24			1	2	1	300.	0		1
174	89	83	53	59	88	82	52	58		
175	25			1	2	1	300.	0		1
176	42	36	6	12	41	35	5	11		
177	36			1	2	1	300.	0		1
178	48	42	12	18	47	41	11	17		
179	27			1	2	1	300.	0		1
180	54	48	18	24	53	47	17	23		
181	38			1	2	1	300.	10		1
182	60	54	24	30	59	53	23	29		
183	39			1	2	1	300.	0		1
184	72	66	36	42	71	65	35	41		
185	30			1	2	1	300.	0		1
186	73	72	42	48	72	71	41	47		
187	31			1	2	1	300.	6		1
188	84	78	48	54	83	77	47	53		
189	32			1	2	1	300.	0		1
190	90	84	54	60	89	83	53	59		
191										
192		1.								
193	10									
194	0	2	0.0							
195	4	2	0.0							
196	9	2	0.0							
197	10	2	0.0							
198	15	2	0.0							
199	16	2	0.0							
200	21	2	0.0							
201	22	2	0.0							
202	27	2	0.0							
203	28	2	0.0							
204	15									
205	0	4	1 0 1							
206	9	10	1 0 1							
207	15	16	1 0 1							
208	21	22	1 0 1							
209	27	28	1 0 1							
210	33	34	1 1 1							
211	39	40	1 1 1							
212	45	46	1 1 1							
213	51	52	1 1 1							
214	57	58	1 1 1							
215	63	64	1 1 1							
216	69	70	1 1 1							

217	75	76	1 1 1	!	63
218	81	82	1 1 1	!	68
219	87	88	1 1 1	!	73
220		0 0 0			
221		1 32			
222	27	28	1	!	23
223	27	28	2	!	23
224	27	28	3	!	23
225		0 0 0			
226		1 32			
227	31	32	1	!	13
228	31	32	2	!	13
229	31	32	3	!	13
230	57	58	1	!	43
231	57	58	2	!	43
232	57	58	3	!	43
233		0 0 0			
234		1 32			
235	5999	5999	0		
236					

MODIFICATIONS FOR CRACK OPENING SEQUENCE

1	10					
2	3	2	0.0			
3	4	2	0.0			
4	9	2	0.0			
5	10	2	0.0			
6	15	2	0.0			
7	16	2	0.0			
8	21	2	0.0			
9	22	2	0.0			
10	27	2	0.0			
11	28	2	0.0			
12	15					
13	3	4	1 0 1	!		3
14	9	10	1 0 1	!		3
15	15	16	1 0 1	!		13
16	21	22	1 0 1	!		18
17	27	28	1 0 1	!		23
18	33	34	1 1 1	!		28
19	39	40	1 1 1	!		33
20	45	46	1 1 1	!		38
21	51	52	1 1 1	!		43
22	57	58	1 1 1	!		48
23	63	64	1 1 1	!		53
24	69	70	1 1 1	!		58
25	75	76	1 1 1	!		63
26	81	82	1 1 1	!		68
27	87	88	1 1 1	!		73
28	0	0	0			
29	1	32				
30	27	28	1	!		23
31	27	28	2	!		23
32	27	28	3	!		23
33	0	0	0			
34	1	32				
35	21	22	1	!		18
36	21	22	2	!		16
37	21	22	3	!		18
38	57	58	1	!		48
39	57	58	2	!		48
40	57	58	3	!		48
41	0	0	0			
42	1	32				
43	9999	9999	0			
44						

OUTPUT FROM KRAP II : 10/30/87

8 NODE EL102/9031a; delin=delin; delin=delin; delin=delin (10/30/87)

CONTROL INFORMATION

8 NUMBER OF MODAL POINTS = 6
 9 NUMBER OF ELEMENT TYPES = 1
 10 NUMBER OF LOAD CASES = 1
 11 NUMBER OF FREQUENCIES = 0
 12 ANALYSIS CODE (NDYN) = 0
 13 EQ.0, STATIC
 14 EQ.1, MODAL EXTRACTION
 15 EQ.2, FORCED RESPONSE
 16 EQ.3, RESPONSE SPECTRUM
 17 EQ.4, DIRECT INTEGRATION
 18 SOLUTION MODE (MODEX) = 0
 19 EQ.0, EXECUTION
 20 EQ.1, DATA CHECK
 21 NUMBER OF SUBSPACE
 22 ITERATION VECTORS (IND) = 0
 23 EQUATIONS PER BLOCK = 0
 24 TAPEIO SAVE FLAG (NIOSV) = 0

MODAL POINT INPUT DATA

29 NODE BOUNDARY CONDITION CODES MODAL POINT COORDINATES

30 NUMBER X Y Z XX YY ZZ X Y Z

31 BOUNDARY CONDITION CODES

34 ELEMENT TYPE = 1
 35 NUMBER OF ELEMENTS = 6

ELEMENT LOAD CASE MULTIPLIERS

41 CASE(A) CASE(B) CASE(C) CASE(D)
 42 1.0000 0.0000 0.0000 0.0000

ELEMENT	NODE	NODES	BOUNDING	CONSTRAINT	DIRECTION	CODE	CODE	GENERATION
NUMBER	(N)	(N1)	(N2)	(N3)	(N4)	KD	KK	CODE (KN)
1	61	1	2	1	7	1	0	0
2	62	1	2	1	7	1	0	0
3	63	1	2	1	7	1	0	0
4	65	1	2	1	7	1	0	0
5	66	1	2	1	7	1	0	0
6	67	1	2	1	7	1	0	0

55	7	68	1	2	1	7	1	0	0
56	8	69	1	2	1	7	1	0	0
57	9	71	1	2	1	7	1	0	0
58	10	72	1	2	1	7	1	0	0
59	11	73	1	2	1	7	1	0	0
60	12	74	1	2	1	7	1	0	0
61	13	75	1	2	1	7	1	0	0
62	14	77	1	2	1	7	1	0	0
63	15	78	1	2	1	7	1	0	0
64	16	79	1	2	1	7	1	0	0
65	17	80	1	2	1	7	1	0	0
66	18	81	1	2	1	7	1	0	0
67	19	83	1	2	1	7	1	0	0
68	20	84	1	2	1	7	1	0	0
69	21	85	1	2	1	7	1	0	0
70	22	86	1	2	1	7	1	0	0
71	23	87	1	2	1	7	1	0	0
72	24	89	1	2	1	7	1	0	0
73	25	90	1	2	1	7	1	0	0
74	26	64	1	2	1	7	1	0	0
75	27	70	1	2	1	7	1	0	0
76	28	76	1	2	1	7	1	0	0
77	29	82	1	2	1	7	1	0	0
78	30	88	1	2	1	7	1	0	0

79 121 - NODE SOLID ELEMENT INPUT DATA

80

81 CONTROL INFORMATION

82

83 NUMBER OF 21-NODE ELEMENTS = 32

84

85 NUMBER OF MATERIAL SETS = 1

86

87 MAXIMUM NUMBER OF MATERIAL
88 TEMPERATURE INPUT POINTS = 1

89

90 NUMBER OF MATERIAL
91 AXIS ORIENTATION SETS = 2

92

93 NUMBER OF DISTRIBUTED LOAD SETS = 0

94

95 MAXIMUM NUMBER OF ELEMENT NODES = 3

96

97 NUMBER OF STRESS OUTPUT SETS = 1

98

99 R,S COORDINATE INTEGRATION ORDER = 2

100

101 T COORDINATE INTEGRATION ORDER = 2

102

103

104

105

106 MATERIAL PROPERTY TABLES

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108

```

109 MATERIAL NUMBER = ( 1 )
110 NUMBER OF
111 TEMPERATURE POINTS = ( 1 )
112 WEIGHT DENSITY = ( 0.0000000 )
113 MASS DENSITY = ( 0.0000000 )
114 IDENTIFICATION = (MATERIAL=0-LAYER=00-LAYER. )
115
116 TEMPERATURE E11 E12 E13 V12 V13 V22 G12
117
118 500.00 21000000.0 1.000000 1.000000 0.024 0.024 0.540 940000.0
119
120

```

121 MATERIAL AXIS ORIENTATION TABLE

```

122
123 SET NODE NODE NODE
124 NUMBER N1 N2 N3
125
126 1 6 30 90
127 2 30 90 0
128
129

```

130 STRESS OUTPUT ELEMENT TABLE

```

131
132 SET POINT POINT POINT POINT POINT POINT POINT
133 NUMBER (1) (2) (3) (4) (5) (6) (7)
134
135 1 21 0 0 0 0 0
136
137
138

```

139 ELEMENT LOAD CASE MULTIPLIERS

```

140
141 CASE A CASE B CASE C CASE D
142
143 X-DIRECTION GRAVITY = 0.00 0.00 0.00 0.00
144 Y-DIRECTION GRAVITY = 0.00 0.00 0.00 0.00
145 Z-DIRECTION GRAVITY = 0.00 0.00 0.00 0.00
146 THERMAL LOADING = 0.00 0.00 0.00 0.00
147 PRESSURE LOADING = 0.00 0.00 0.00 0.00
148
149

```

150 12 / 0 3 T 0 3 1 NODES SELECTED ELEMENT DATA

```

151
152 ELEMENT NODES NODES MATL. MATL. STRESS STRESS NODE GAUSS GAUSS
153 NUMBER -N1S- -N1S- TAILC MASS OUTPUT FREE INC. PTS. PTS. MARK
154 NO. SET SET TEMP. -KC- -K,S- -I- RE-U
155 1 8 8 1 1 1 500.0 1 2 0
156
157 2 8 8 1 1 1 500.0 6 2 0
158
159 3 8 8 1 1 1 500.0 6 2 0
160
161 4 8 8 1 1 1 500.0 12 2 0
162

```

163	5	3	8	1	1	1	300.0	6	2	0
164										
165	6	8	2	1	1	1	300.0	6	2	0
166										
167	7	8	8	1	1	1	300.0	6	2	0
168										
169	8	6	8	1	1	1	300.0	1	2	0
170										
171	9	8	8	1	1	1	300.0	6	2	0
172										
173	10	3	8	1	1	1	300.0	6	2	0
174										
175	11	8	8	1	1	1	300.0	6	2	0
176										
177	12	8	8	1	1	1	300.0	12	2	0
178										
179	13	8	8	1	1	1	300.0	6	2	0
180										
181	14	8	8	1	1	1	300.0	6	2	0
182										
183	15	8	8	1	1	1	300.0	6	2	0
184										
185	16	8	8	1	1	1	300.0	1	2	0
186										
187	17	8	8	1	2	1	300.0	1	2	0
188										
189	18	8	8	1	2	1	300.0	6	2	0
190										
191	19	8	8	1	2	1	300.0	6	2	0
192										
193	20	8	8	1	2	1	300.0	12	2	0
194										
195	21	8	8	1	2	1	300.0	6	2	0
196										
197	22	8	8	1	2	1	300.0	6	2	0
198										
199	23	8	8	1	2	1	300.0	6	2	0
200										
201	24	8	8	1	2	1	300.0	1	2	0
202										
203	25	8	8	1	2	1	300.0	6	2	0
204										
205	26	8	8	1	2	1	300.0	6	2	0
206										
207	27	8	8	1	2	1	300.0	6	2	0
208										
209	28	8	8	1	2	1	300.0	12	2	0
210										
211	29	8	8	1	2	1	300.0	6	2	0
212										
213	30	8	8	1	2	1	300.0	6	2	0
214										
215	31	8	8	1	2	1	300.0	6	2	0
216										

```

111      32      6      6      .      .      .      1000.0      1      2      0
112
113 EQUATION PARAMETERS
114
115 TOTAL NUMBER OF EQUATIONS
116 BANDWIDTH
117 NUMBER OF EQUATIONS IN A LEVEL
118 NUMBER OF BLOCKS
119 INDIVIDUAL LOADS (DYNAMIC)

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120
121 NODE LOAD 2-AXIS 2-AXIS 2-AXIS 2-AXIS
122 NUMBER CASE FORCE FORCE FORCE FORCE
123
124
125
126
127

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```

128 STRUCTURE ELEMENTS MULTIPLIER
129 LOAD CASE A B C D

```

```

130
131 1 1.000 0.000 0.000 0.000
132
133 3 0.0000000000000000E+00
134 4 0.0000000000000000E+00
135 9 0.0000000000000000E+00
136 10 0.0000000000000000E+00
137 15 0.0000000000000000E+00
138 16 0.0000000000000000E+00
139 21 0.0000000000000000E+00
140 22 0.0000000000000000E+00
141 27 0.0000000000000000E+00
142 28 0.0000000000000000E+00
143
144 10
145 3 4 1 0 1
146 9 10 1 0 1
147 15 10 1 0 1
148 21 10 1 0 1
149 27 10 1 0 1
150 33 34 1 1 1
151 39 40 1 1 1
152 45 46 1 1 1
153 51 52 1 1 1
154 57 58 1 1 1
155 63 64 1 1 1
156 69 70 1 1 1
157 75 76 1 1 1
158 81 82 1 1 1
159 87 88 1 1 1

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160 --- NODES ---
161 0 0
162 STEP 1 0

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163
164
165
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AAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAAA

271 ---- NODAL DISPLACEMENTS AND FORCES IN COLEU----

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NODE	U	V	W	Fx	Fy	Fz
1	0.00000E+00	0.11463E-02	0.00000E+00	0.00000E+00	0.54179E-13	0.00000E+00
2	0.00000E+00	0.66914E-03	-0.23028E-03	0.00000E+00	0.31974E-13	-0.21316E-13
3	0.00000E+00	0.00000E+00	-0.48447E-03	0.00000E+00	-1254.5	-12.449
4	0.00000E+00	0.00000E+00	-0.48447E-03	0.00000E+00	-5360.6	12.449
5	0.00000E+00	0.00000E+00	-0.70911E-03	0.00000E+00	0.00000E+00	-0.99476E-13
6	0.00000E+00	0.00000E+00	-0.92034E-03	0.00000E+00	0.00000E+00	-0.99476E-13
7	-0.73276E-04	0.11457E-02	0.00000E+00	-0.12434E-13	0.25892E-12	0.00000E+00
8	-0.75126E-04	0.86872E-03	-0.23072E-03	0.53391E-13	-0.92371E-13	-0.92371E-13
9	-0.78116E-04	0.00000E+00	-0.48515E-03	-14.750	-2505.8	-27.415
10	-0.78116E-04	0.00000E+00	-0.48515E-03	14.750	-10721.	27.415
11	-0.80760E-04	0.00000E+00	-0.70685E-03	0.49738E-13	0.00000E+00	-0.56343E-13
12	-0.82216E-04	0.00000E+00	-0.92999E-03	-0.71054E-14	0.00000E+00	-0.85265E-13
13	-0.13991E-03	0.11383E-02	0.00000E+00	-0.29310E-13	0.30509E-12	0.00000E+00
14	-0.14765E-03	0.98319E-03	-0.23491E-03	0.88818E-13	-0.87041E-13	-0.37659E-12
15	-0.15902E-03	0.00000E+00	-0.43065E-03	-74.970	-2492.4	-64.931
16	-0.15902E-03	0.00000E+00	-0.43065E-03	74.970	-10718.	64.931
17	-0.17076E-03	0.00000E+00	-0.71501E-03	-0.71054E-14	0.00000E+00	-0.14211E-13
18	-0.16825E-03	0.00000E+00	-0.93503E-03	0.71054E-14	0.00000E+00	-0.54001E-12
19	-0.18363E-03	0.11366E-02	0.00000E+00	0.10658E-13	0.78160E-13	0.00000E+00
20	-0.20636E-03	0.88057E-03	-0.25799E-03	-0.65725E-13	0.18296E-12	0.00000E+00
21	-0.25376E-03	0.00000E+00	-0.53085E-03	-224.03	-2509.2	-90.916
22	-0.25376E-03	0.00000E+00	-0.53085E-03	224.03	-10719.	90.916
23	-0.30927E-03	0.00000E+00	-0.74951E-03	-0.10658E-12	0.00000E+00	0.55422E-12
24	-0.35406E-03	0.00000E+00	-0.94472E-03	-0.39080E-13	0.00000E+00	-0.22737E-12
25	-0.20854E-03	0.12250E-02	0.00000E+00	0.16376E-13	-0.55067E-13	0.00000E+00
26	-0.23900E-03	0.94963E-03	-0.17825E-03	0.35527E-14	0.81712E-13	-0.28422E-13
27	-0.37387E-03	0.00000E+00	-0.39000E-03	-389.31	-1305.8	104.43
28	-0.37287E-03	0.00000E+00	-0.39000E-03	389.81	-5368.9	-104.43
29	-0.55338E-03	0.00000E+00	-0.57126E-03	-0.97700E-13	0.00000E+00	0.22737E-12
30	-0.77311E-03	0.00000E+00	-0.74140E-03	-0.46185E-13	0.00000E+00	-0.14211E-12
31	0.00000E+00	0.30734E-02	0.00000E+00	0.00000E+00	0.23093E-13	0.00000E+00
32	0.00000E+00	0.30562E-02	-0.18195E-03	0.00000E+00	0.17941E-12	-0.16342E-12
33	0.00000E+00	0.30773E-02	-0.41630E-03	0.00000E+00	-444.89	-53.583
34	0.00000E+00	0.30773E-02	-0.41630E-03	0.00000E+00	444.89	53.583
35	0.00000E+00	0.30191E-02	-0.64568E-03	0.00000E+00	0.17935E-11	0.11369E-12
36	0.00000E+00	0.30104E-02	-0.86908E-03	0.00000E+00	-0.11990E-12	-0.24158E-12
37	-0.84036E-04	0.30730E-02	0.00000E+00	0.71054E-14	0.41656E-12	0.00000E+00
38	-0.85401E-04	0.30559E-02	-0.18228E-03	0.74607E-13	-0.52935E-12	-0.41922E-12
39	-0.87090E-04	0.30771E-02	-0.41734E-03	-19.334	-888.27	-112.18
40	-0.87090E-04	0.30771E-02	-0.41734E-03	19.334	888.27	112.18
41	-0.87638E-04	0.30190E-02	-0.64533E-03	-0.36818E-13	0.18510E-11	0.17053E-12
42	-0.88506E-04	0.30104E-02	-0.86809E-03	0.71054E-14	0.11431E-11	-0.52580E-12
43	-0.16394E-03	0.30731E-02	0.00000E+00	-0.35527E-13	0.29488E-12	0.00000E+00
44	-0.17014E-03	0.30560E-02	-0.18640E-03	0.11724E-12	-0.31442E-12	-0.83844E-12
45	-0.17822E-03	0.30769E-02	-0.42291E-03	-125.37	-835.34	-186.00
46	-0.17822E-03	0.30769E-02	-0.42291E-03	125.37	885.34	186.00
47	-0.18532E-03	0.30190E-02	-0.65904E-03	0.31974E-13	-0.91357E-12	0.15632E-12

325	48-0.18212E-03	0.30163E-02	0.15120E-03	0.158020E-13	0.22280E-11	-0.63949E-12
326	49-0.22847E-03	0.30765E-02	0.00000E+00	0.15721E-12	0.98588E-13	0.00000E+00
327	50-0.24589E-03	0.30589E-02	0.10490E-03	0.20468E-12	0.43165E-12	0.20606E-12
328	51-0.28415E-03	0.30775E-02	0.40000E-03	408.01	-900.19	-243.72
329	52-0.28415E-03	0.30775E-02	0.40000E-03	408.01	900.19	243.72
330	53-0.33013E-03	0.30190E-02	0.10000E-03	0.14921E-12	0.13074E-11	0.48317E-12
331	54-0.37055E-03	0.30099E-02	0.07000E-03	0.14211E-12	0.15103E-11	-0.44054E-12
332	55-0.26533E-03	0.30814E-02	0.00000E+00	0.10630E-12	0.88818E-15	0.00000E+00
333	56-0.30213E-03	0.30655E-02	0.10200E-03	0.21054E-14	0.12923E-12	0.48850E-13
334	57-0.40790E-03	0.30651E-02	0.30000E-03	-741.90	-469.95	173.82
335	58-0.40790E-03	0.30651E-02	0.30000E-03	-741.90	469.95	-173.82
336	59-0.67171E-03	0.30522E-02	0.31000E-03	0.70166E-13	0.15967E-11	0.14744E-12
337	60-0.78089E-03	0.30100E-02	0.00770E-03	0.12012E-12	0.25624E-12	-0.16375E-12
338	61	0.00000E+00	0.00000E+00	1.00000E+00	0.00000E+00	-16.000
339	62	0.00000E+00	0.00000E+00	0.00000E+00	32.000	-0.85265E-13
340	63	0.00000E+00	0.00000E+00	0.00000E+00	-2416.0	63.375
341	64	0.00000E+00	0.00000E+00	0.00000E+00	2416.0	-63.375
342	65	0.00000E+00	0.00000E+00	0.00000E+00	-16.000	0.39790E-12
343	66	0.00000E+00	0.00000E+00	0.00000E+00	32.000	-0.56843E-13
344	67-0.86973E-04	0.60000E-02	0.00000E+00	0.14211E-13	32.000	0.00000E+00
345	68-0.90275E-04	0.60000E-02	0.00000E+00	0.10658E-13	80.000	-0.29843E-12
346	69-0.91885E-04	0.60000E-02	0.54214E-03	-7.6169	-4848.0	124.28
347	70-0.91885E-04	0.60000E-02	0.54214E-03	7.6169	4848.0	-124.28
348	71-0.92281E-04	0.60000E-02	0.74309E-03	0.39080E-13	48.000	-0.71054E-13
349	72-0.92165E-04	0.60000E-02	0.90005E-03	0.42633E-13	16.000	-0.56843E-12
350	73-0.17348E-03	0.60000E-02	0.00000E+00	0.09278E-13	0.00000E+00	0.00000E+00
351	74-0.17961E-03	0.60000E-02	0.00702E-03	0.30198E-13	32.000	-0.32552E-12
352	75-0.18760E-03	0.60000E-02	0.54005E-03	-58.135	-4928.0	87.420
353	76-0.18760E-03	0.60000E-02	0.54005E-03	58.135	4916.0	-87.420
354	77-0.19428E-03	0.60000E-02	0.74810E-03	0.12479E-12	32.000	0.10747E-12
355	78-0.19082E-03	0.60000E-02	0.90151E-03	0.10214E-13	-32.000	0.13189E-12
356	79-0.24146E-03	0.60000E-02	0.00000E+00	0.14388E-12	32.000	0.00000E+00
357	80-0.25882E-03	0.60000E-02	0.00000E+00	0.00000E-13	32.000	-0.15610E-12
358	81-0.29749E-03	0.60000E-02	0.50000E-03	-200.15	-4848.0	58.284
359	82-0.29749E-03	0.60000E-02	0.50000E-03	200.15	4848.0	-58.284
360	83-0.34226E-03	0.60000E-02	0.77813E-03	0.40650E-13	16.000	-0.13569E-12
361	84-0.38280E-03	0.60000E-02	0.90023E-03	0.55955E-13	48.000	0.41478E-12
362	85-0.30196E-03	0.60000E-02	0.00000E+00	0.10658E-13	32.000	0.00000E+00
363	86-0.31903E-03	0.60000E-02	0.24017E-03	0.03949E-13	48.000	-0.14211E-12
364	87-0.42430E-03	0.60000E-02	0.40000E-03	-374.35	-2432.0	179.58
365	88-0.42430E-03	0.60000E-02	0.40000E-03	374.35	2432.0	-179.58
366	89-0.69559E-03	0.60000E-02	0.01700E-03	0.50643E-13	-16.000	0.88818E-13
367	90-0.80112E-03	0.60000E-02	0.70100E-03	0.00396E-13	-32.000	0.12079E-12

370	-----		
371	27	28	1
372	27	28	2
373	27	28	3
374	0	0	0
375	1	STEP 1	1
376	-----		
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433	47-0.18275E-03	0.30000E-02	-0.04400E-03	-0.00000E-13	0.93792E-12	-0.63949E-12
434	48-0.17762E-03	0.30000E-02	-0.04400E-03	-0.11569E-12	0.23170E-11	0.36948E-12
435	49-0.24254E-03	0.32160E-02	-0.00000E-03	-0.13923E-13	-0.13145E-12	0.00000E+00
436	50-0.26493E-03	0.31994E-02	-0.10154E-03	-0.23981E-12	0.40945E-12	-0.99476E-13
437	51-0.32906E-03	0.31915E-02	-0.44950E-03	-0.00000E-13	-808.58	-347.65
438	52-0.32906E-03	0.31915E-02	-0.44950E-03	-0.00000E-13	808.58	347.65
439	53-0.36051E-03	0.30618E-02	-0.07092E-03	-0.19540E-13	0.20961E-12	0.24158E-12
440	54-0.37399E-03	0.29663E-02	-0.37057E-03	-0.10845E-14	0.18332E-11	-0.36948E-12
441	55-0.30339E-03	0.33490E-02	0.00000E+00	-0.13076E-12	0.34417E-13	0.00000E+00
442	56-0.32779E-03	0.33772E-02	-0.01875E-04	-0.19302E-12	-0.65281E-13	0.18874E-12
443	57-0.45400E-03	0.36043E-02	-0.25000E-03	-0.00000E-13	-79.197	368.55
444	58-0.46466E-03	0.36043E-02	-0.25000E-03	-0.00000E-13	79.197	-368.55
445	59-0.67718E-03	0.35075E-02	-0.41500E-03	-0.20095E-12	-0.12574E-11	-0.23093E-12
446	60-0.72348E-03	0.36894E-02	-0.60040E-03	-0.42166E-14	0.13501E-11	-0.50981E-12
447	61 0.00000E+00	0.60000E-02	0.00000E+00	0.00000E+00	-16.000	0.00000E+00
448	62 0.00000E+00	0.60000E-02	-0.00000E-03	0.00000E+00	32.000	-0.12790E-12
449	63 0.00000E+00	0.60000E-02	-0.00000E-03	0.00000E+00	-2416.0	62.806
450	64 0.00000E+00	0.60000E-02	-0.00000E-03	0.00000E+00	2416.0	-62.806
451	65 0.00000E+00	0.60000E-02	-0.00000E-03	0.00000E+00	-16.000	0.85265E-13
452	66 0.00000E+00	0.60000E-02	-0.00000E-03	0.00000E+00	32.000	0.11369E-12
453	67-0.10239E-03	0.60000E-02	0.00000E+00	0.21054E-14	32.000	0.00000E+00
454	68-0.10480E-03	0.60000E-02	-0.26521E-03	-0.63949E-13	48.000	-0.34106E-12
455	69-0.10772E-03	0.60000E-02	-0.54500E-03	-4.8732	-4864.0	121.46
456	70-0.10772E-03	0.60000E-02	-0.54500E-03	-4.8732	4896.0	-121.46
457	71-0.10817E-03	0.60000E-02	-0.74330E-03	-0.24369E-13	16.000	-0.12221E-11
458	72-0.10861E-03	0.60000E-02	-0.95360E-03	-0.42633E-13	48.000	0.14211E-12
459	73-0.19891E-03	0.60000E-02	0.00000E+00	0.31974E-13	-32.000	0.00000E+00
460	74-0.20703E-03	0.60000E-02	-0.26553E-03	0.51514E-13	64.000	-0.24514E-12
461	75-0.22297E-03	0.60000E-02	-0.54440E-03	-59.615	-4928.0	100.92
462	76-0.22297E-03	0.60000E-02	-0.54440E-03	-59.615	4832.0	-100.92
463	77-0.24100E-03	0.60000E-02	-0.74532E-03	-0.47513E-13	64.000	-0.82645E-12
464	78-0.24622E-03	0.60000E-02	-0.95450E-03	-0.73375E-13	-16.000	0.41256E-12
465	79-0.27460E-03	0.60000E-02	0.00000E+00	0.55307E-13	-16.000	0.00000E+00
466	80-0.29576E-03	0.60000E-02	-0.27050E-03	-0.15145E-12	64.000	-0.77272E-13
467	81-0.34303E-03	0.60000E-02	-0.52275E-03	-221.86	-4640.0	206.30
468	82-0.34303E-03	0.60000E-02	-0.52275E-03	-221.86	4688.0	-206.30
469	83-0.42417E-03	0.60000E-02	-0.70000E-03	-0.10099E-13	80.000	0.33573E-12
470	84-0.50061E-03	0.60000E-02	-0.87791E-03	-0.41744E-13	16.000	0.22826E-12
471	85-0.33558E-03	0.60000E-02	0.00000E+00	0.00949E-13	16.000	0.00000E+00
472	86-0.35735E-03	0.60000E-02	-0.18971E-03	-0.10056E-12	0.00000E+00	-0.29310E-13
473	87-0.47481E-03	0.60000E-02	-0.82597E-03	-593.50	-2096.0	354.50
474	88-0.47481E-03	0.60000E-02	-0.82597E-03	-593.50	2112.0	-354.50
475	89-0.31700E-03	0.60000E-02	-0.55231E-03	-0.21310E-13	0.00000E+00	0.10658E-12
476	90-0.10120E-02	0.60000E-02	-0.40711E-03	-0.40856E-13	16.000	0.26645E-13

477			
478			
479			
480	21	22	1
481	21	22	1
482	21	22	1
483	57	58	1
484	57	58	1
485	57	58	1
486	0	0	0

487 1 11111 STEP 1 2 1111111

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492 ---- NODAL DISPLACEMENTS AND FORCES IN SOLEO----

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NODE	U	V	W	Fx	Fy	Fz
1	0.00000E+00	0.11479E-02	0.00000E+00	0.00000E+00	0.47518E-13	0.00000E+00
2	0.00000E+00	0.86565E-03	-0.22518E-03	0.00000E+00	-0.17764E-12	-0.11369E-12
3	0.00000E+00	0.00000E+00	-0.48940E-03	0.00000E+00	-1257.5	19.659
4	0.00000E+00	0.00000E+00	-0.48940E-03	0.00000E+00	-5347.2	-19.659
5	0.00000E+00	0.00000E+00	-0.72839E-03	0.00000E+00	0.00000E+00	-0.39476E-13
6	0.00000E+00	0.00000E+00	-0.95189E-03	0.00000E+00	0.00000E+00	-0.11369E-12
7	-0.43660E-04	0.11561E-02	0.00000E+00	0.14211E-13	0.11102E-12	0.00000E+00
8	-0.51390E-04	0.89754E-03	-0.21430E-03	0.35527E-14	0.22560E-12	-0.16342E-12
9	-0.26748E-04	0.00000E+00	-0.43272E-03	95.531	-2546.6	77.612
10	-0.26748E-04	0.00000E+00	-0.43272E-03	-95.531	-10733.	-77.612
11	0.24181E-04	0.00000E+00	-0.64642E-03	-0.49738E-13	0.00000E+00	0.85265E-13
12	0.17142E-04	0.00000E+00	-0.90242E-03	-0.10658E-13	0.00000E+00	-0.56843E-13
13	-0.66606E-04	0.11869E-02	0.00000E+00	-0.91432E-13	0.58620E-13	0.00000E+00
14	-0.66621E-04	0.91686E-03	-0.34559E-03	0.00000E+00	0.17764E-14	-0.19185E-12
15	-0.14033E-03	0.00000E+00	-0.72966E-03	-666.80	-2564.5	-666.75
16	-0.14033E-03	0.00000E+00	-0.72966E-03	666.80	-9013.7	666.75
17	0.17186E-03	0.00000E+00	-0.10019E-02	-0.17408E-12	0.00000E+00	-0.48317E-12
18	0.67653E-03	0.00000E+00	-0.12980E-02	-0.13500E-12	0.00000E+00	0.21316E-12
19	-0.84855E-04	0.13738E-02	0.00000E+00	0.39080E-13	0.36415E-12	0.00000E+00
20	-0.96202E-04	0.10742E-02	-0.23259E-03	-0.17764E-13	-0.22915E-12	0.24158E-12
21	-0.70549E-04	0.00000E+00	-0.51319E-03	-0.29310E-13	-2778.4	-0.10658E-12
22	-0.13701E-02	0.22522E-02	-0.57535E-02	0.14211E-12	0.31974E-12	-0.10232E-11
23	-0.22838E-03	0.00000E+00	-0.56503E-02	-0.55422E-12	0.00000E+00	-0.26432E-11
24	-0.59570E-03	0.00000E+00	-0.57034E-02	-0.71054E-14	0.00000E+00	0.36380E-11
25	-0.11507E-03	0.14984E-02	0.00000E+00	0.29921E-13	0.47962E-13	0.00000E+00
26	-0.12385E-02	0.11694E-02	-0.27357E-03	-0.39706E-13	0.27303E-13	0.11269E-12
27	-0.15967E-03	0.00000E+00	-0.60236E-03	0.10125E-12	-1498.4	-0.71054E-13
28	-0.16199E-02	0.28741E-02	-0.88058E-02	0.67502E-13	0.29843E-12	0.68312E-12
29	-0.10713E-02	0.00000E+00	-0.89213E-02	0.14211E-13	0.00000E+00	-0.41496E-11
30	-0.29968E-03	0.00000E+00	-0.89224E-02	0.39790E-12	0.00000E+00	0.30127E-11
31	0.00000E+00	0.30553E-02	0.00000E+00	0.00000E+00	0.26823E-12	0.00000E+00
32	0.00000E+00	0.30423E-02	-0.17981E-03	0.00000E+00	-0.17941E-12	-0.49027E-12
33	0.00000E+00	0.30750E-02	-0.41112E-03	0.00000E+00	-433.39	-31.994
34	0.00000E+00	0.30750E-02	-0.41112E-03	0.00000E+00	433.39	31.994
35	0.00000E+00	0.30189E-02	-0.64004E-03	0.00000E+00	0.21174E-11	0.42633E-13
36	0.00000E+00	0.30085E-02	-0.86547E-03	0.00000E+00	0.21272E-12	-0.32685E-12
37	-0.93072E-04	0.30743E-02	0.00000E+00	0.39080E-13	-0.24336E-12	0.00000E+00
38	-0.97256E-04	0.30510E-02	-0.18087E-03	0.10481E-12	0.44764E-12	-0.67502E-12
39	-0.96425E-04	0.30616E-02	-0.41379E-03	79.605	-958.20	-46.563
40	-0.96425E-04	0.30616E-02	-0.41379E-03	-79.605	958.20	46.563
41	-0.83153E-04	0.30059E-02	-0.63399E-03	-0.85265E-13	0.14992E-11	-0.18474E-12
42	-0.77701E-04	0.30000E-02	-0.86052E-03	-0.15987E-13	0.13709E-11	-0.46896E-12

541	43-0.17673E-03	0.32280E-03	0.00000E+00	-0.15721E-12	-0.25580E-12	0.00000E+00
542	44-0.19037E-03	0.32062E-02	-0.18322E-03	0.18620E-13	0.59686E-13	-0.41922E-12
543	45-0.22271E-03	0.31866E-02	-0.42200E-03	-579.02	-819.24	-408.11
544	46-0.22271E-03	0.31866E-02	-0.42200E-03	579.02	819.24	408.11
545	47-0.20486E-03	0.30612E-02	-0.64864E-03	-0.10303E-12	-0.44942E-12	-0.59686E-12
546	48-0.18848E-03	0.29922E-02	-0.56920E-03	0.35527E-14	0.32807E-11	0.15632E-12
547	49-0.24670E-03	0.33942E-02	0.00000E+00	-0.76100E-13	0.44231E-12	0.00000E+00
548	50-0.25784E-03	0.34314E-02	-0.18202E-03	0.20901E-12	-0.87574E-12	0.60396E-13
549	51-0.35287E-03	0.36820E-02	-0.43007E-03	-1021.9	72.417	-185.66
550	52-0.35287E-03	0.36820E-02	-0.43007E-03	1021.9	-72.417	185.66
551	53-0.38131E-03	0.34879E-02	-0.65100E-03	-0.39900E-12	0.18048E-11	-0.14140E-11
552	54-0.22594E-03	0.26038E-02	-0.94200E-03	-0.10039E-12	-0.11637E-11	0.16129E-11
553	55-0.25875E-03	0.35344E-02	0.10000E+00	-0.24009E-13	0.12068E-12	0.00000E+00
554	56-0.32107E-03	0.35647E-02	-0.15000E-03	-0.15000E-12	-0.56621E-14	0.25846E-12
555	57-0.31717E-03	0.37435E-02	-0.38855E-03	-0.51514E-13	0.19318E-13	-0.45119E-12
556	58-0.11466E-02	0.39825E-02	-0.17455E-02	-0.35527E-13	0.10107E-11	0.44764E-12
557	59-0.71109E-03	0.34814E-02	-0.19112E-02	0.35527E-13	0.20410E-11	-0.30198E-11
558	60-0.38445E-03	0.25762E-02	-0.21901E-02	0.35500E-12	-0.37104E-12	0.13705E-11
559	61	0.00000E+00	0.60000E-02	0.00000E+00	0.00000E+00	0.00000E+00
560	62	0.00000E+00	0.60000E-02	-0.28070E-03	0.99000E+00	48.000
561	63	0.00000E+00	0.60000E-02	-0.54430E-03	0.00000E+00	-2448.0
562	64	0.00000E+00	0.60000E-02	-0.54430E-03	0.00000E+00	2432.0
563	65	0.00000E+00	0.60000E-02	-0.74325E-03	0.00000E+00	-32.000
564	66	0.00000E+00	0.60000E-02	-0.95227E-03	0.00000E+00	32.000
565	67-0.10561E-03	0.60000E-02	0.00000E+00	0.71054E-14	16.000	0.00000E+00
566	68-0.10613E-03	0.60000E-02	-0.28179E-03	0.35422E-13	48.000	-0.30553E-12
567	69-0.11717E-03	0.60000E-02	-0.53992E-03	-1.1518	-4896.0	142.11
568	70-0.11717E-03	0.60000E-02	-0.53992E-03	1.1518	4848.0	-142.11
569	71-0.13113E-03	0.60000E-02	-0.74081E-03	-0.67902E-13	32.000	-0.42633E-13
570	72-0.14211E-03	0.60000E-02	-0.95039E-03	-0.21310E-13	32.000	-0.54001E-12
571	73-0.20521E-03	0.60000E-02	0.00000E+00	-0.39903E-13	16.000	0.00000E+00
572	74-0.21400E-03	0.60000E-02	-0.24712E-03	0.44409E-13	64.000	-0.20384E-12
573	75-0.22667E-03	0.60000E-02	-0.40679E-03	-49.501	-4656.0	270.56
574	76-0.22667E-03	0.60000E-02	-0.40679E-03	49.501	4608.0	-270.56
575	77-0.25849E-03	0.60000E-02	-0.64762E-03	-0.74007E-13	-16.000	-0.16875E-12
576	78-0.29616E-03	0.60000E-02	-0.85435E-03	-0.30198E-13	48.000	0.47740E-12
577	79-0.28233E-03	0.60000E-02	0.00000E+00	-0.12454E-13	16.000	0.00000E+00
578	80-0.30470E-03	0.60000E-02	-0.24014E-03	-0.39080E-13	48.000	-0.13767E-12
579	81-0.34939E-03	0.60000E-02	-0.41262E-03	-357.03	-3872.0	420.80
580	82-0.34939E-03	0.60000E-02	-0.41262E-03	357.03	3904.0	-420.80
581	83-0.40527E-03	0.60000E-02	-0.49364E-03	-0.95923E-13	64.000	-0.64926E-12
582	84-0.46693E-03	0.60000E-02	-0.60819E-03	-0.75495E-13	48.000	0.80203E-12
583	85-0.34344E-03	0.60000E-02	0.00000E+00	0.24007E-13	16.000	0.00000E+00
584	86-0.36462E-03	0.60000E-02	-0.14797E-03	-0.67902E-13	16.000	0.35527E-13
585	87-0.49756E-03	0.60000E-02	-0.24049E-03	-332.14	-1680.0	347.53
586	88-0.49756E-03	0.60000E-02	-0.24049E-03	332.14	1664.0	-347.53
587	89-0.80121E-03	0.60000E-02	-0.37001E-03	-0.66990E-13	-32.000	-0.28422E-12
588	90-0.92550E-03	0.60000E-02	-0.37100E-03	0.24809E-13	48.000	0.35705E-12

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592 9999 9999 0
593 10 V E R A L L T I M E 0.0
594

585	NODAL POINT INPUT	=	0.00
586	ELEMENT STIFFNESS FORMATION	=	0.00
587	NODAL LOAD INPUT	=	0.00
588	TOTAL STIFFNESS FORMATION	=	0.00
589	STATIC ANALYSIS	=	0.00
590	EIGENVALUE EXTRACTION	=	0.00
601	FORCED RESPONSE ANALYSIS	=	0.00
602	RESPONSE SPECTRUM ANALYSIS	=	0.00
603	STEP-BY-STEP INTEGRATION	=	0.00
604			
605	TOTAL SOLUTION TIME	=	0.00
606			

OUTPUT FROM 'KSAF 11' : KSAF001.DAT

1
2 5 Node EL102/9023s; 31125 MESH 1000; 51045 MESH 440.100 (10/30/87)
3

4 ---STRESS OUTPUT LOCATIONS---

5 1 21 0 0 0 0 0
6 121 - N O D E S O L I D E L E M E N T S T R E S S
7

8 ELEMENT	LOAD LOCATION	SIG-XX	SIG-YY	SIG-ZZ	SIG
11 1 1 1	1	0.000000E+00	0.000000E+00	-0.135072E+03	-0.183894E
12 2 1 1	1	-0.013450E+03	0.000000E+00	-0.142729E+03	-0.680401E
13 3 1 1	1	-0.000000E+00	0.000000E+00	-0.192748E+03	-0.100091E
14 4 1 1	1	-0.000000E+00	0.000000E+00	-0.617359E+02	0.205791E
15 5 1 1	1	-0.000000E+00	0.000000E+00	0.130826E+03	-0.847874E
16 6 1 1	1	-0.000000E+00	0.000000E+00	0.122555E+03	-0.324551E
17 7 1 1	1	-0.000000E+00	0.000000E+00	0.742566E+02	-0.278600E
18 8 1 1	1	-0.000000E+00	0.000000E+00	0.204648E+03	-0.346053E
19 9 1 1	1	-0.000000E+00	0.000000E+00	-0.648239E+02	-0.160910E
20 10 1 1	1	-0.000000E+00	0.000000E+00	-0.681815E+02	-0.545370E
21 11 1 1	1	-0.000000E+00	0.000000E+00	-0.110922E+03	-0.864481E
22 12 1 1	1	-0.000000E+00	0.000000E+00	-0.106091E+02	-0.352275E
23 13 1 1	1	-0.000000E+00	0.000000E+00	0.625469E+02	-0.809481E
24 14 1 1	1	-0.000000E+00	0.000000E+00	0.590736E+02	-0.224869E
25 15 1 1	1	-0.000000E+00	0.000000E+00	0.164611E+02	-0.312321E
26 16 1 1	1	-0.000000E+00	0.000000E+00	0.116455E+03	-0.322742E
27 17 1 1	1	0.447563E+03	0.214096E+05	-0.603122E+01	-0.126553E
28 18 1 1	1	0.440140E+03	0.214056E+05	-0.926818E+01	-0.392549E
29 19 1 1	1	0.448288E+03	0.214536E+05	-0.189085E+02	-0.660276E
30 20 1 1	1	0.227405E+03	0.214531E+05	0.876635E+01	-0.637688E
31 21 1 1	1	0.432792E+03	0.207950E+05	0.692551E+01	-0.763006E
32 22 1 1	1	0.425190E+03	0.207926E+05	0.386937E+01	-0.219859E
33 23 1 1	1	0.393862E+03	0.207794E+05	-0.648407E+01	-0.335985E
34 24 1 1	1	0.314568E+03	0.207430E+05	0.211307E+02	-0.384381E
35 25 1 1	1	0.444064E+03	0.212235E+05	0.184790E+00	-0.104566E
36 26 1 1	1	0.437838E+03	0.212341E+05	-0.135654E+00	-0.326547E
37 27 1 1	1	0.250875E+03	0.212204E+05	0.124554E+02	-0.518971E
38 28 1 1	1	0.134009E+03	0.211650E+05	-0.132877E+02	-0.369496E
39 29 1 1	1	0.424514E+03	0.210275E+05	0.709911E+00	-0.742767E
40 30 1 1	1	0.421833E+03	0.210253E+05	0.245991E+00	-0.212575E
41 31 1 1	1	0.274490E+03	0.210144E+05	0.127670E+02	-0.333745E
42 32 1 1	1	0.180990E+03	0.209958E+05	-0.129398E+02	-0.451993E

43 ----- ENERGY RELEASED IN X, Y, Z Directions -----

44 0.24492099 9.070000E-01 -0.000000E+00

45 -----

46 121 - N O D E S O L I D E L E M E N T S T R E S S

47 ELEMENT	LOAD LOCATION	SIG-XX	SIG-YY	SIG-ZZ	SIG
51 1 1 1	1	-0.429445E+03	0.000000E+00	-0.131187E+03	-0.677179E
52 2 1 1	1	-0.435305E+03	0.000000E+00	-0.118742E+03	-0.147760E
53 3 1 1	1	-0.250007E+03	0.000000E+00	-0.244673E+03	0.945992E
54 4 1 1	1	-0.318020E+03	0.000000E+00	-0.112547E+03	0.384709E

55	5	1	1	-0.461091E+03	0.172685E+04	0.130259E+03	-0.313154E
56	6	1	1	-0.455280E+03	0.172218E+04	0.122723E+03	-0.363426E
57	7	1	1	-0.298799E+03	0.165999E+04	0.921526E+02	0.256255E
58	8	1	1	-0.595150E+02	0.168531E+04	0.272012E+03	0.265112E
59	9	1	1	-0.330395E+03	0.144035E+04	-0.657823E+02	-0.634614E
60	10	1	1	-0.339405E+03	0.145545E+04	-0.319520E+02	-0.265199E
61	11	1	1	-0.547364E+03	0.140766E+04	-0.172534E+03	0.148727E
62	12	1	1	-0.444362E+02	0.157723E+04	-0.523599E+02	0.355030E
63	13	1	1	-0.509716E+03	0.166601E+04	0.623993E+02	-0.264511E
64	14	1	1	-0.550907E+03	0.168559E+04	0.598138E+02	-0.325761E
65	15	1	1	-0.631592E+03	0.162664E+04	0.232740E+02	0.246813E
66	16	1	1	-0.497861E+03	0.158964E+04	0.179141E+03	0.626181E
67	17	1	1	0.454919E+02	0.214599E+05	-1.942930E+01	-0.491934E
68	18	1	1	0.477618E+03	0.214393E+05	0.136786E+02	-0.257362E
69	19	1	1	0.445130E+03	0.212855E+05	-0.246355E+02	-0.365693E
70	20	1	1	-0.599666E+02	0.172619E+05	-0.645275E+02	0.520656E
71	21	1	1	0.425392E+03	0.209036E+05	0.678539E+01	-0.255128E
72	22	1	1	0.415775E+03	0.208448E+05	0.596236E+01	-0.134598E
73	23	1	1	0.365666E+03	0.205495E+05	-0.344725E+01	0.873985E
74	24	1	1	0.285974E+03	0.187193E+05	0.716737E+02	0.930356E
75	25	1	1	0.451999E+03	0.212285E+05	-0.209313E+00	-0.390673E
76	26	1	1	0.471023E+02	0.212024E+05	-0.923305E+01	-0.246273E
77	27	1	1	0.562264E+03	0.212840E+05	0.285149E+02	-0.109388E
78	28	1	1	0.316123E+03	0.218731E+05	-0.226633E+01	-0.175313E
79	29	1	1	0.428236E+03	0.210360E+05	0.883838E+00	-0.308181E
80	30	1	1	0.416479E+03	0.210620E+05	0.254047E+01	-0.152305E
81	31	1	1	0.343974E+03	0.210659E+05	0.111353E+02	-0.202870E
82	32	1	1	0.141545E+03	0.202542E+05	-0.312466E+02	-0.747797E

83 ----- ENERGY RELEASED in (m.) . z / directions -----

84 0.64558025 14.140609 1.3794686

85 =====

86 121 - N O D E S O L I D E L E M E N T S T R E S S

87

88	ELEMENT	LOAD	LOCATION	SIG-XX	SIG-YY	SIG-ZZ	SIG
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89

90

91	1	1	1	-0.455877E+03	0.108435E+04	-0.104790E+03	-0.231523E
92	2	1	1	-0.312688E+03	0.106184E+04	-0.223837E+03	0.154030E
93	3	1	1	-0.214170E+03	0.111562E+04	-0.208921E+03	0.427663E
94	4	1	1	-0.108979E+03	0.121222E+04	-0.612078E+02	0.279814E
95	5	1	1	-0.502761E+03	0.172726E+04	0.131473E+03	0.142089E
96	6	1	1	-0.458382E+03	0.168180E+04	0.133735E+03	0.303849E
97	7	1	1	-0.293367E+03	0.157673E+04	0.115431E+03	0.354324E
98	8	1	1	-0.916770E+02	0.154290E+04	0.218118E+03	0.187686E
99	9	1	1	-0.262870E+03	0.145925E+04	-0.245710E+02	-0.867483E
100	10	1	1	-0.536140E+03	0.141648E+04	-0.157500E+03	0.999497E
101	11	1	1	0.151097E+02	0.155991E+04	-0.132885E+03	0.522352E
102	12	1	1	0.169432E+03	0.173573E+04	0.118579E+02	-0.291787E
103	13	1	1	-0.574305E+03	0.168757E+04	0.631362E+02	-0.302754E
104	14	1	1	-0.623795E+03	0.165204E+04	0.718584E+02	0.282842E
105	15	1	1	-0.614966E+03	0.149768E+04	0.481470E+02	0.791060E
106	16	1	1	-0.156371E+03	0.141790E+04	0.119957E+03	0.196302E
107	17	1	1	0.490080E+03	0.214341E+05	0.147009E+02	-0.169694E
108	18	1	1	0.474299E+03	0.216301E+05	-0.125472E+02	-0.286492E

109	19	1	1	-0.425933E+02	0.177421E+05	-0.115167E+03	0.522225E
110	20	1	1	-0.852782E+02	0.148759E+05	0.447546E+02	0.123125E
111	21	1	1	0.419800E+03	0.208458E+05	0.784515E+01	-0.848678E
112	22	1	1	0.402690E+03	0.205723E+05	0.107676E+02	0.112853E
113	23	1	1	0.353939E+03	0.186384E+05	0.434208E+02	0.102231E
114	24	1	1	0.422415E+02	0.164075E+05	0.722564E+01	0.767311E
115	25	1	1	0.483730E+03	0.212003E+05	-0.110830E+02	-0.183662E
116	26	1	1	0.855943E+03	0.213080E+05	0.260463E+02	-0.107525E
117	27	1	1	0.411701E+03	0.217331E+05	0.185557E+02	-0.153453E
118	28	1	1	0.493017E+02	0.216361E+05	0.233374E+02	-0.900981E
119	29	1	1	0.421230E+03	0.210689E+05	0.288468E+01	-0.113321E
120	30	1	1	0.400005E+03	0.210154E+05	-0.260228E+01	-0.158576E
121	31	1	1	0.384507E+03	0.205108E+05	-0.206248E+01	-0.540002E
122	32	1	1	0.125770E+03	0.204220E+05	-0.650713E+02	-0.976947E

123

124

125

126

127 STATIC SOLUTION TIME LOG

128

129 EQUATION SOLUTION = 1.00

130 DISPLACEMENT OUTPUT = 0.00

131 STRESS RECOVERY = 0.00

132

END

DATE

FILMED

DTIC

JULY 88