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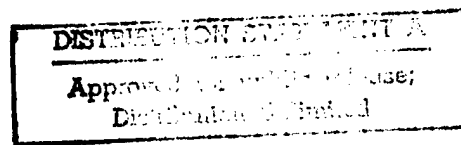
Tunnel Closure Experiment 1997 Test Programme

**Core Logging and Laboratory Tests on
Diamond Drilled Holes at Jerntoppen**

Post-Final Rep

923033-13

8 June 1998



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Tunnel Closure Experiment 1997 Test Programme

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Diamond Drilled Holes at Jerntoppen

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8 June 1998

Client: Waterways Experiment Station

Contact person: Will McMahon
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For the Norwegian Geotechnical Institute

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Summary

The Tunnel Closure Experiment - 1997 Test Programme included seven tunnel response tests and two tunnel portal tests. The tests were carried out in August 1997 (eight tests) and September 1997 (one test) in AS Sydvaranger's open pit mine at Bjørnevatn, Kirkenes, Norway.

Rock cores were collected for each of the tunnel response tests previous to drilling of the large diameter boreholes for bomb emplacements.

The rock cores have been analysed with respect to rock type, Rock Quality Designation (RQD, apparent and true), joint frequency, Joint Alteration Factor (J_a), Joint Roughness Factor (J_r), joint filling, joint angle related to the core, crushed core and core loss.

Laboratory tests on specimens from the rock cores have been carried out. The tests include pressure-wave velocity (V_p), shear-wave velocity (V_s), density, Uniaxial Compressive Strength (UCS), Young's modulus (E-modulus) and Poisson's ratio.

This report gives the results of the core logging and the results of the laboratory tests.



Contents

1 INTRODUCTION.....	4
2 CORE LOGGING.....	4
2.1 General.....	4
2.2 General geological description based on the core logging	5
2.3 Detailed geological description of each borehole.....	6
3 LABORATORY TEST.....	7

Review and reference document

List of Figures

1	Map of the site Jerntoppen. Locations of boreholes
2.1-8.2	Core logging. Photos of cores
9	Q-system parameters
10.1-10.33	Unconfined compression tests
11.1-11.4	Photos of specimens for laboratory testing



1 INTRODUCTION

The Tunnel Closure Experiment - 1997 Test Programme included seven tunnel response tests and two tunnel portal tests. The tests were carried out in August 1997 (eight tests) and September 1997 (one test) in AS Sydvaranger's open pit mine at Bjørnevåtn, Kirkenes, Norway. The locations of the tunnel response tests at the site Jerntoppen tunnel are shown in Figure 1.

Rock cores were collected for each of the tunnel response tests previous to drilling of the large diameter boreholes for bomb emplacements. The locations of the diamond drilled boreholes were almost similar to the locations of the large diameter boreholes, *i.e.* similar start point, similar direction, but the diamond drilling was generally stopped approx. 1 m above the tunnel ceiling. The diameter of the rock cores was 35 mm.

This report gives the results of the core logging and the results of the laboratory tests on specimens collected from the cores. Laboratory tests include pressure-wave velocity (V_P), shear-wave velocity (V_S), density, Uniaxial Compressive Strength (UCS), Young's modulus (E-modulus) and Poisson's ratio.

2 CORE LOGGING

2.1 General

Seven boreholes have been analysed. Table 1 summarises the boreholes.

Table 1 Diamond drilled holes at Jerntoppen

Borehole No.	Core length	Location
97-1	11.0 m	Jerntoppen adit extension
97-2	20.0 m	Jerntoppen adit extension
97-3	20.0 m	"Old" Jerntoppen tunnel
97-5	15.5 m	Jerntoppen adit extension
97-6	17.0 m	Jerntoppen adit extension
97-7	14.5 m	Jerntoppen adit extension
97-8	8.2 m	Jerntoppen adit extension

The rock cores have been analysed with respect to rock type, Rock Quality Designation (RQD, apparent and true), joint frequency, Joint Alteration Factor (J_a), Joint Roughness Factor (J_r), joint filling, joint angle related to the core, crushed core and core loss. Reference to the Q-system parameters included in the analysis is given in Figure 9.



The true RQD, which is a measure of the degree of natural jointing, is supplemented by the apparent RQD. The apparent RQD also includes cracks which have been induced by previous blasting and by the diamond drilling itself. It should be noted that it has been difficult to determine the true RQD and joint frequency in sections which include crushed core.

2.2 General geological description based on the core logging

The main rock type in the boreholes is a fine grained, dark grey or greenish gneiss usually containing hornblende and biotite. There are random bands of quartz veins. Some quartz veins have a thickness of a few centimetres while others are only a few millimetres thick. The rock has generally a wavy foliation due to small scale folding. Predominant RQD values are in the range 90-100%.

The following joint sets have been identified:

Foliation joints, angle 70-80°.

Joints, angle 10-30°.

Joints, angle 30-45°.

The angles given above and in the following, are generally the smallest angle between the joint and the core.

The frequency of the foliation joints seems to be dependent on the rock type to some extent. The angle is mostly 70-80°, but varies due to folding between 60° and 90°. In the cores, several breakage's per meter parallel to the foliation have been registered, but few of these are natural joints. The spacing for the foliation joints in the range 0.75-2 m seems to be realistic. The joints are undulating due to small scale folding, occasionally rough, and mainly calcite and to a less extent biotite coating, have been observed. Rust stains have been registered, but there is no clay coating on foliation joints.

The joints with angles in the range 10-30° are rough and mostly slightly undulating. Mostly, they have a coating of calcite and/or rust. In sections, in some of the boreholes, several of these joints per meter have been found, and since they have an acute angle to the borehole axis the spacing is assumed to be small. Based on survey in the tunnel below we assume that only a few of these joints are pervasive, and the majority are only short cracks. A realistic spacing is therefore estimated to be 1-2 m.

The third joint set has angles in the range 30-40°. The majority is rough and undulating to planar. They also have mineral coating; calcite, biotite and rust stains. From core logging observations these joints seem to be short (a few meters) and the spacing is in the range 0.5-2 m.



2.3 Detailed geological description of each borehole

Borehole 97-1 (Figure 2). The rock is a grey biotite gneiss. There is significant blasting damage in the upper two meters of the borehole, RQD below two meters is 95-100% and joint frequency is largely 1-5 per meter. The main joint set has an angle of 45° and has often calcite or rust coating. Foliation joints are rare, but the foliation has an angle of approx. 70°. Some joints with angle 20-30° without coating have also been found.

Borehole 97-2 (Figure 3). The rock in this borehole is a grey biotite gneiss. The whole borehole shows a rather low apparent RQD, probably due to blast damage. True RQD is normally 100%, and the joint frequency is 1-4 per meter. Predominant joints have an acute angle related to the core and they have calcite coating or rust stains. Foliation joints with angle 70-80° may have similar coating. In addition, some joints with angle 30-45° have been found.

Borehole 97-3 (Figure 4). The rock in this borehole is a grey to greenish biotite gneiss. At about 11 m depth there is a distinct white coloured band. The whole borehole shows a rather low apparent RQD number, due to blast damage or induced by the core drilling. True RQD is normally in the interval 90-100%, and the joint frequency is 1-5 per meter. The predominant joint set has angle 60° and calcite coating or rust stains. Also foliation joints with angle 10-30° may have such coating. In addition, some joints with angle 30-45° with the same coating have been found.

Borehole 97-5 (Figure 5). The rock in the upper part of the borehole (0-3.0 m) is a grey biotite gneiss. The lower part of the borehole (4.0-15.5 m) is a greenish amphibolitic gneiss. There is significant material diskings, especially at depth 4.0 - 15.5 m. The joint frequency is 0-1 per meter at depth 0-5 m where RQD is 100%. At depth 5-15 m RQD is 50-100%. The major joint set has angle 70-90° and often biotite coating. Also, some foliation joints with angle 30-45°, and some joints with angle 20° have been registered.

Borehole 97-6 (Figure 6). The rock in this borehole is mainly grey biotite gneiss. The joint frequency varies between 1 and 3 per meter and RQD varies between 60% and 100% in the upper part of the borehole. In the lower part of the borehole the joint frequency is largely 1-5 per meter and RQD varies between 40% and 100%. The major joint set has angle 70-90° and coating of biotite, calcite or rust stains is common. A few foliation joints with angle 30-45° with rust stains or calcite have been registered.

Borehole 97-7 (Figure 7). The rock in this borehole is a grey biotite gneiss with relatively few joints. RQD is normally 100%, and the joint frequency is largely 1-2 per meter in the whole borehole. The predominant joint set has angle 30-45° and calcite coating or rust stains. In addition, some joints with angle 30-45°



with the same coating have been observed. Only one foliation joint has been registered in this borehole.

Borehole 97-8 (Figure 8). The rock in this borehole is mainly grey biotite gneiss alternating with grey/greenish biotite gneiss. There is some material diskings at depth 6.0-8.0 m. The joint frequency varies between 1 and 5 per meter in the upper two meters of the borehole and RQD is 60-80%. The joint frequency is 1-3 per meter in the lower part of the borehole and RQD is 80-100%. The major joint set has angle 20-30°. The coating is mainly rust stains and occasionally biotite or calcite. A few joints with angle 30-60° with biotite and calcite coating have been registered.

3 LABORATORY TEST

Specimens for laboratory test are generally collected from the deepest two meters of the cores. Prior to the uniaxial compression testing, P-wave and S-wave measurements were carried out. Unconfined compression tests and photos of the specimens are shown in Figures 10.1-10.33 and Figures 11.1-11.4 respectively.

A summary of the test results is given in Table 2. P-wave velocities range from 5,001 to 5,819 m/s with a mean value of 5,401 m/s. S-wave velocities range from 2,919 to 3,805 m/s with a mean value of 3,324 m/s.

Density varies between 2,724 and 3,573 kg/m³ with a mean value of 3,035 kg/m³.

Uniaxial compressive strength varies between 85 MPa and 446 MPa with a mean value at 248 MPa. Young's modulus varies between 23.4 GPa and 80.1 GPa with a mean value of 55.4 GPa. Young's modulus was taken as secant modulus between 10% and 40% of the first 100 MPa (or lower for a few specimens with UCS lower than 100 MPa).

The measured mean value for Poisson's ratio is 0.30. Poisson's ratio varies between 0.18 and 0.48. Poisson's ratio was calculated between 60% and 90% of the first 100 MPa of each specimen for boreholes 97-1 and 97-2. For the other boreholes Poisson's ratio was calculated between 10% and 40% of the first 100 MPa.



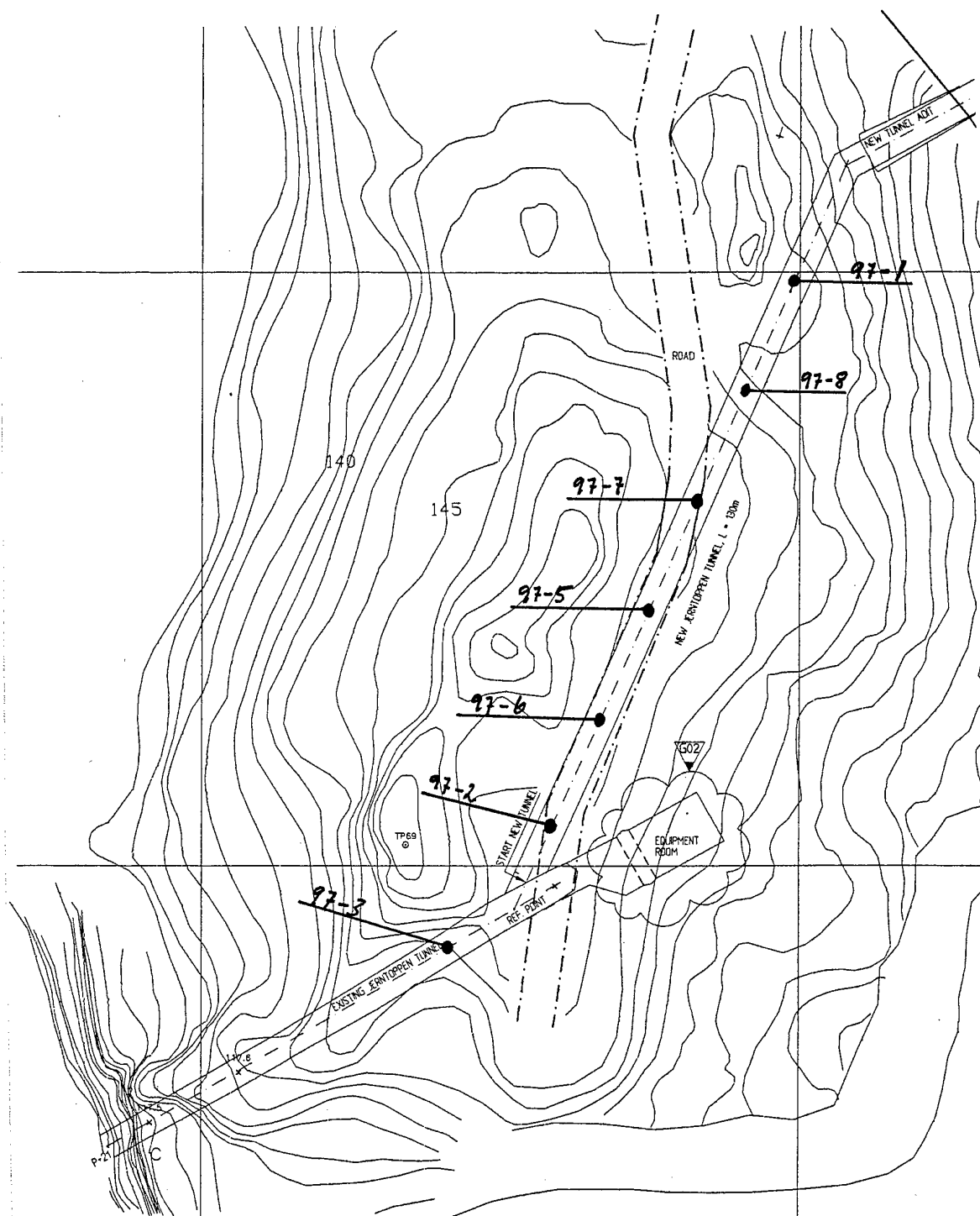
Table 2 The results of the laboratory tests

Test	Sample	Depth	Length	Diameter	Vp	Vs	Density	UCS	E mod	Poisson
no	no	m	mm	mm	m/s	m/s	kg/m3	MPa	GPa	ratio
UCT352/1	97/1-1	7.11	84.34	35.05	5365	3501	3502	361.70	80.06	0.306
UCT353/2	97/1-2	9.19	83.70	35.08	5449	3459	3517	445.83	78.97	0.413
UCT354/3	97/1-3	9.28	83.71	35.07	5471	3805	3422	357.27	75.29	0.43
UCT355/4	97/1-4	10.38	84.05	34.86	5388	3445	3573	269.37	73.58	0.343
UCT356/5	97/1-5	10.56	84.35	34.95	5339	3361	3501	306.80	74.1	0.361
maximum	maximum		84.35	35.08	5471	3805	3573	445.83	80.06	0.43
mean	average		84.03	35.00	5402	3514	3503	348.19	76.40	0.37
minimum	minimum		83.70	34.86	5339	3361	3422	269.37	73.58	0.31
UCT357/6	97/2-1	17.02	83.46	35.02	5197	3371	3139	376.50		
UCT358/7	97/2-2	17.10	84.06	34.85	5538	3479	3130	337.26	76.79	0.257
UCT359/8	97/2-3	19.15	83.46	34.87	5455	2939	3283	290.87	66.13	0.355
UCT360/9	97/2-4	19.39	84.09	34.86	5689	3587	3354	197.93	68.83	0.304
UCT361/10	97/2-5	19.50	82.87	34.90	5532	3447	3241	246.88	75.42	0.253
maximum	maximum		84.09	35.02	5689	3587	3354	376.50	76.79	0.36
mean	average		83.59	34.90	5482	3365	3229	289.89	71.79	0.29
minimum	minimum		82.87	34.85	5197	2939	3130	197.93	66.13	0.25
UCT362/11	97/3-1	10.70	82.86	35.04	5159	3196	2764	152.62	31.58	0.21
UCT363/12	97/3-2	13.20	82.89	35.15	5266	2919	2724	85.02	23.42	0.298
UCT364/13	97/3-3	17.44	82.92	35.02	5001	3153	2753	107.46	24.67	0.37
UCT365/14	97/3-4	17.53	82.94	34.98	5039	3142	2755	99.29	24.01	0.18
UCT366/15	97/3-5	17.71	82.90	35.09	5061	3251	2728	88.14	26.88	0.211
maximum	maximum		82.94	35.15	5266	3251	2764	152.62	31.58	0.37
mean	average		82.90	35.06	5105	3132	2745	106.51	26.11	0.25
minimum	minimum		82.86	34.98	5001	2919	2724	85.02	23.42	0.18
UCT367/16	97/5-1	14.14	83.24	34.67	5356	3083	2982	413.58	75.69	0.244
UCT368/17	97/5-2	14.23	83.26	34.69	5317	3263	3013	362.63	41.26	0.22
UCT369/18	97/5-3	14.82	83.85	34.53	5410	3448	3170	309.12	49.25	0.28
UCT370/19	97/5-4	14.91	83.86	34.56	5341	3420	3157	353.61	54.86	0.293
UCT371/20	97/5-5	15.16	83.95	34.50	5545	3144	3149	267.03	57.5	0.406
maximum	maximum		83.95	34.69	5545	3448	3170	413.58	75.69	0.41
mean	average		83.63	34.59	5394	3272	3094	341.19	55.71	0.29
minimum	minimum		83.24	34.50	5317	3083	2982	267.03	41.26	0.22
UCT372/21	97/6-1	10.46	83.76	35.22	5376	3149	2784	345.16	73.52	0.342
UCT373/22	97/6-2	10.55	83.79	35.20	5448	3394	2858	123.40	51.72	0.277
UCT374/23	97/6-3	11.18	83.84	35.10	5711	3598	3305	259.83	52.94	0.296
UCT375/24	97/6-4	11.74	83.92	34.85	5366	3453	3147	424.50		
UCT376/25	97/6-5	11.83	83.80	34.83	5218	3139	2912	309.94	53.27	0.24
maximum	maximum		83.92	35.22	5711	3598	3305	424.50	73.52	0.34
mean	average		83.82	35.04	5424	3347	3001	292.57	57.86	0.29
minimum	minimum		83.76	34.83	5218	3139	2784	123.40	51.72	0.24



Table 2 (continued)

Test	Sample	Depth	Length	Diameter	Vp	Vs	Density	UCS	E mod	Poisson
no	no	m	mm	mm	m/s	m/s	kg/m3	MPa	GPa	ratio
UCT378/27	97/7-1	13.25	82.86	34.87	5819	3603	2955	178.40	58.49	0.472
UCT378/27	97/7-2	13.32	83.13	34.89	5702	3325	2820	319.60	65.63	0.184
UCT379/28	97/7-3	13.64	83.10	34.85	5555	3208	2883	273.10	72.38	0.197
UCT380/29	97/7-4	13.73	83.11	34.94	5418	3120	2936	205.74	65.39	0.241
UCT381/30	97/7-5	13.82	83.12	34.95	5534	3079	2872	245.73	60.39	0.18
maximum	maximum		83.13	34.95	5819	3603	2955	319.60	72.38	0.47
mean	average		83.06	34.90	5605	3267	2893	244.51	64.46	0.25
minimum	minimum		82.86	34.85	5418	3079	2820	178.40	58.49	0.18
UCT382/31	97/8-1	7.00	84.08	35.17	5355	3284	2774	136.13	32.14	0.482
UCT383/32	97/8-2	7.23	84.07	35.15	5368	3320	2811	97.99	36.17	0.323
UCT384/33	97/8-3	7.70	83.91	35.13	5291	3367	2789	-	-	-
UCT385/34	97/8-4	7.79	83.94	35.17	5402	3423	2757	95.56	35.98	0.373
UCT386/35	97/8-5	7.88	83.92	35.14	5558	3456	2765	134.27	38.42	0.33
maximum	maximum		84.08	35.17	5558	3456	2811	136.13	38.42	0.48
mean	average		83.98	35.15	5395	3370	2779	115.99	35.68	0.38
minimum	minimum		83.91	35.13	5291	3284	2757	95.56	32.14	0.32



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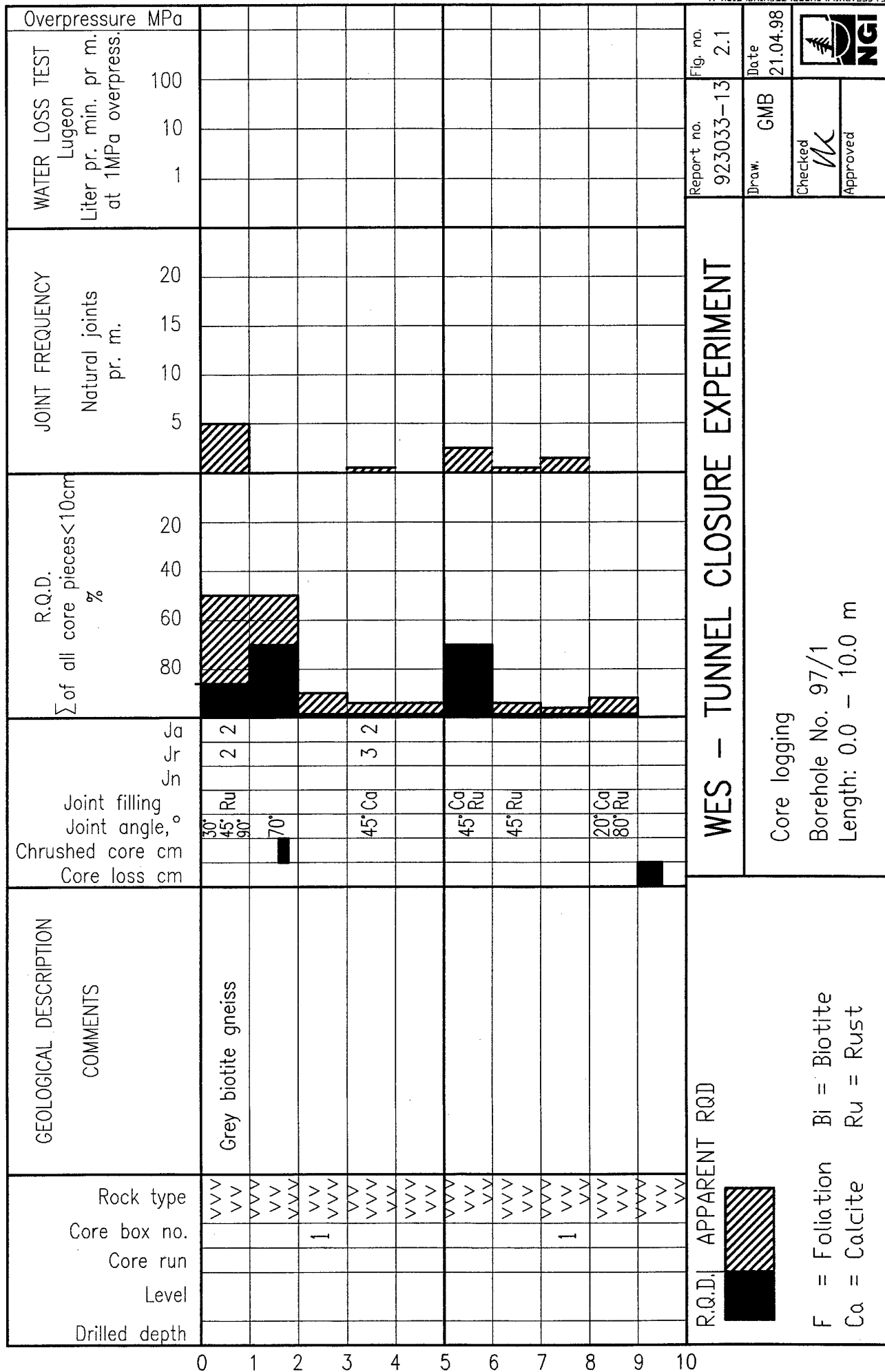
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TUNNEL RESPONSE TESTS
Map of the site Jerntoppen
Locations of boreholes
Scale 1:1000

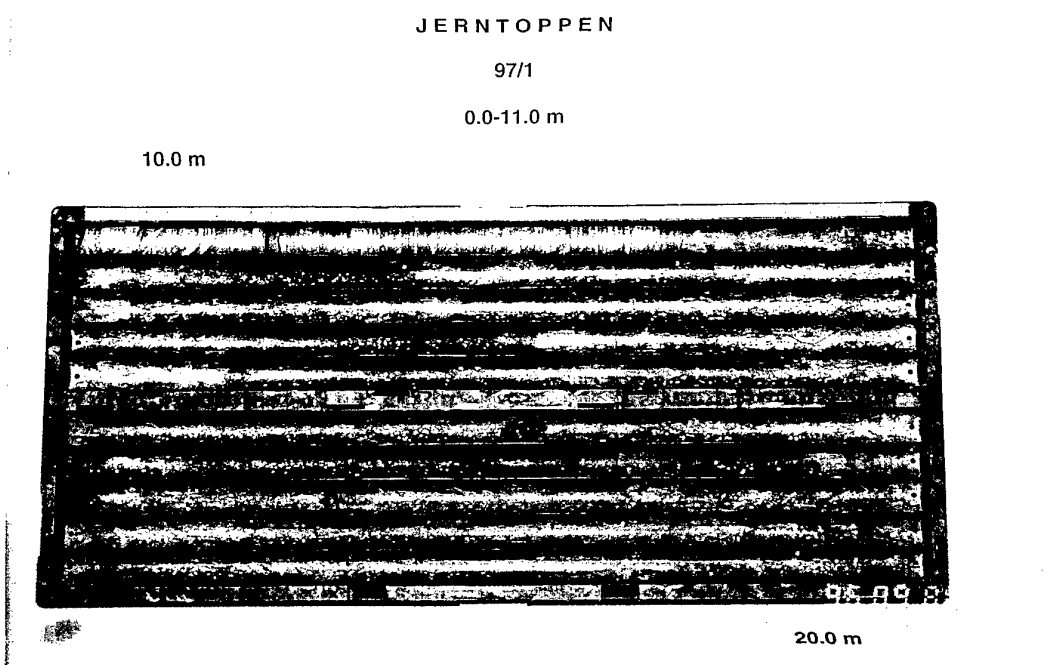
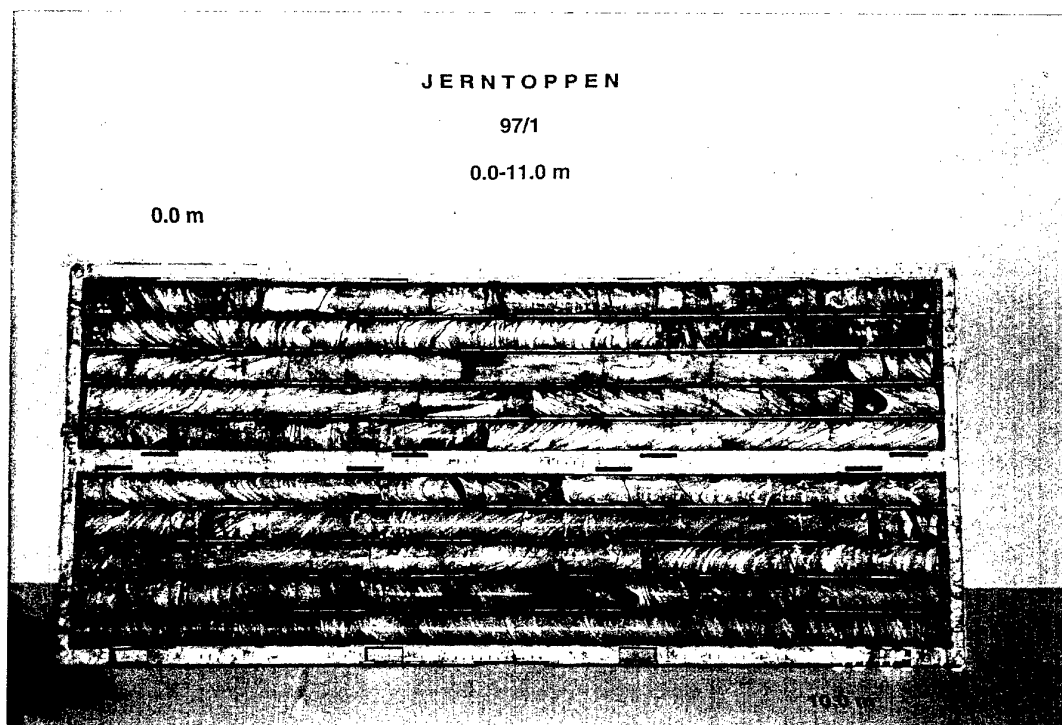
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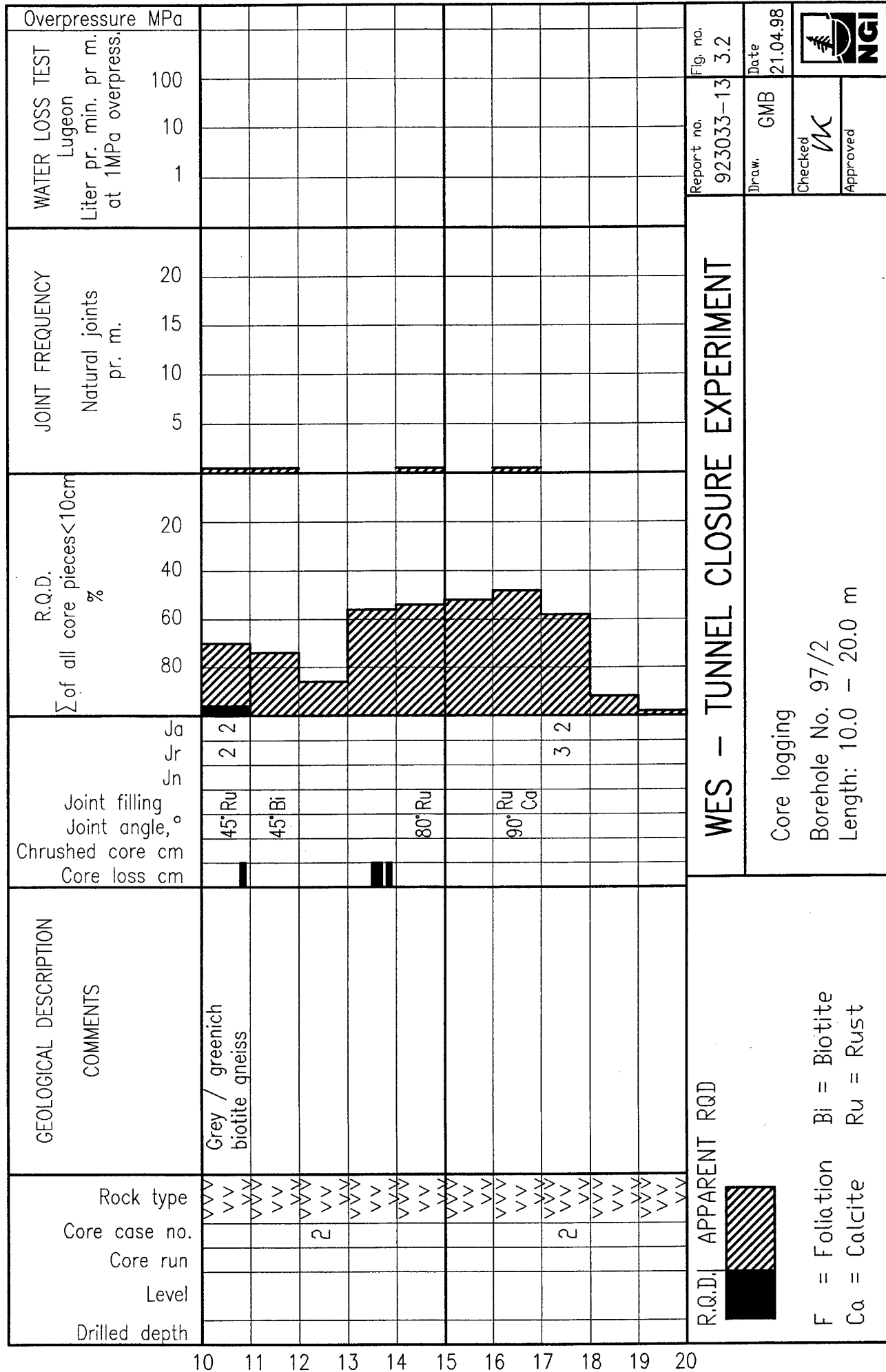


Overpressure MPa		WATER LOSS TEST		JOINT FREQUENCY				R.Q.D.				Joint filling				GEOLOGICAL DESCRIPTION		Rock type	
Lugeon		Liter pr. min. pr m. at 1MPa overpress.		Natural joints pr. m.				Σ of all core pieces<10cm %				Joint angle, °				COMMENTS		Core box no.	
100		10		20				20				Ja				Grey biotite gneiss		VVV	
1				15				40				Jr						VVV	
				10				60				Jn						VVV	
				5				80											

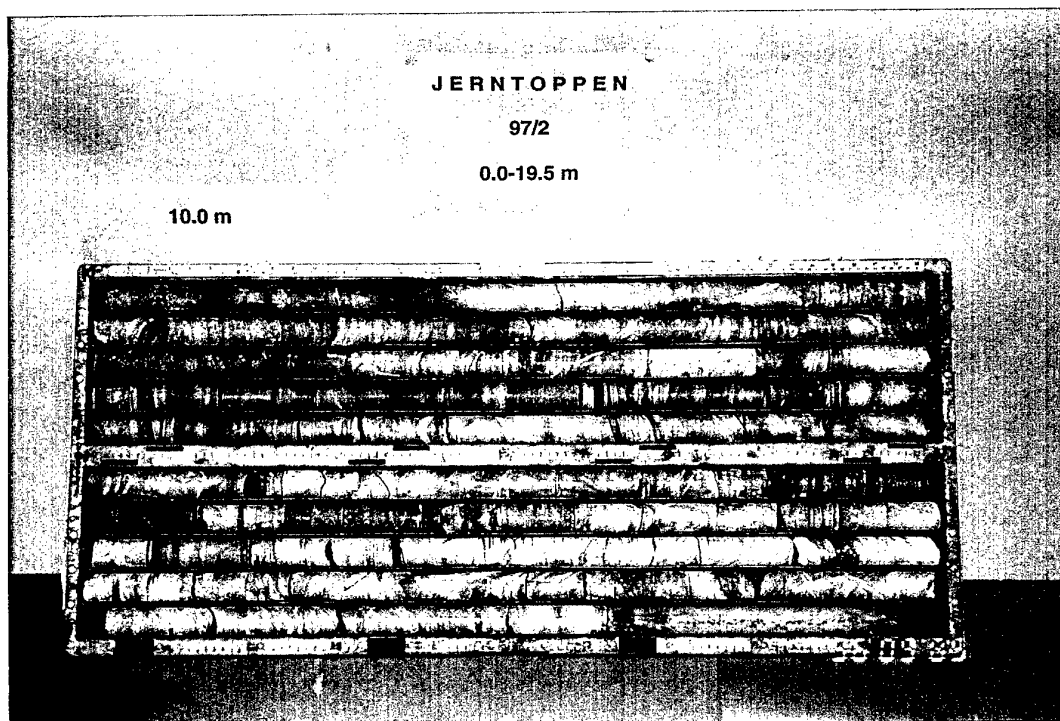
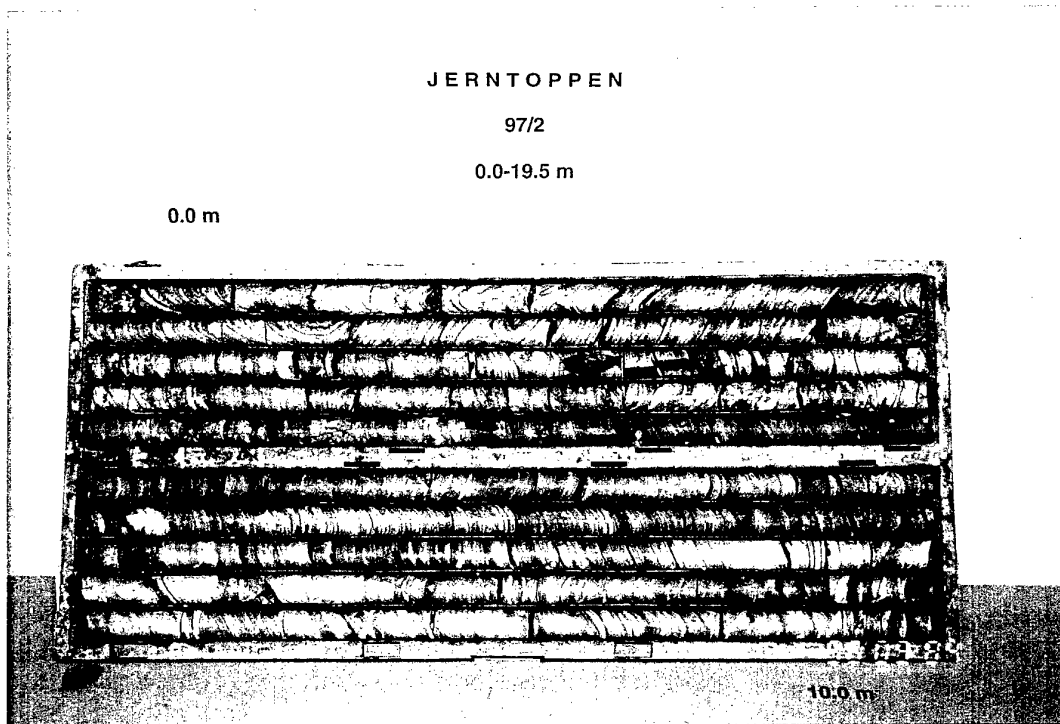


WES - TUNNEL CLOSURE EXPERIMENT	Report No. 933033-13	Figure No. 2.3
Photos of diamond drilled cores Borehole 97/1	Drawn by PC	Date 97-02-28
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	Approved	

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WES - TUNNEL CLOSURE EXPERIMENT

Report No.
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Figure No.
3.3

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PC

Date
97-02-28

Photos of diamond drilled cores.
Borehole 97/2

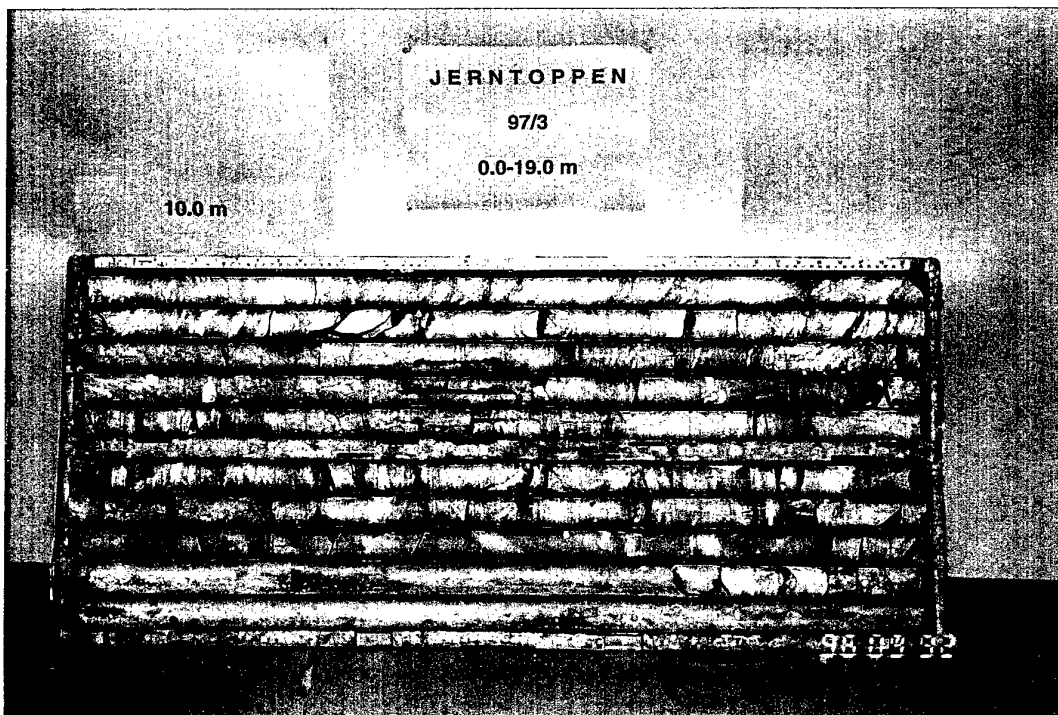
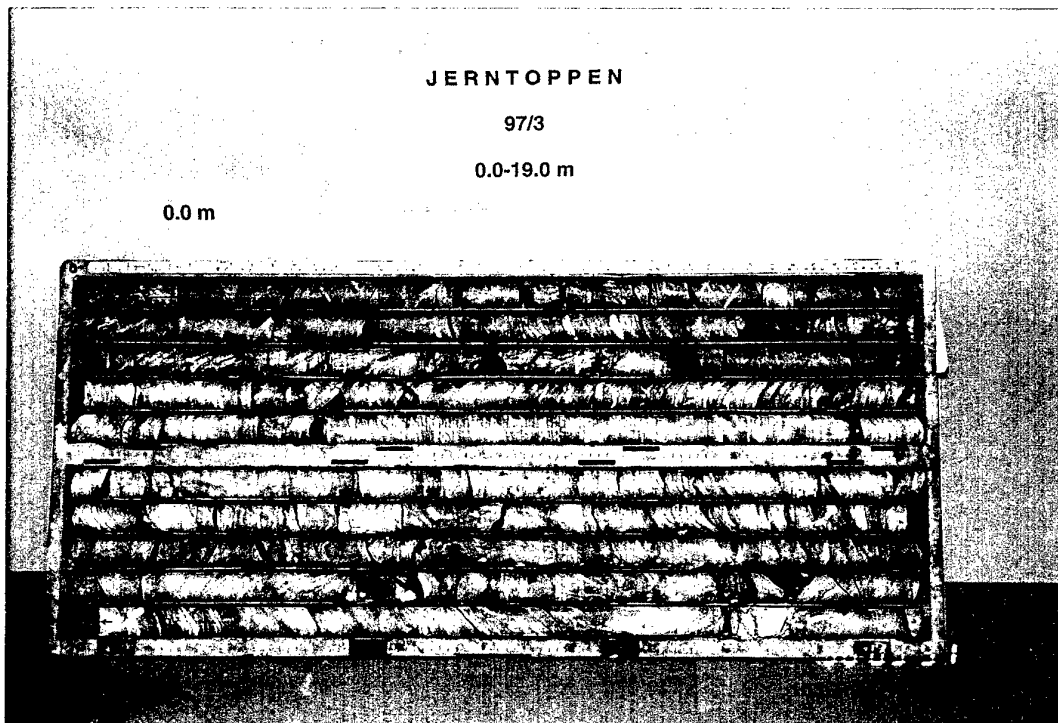
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Overpressure MPa				WATER LOSS TEST				JOINT FREQUENCY				R.Q.D.				Joint filling				GEOLOGICAL DESCRIPTION				Rock type			
Lugeon				Liter pr. min. pr m. at 1MPa overpress.				Natural joints pr. m.				Σ of all core pieces < 10cm %				Joint angle, °				COMMENTS				Core box no.			
100				10				5				20				Ja				Grey biotite gneiss with				vvvv			
1												40				Jr				light bands							
												60				Jn				Grey biotite gneiss				vvvv			
												80				60° Ca								vvvv			
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WES - TUNNEL CLOSURE EXPERIMENT

Report No.
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Figure No.
4.3

Drawn by
PC

Date
97-02-28

Photos of diamond drilled cores
Borehole 97/3

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R.Q.D./ APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT				Report no. 923033–13		Fig. no. 5.2	
R.Q.D./ APPARENT RQD				Core logging				Draw. GMB		Date 21.04.98	
F = Foliation Ca = Calcite				Borehole No. 97/5 Length: 10.0 – 15.5 m				Checked <i>M</i>		Approved	
Greenish amphibolitic gneiss				Core logging				Borehole No. 97/5		Length: 10.0 – 15.5 m	
F = Foliation Ca = Calcite				Borehole No. 97/5				Length: 10.0 – 15.5 m			
F = Foliation Ca = Calcite				Borehole No. 97/5				Length: 10.0 – 15.5 m			
F = Foliation Ca = Calcite				Borehole No. 97/5				Length: 10.0 – 15.5 m			
F = Foliation Ca = Calcite				Borehole No. 97/5				Length: 10.0 – 15.5 m			
F = Foliation Ca = Calcite				Borehole No. 97/5				Length: 10.0 – 15.5 m			
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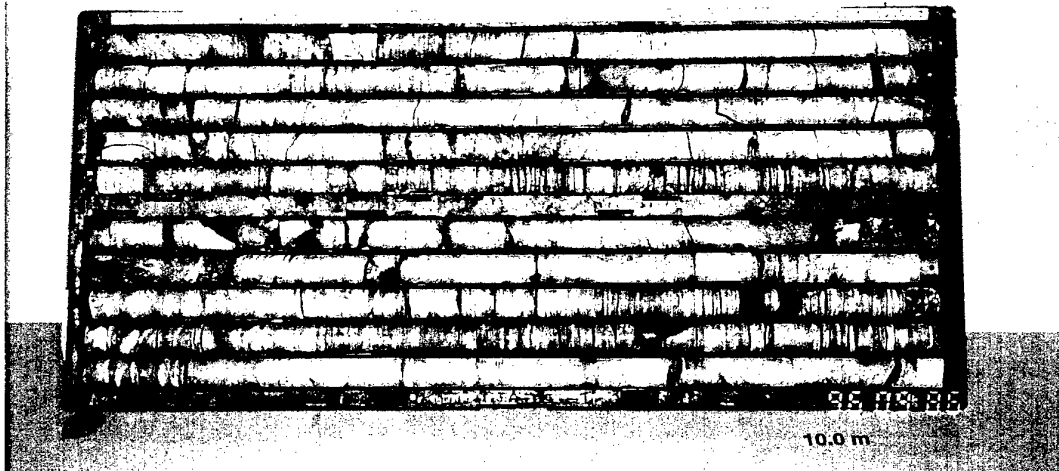
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JERNTOPPEN

97/5

0.0-15.5 m

0.0 m

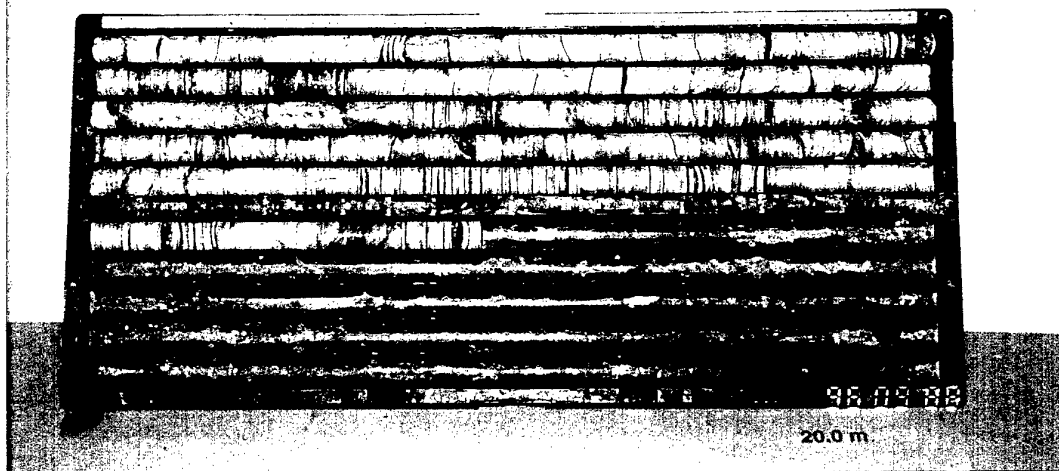


JERNTOPPEN

97/5

0.0-15.5 m

10.0 m



WES - TUNNEL CLOSURE EXPERIMENT

Report No.
933033-13

Figure No.
5.3

Drawn by
PC

Date
97-02-28

Photos of diamond drilled cores
Borehole 97/5

Checked
VK
Approved



Overpressure MPa		WATER LOSS TEST Lugeon Liter pr. min. pr m. at 1MPa overpress.		JOINT FREQUENCY Natural joints pr. m.		R.Q.D. Σ of all core pieces < 10cm %		Joint filling Joint angle, ° Crushed core cm Core loss cm		GEOLOGICAL DESCRIPTION COMMENTS		Rock type Core box no. Core run Level Drilled depth	
100	10	1	20	20	80	Ja Jr Jn	45° Ru	2	2	2	2	1	0
							70° Ru	2	2	2	2	1	1
							80° Ru	2	2	2	2	1	2
							45° Ru	3	2	2	2	1	3
							80° Ca	1.5	2	2	2	1	7
							90° Bi			2	2	1	8
							80° Ca			2	2	1	9
							90° Bi			2	2	1	10

WES - TUNNEL CLOSURE EXPERIMENT

Core logging

Borehole No. 97/6

Length: 0.0 - 10.0 m

Report no. 923033-13

Fig. no. 6.1

Draw. GMB

Date 21.04.98

Checked

Approved

R.Q.D. APPARENT RQD

APPARENT RQD

F = Foliation Bi = Biotite

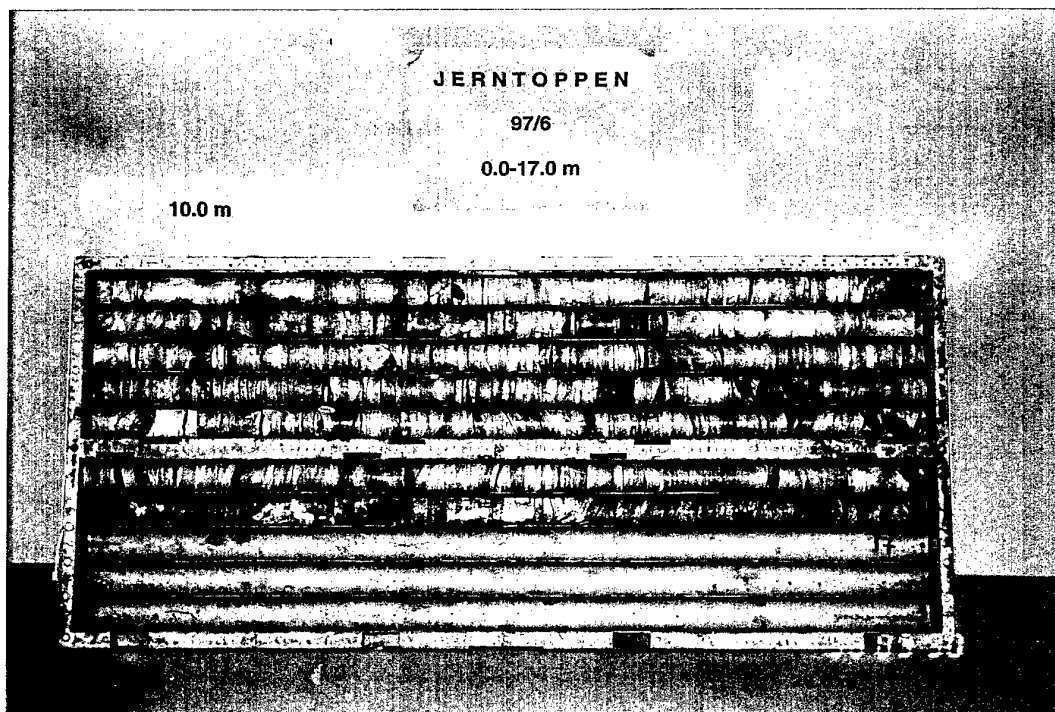
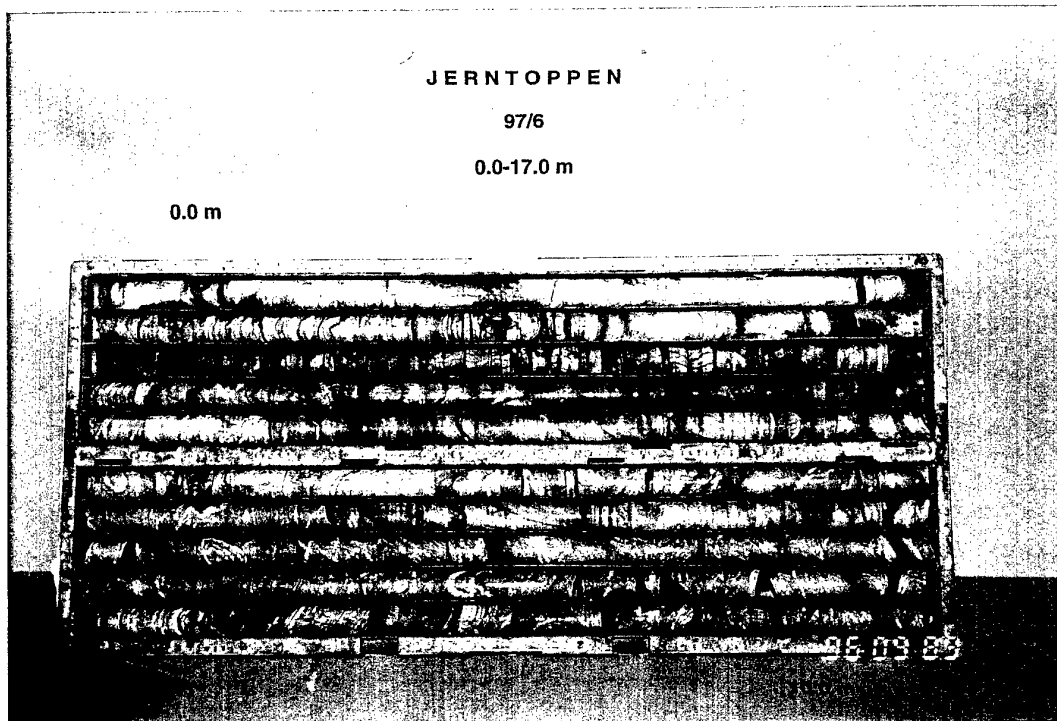
Ca = Calcite Ru = Rust

Core logging

Borehole No. 97/6

Length: 0.0 - 10.0 m

[illegible]



WES - TUNNEL CLOSURE EXPERIMENT

Report No.
933033-13

Figure No.
6.3

Drawn by
PC

Date
97-02-28

Photos of diamond drilled cores
Borehole 97/6

Checked
VK
Approved



WES - TUNNEL CLOSURE EXPERIMENT			
Core logging			
Borehole No. 97/7			
Length: 0.0 - 10.0 m			
Rock type Core box no. Core run Level Drilled depth Geological Description Comments Joint filling Joint angle, ° Crushed core cm Core loss cm R.Q.D. Σ of all core pieces < 10cm % JOINT FREQUENCY Natural joints pr. m. WATER LOSS TEST Lugeon Liter pr. min. pr m. at 1MPa overpress. Overpressure MPa			

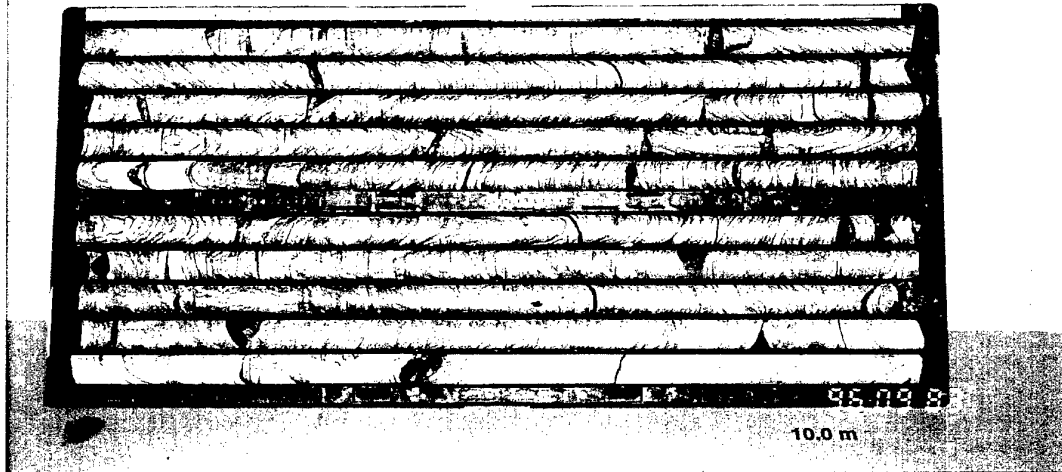
5NGEDARKIV\SKRED\TEGNING\FISK-A

JERNTOPPEN

97/7

0.0-15.0 m

0.0 m



JERNTOPPEN

97/7

0.0-15.0 m

10.0 m



WES - TUNNEL CLOSURE EXPERIMENT

Report No.
933033-13

Figure No.
7.3

Drawn by
PC

Date
97-02-28

Photos of diamond drilled cores
Borehole 97/7

Checked
VK
Approved



R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										Report no.	Fig. no.
R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										923033–13	8.1
R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										Draw.	Date
R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										GMB	21.04.98
R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										Checked	Approved
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R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										923033–13	8.1
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R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										GMB	21.04.98
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R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										Report no.	Fig. no.
R.Q.D. APPARENT RQD				WES – TUNNEL CLOSURE EXPERIMENT										923033–13	



F = Foliation Bi = Biotite
Ca = Calcite Ru = Rust

Core logging
Borehole No. 97/8
Length: 0.0 - 8.2 m



Draw. GMB
Checked *W*
Approved

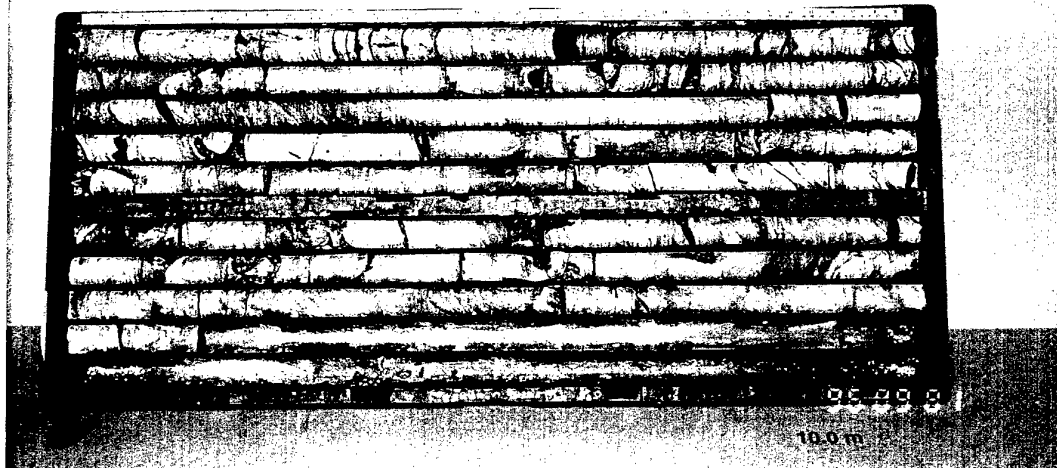
Date 21.04.98
Fig. no. 8.1

JERNTOPPEN

97/8

0.0-9.0 m

0.0 m



WES - TUNNEL CLOSURE EXPERIMENT

Report No.
933033-13

Figure No.
8.2

Drawn by
PC

Date
97-02-28

Photo of diamond drilled cores
Borehole 97/8

Checked
W
Approved



1. Rock Quality Designation	RQD
A Very poor	0 - 25
B Poor	25 - 50
C Fair	50 - 75
D Good	75 - 90
E Excellent	90 - 100

Note: i) Where RQD is reported or measured as ≤ 10 (including 0), a nominal value of 10 is used to evaluate Q.
ii) RQD intervals of 5, i.e., 100, 95, 90, etc., are sufficiently accurate.

2. Joint Set Number	J_n
A Massive, no or few joints	0.5 - 1.0
B One joint set	2
C One joint set plus random joints	3
D Two joint sets	4
E Two joint sets plus random joints	6
F Three joint sets	9
G Three joint sets plus random joints	12
H Four or more joint sets, random, heavily jointed, "sugar cube", etc.	15
J Crushed rock, earthlike	20

Note: i) For intersections, use $(3.0 \times J_n)$
ii) For portals, use $2.0 \times J_n$

3. Joint Roughness Number	J_r
a) Rock-wall contact, and b) rock-wall contact before 10 cm shear	
A Discontinuous joints	4
B Rough or irregular, undulating	3
C Smooth, undulating	2
D Slickensided, undulating	1.5
E Rough or irregular, planar	1.5
F Smooth, planar	1.0
G Slickensided, planar	0.5
Note: i) Descriptions refer to small scale features and intermediate scale features, in that order.	
c) No rock-wall contact when sheared	
H Zone containing clay minerals thick enough to prevent rock-wall contact	1.0
J Sandy, gravelly or crushed zone thick enough to prevent rock-wall contact	1.0
Note: i) Add 1.0 if the mean spacing of the relevant joint set is greater than 3m. ii) $J_r = 0.5$ can be used for planar slickensided joints having lineations, provided the lineations are oriented for minimum strength.	

4. Joint Alteration Number	ϕ_r approx.	J_a
a) Rock-wall contact (no mineral fillings, only coatings)		
A Tightly healed, hard, non-softening, impermeable filling, i.e., quartz or epidote		0.75
B Unaltered joint walls, surface staining only	25-35°	1.0
C Slightly altered joint walls. Non-softening mineral coatings, sandy particles, clay-free disintegrated rock, etc.	25-30°	2.0
D Silty- or sandy-clay coatings, small clay fraction (non-softening)	20-25°	3.0
E Softening or low friction clay mineral coatings, i.e., kaolinite or mica. Also chlorite, talc, gypsum, graphite, etc., and small quantities of swelling clays.	8-16°	4.0
b) Rock-wall contact before 10 cm shear (thin mineral fillings)		
F Sandy particles, clay-free disintegrated rock, etc.	25-30°	4.0
G Strongly over-consolidated non-softening clay mineral fillings (continuous, but < 5 mm thickness)	16-24°	6.0
H Medium or low over-consolidation, softening, clay mineral fillings (continuous, but < 5 mm thickness)	12-16°	8.0
J Swelling-clay fillings, i.e., montmorillonite (continuous, but < 5 mm thickness). Value of J_a depends on percent of swelling clay-size particles, and access to water, etc.	6-12°	8-12
c) No rock-wall contact when sheared (thick mineral fillings)		
KLM Zones or bands of disintegrated or crushed rock and clay (see G, H, J for description of clay condition)	6-24°	6, 8, or 8-12
N Zones or bands of silty- or sandy-clay, small clay fraction (non-softening)	-	5.0
OPR Thick, continuous zones or bands of clay (see G, H, J for description of clay condition)	6-24°	10, 13, or 13-20

5. Joint Water Reduction Factor	approx. water pres. (kg/cm ²)	J_w
A Dry excavations or minor inflow, i.e., < 5 l/min locally	< 1	1.0
B Medium inflow or pressure, occasional outwash of joint fillings	1-2.5	0.66
C Large inflow or high pressure in competent rock with unfilled joints	2.5-10	0.5
D Large inflow or high pressure, considerable outwash of joint fillings	2.5-10	0.33
E Exceptionally high inflow or water pressure at blasting, decaying with time	> 10	0.2-0.1
F Exceptionally high inflow or water pressure continuing without noticeable decay	> 10	0.1-0.05

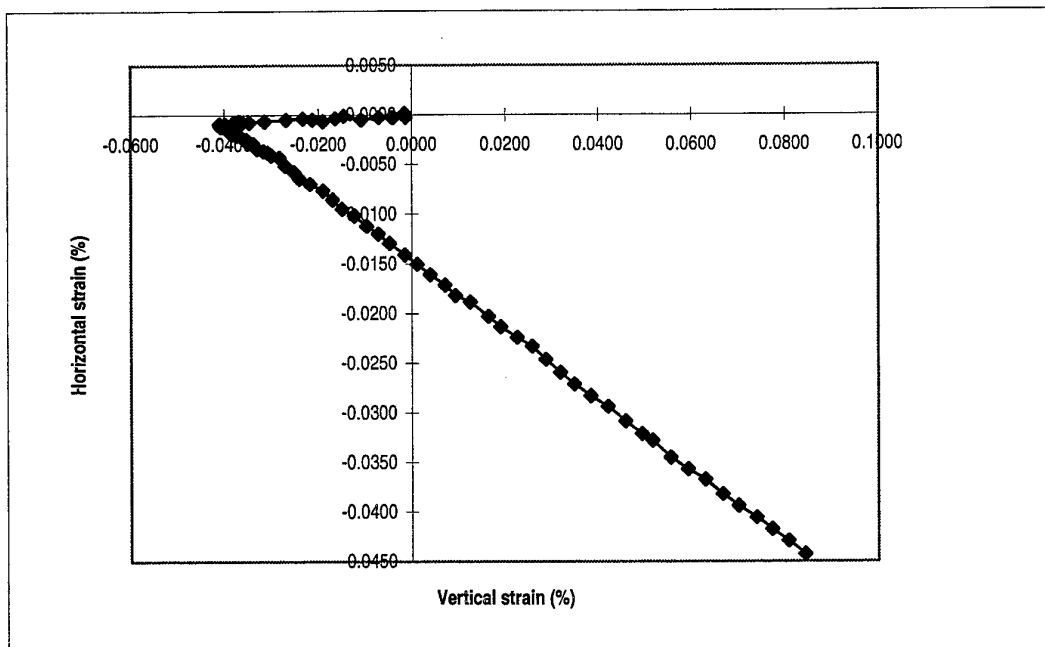
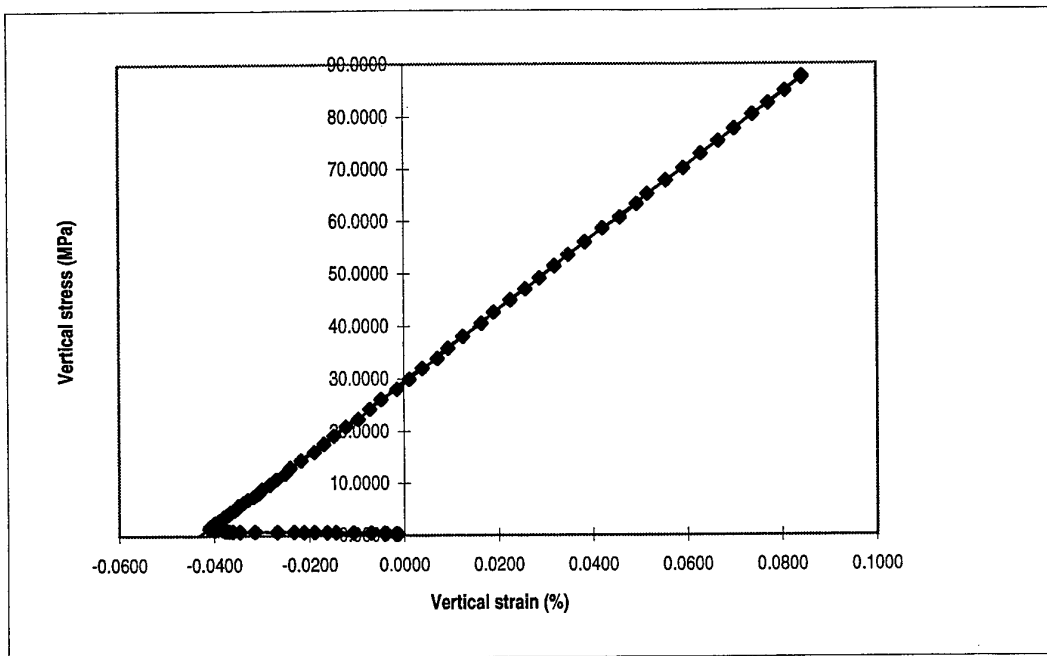
Note: i) Factors C to F are crude estimates. Increase J_w if drainage measures are installed.
ii) Special problems caused by ice formation are not considered.

6. Stress Reduction Factor				SRF
a) Weakness zones intersecting excavation, which may cause loosening of rock mass when tunnel is excavated				
A	Multiple occurrences of weakness zones containing clay or chemically disintegrated rock, very loose surrounding rock (any depth)			10
B	Single weakness zones containing clay or chemically disintegrated rock (depth of excavation ≤ 50 m)			5
C	Single weakness zones containing clay or chemically disintegrated rock (depth of excavation > 50 m)			2.5
D	Multiple shear zones in competent rock (clay-free), loose surrounding rock (any depth)			7.5
E	Single shear zones in competent rock (clay-free) (depth of excavation ≤ 50 m)			5.0
F	Single shear zones in competent rock (clay-free) (depth of excavation > 50 m)			2.5
G	Loose, open joints, heavily jointed or "sugar cube", etc. (any depth)			5.0
Note: i) Reduce these values of SRF by 25-50% if the relevant shear zones only influence but do not intersect the excavation.				
b) Competent rock, rock stress problems				
		σ_1/σ_3	σ_3/σ_c	SRF
H	Low stress, near surface, open joints	> 200	< 0.01	2.5
J	Medium stress, favourable stress condition	200-10	0.01-0.3	1
K	High stress, very tight structure. Usually favourable to stability, may be unfavourable for wall stability.	10-5	0.3-0.4	0.5-2
L	Moderate slabbing after > 1 hour in massive rock	5-3	0.5-0.65	5-50
M	Slabbing and rock burst after a few minutes in massive rock	3-2	0.65-1	50-200
N	Heavy rock burst (strain-burst) and immediate dynamic deformations in massive rock	< 2	> 1	200-400
Note: ii) For strongly anisotropic virgin stress field (if measured): when $5 \leq \sigma_1/\sigma_3 \leq 10$, reduce σ_c to $0.75\sigma_c$. When $\sigma_1/\sigma_3 > 10$, reduce σ_c to $0.5\sigma_c$, where σ_c = unconfined compression strength, σ_1 and σ_3 are the major and minor principal stresses, and σ_3 = maximum tangential stress (estimated from elastic theory). iii) Few case records available where depth of crown below surface is less than span width. Suggest SRF increase from 2.5 to 5 for such cases (see H).				
c) Squeezing rock: plastic flow of incompetent rock under the influence of high rock pressure				
		σ_3/σ_c		SRF
O	Mild squeezing rock pressure	1-5		5-10
P	Heavy squeezing rock pressure	> 5		10-20
Note: iv) Cases of squeezing rock may occur for depth $H > 350 Q^{1/3}$ (Singh et al., 1992). Rock mass compression strength can be estimated from $q = 0.7 \gamma Q^{1/3}$ (MPa) where γ = rock density in kN/m ³ (Singh, 1993).				
d) Swelling rock: chemical swelling activity depending on presence of water				
R	Mild swelling rock pressure			5-10
S	Heavy swelling rock pressure			10-15

Note: J_r and J_a classification is applied to the joint set or discontinuity that is least favourable for stability both from the point of view of orientation and shear resistance, τ (where $\tau = \sigma_n \tan^{-1} (J_r/J_a)$). Choose the most likely feature to allow failure to initiate.

$$Q = \frac{RQD}{J_n} \times \frac{J_r}{J_a} \times \frac{J_w}{SRF}$$

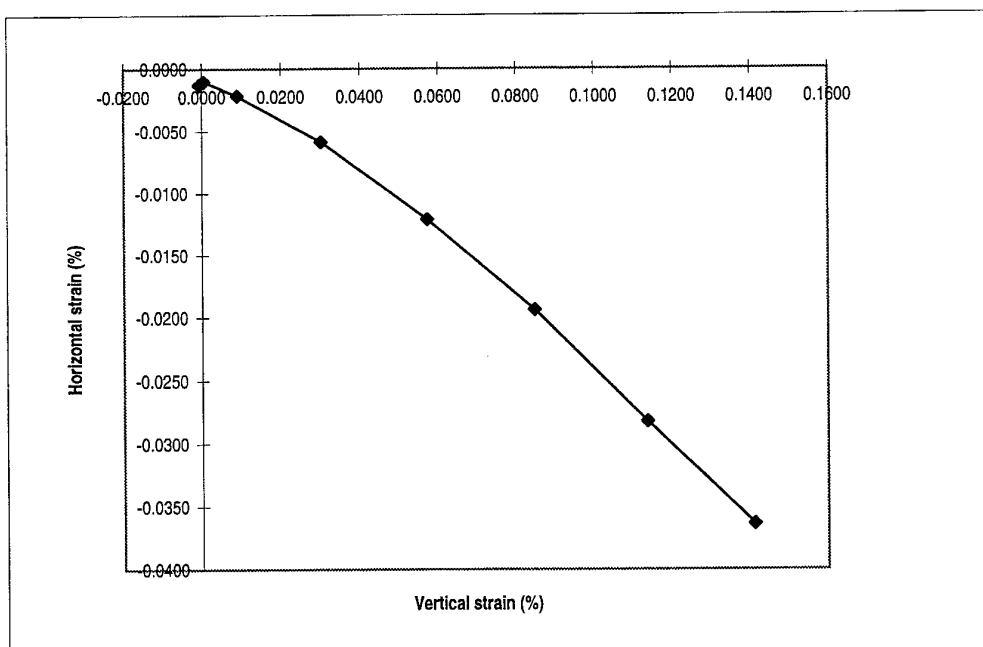
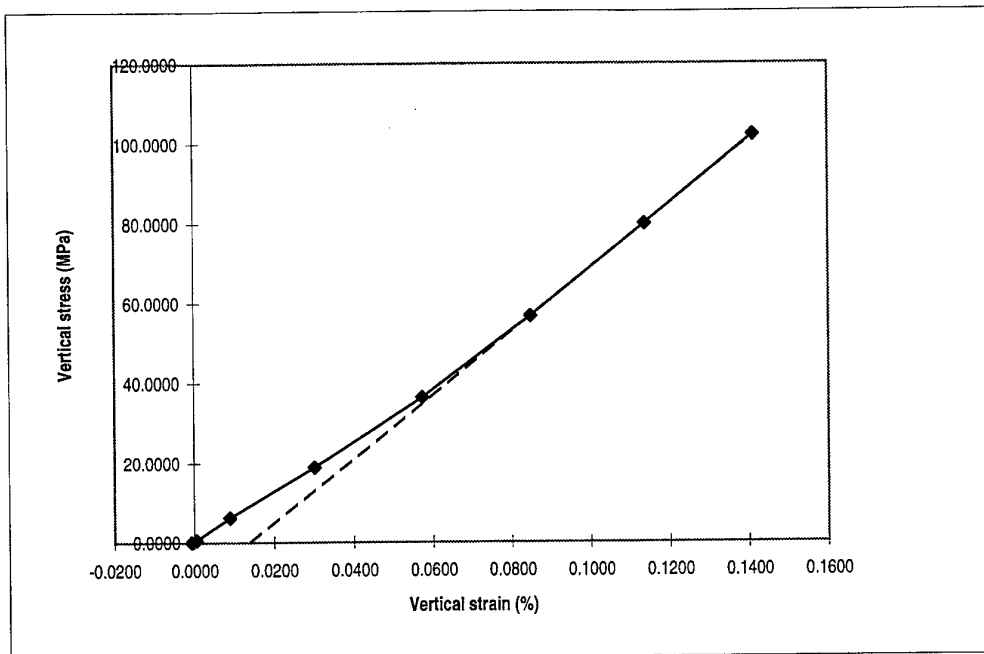
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 933033-13	Figure No. 9
Q-system parameters		Drawn by VK	Date 97-04-30
		Checked	
		Approved	



Uniaxial compressive strength: 87.61 MPa
 Deformation modulus: 68.62 GPa
 Poisson's ratio : 0.348

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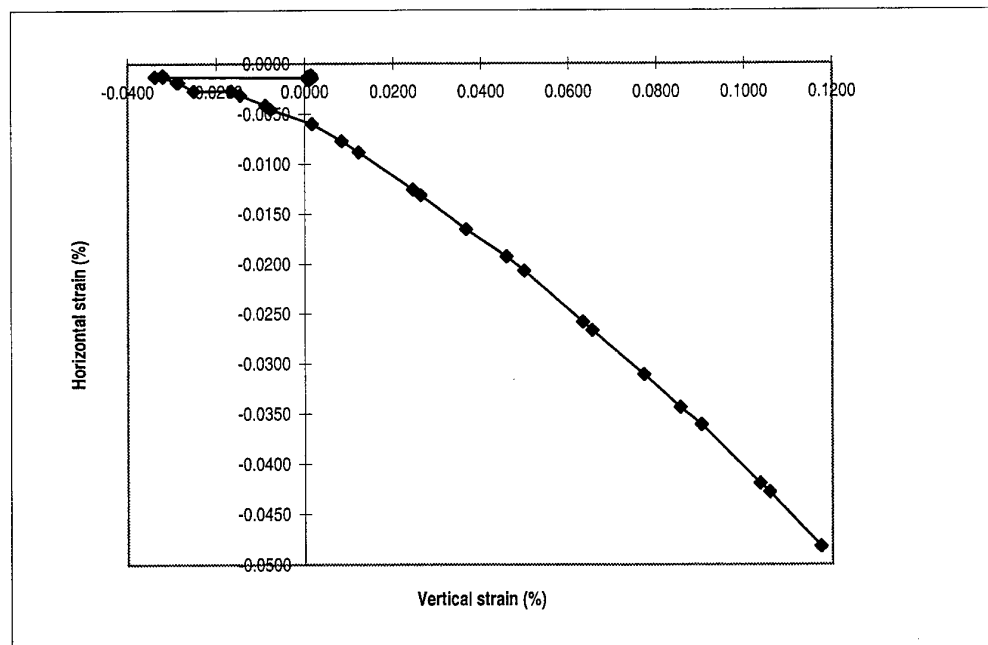
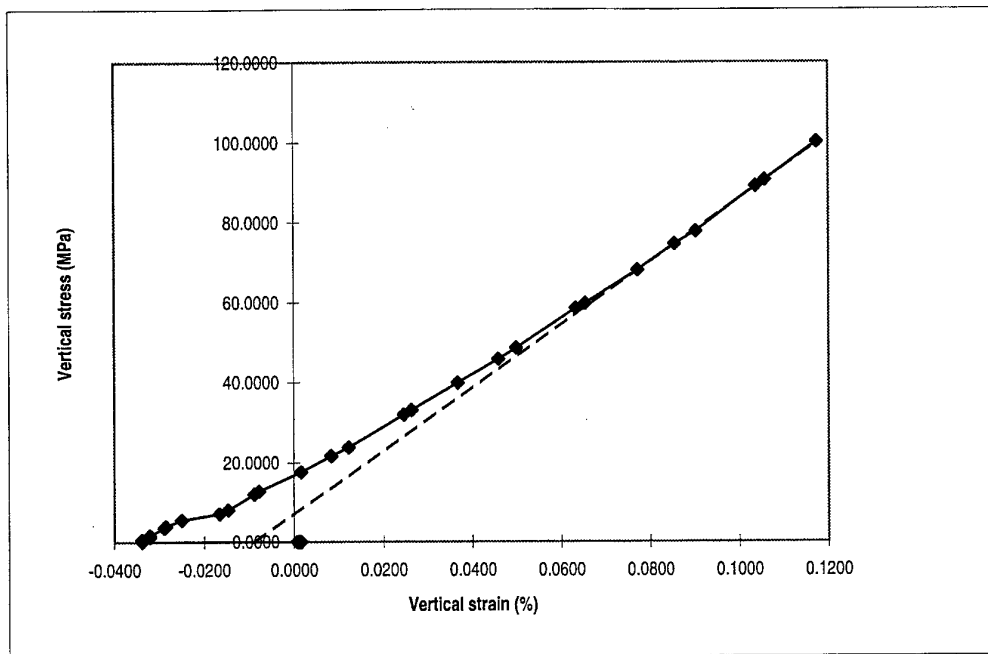
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.1
Unconfined Compression Test		Drawn by PC	Date 04.02.97
Boring : 0	Tube :	Checked	
Part : 0	Test : 0	Approved	
Depth : 0.0 m		UCT No. : dummy	



Uniaxial compressive strength: 102.12 MPa
 Deformation modulus: 80.06 GPa
 Poisson's ratio : 0.306

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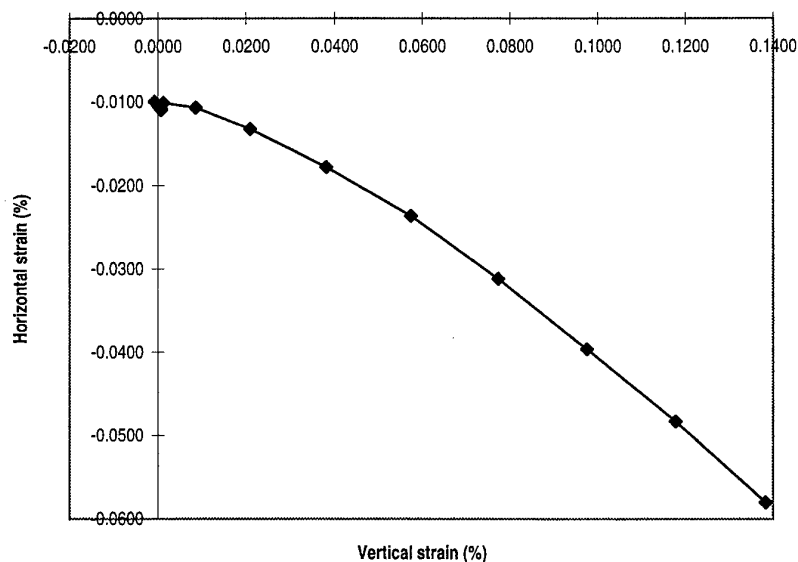
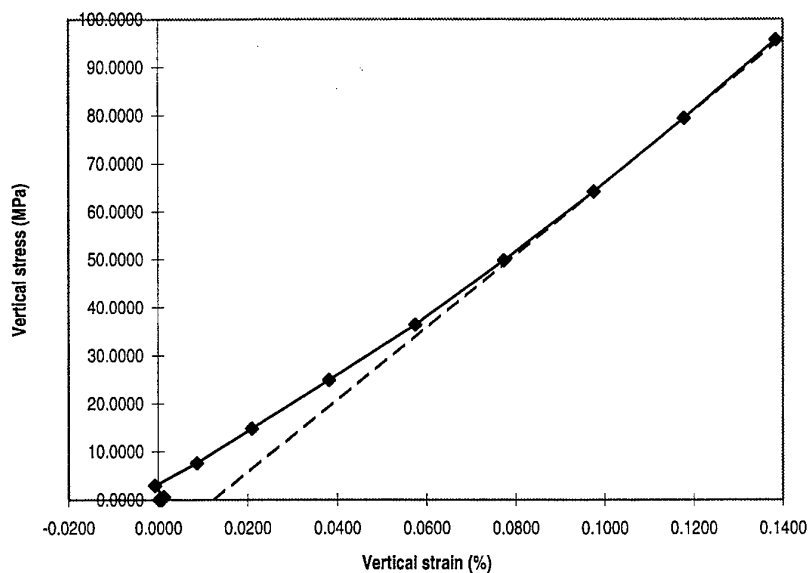
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.2
Unconfined Compression Test		Depth : 7.1 m	Drawn by PC	Date 13.12.96
Boring : 97/1	Tube :		Checked	
Part : 1	Test : 1	UCT No. : 352	Approved	



Uniaxial compressive strength: 100.10 MPa
 Deformation modulus: 78.97 GPa
 Poisson's ratio : 0.413

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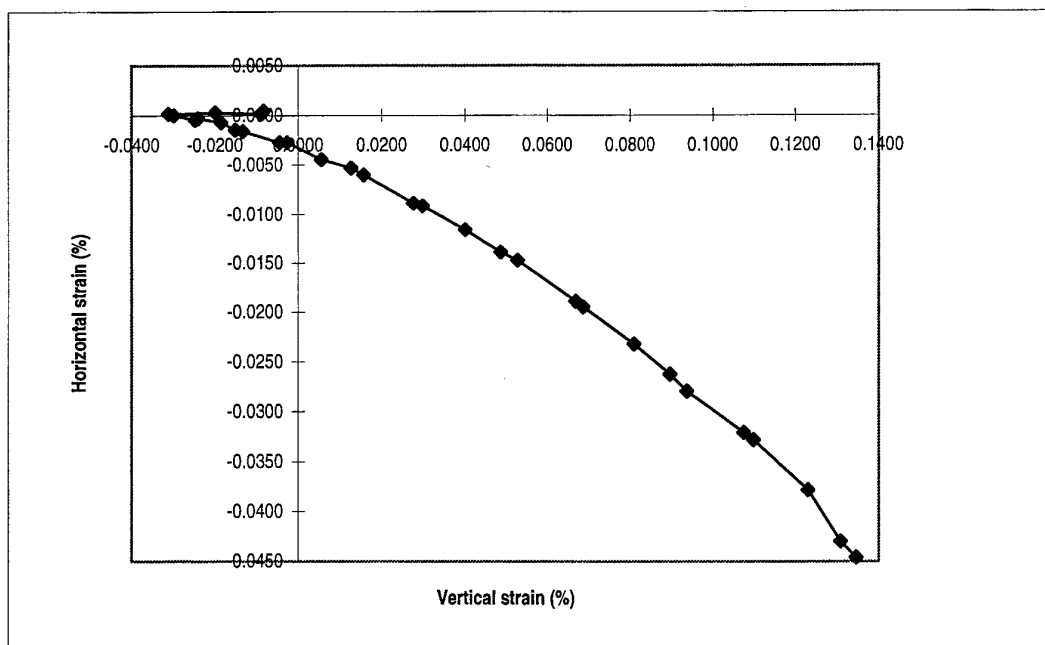
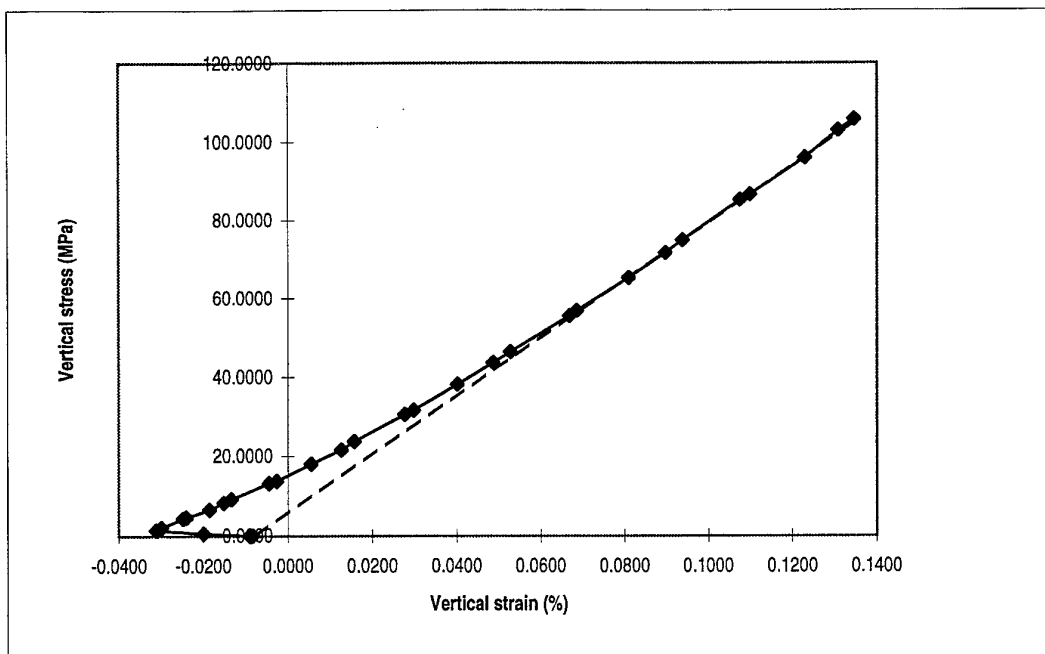
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.3
Unconfined Compression Test			Drawn by PC	Date 13.12.96
Boring : 97/1			Checked	
Part : 2			Approved	
Tube :			UCT No. : 353	



Uniaxial compressive strength: 95.72 MPa
 Deformation modulus: 75.29 GPa
 Poisson's ratio : 0.430

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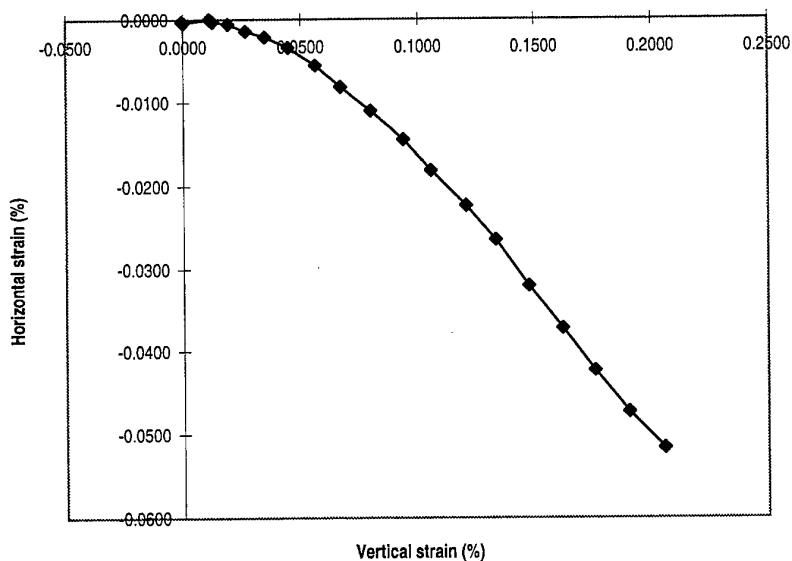
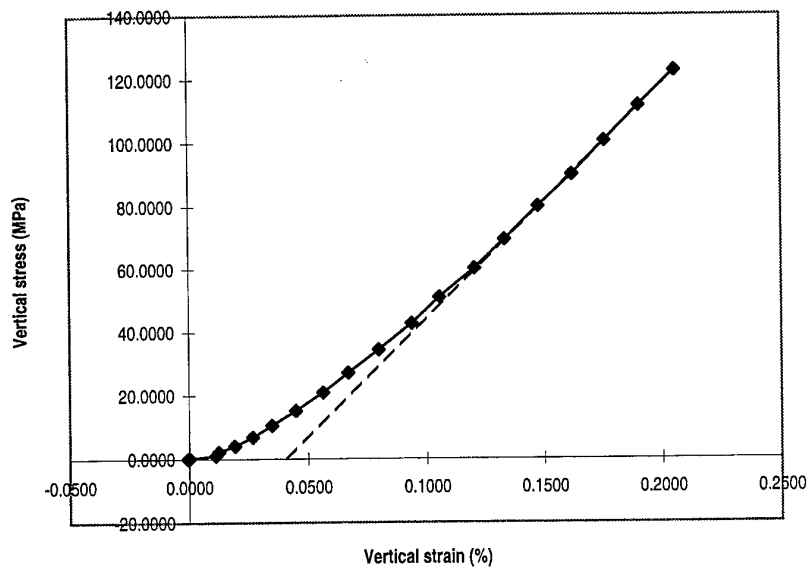
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.4
Unconfined Compression Test			Drawn by PC	Date 18.12.96
Boring : 97/1	Tube :		Checked	
Part : 3	Test : 3	UCT No. : 354	Approved	



Uniaxial compressive strength: 105.77 MPa
 Deformation modulus: 73.58 GPa
 Poisson's ratio : 0.343

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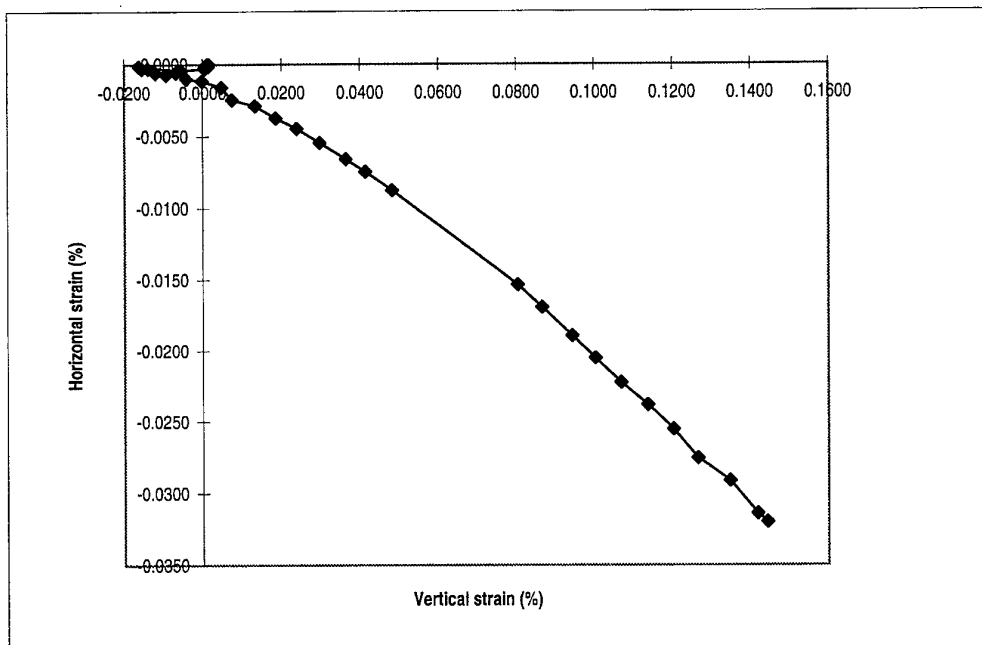
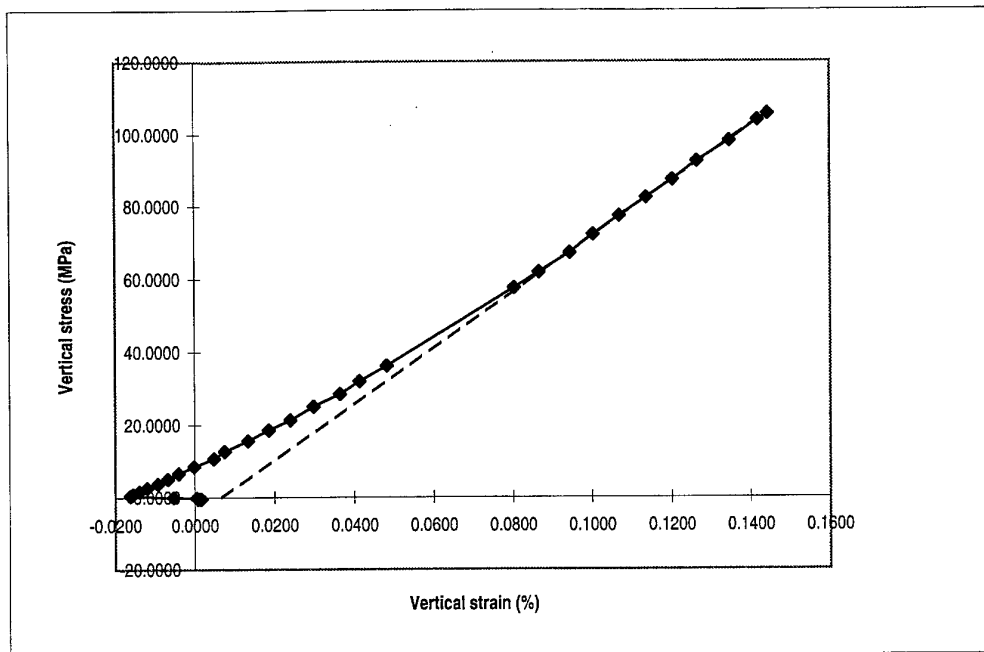
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.5
Unconfined Compression Test		Drawn by PC	Date 18.12.96
Boring : 97/1	Tube :	Checked	
Part : 4	Test : 4	Approved	
UCT No. : 355			



Uniaxial compressive strength: 122.56 MPa
 Deformation modulus: 74.41 GPa
 Poisson's ratio : 0.361

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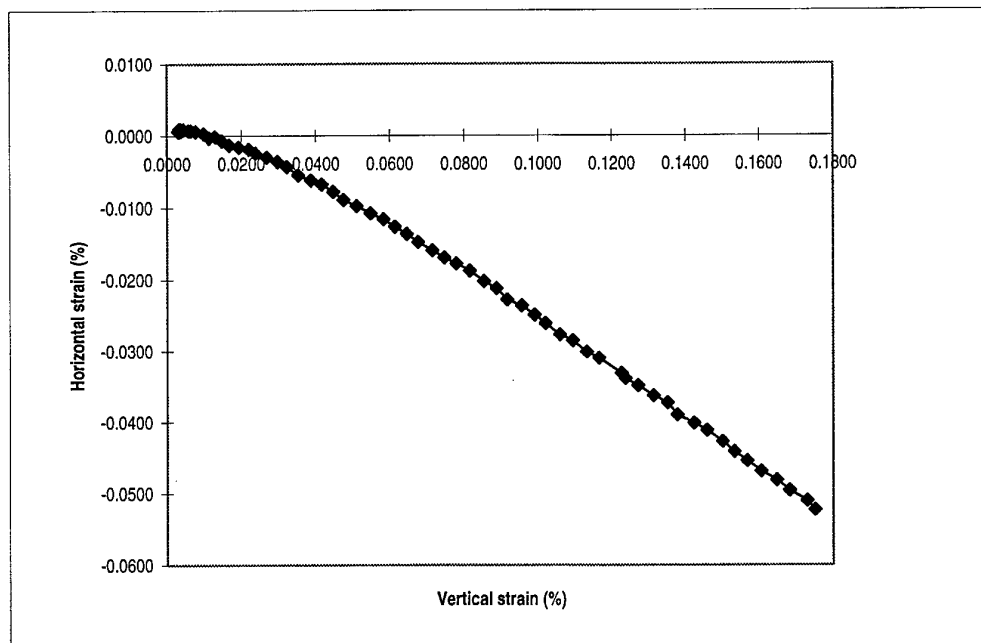
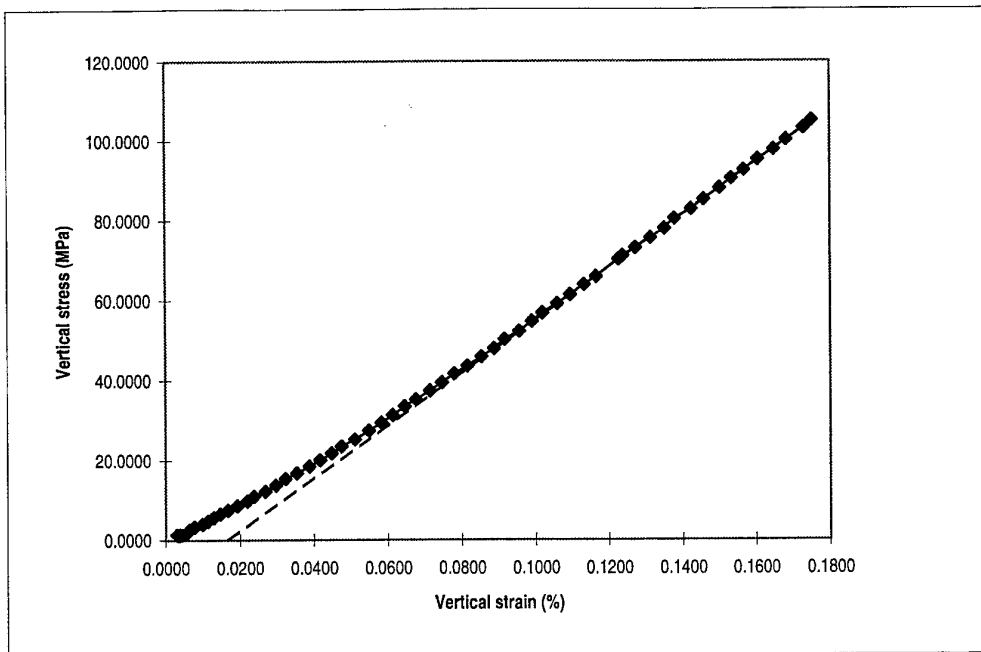
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.6
Unconfined Compression Test			Drawn by PC	Date 19.12.96
Boring : 97/1	Tube :		Checked	
Part : 5	Test : 5	UCT No. : 356	Approved	



Uniaxial compressive strength: 105.57 MPa
 Deformation modulus: 76.79 GPa
 Poisson's ratio : 0.257

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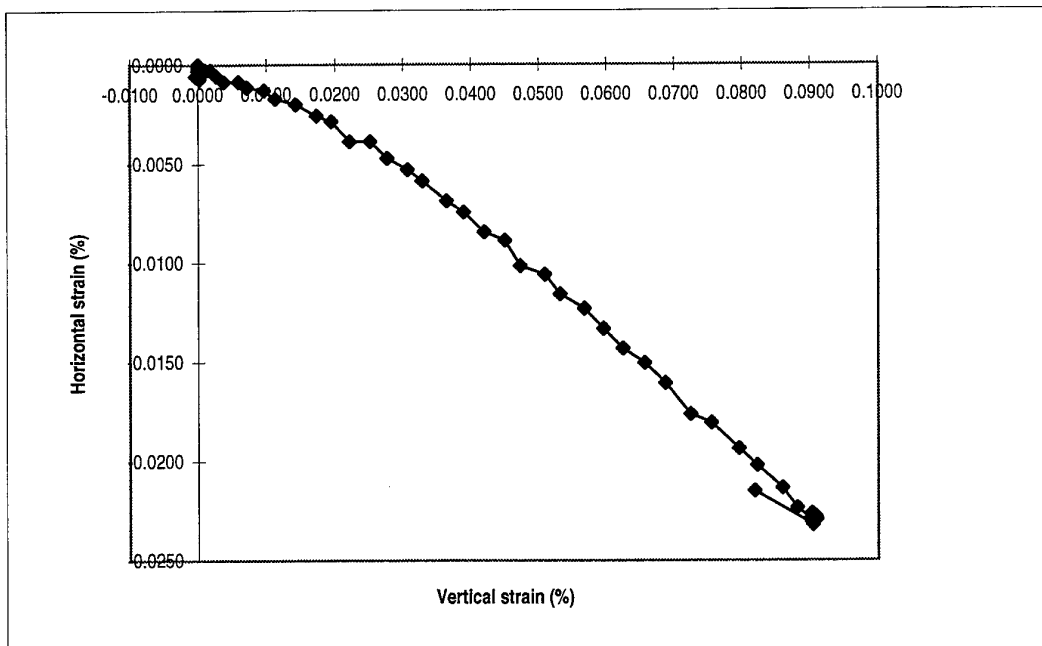
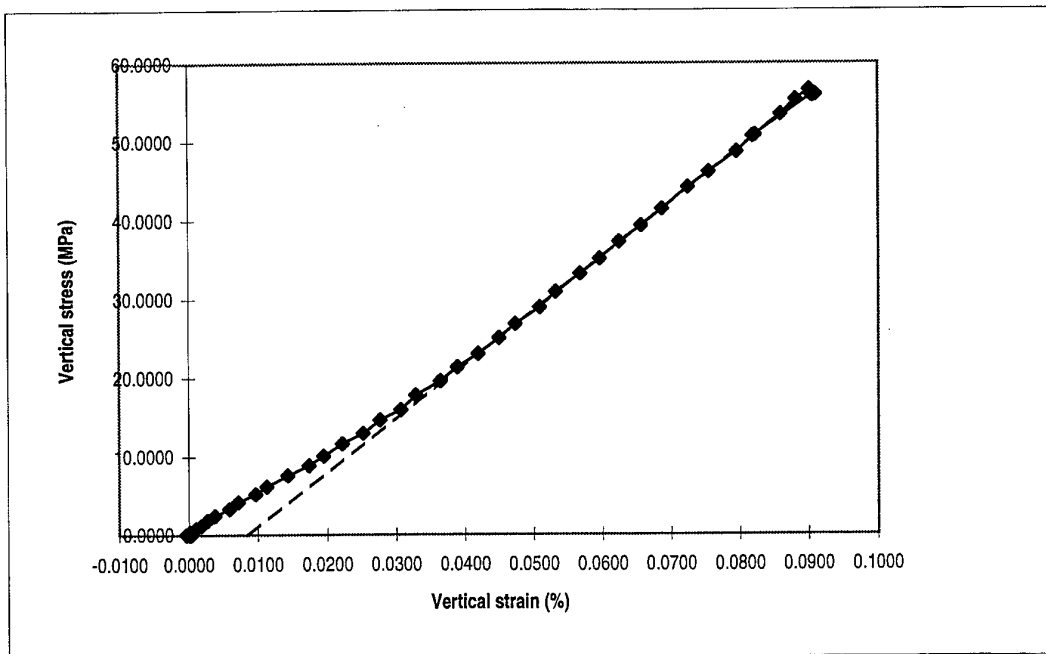
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.7
Unconfined Compression Test		Drawn by PC	Date 23.12.96
Boring : 97/2	Tube :	Checked	
Part : 2	Test : 7	Approved	
UCT No. : 358			



Uniaxial compressive strength: 105.11 MPa
 Deformation modulus: 66.13 GPa
 Poisson's ratio : 0.355

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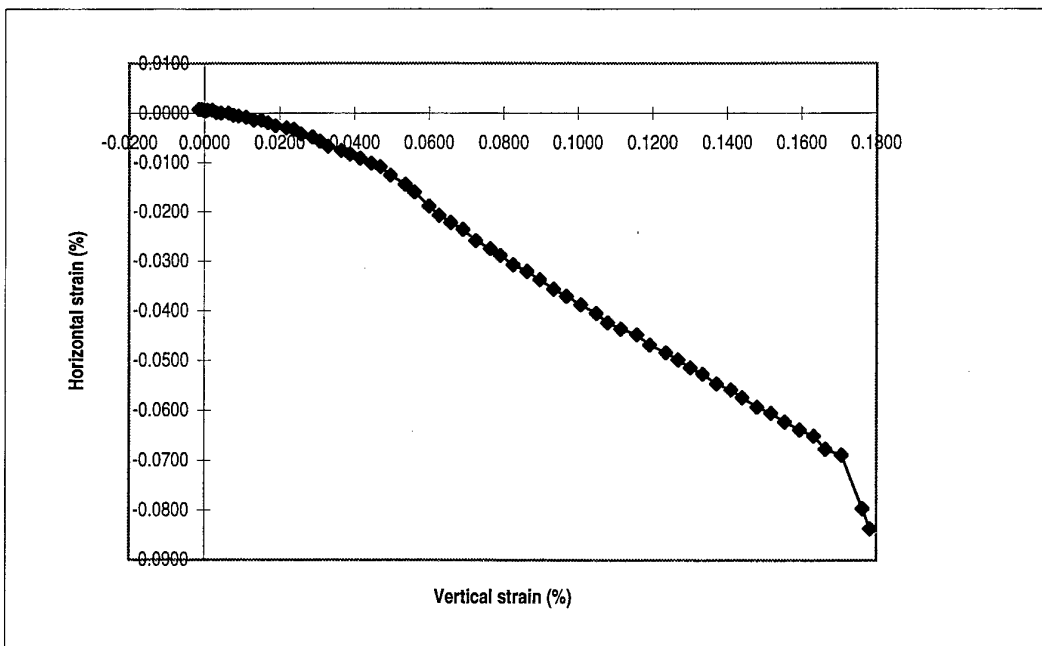
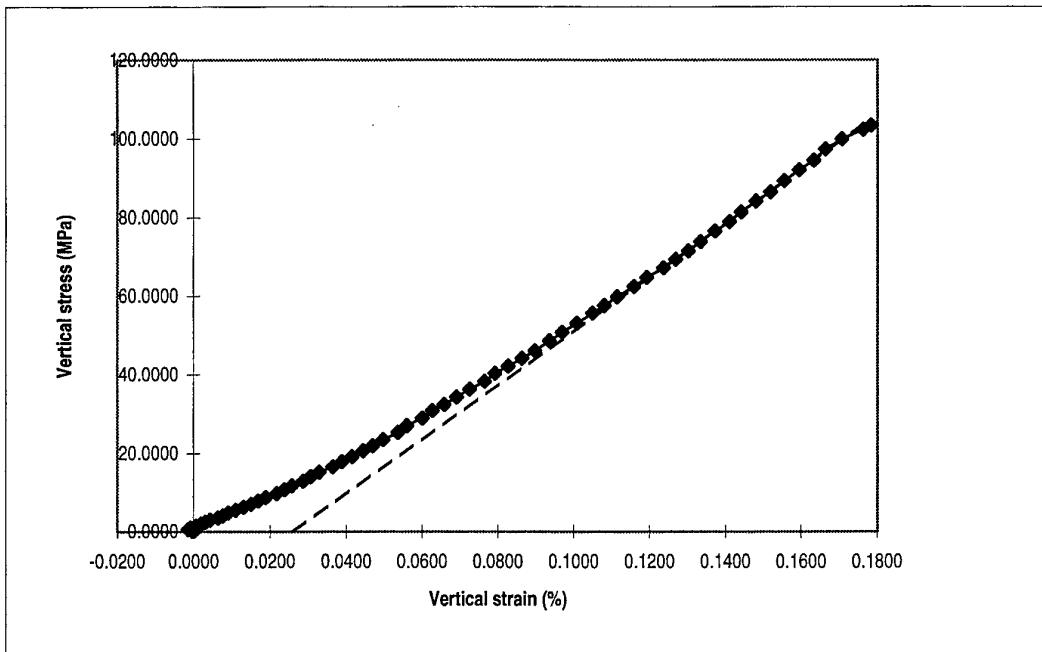
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.8
Unconfined Compression Test		Drawn by PC	Date 13.01.97
Boring : 97/2	Tube :	Checked	
Part : 3	Test : 8	Approved	
UCT No. : 359			



Uniaxial compressive strength: 56.56 MPa
 Deformation modulus: 68.83 GPa
 Poisson's ratio : 0.304

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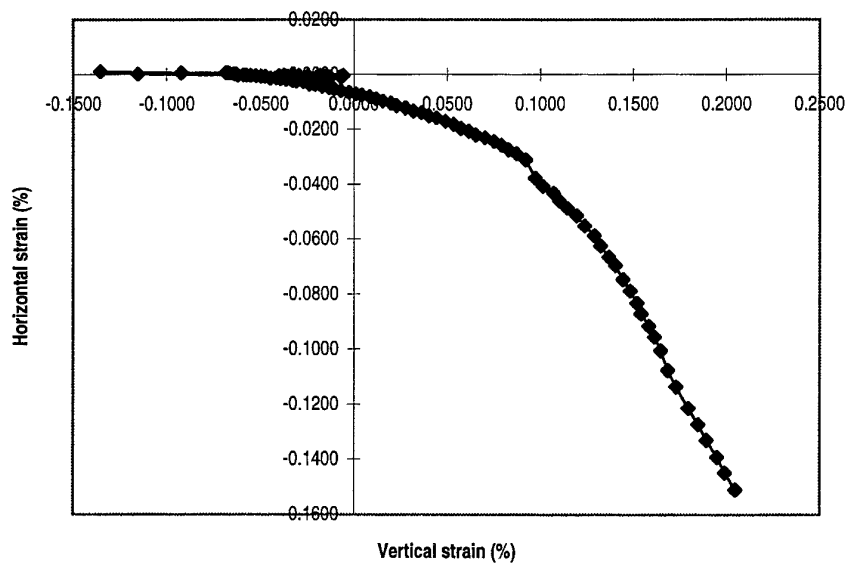
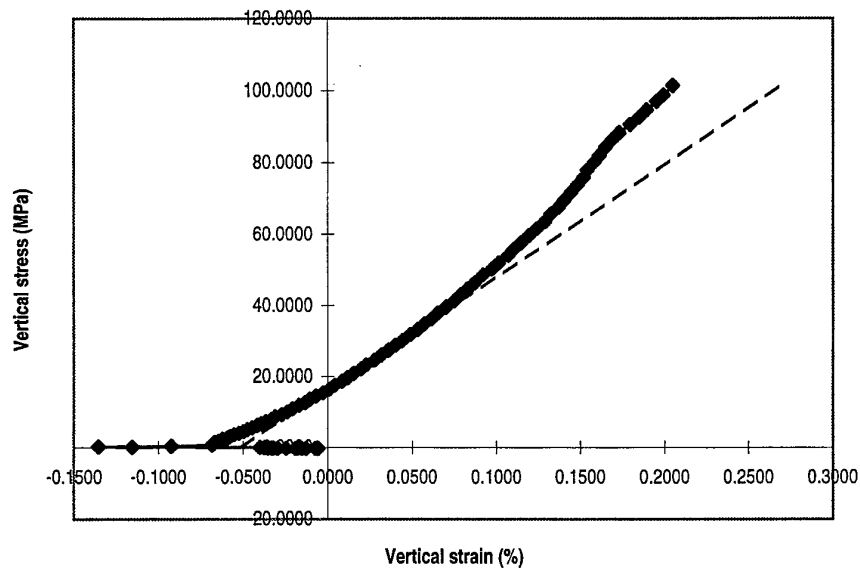
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.9
Unconfined Compression Test		Drawn by PC	Date 10.12.96
Boring : 97/2	Tube :	Checked	
Part : 4	Test : 9	Approved	
UCT No. : 360			



Uniaxial compressive strength: 103.41 MPa
 Deformation modulus: 68.86 GPa
 Poisson's ratio : 0.436

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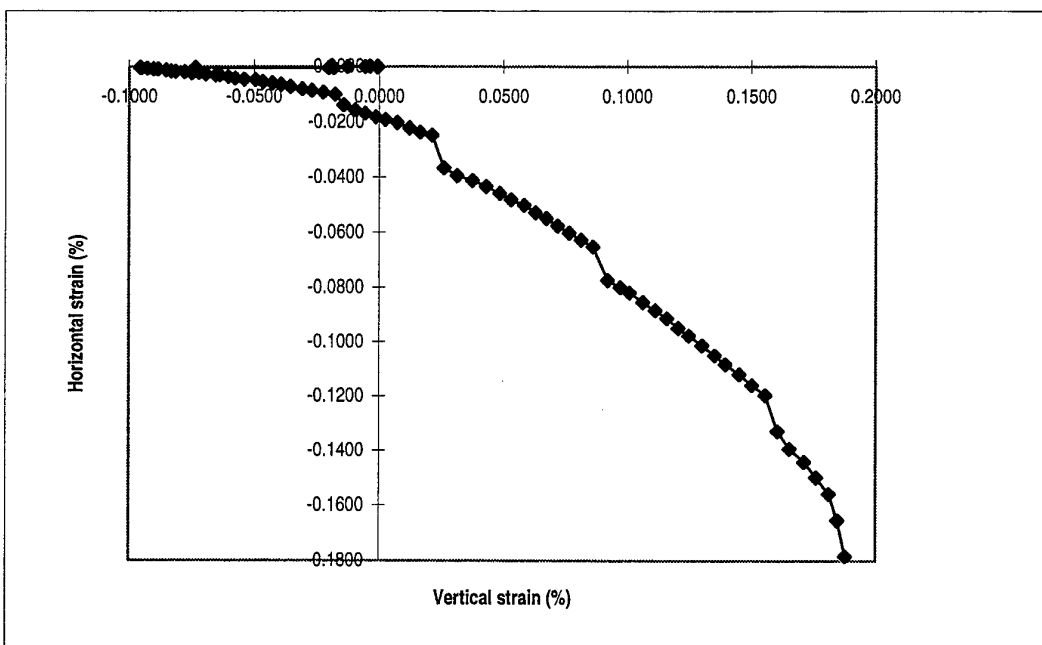
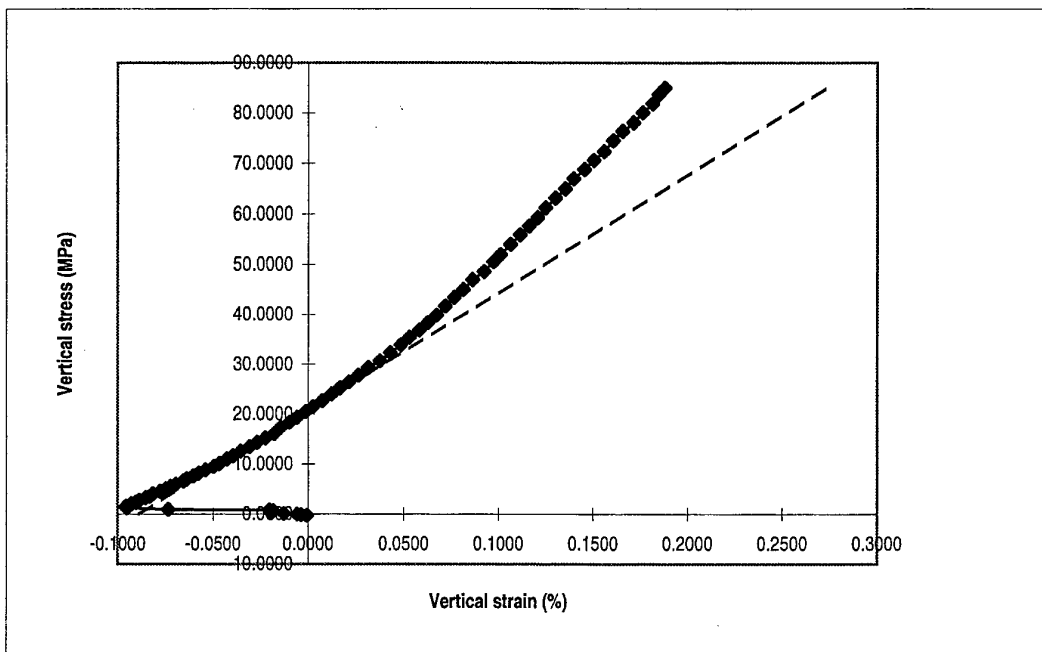
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.10
Unconfined Compression Test		Drawn by PC	Date 10.01.97
Boring : 97/2	Tube :	Checked	
Part : 5	Test : 10	Approved	
UCT No. : 361			



Uniaxial compressive strength: 101.37 MPa
 Deformation modulus: 31.58 GPa
 Poisson's ratio : 0.213

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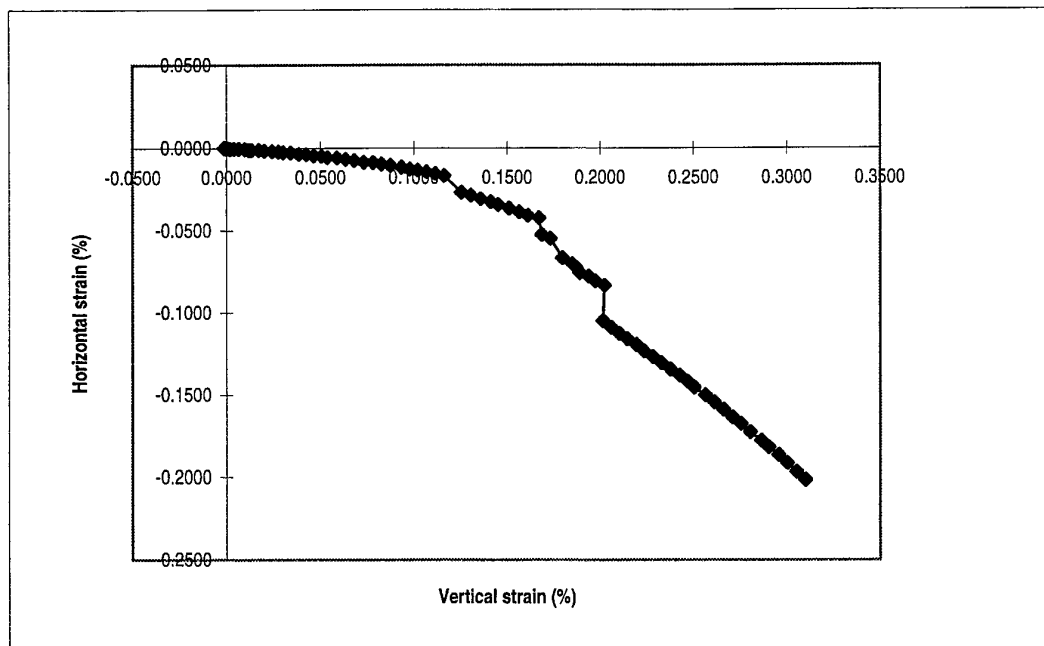
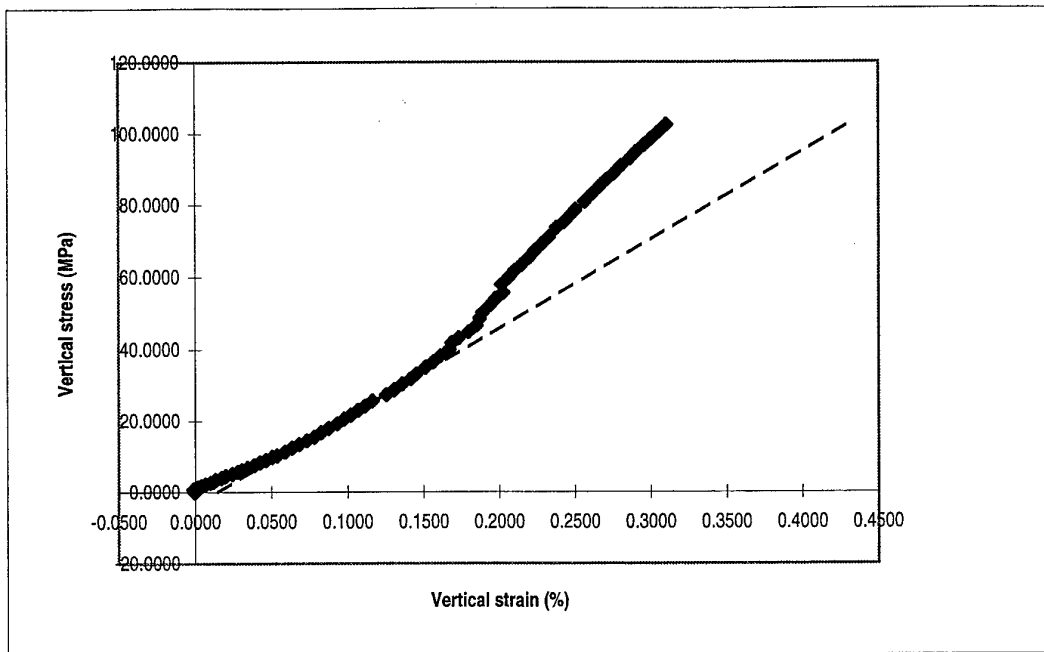
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.11	
Unconfined Compression Test			Depth : 10.7 m	Drawn by PC	Date 10.01.97
Boring : 97/3			Tube :	Checked	
Part : 1			Test : 11	UCT No. : 362	



Uniaxial compressive strength: 85.02 MPa
 Deformation modulus: 23.42 GPa
 Poisson's ratio : 0.298

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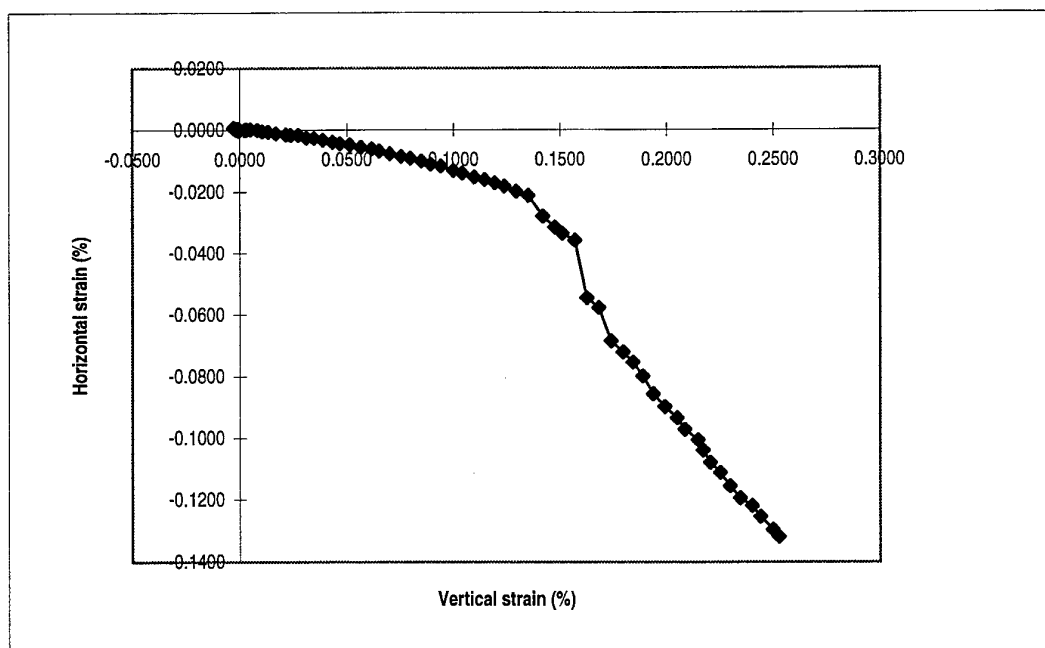
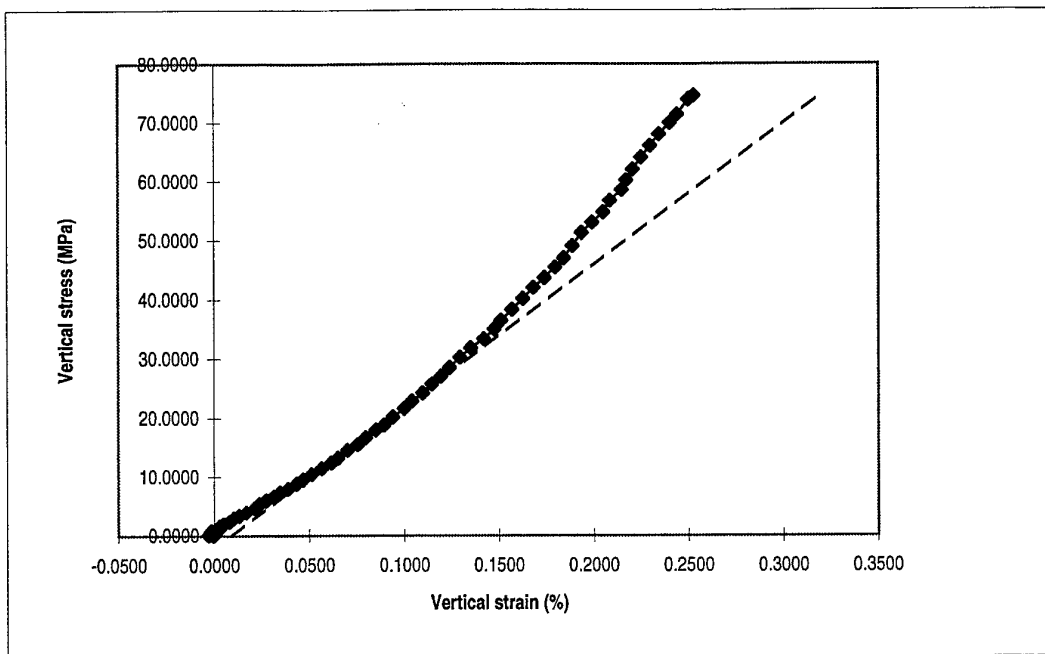
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.12
Unconfined Compression Test			Drawn by PC	Date 17.01.97
Boring : 97/3			Checked	
Part : 2			Approved	
Tube :			UCT No. : 363	



Uniaxial compressive strength: 102.42 MPa
 Deformation modulus: 24.67 GPa
 Poisson's ratio : 0.371

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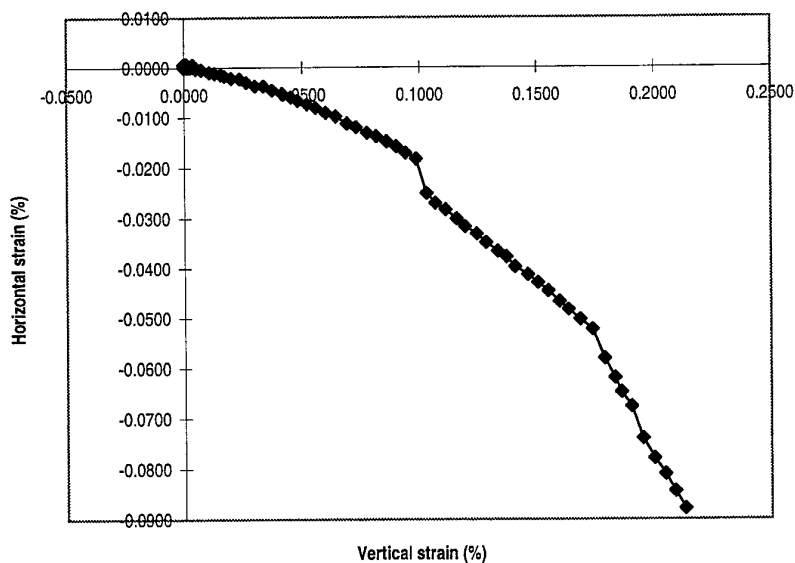
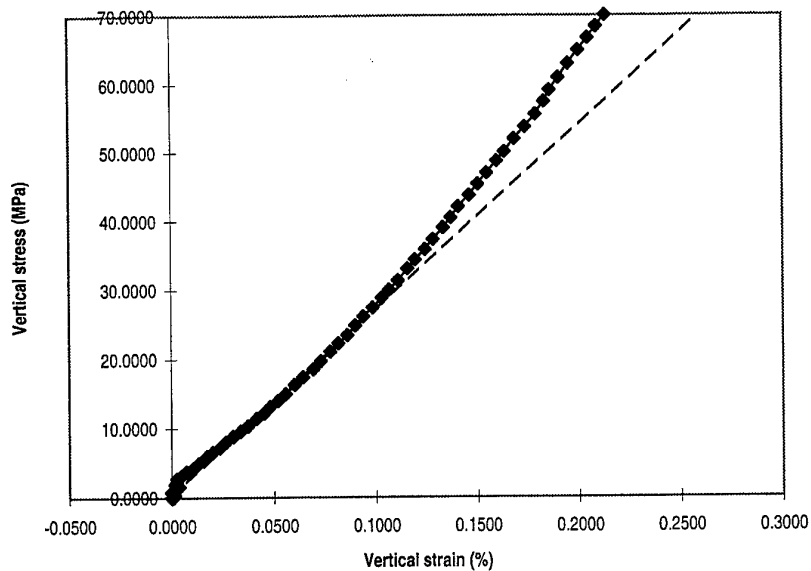
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.13
Unconfined Compression Test		Drawn by PC	Date 20.01.97
Boring : 97/3	Tube :	Checked	
Part : 3	Test : 13	Approved	
UCT No. : 364			



Uniaxial compressive strength: 74.60 MPa
 Deformation modulus: 24.01 GPa
 Poisson's ratio : 0.177

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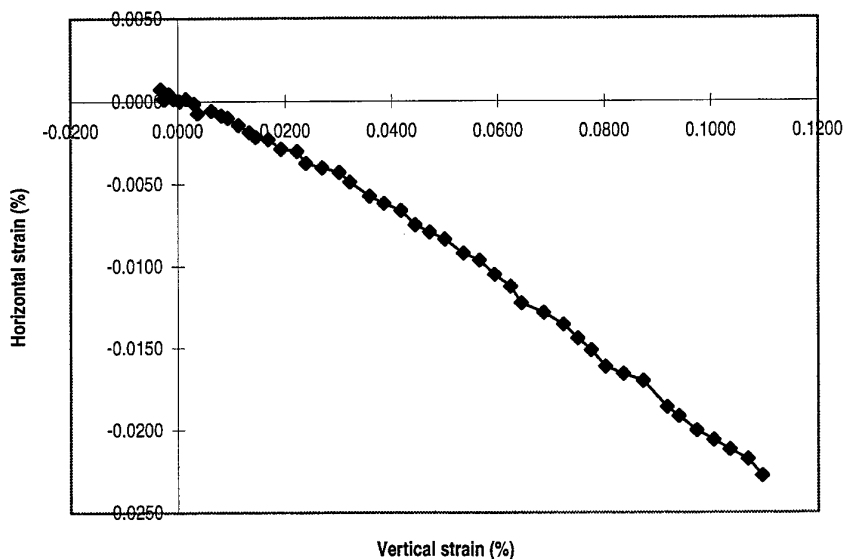
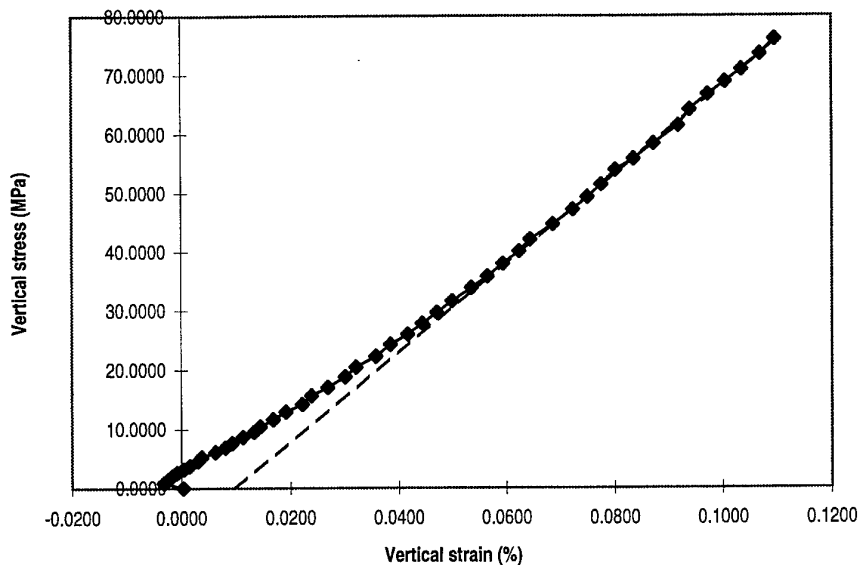
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.14
Unconfined Compression Test		Depth : 17.5 m	Drawn by PC
Boring : 97/3	Tube :	Checked	Date 20.01.97
Part : 4	Test : 14	Approved	
	UCT No. : 365		



Uniaxial compressive strength: 69.99 MPa
 Deformation modulus: 26.86 GPa
 Poisson's ratio : 0.211

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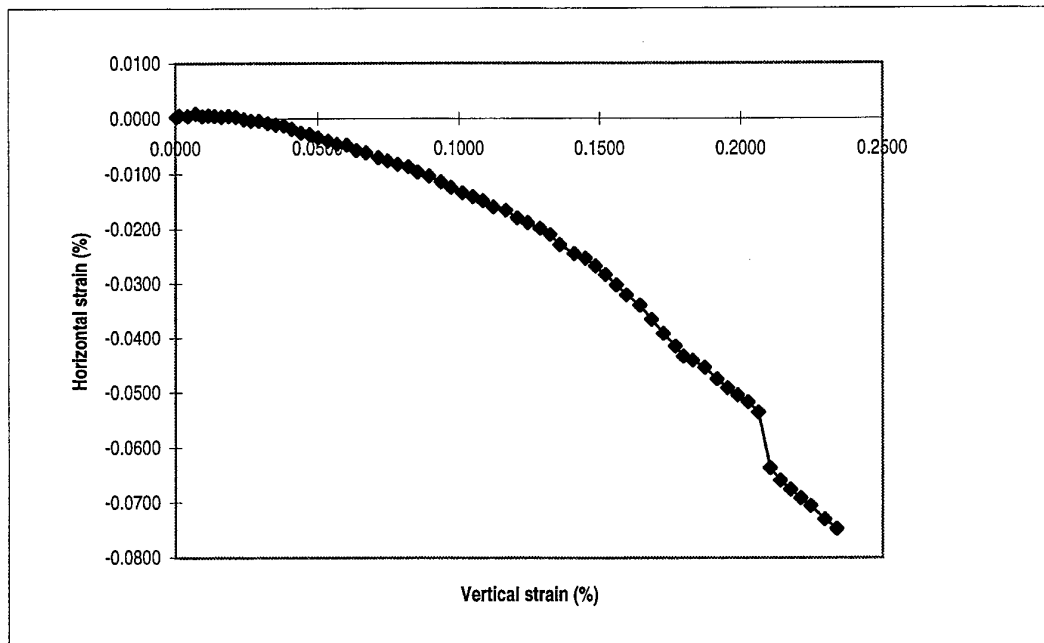
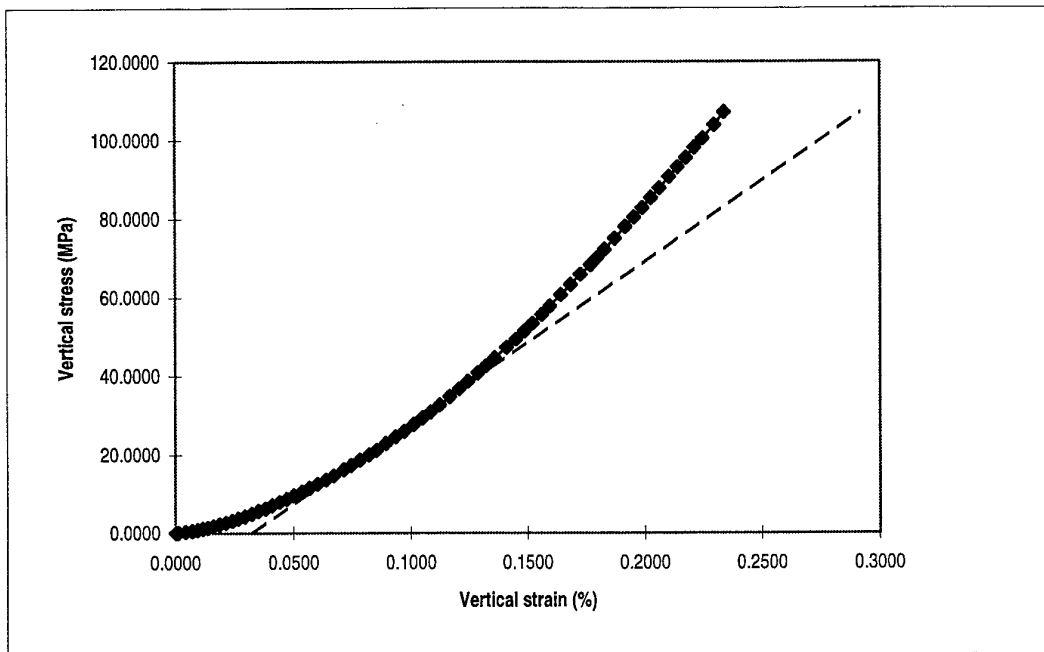
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.15
Unconfined Compression Test		Depth : 17.7 m	Drawn by PC
Boring : 97/3	Tube :	Checked	Date 20.01.97
Part : 5	Test : 15	Approved	
UCT No. : 366			



Uniaxial compressive strength: 76.05 MPa
 Deformation modulus: 75.69 GPa
 Poisson's ratio : 0.244

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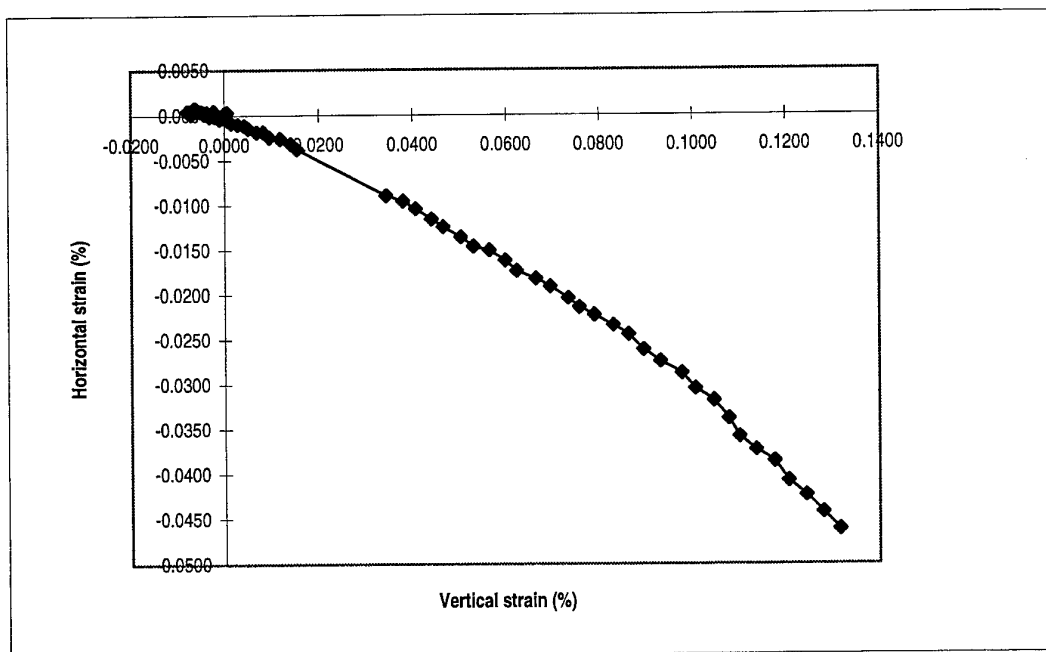
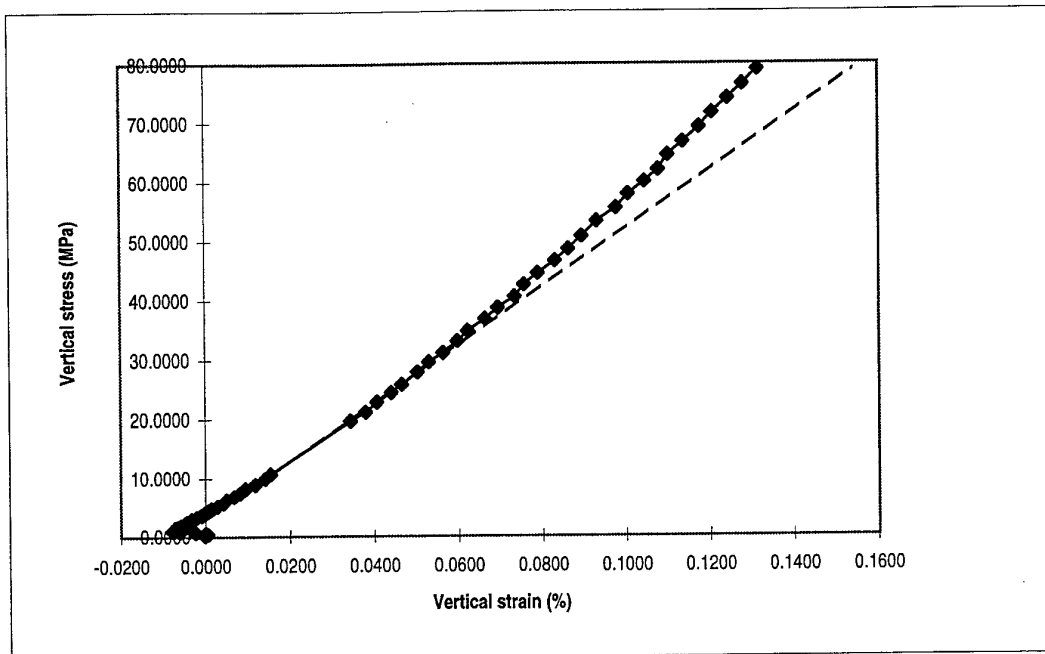
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.16
Unconfined Compression Test		Drawn by PC	Date 20.01.97
Boring : 97/5	Tube :	Checked	
Part : 1	Test : 16	Approved	
UCT No. : 367			



Uniaxial compressive strength: 107.12 MPa
 Deformation modulus: 41.26 GPa
 Poisson's ratio : 0.220

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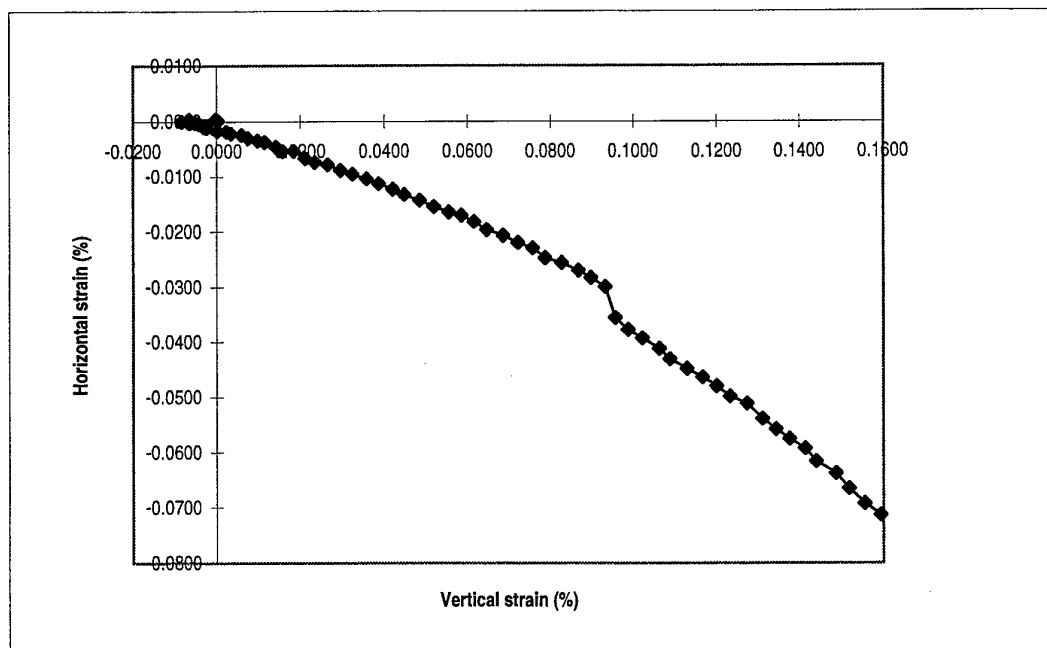
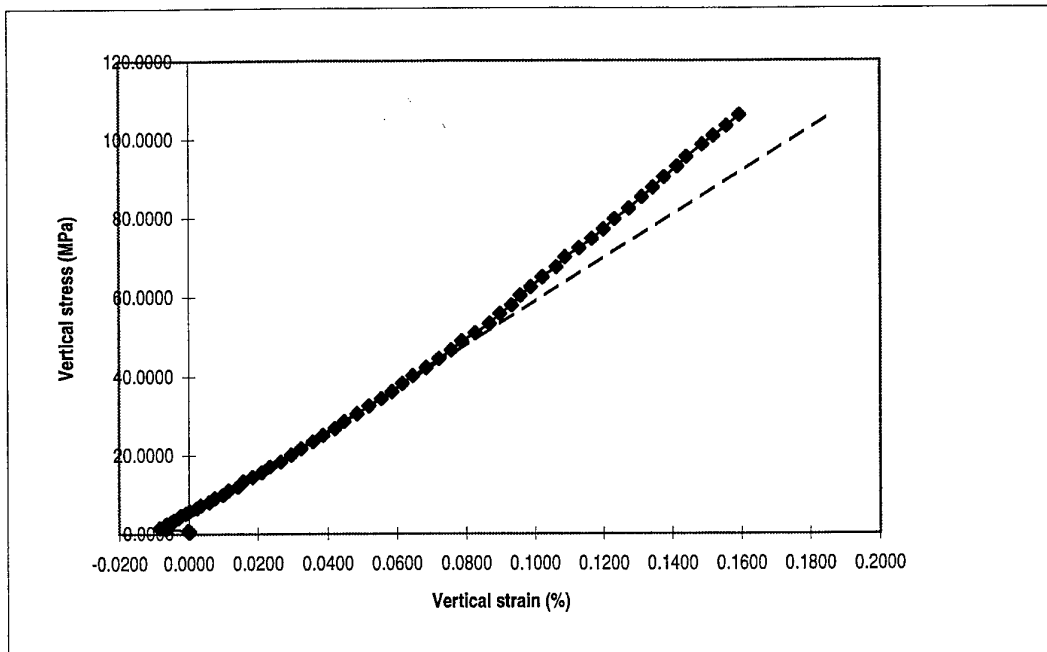
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.17
Unconfined Compression Test		Drawn by PC	Date 20.01.97
Boring : 97/5	Tube :	Checked	
Part : 2	Test : 17	Approved	
UCT No. : 368			



Uniaxial compressive strength: 78.99 MPa
 Deformation modulus: 49.25 GPa
 Poisson's ratio : 0.280

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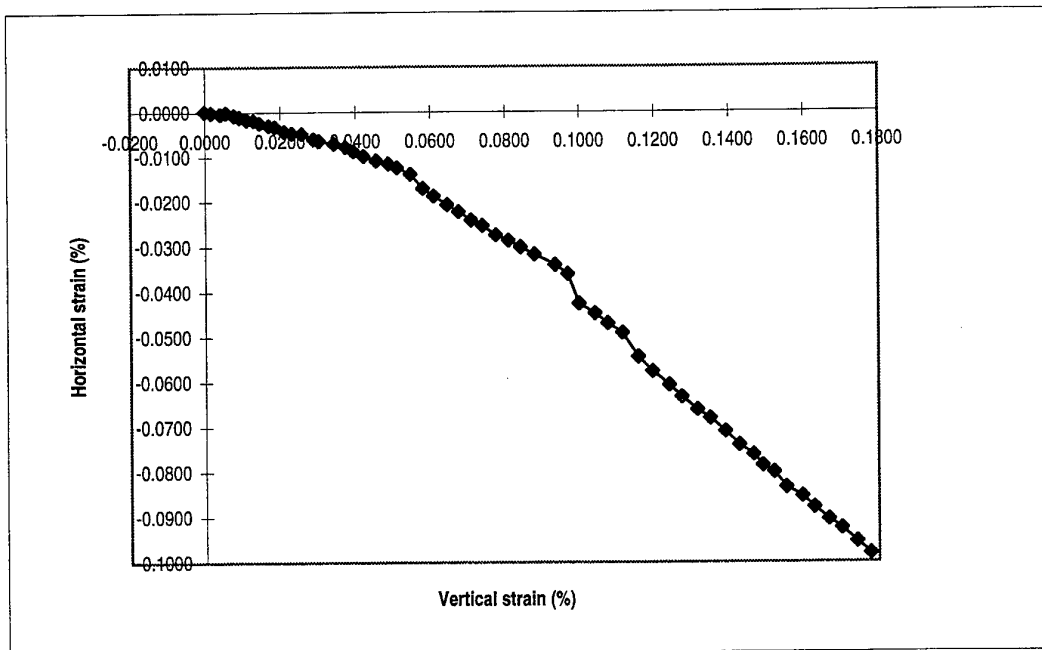
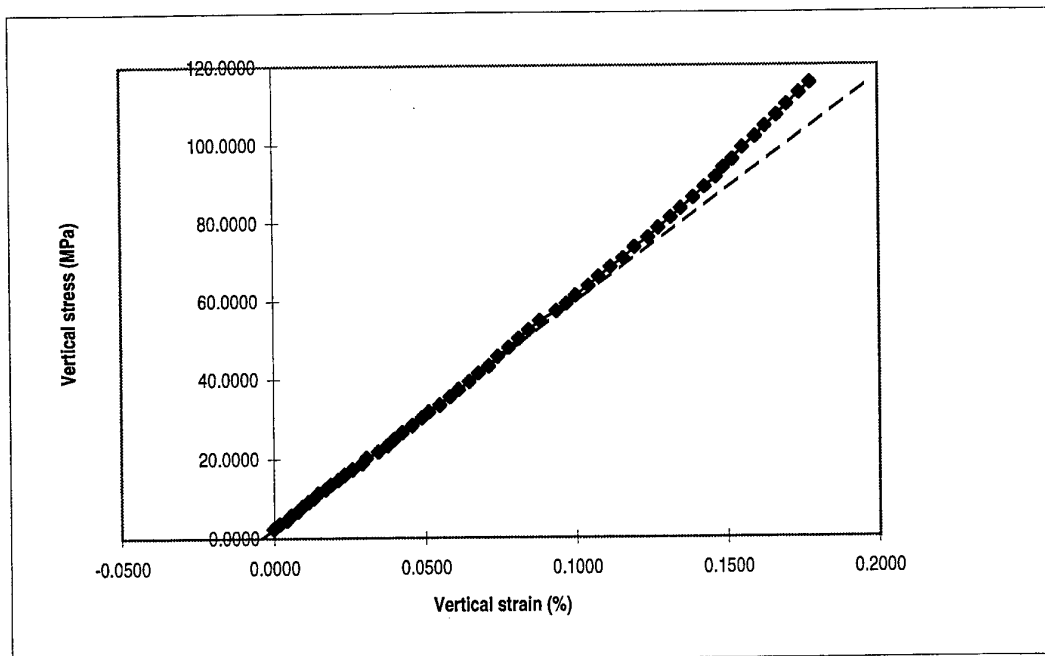
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.18
Unconfined Compression Test			Depth : 14.8 m	Drawn by PC
Boring : 97/5			Tube :	Date 20.01.97
Part : 3			Test : 18	Checked
UCT No. : 369			Approved	



Uniaxial compressive strength: 106.06 MPa
 Deformation modulus: 54.86 GPa
 Poisson's ratio : 0.293

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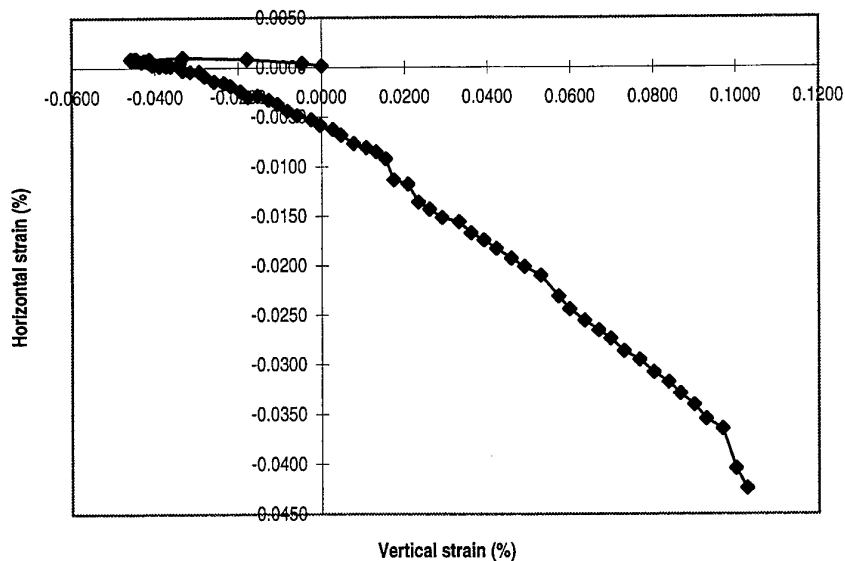
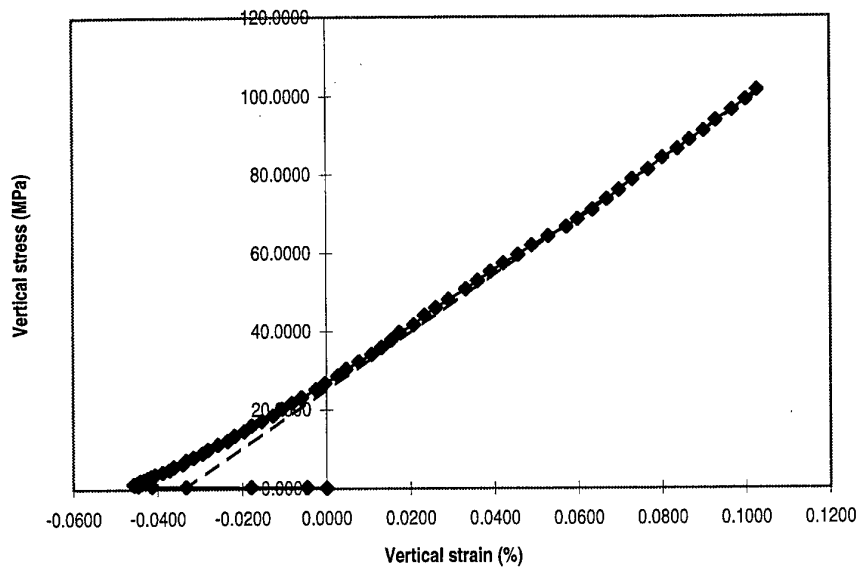
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.19	
Unconfined Compression Test			Depth : 14.9 m	Drawn by PC	Date 21.01.97
Boring : 97/5		Tube :		Checked	
Part : 4		Test : 19		Approved	
UCT No. : 370					



Uniaxial compressive strength: 115.55 MPa
 Deformation modulus: 57.51 GPa
 Poisson's ratio : 0.406

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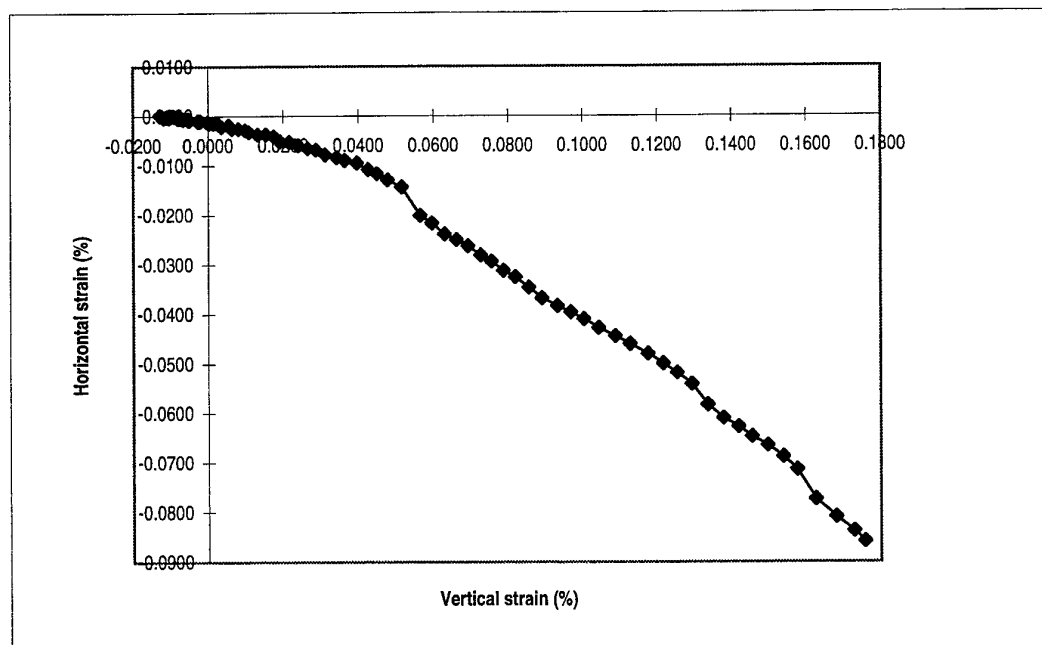
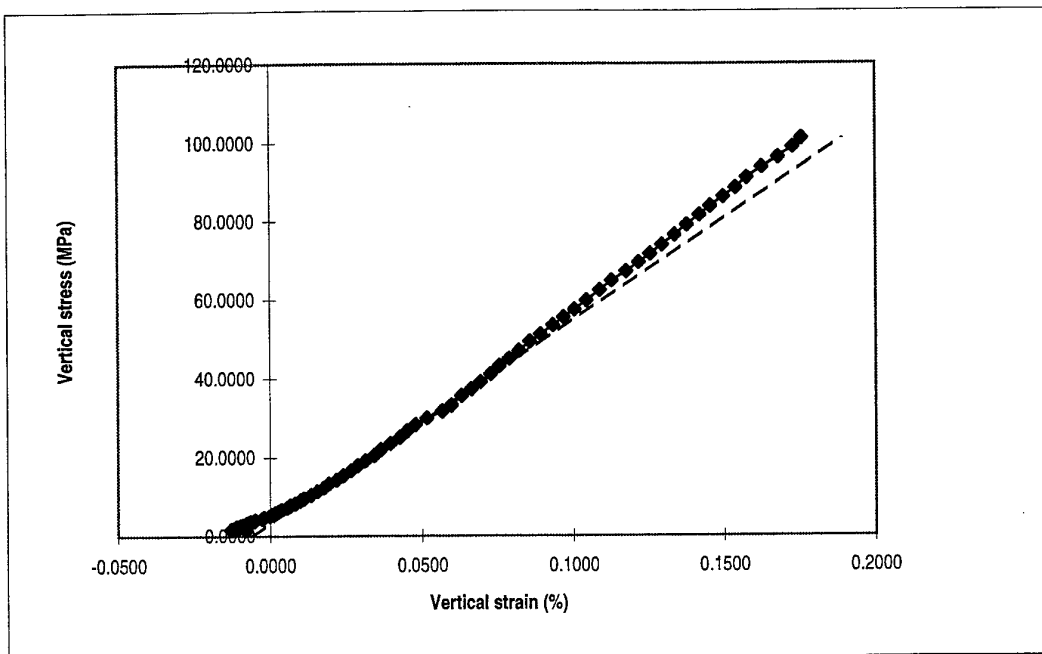
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.20
Unconfined Compression Test		Drawn by PC	Date 20.01.97
Boring : 97/5	Tube :	Checked	
Part : 5	Test : 20	Approved	
UCT No. : 371			



Uniaxial compressive strength: 101.47 MPa
 Deformation modulus: 73.52 GPa
 Poisson's ratio : 0.342

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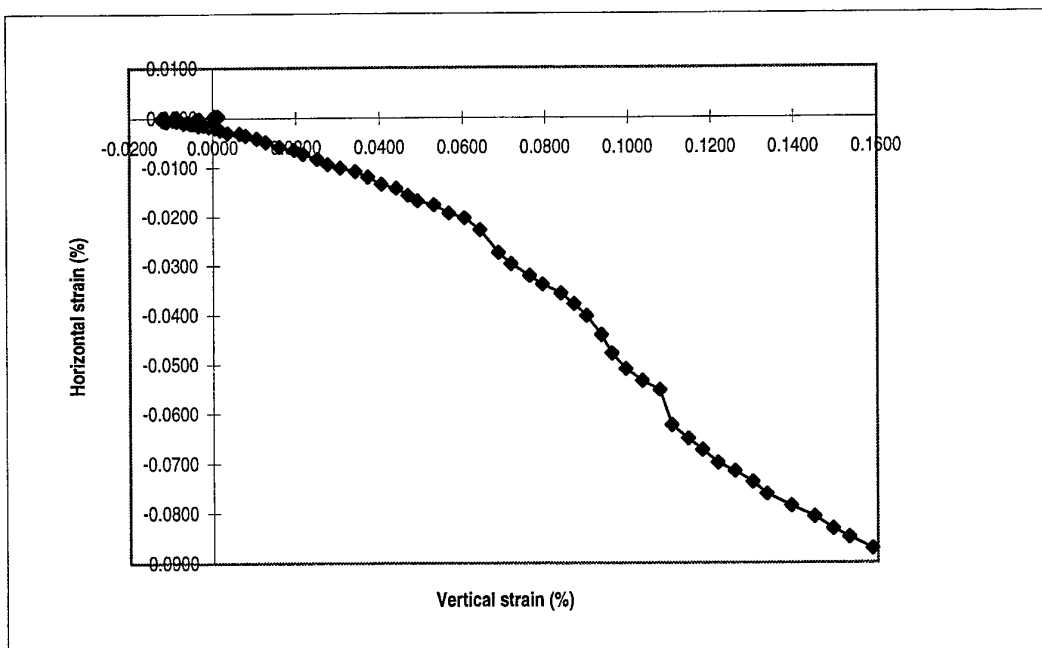
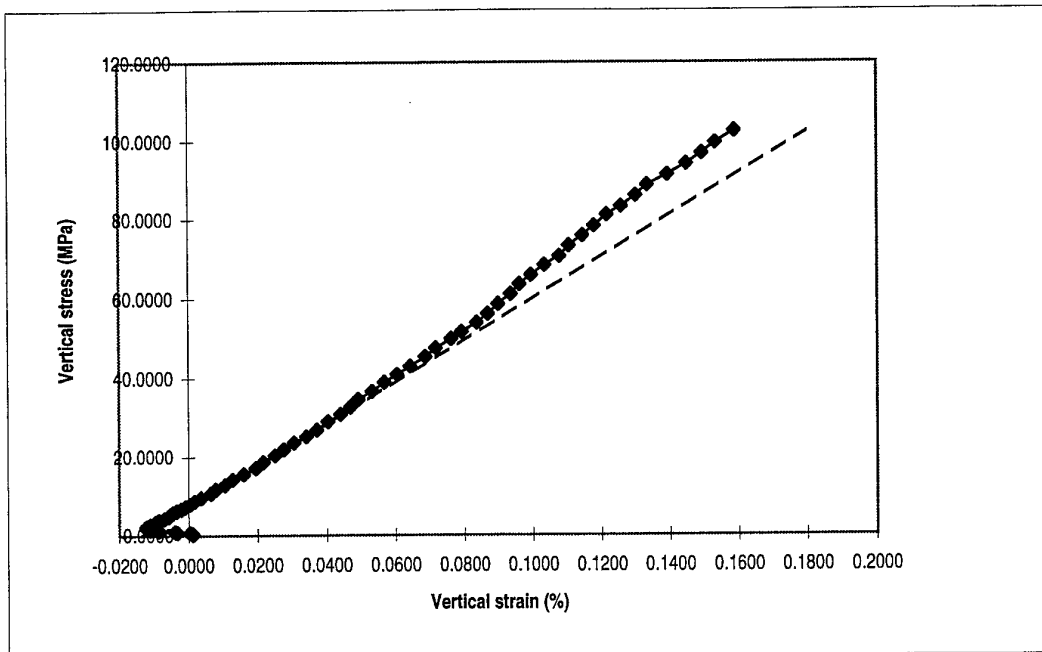
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.21
Unconfined Compression Test		Drawn by PC	Date 22.01.97
Boring : 97/6	Tube :	Checked	
Part : 1	Test : 21	Approved	
UCT No. : 372			



Uniaxial compressive strength: 101.02 MPa
 Deformation modulus: 51.72 GPa
 Poisson's ratio : 0.277

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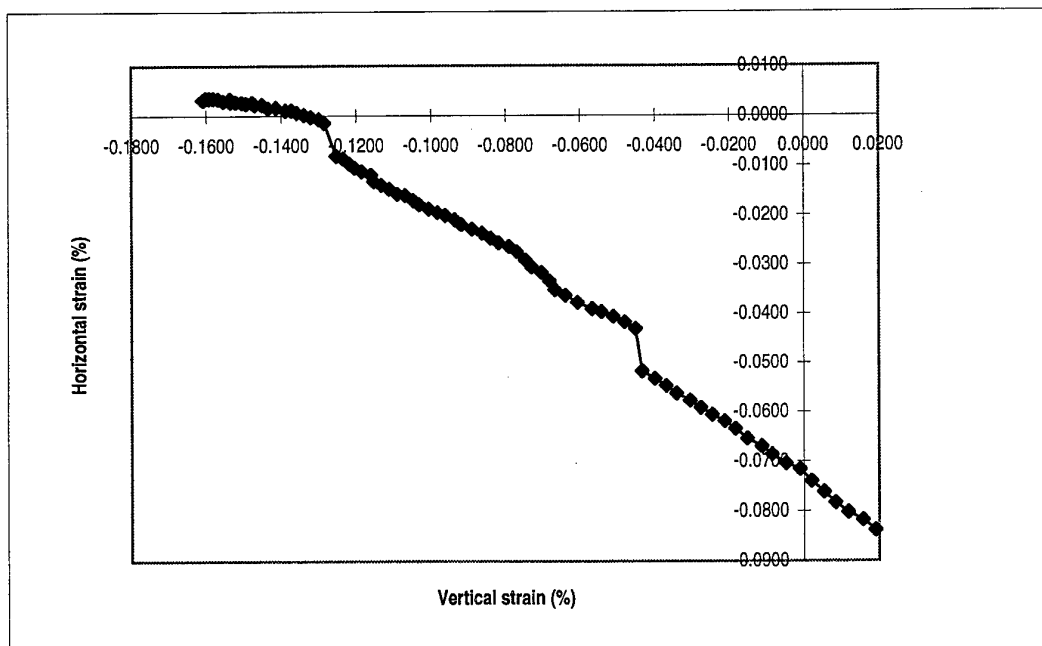
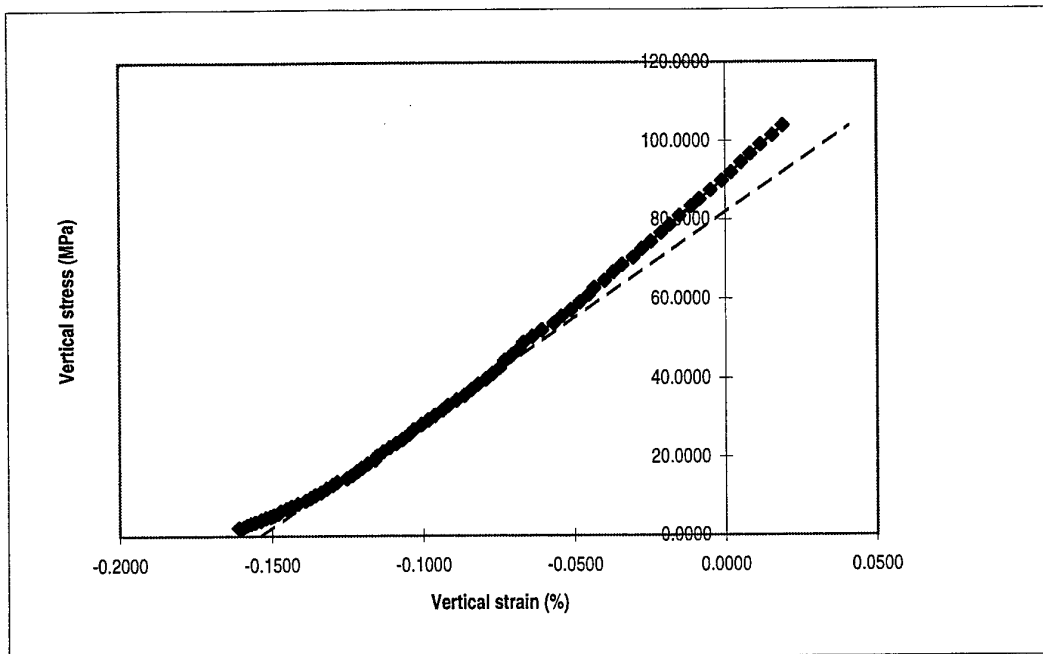
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.22
Unconfined Compression Test			Drawn by PC	Date 22.01.97
Boring : 97/6	Tube :		Checked	
Part : 2	Test : 22	UCT No. : 373	Approved	



Uniaxial compressive strength: 102.61 MPa
 Deformation modulus: 52.93 GPa
 Poisson's ratio : 0.296

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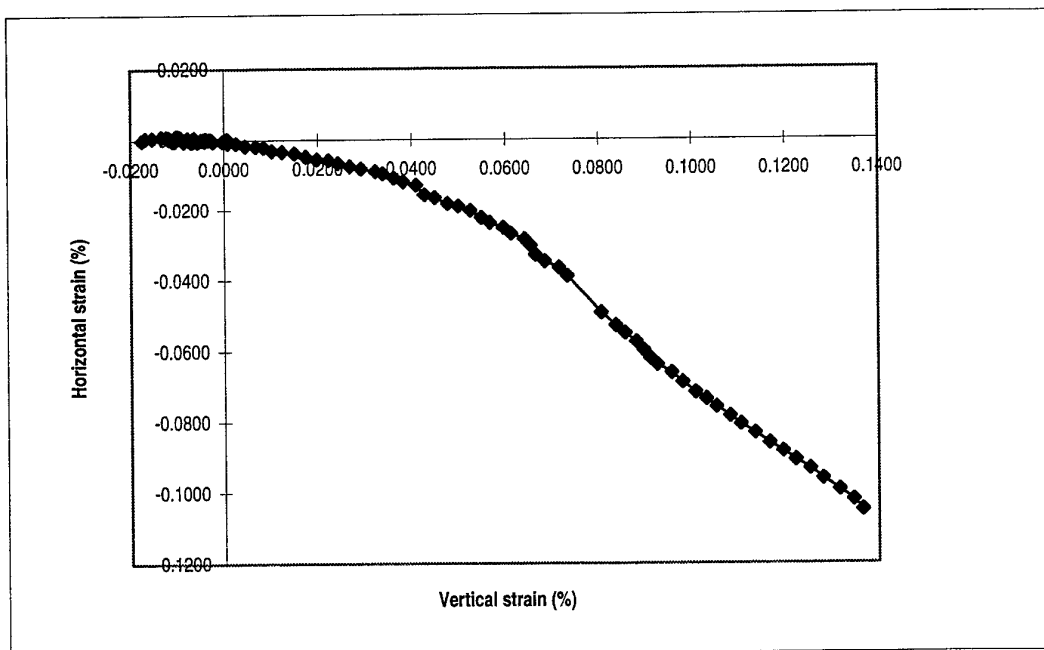
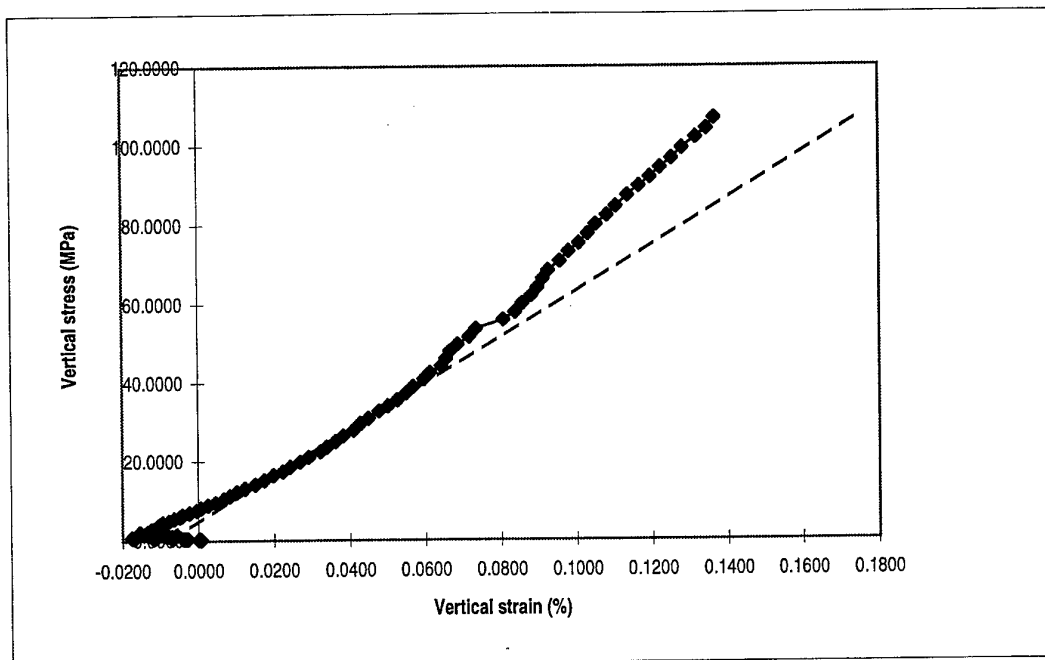
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.23
Unconfined Compression Test			Drawn by PC	Date 24.01.97
Boring : 97/6			Checked	
Tube :			Approved	
Part : 3				
Test : 23			UCT No. : 374	



Uniaxial compressive strength: 103.78 MPa
 Deformation modulus: 53.27 GPa
 Poisson's ratio : 0.240

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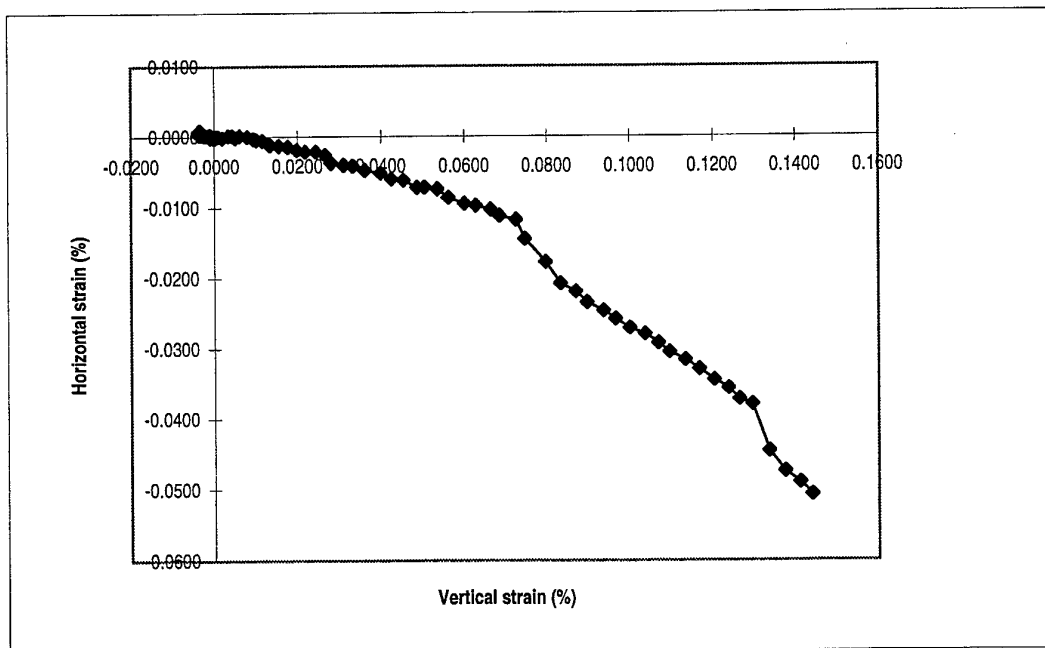
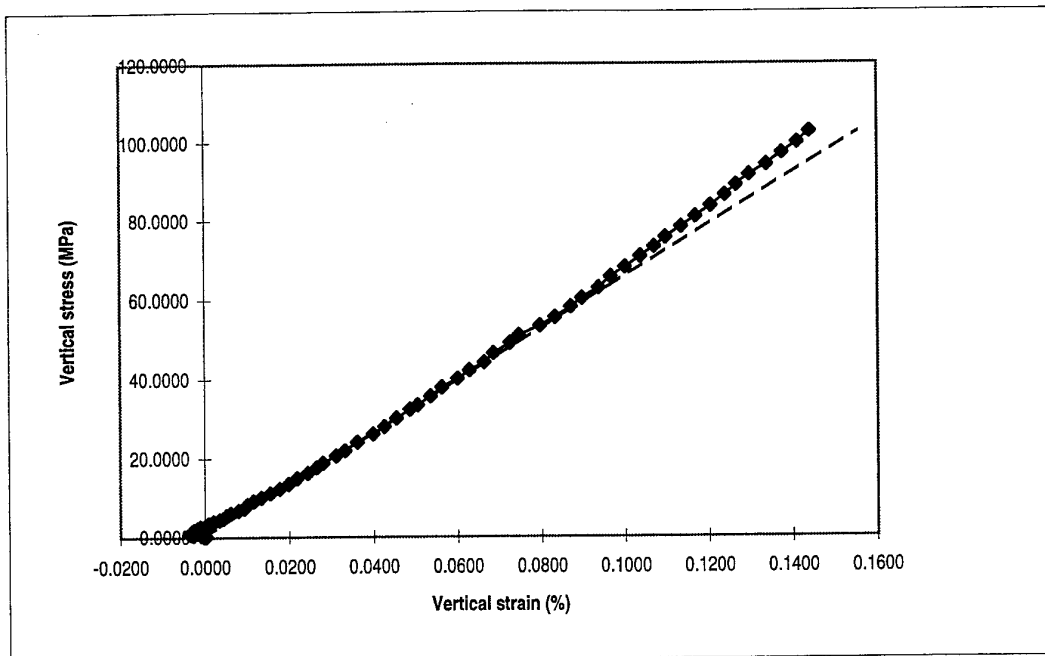
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.24
Unconfined Compression Test		Drawn by PC	Date 24.01.97
Boring : 97/6	Tube :	Checked	
Part : 5	Test : 25	Approved	
UCT No. : 376			



Uniaxial compressive strength: 106.74 MPa
 Deformation modulus: 58.49 GPa
 Poisson's ratio : 0.472

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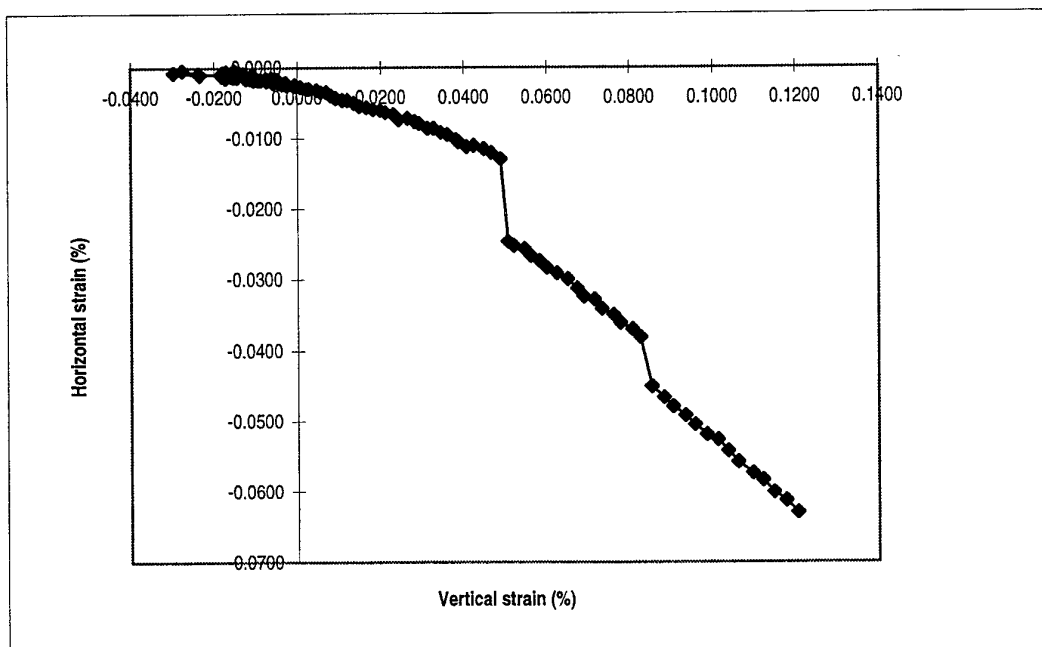
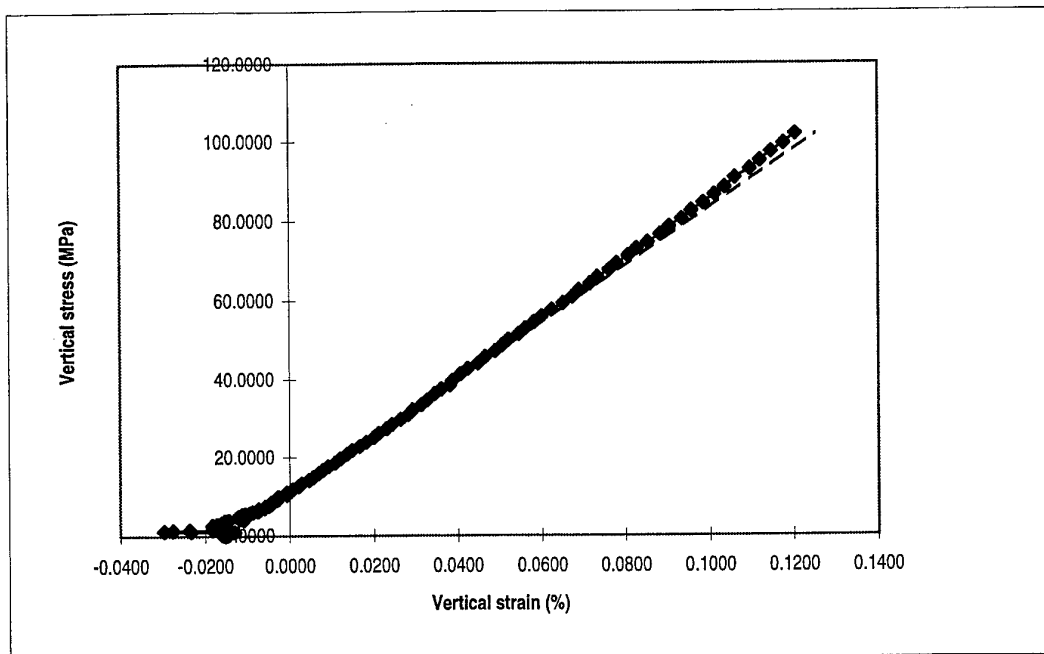
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.25
Unconfined Compression Test		Drawn by PC	Date 24.01.97
Boring : 97/7	Tube :	Checked	
Part : 1	Test : 26	Approved	
UCT No. : 377			



Uniaxial compressive strength: 102.61 MPa
 Deformation modulus: 65.63 GPa
 Poisson's ratio : 0.184

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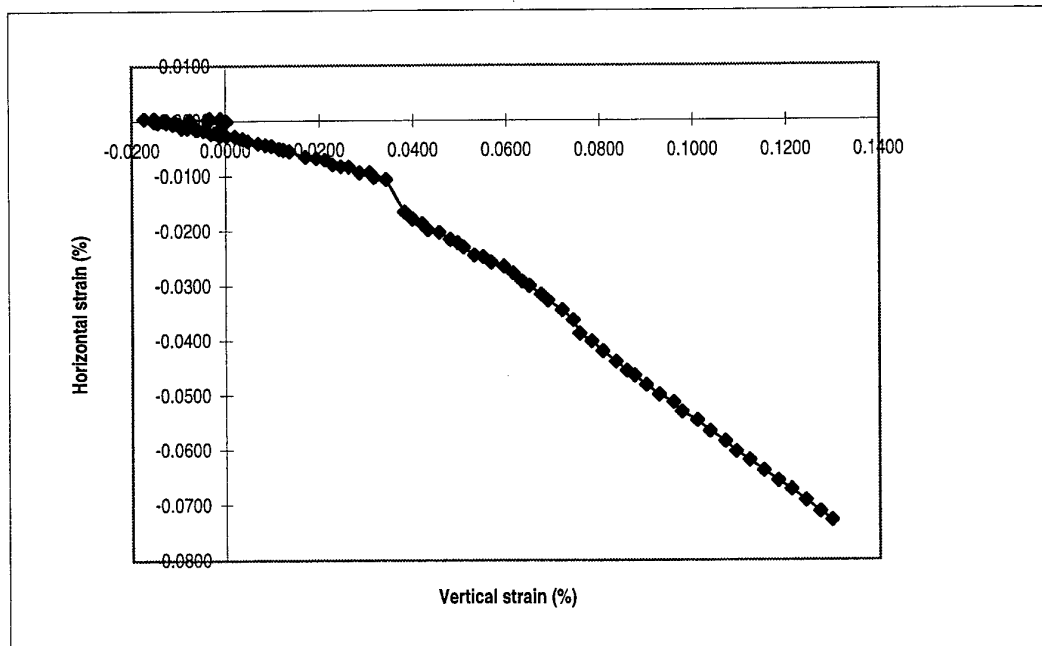
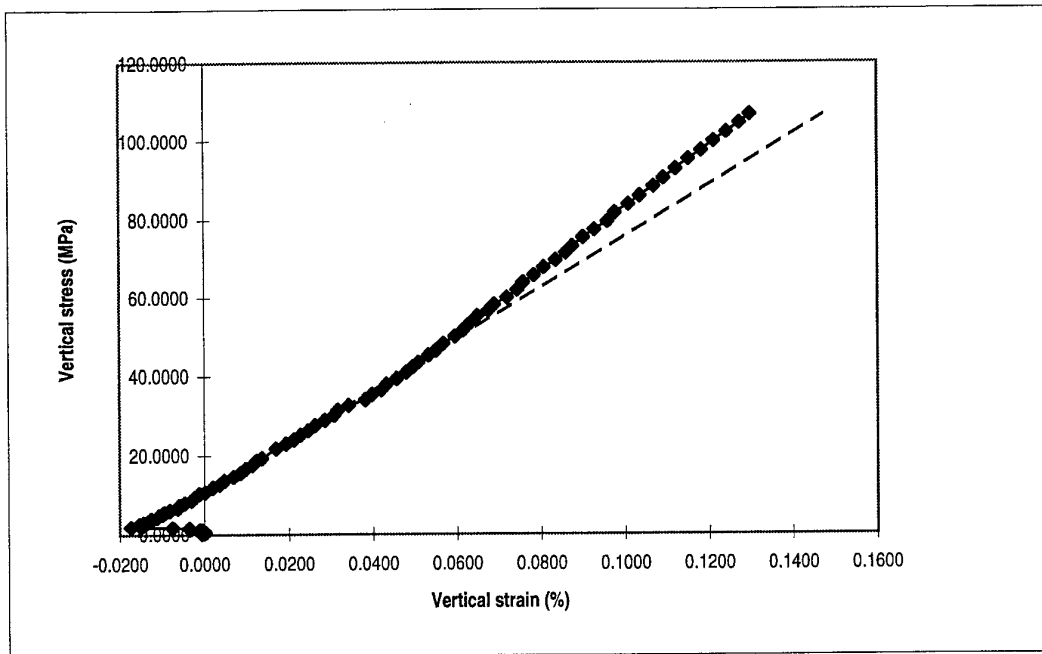
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.26
Unconfined Compression Test		Drawn by PC	Date 24.01.97
Boring : 97/7	Tube :	Checked	
Part : 2	Test : 27	Approved	
UCT No. : 378			



Uniaxial compressive strength: 101.78 MPa
 Deformation modulus: 72.38 GPa
 Poisson's ratio : 0.197

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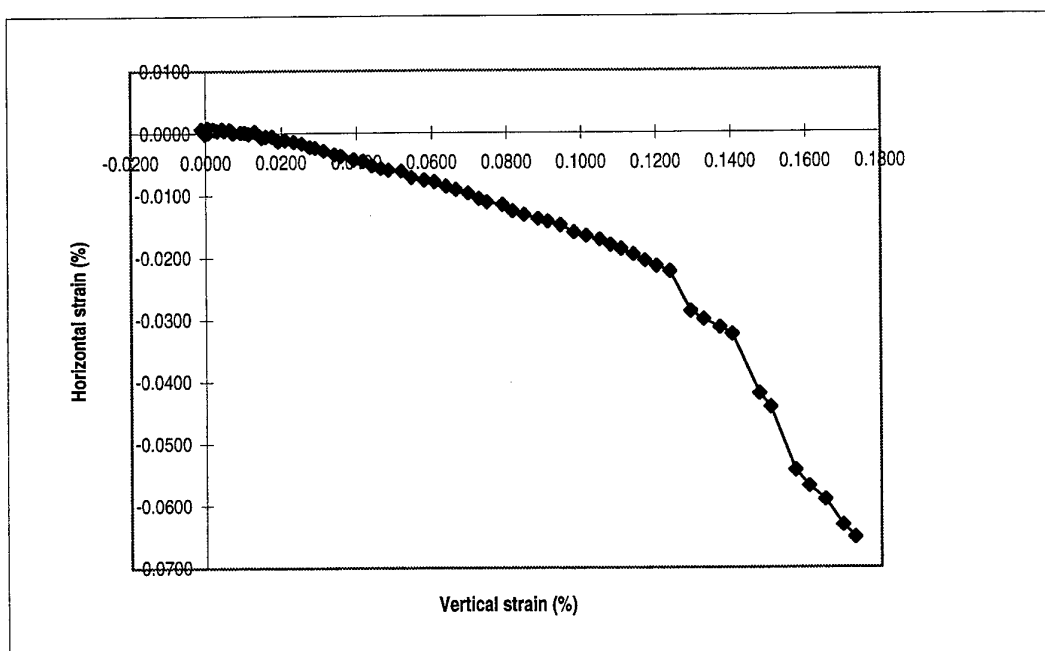
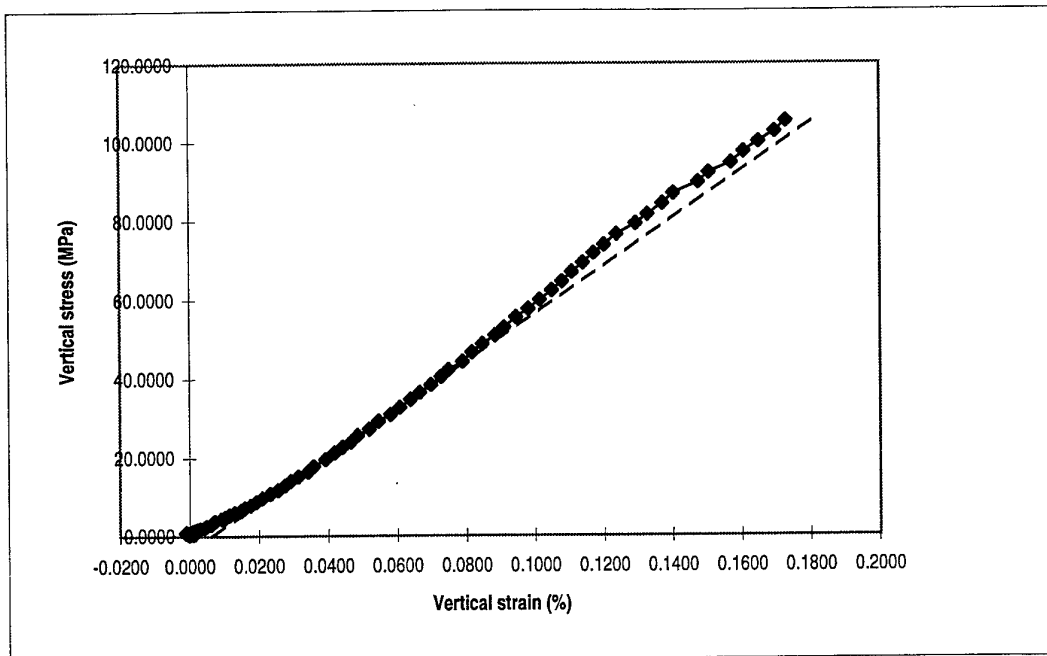
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.27
Unconfined Compression Test		Drawn by PC	Date 28.01.97
Boring : 97/7	Tube :	Checked	
Part : 3	Test : 28	Approved	
UCT No. : 379			



Uniaxial compressive strength: 106.70 MPa
 Deformation modulus: 65.39 GPa
 Poisson's ratio : 0.241

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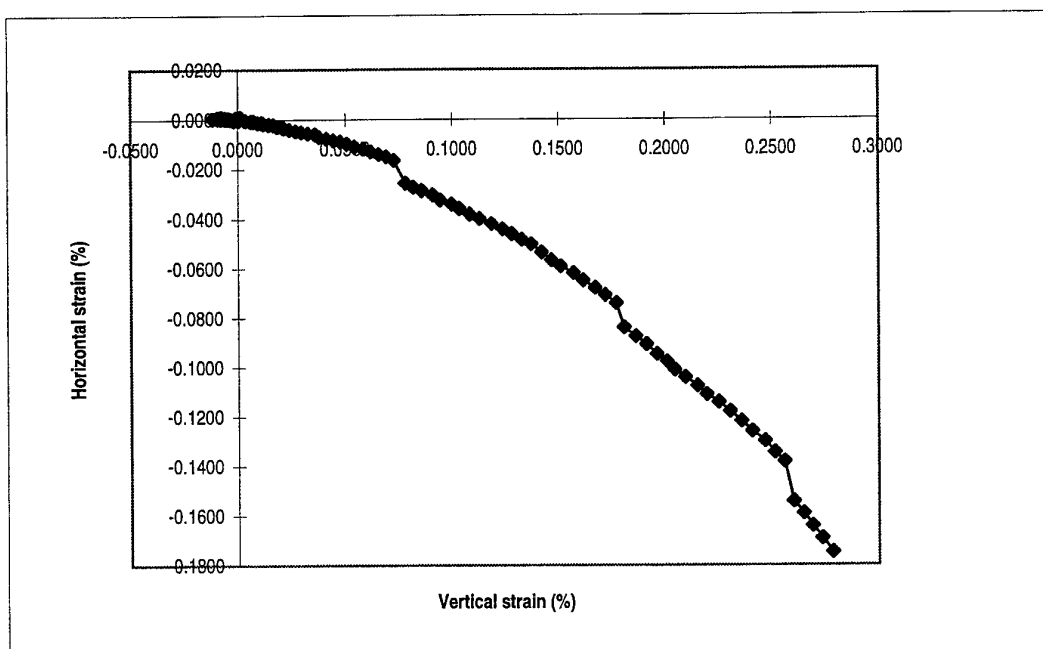
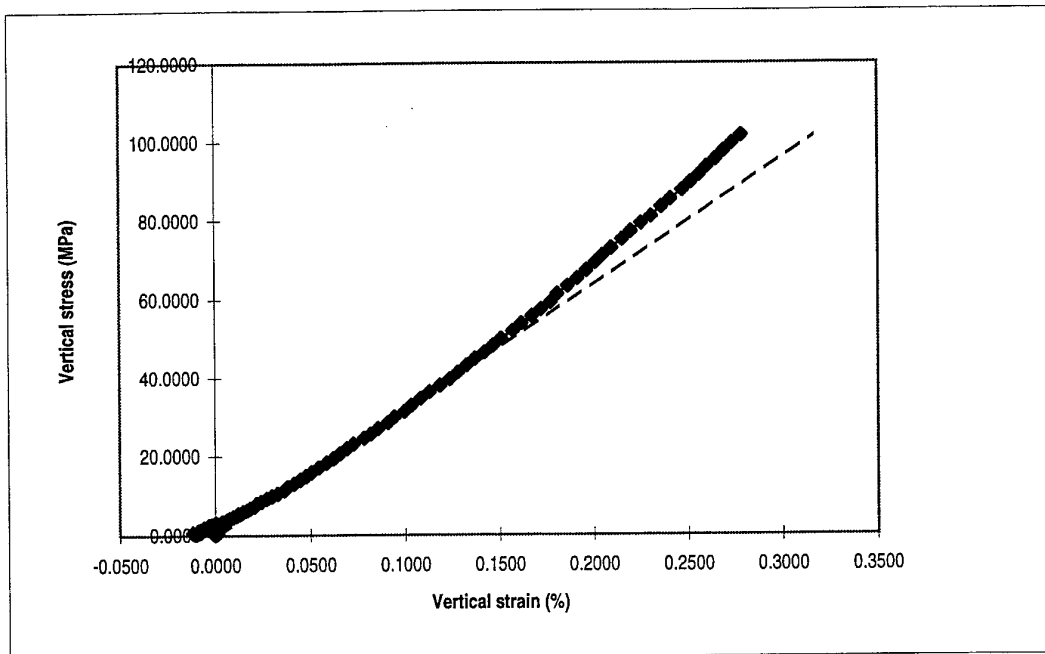
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.28
Unconfined Compression Test		Depth : 13.7 m	Drawn by PC
Boring : 97/7		Tube :	Date 20.02.97
Part : 4		Test : 29	Checked
		UCT No. : 380	Approved
			



Uniaxial compressive strength: 105.20 MPa
 Deformation modulus: 60.39 GPa
 Poisson's ratio : 0.180

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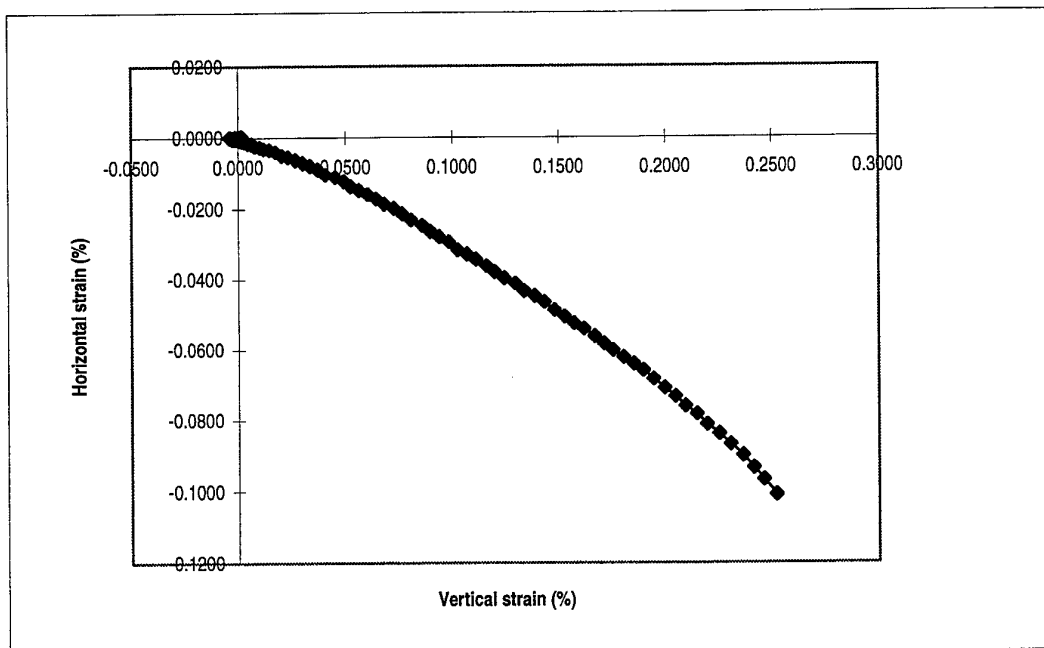
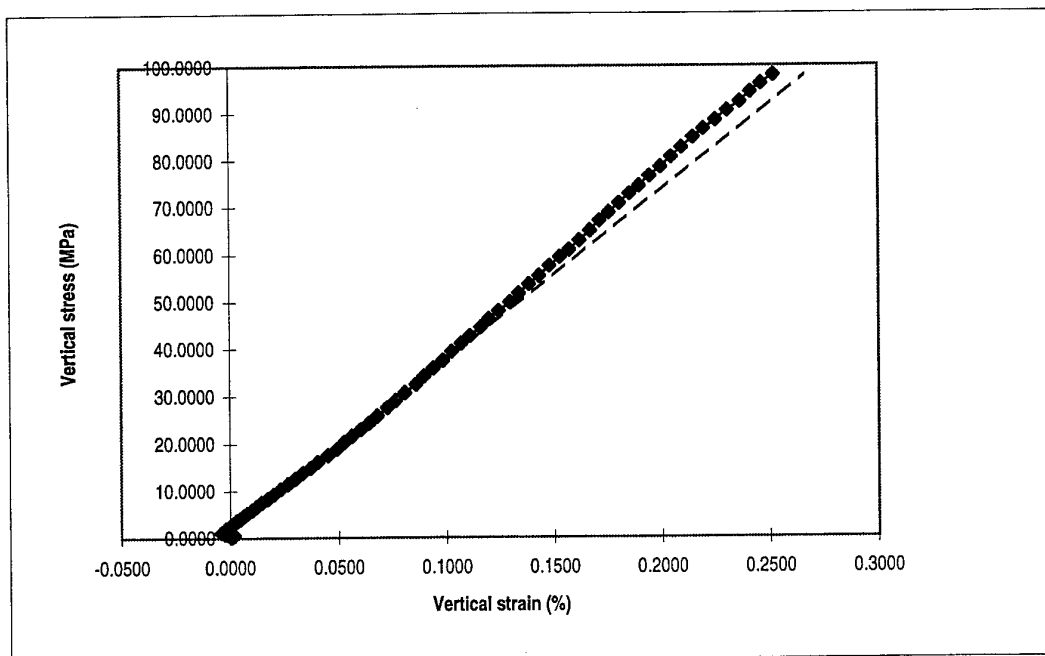
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.29
Unconfined Compression Test			Drawn by PC	Date 20.02.97
Boring : 97/7	Tube :		Checked	
Part : 5	Test : 30	UCT No. : 381	Approved	



Uniaxial compressive strength: 101.54 MPa
 Deformation modulus: 32.14 GPa
 Poisson's ratio : 0.482

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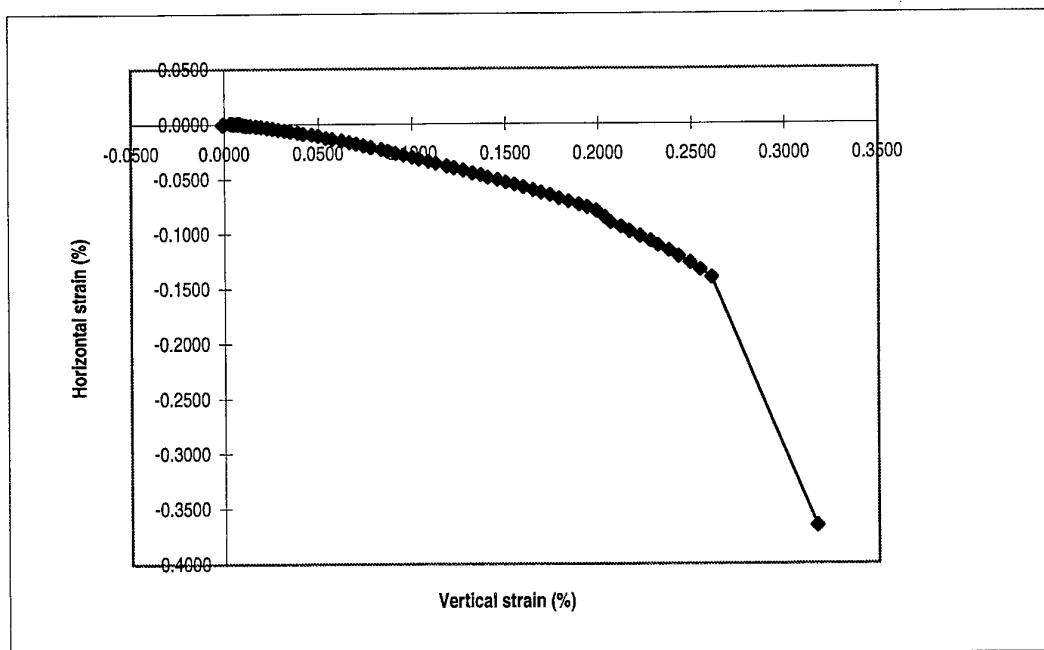
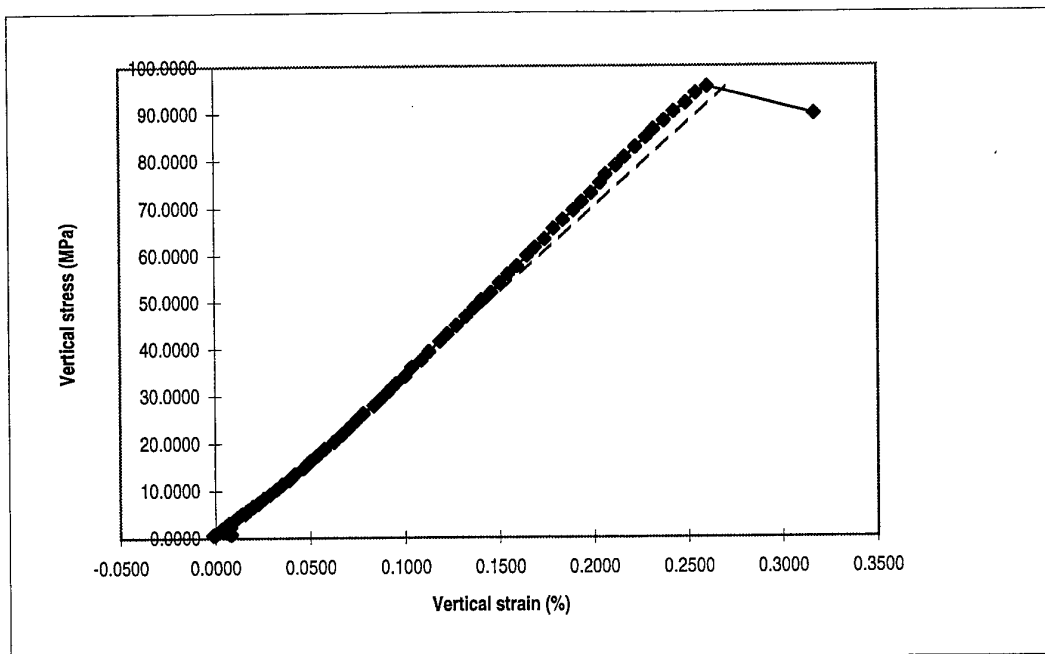
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.30
Unconfined Compression Test			Drawn by PC	Date 24.02.97
Boring : 97/8			Checked	
Tube :			Approved	
Part : 1				
Test : 31			UCT No. : 382	



Uniaxial compressive strength: 97.99 MPa
 Deformation modulus: 36.17 GPa
 Poisson's ratio : 0.323

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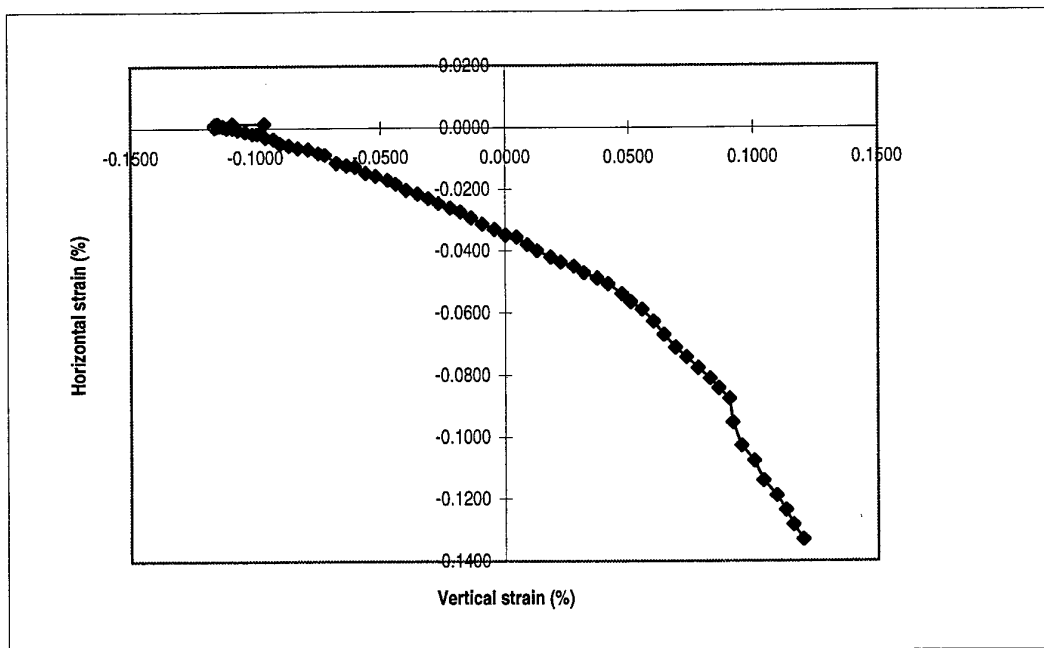
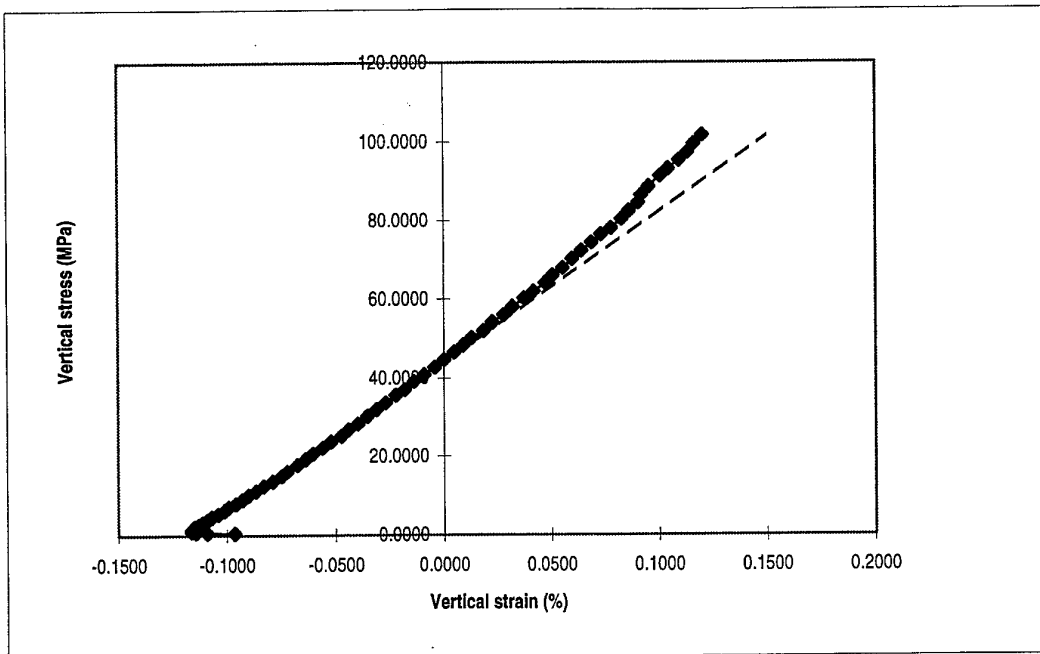
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.31
Unconfined Compression Test		Depth : 7.2 m	Drawn by PC	Date 24.02.97
Boring : 97/8	Tube :		Checked	
Part : 2	Test : 32	UCT No. : 383	Approved	



Uniaxial compressive strength: 95.56 MPa
 Deformation modulus: 35.99 GPa
 Poisson's ratio : 0.373

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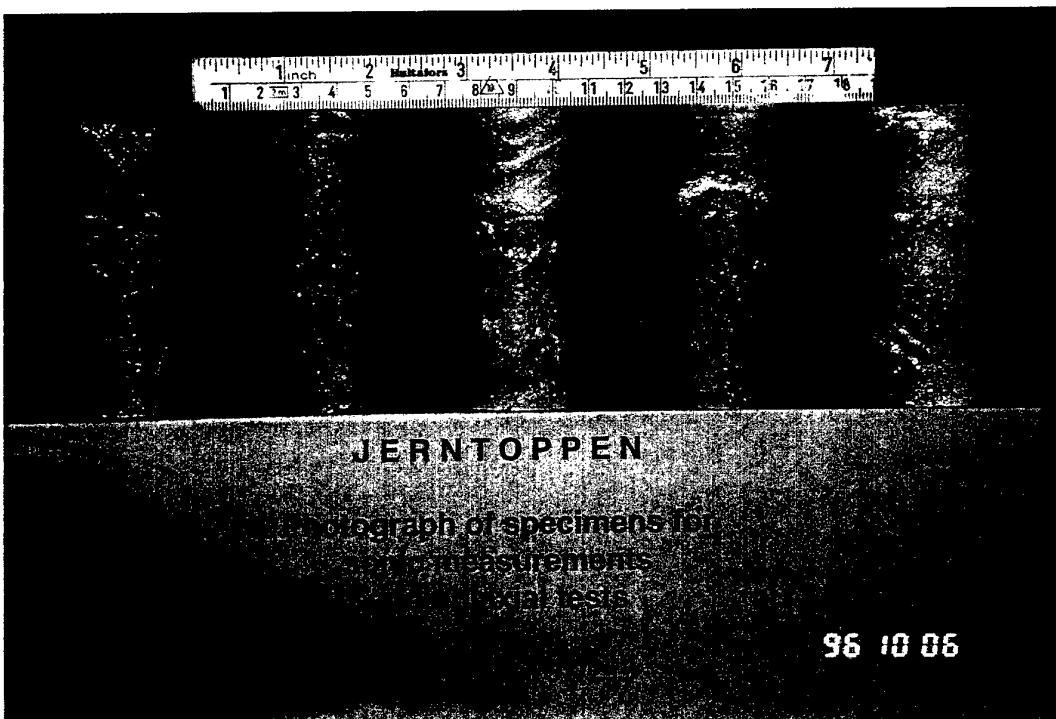
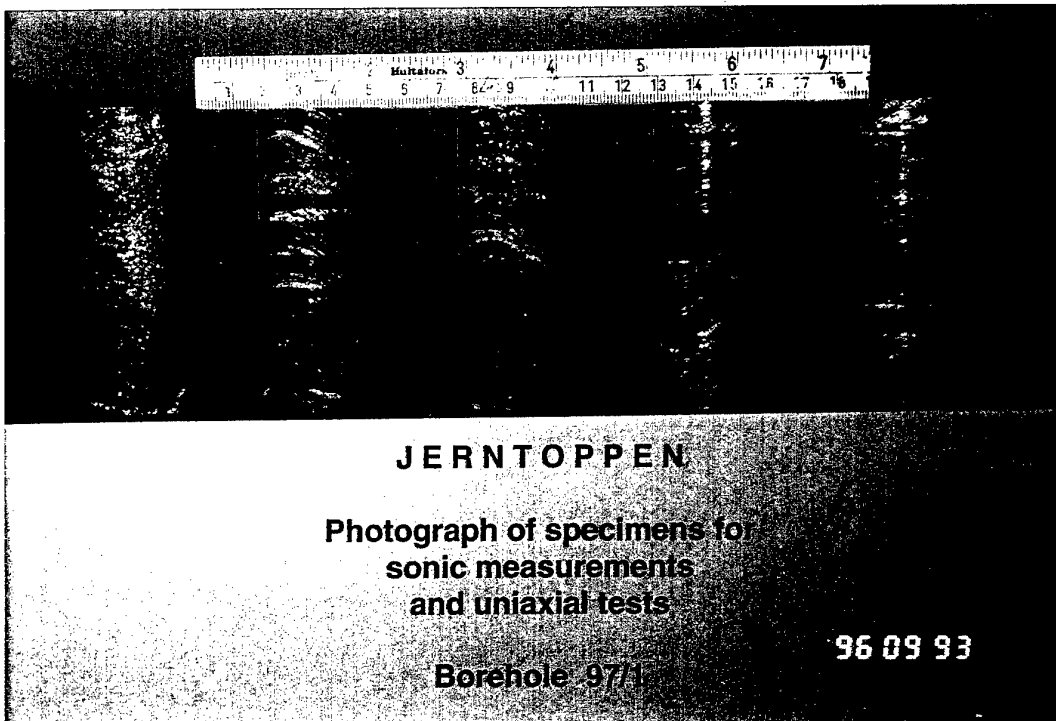
WES - TUNNEL CLOSURE EXPERIMENT		Report No. 923033-13	Figure No. 10.32
Unconfined Compression Test		Drawn by PC	Date 24.02.97
Boring : 97/8	Tube :	Checked	
Part : 4	Test : 34	Approved	
UCT No. : 384			



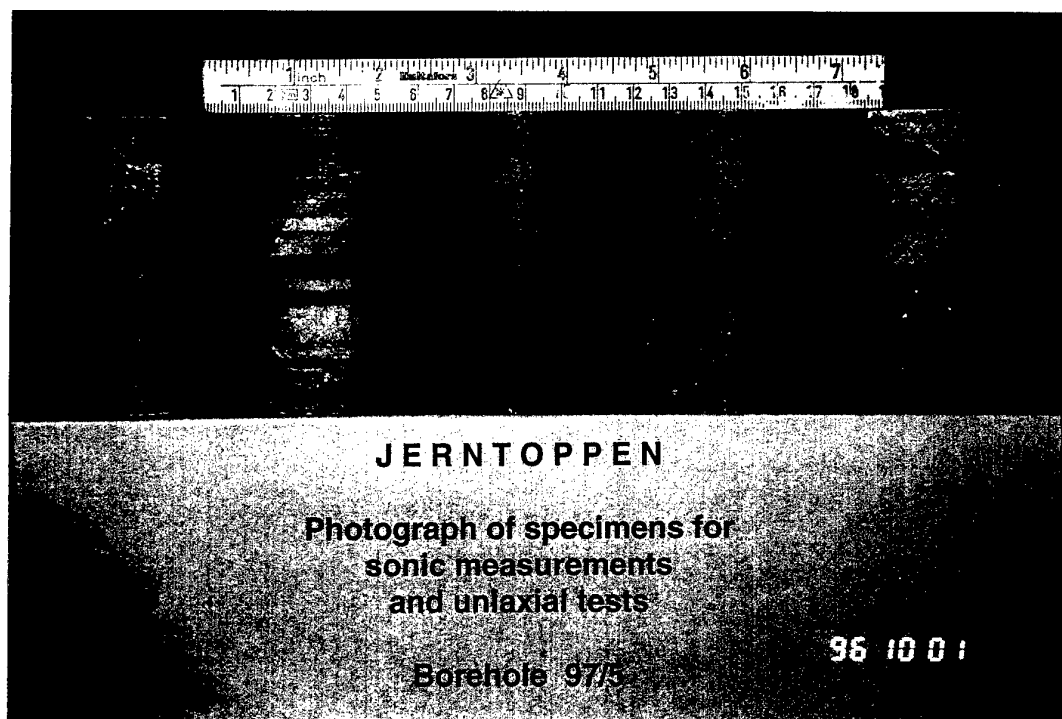
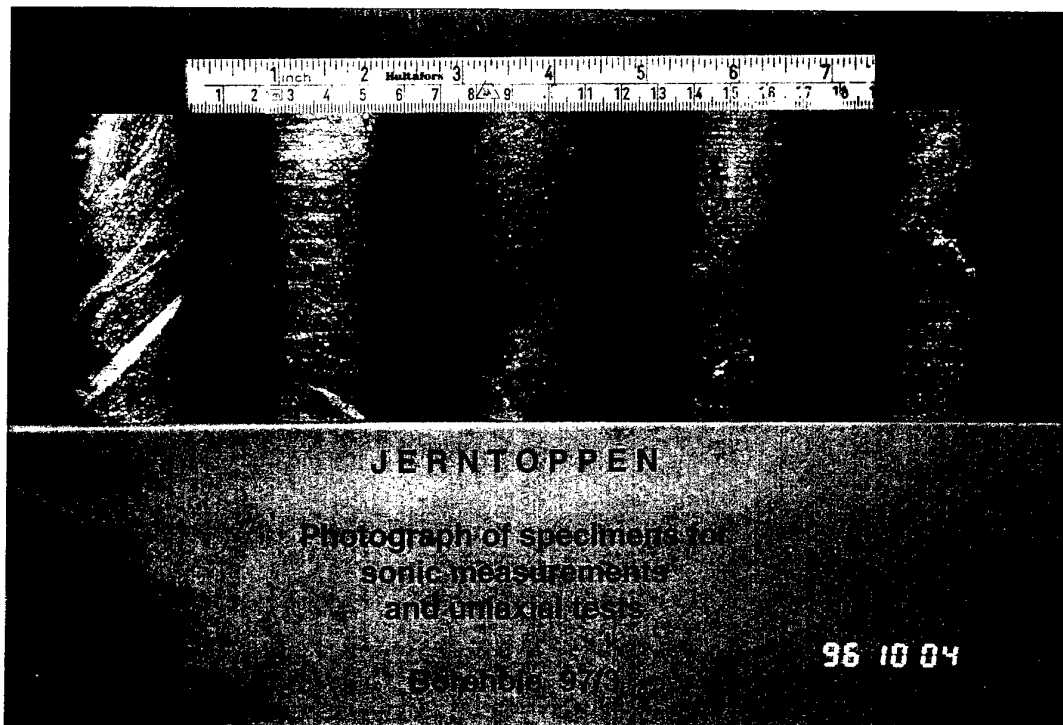
Uniaxial compressive strength: 101.62 MPa
 Deformation modulus: 38.41 GPa
 Poisson's ratio : 0.330

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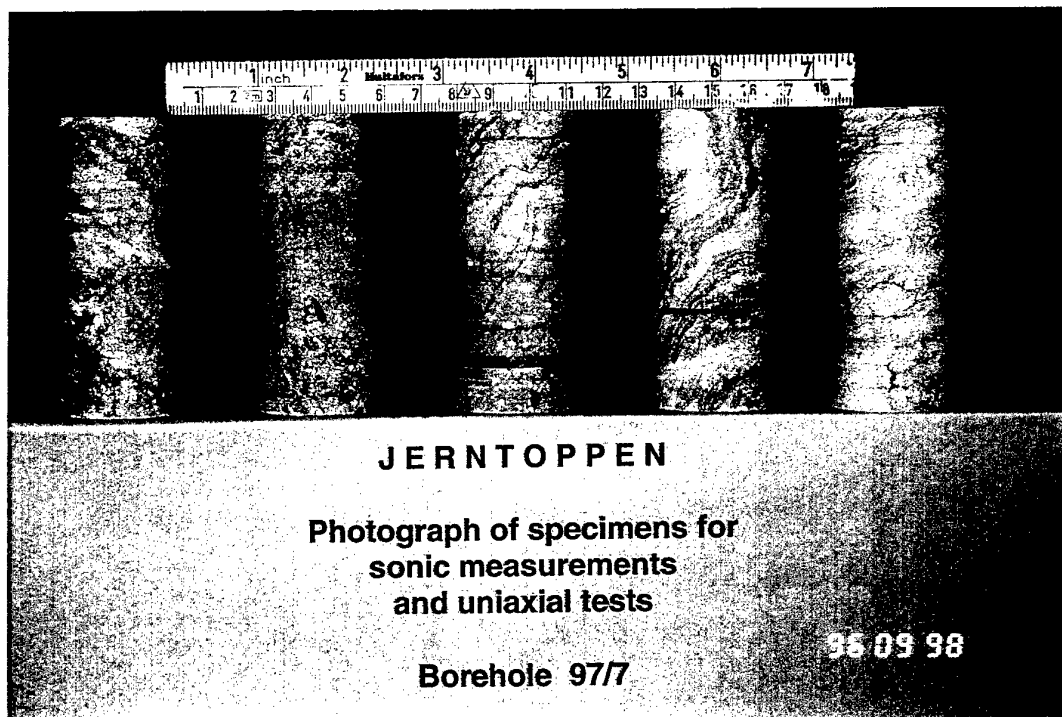
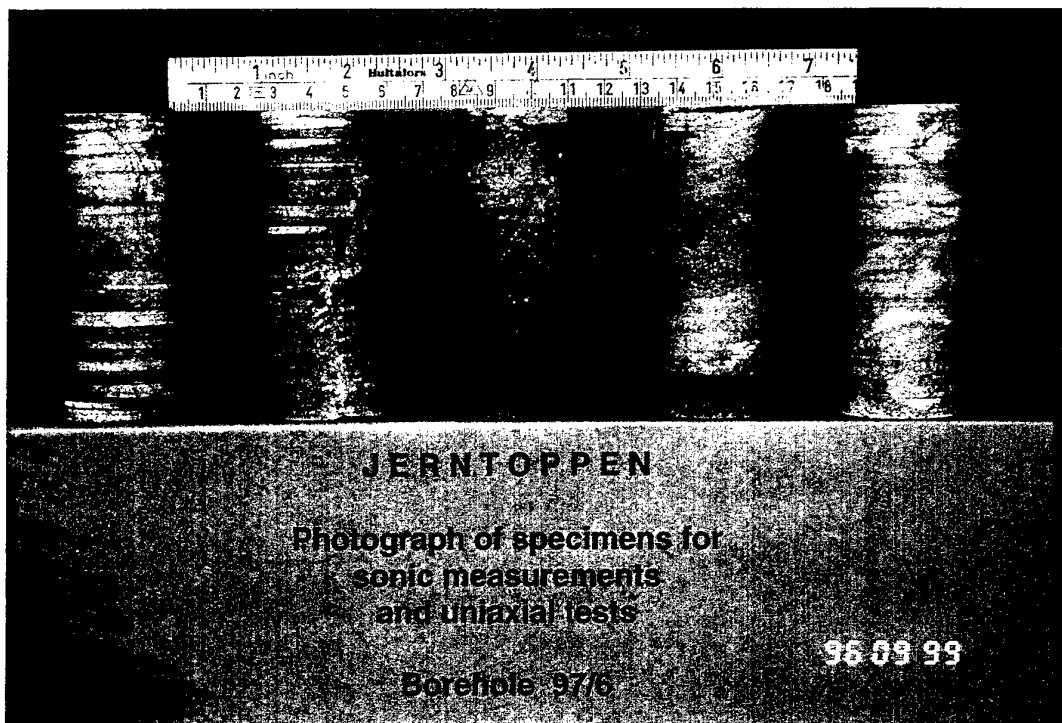
WES - TUNNEL CLOSURE EXPERIMENT			Report No. 923033-13	Figure No. 10.33
Unconfined Compression Test			Drawn by PC	Date 24.02.97
Boring : 97/8	Tube :	Depth : 7.9 m	Checked	
Part : 5	Test : 35	UCT No. : 386	Approved	



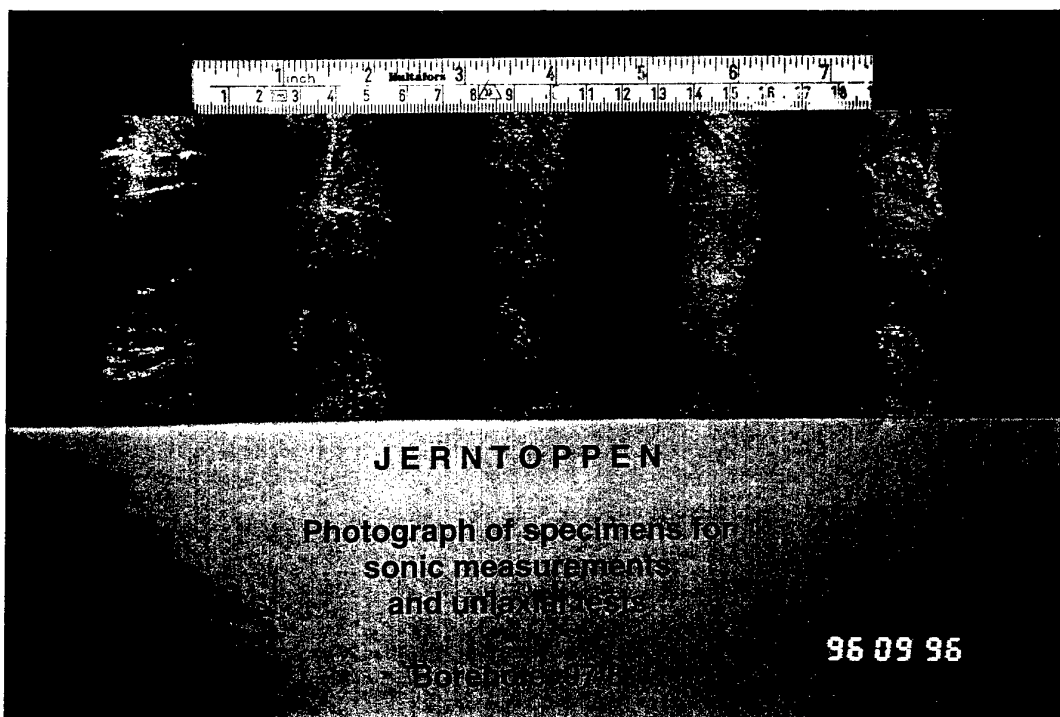
WES - TUNNEL CLOSURE EXPERIMENT	Report No. 923033-13	Figure No. 11.1
Photos of specimens for laboratory testing Boreholes 97/1 and 97/2	Drawn by PC	Date 97-02-28
	Checked <i>W</i>	
	Approved	



WES - TUNNEL CLOSURE EXPERIMENT	Report No. 923033-13	Figure No. 11.2
Photos of specimens for laboratory testing Boreholes 97/3 and 97/5	Drawn by PC	Date 97-02-28
	Checked VK	
	Approved	



WES - TUNNEL CLOSURE EXPERIMENT	Report No. 923033-13	Figure No. 11.3
Photos of specimens for laboratory testing Boreholes 97/6 and 97/7	Drawn by PC	Date 97-02-28
	Checked VK	
	Approved	



WES - TUNNEL CLOSURE EXPERIMENT	Report No. 923033-13	Figure No. 11.4
Photo of specimens for laboratory testing Borehole 97/8	Drawn by PC	Date 97-02-28
	Checked <i>VK</i>	
	Approved	

Kontroll- og referanseside/ Review and reference page



Oppdragsgiver/Client Waterways Experiment Station		Dokument nr/Document No. 923033-13
Kontraksreferanse/ N68171-97-C-9005 Contract reference		Dato/Date 8 June 1998
Dokumenttittel/Document title Tunnel Closure Experiment - 1997 Test Programme. Core Logging and Laboratory Tests on Diamond Drilled Holes at Jerntoppen. Prosjektleder/Project Manager Vidar Kveldsvik. Utarbeidet av/Prepared by Panayiotis Chryssanthakis, Vidar Kveldsvik		Distribusjon/Distribution ☐ Fri/Unlimited ☒ Begrenset/Limited ☐ Ingen/None
Emneord/Keywords Core logging, pressure-wave velocity, shear-wave velocity, uniaxial compressive strength, Young's modulus, Poisson's ratio.		
Land, fylke/Country, County Norway, Finnmark. Kommune/Municipality Sørvaranger. Sted/Location Bjørnevatn, Kirkenes. Kartblad/Map M 711-series. Map 2434 II UTM-koordinater/UTM-coordinates		Havområde/Offshore area Feltnavn/Field name Sted/Location Felt, blokknr./Field, Block No.

Kvalitetssikring i henhold til/Quality assurance according to NS-EN ISO9001							
Kon- trollert av/ Reviewed by	Kontrolltype/ Type of review	Dokument/Document		Revisjon 1/Revision 1		Revisjon 2/Revision 2	
		Kontrollert/Reviewed		Kontrollert/Reviewed		Kontrollert/Reviewed	
		Dato/Date	Sign.	Dato/Date	Sign.	Dato/Date	Sign.
VK	Helhetsvurdering/ General Evaluation *	08-06-98	VK				
THa	Språk/Style	08-06-98	THa				
	Teknisk/Technical - Skjønn/Intelligence - Total/Extensive - Tverrfaglig/ Interdisciplinary						
THa	Utforming/Layout	08-06-98	THa				
VK	Slutt/Final	08-06-98	VK				
JGS	Kopiering/Copy quality	5/6-98	JGS				
* Gjennomlesning av hele rapporten og skjønnsmessig vurdering av innhold og presentasjonsform/ On the basis of an overall evaluation of the report, its technical content and form of presentation							
Dokument godkjent for utsendelse/ Document approved for release		Dato/Date 08-06-98		Sign. <i>VK</i>			

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- * Bergrom og tunneler
- * Dammer
- * Sikring mot skred
- * Miljøvern og miljøgeoteknologi
- * Reservoarmekanikk og borhullsteknologi
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- * Modell- og feltforsøk
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- * Avalanches, landslides and safety measures*
- * Environmental geotechnical engineering*
- * Petroleum reservoir mechanics and borehole technology*
- * Site investigations and laboratory testing*
- * Model and field testing*
- * Field instrumentation and performance evaluation*