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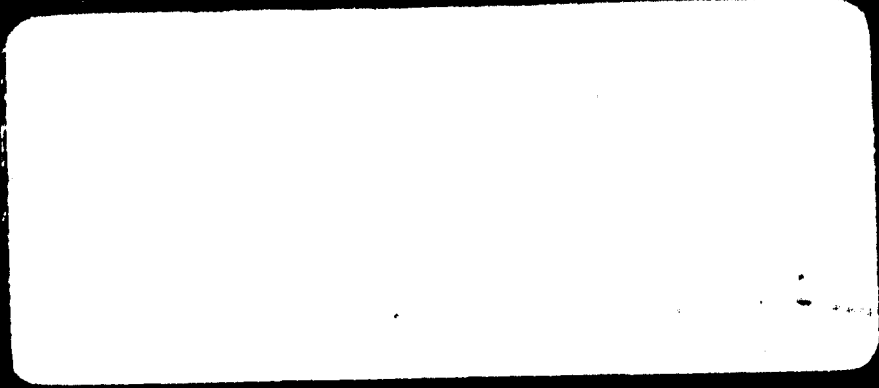
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27-771-92 FPO 7601, Appendix B

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Crest Engineering, Inc. Ocean Engineering &
Construction Project Office
CHESNAVFACENGCOM

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This report is a study of four-pile templet type offshore structure. It
constitutes a portion of the total project report on the structural concept
analysis of an ocean structure for the U.S. Navy Air Combat Maneuvering Range
(ACMR) offshore North Carolina, U.S.A. (con't)

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BLOCK 19 (cont.)

The objective of this report is to establish the most efficient four-pile ocean structure under the environmental loading conditions given. The conditions are the same as those used in the previous design of skirt-pile structures.

The four-pile structure is similar to standard templet type structures used by the oil industry. It has four similar faces with equally spaced jacket legs battered at a true slope of 1 to 7. The structure is to be designed for a water depth of 84 feet (MLW). The anchoring of the structure is to be achieved by driving piles through the jacket legs into the seabed. Securing of the jacket is then accomplished by welding of shim plates along the annulus between the jacket leg and piling at the top of the jacket legs. A superstructure consisting of an equipment deck and a top deck will be installed to the piling above the jacket. A boatlanding is to be furnished with the jacket. A set of stairways attached to the superstructure shall be furnished to provide connections among the boatlanding, equipment deck and the top deck.

FPO 7601 APP B

STRUCTURAL CONCEPT ANALYSIS REPORT

FOR THE

Line

EAST COAST AIR COMBAT MANEUVERING RANGE

OFFSHORE KITTY HAWK, NORTH CAROLINA

CONTRACT NO. N62477-76-C-0179

REPORT NO. 27-771-92

APPENDIX B. FOUR-PILE CONCEPT CALCULATIONS

Prepared for

NAVAL FACILITIES ENGINEERING COMMAND
DEPARTMENT OF THE NAVY
CHESAPEAKE DIVISION

By

CREST ENGINEERING, INC.
TULSA, OKLAHOMA

May 1976

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SECTION 1
INTRODUCTION

1.1 INTRODUCTION

This report is a study of a four-pile templet type offshore structure. It constitutes a portion of the total project report on the structural concept analysis of an ocean structure for the U.S. Navy Air Combat Maneuvering Range (ACMR) offshore North Carolina, U.S.A.

The objective of this report is to establish the most efficient four-pile ocean structure under the environmental loading conditions given. The conditions are the same as those used in the previous design of skirt-pile structures.

The four-pile structure is similar to standard templet type structures used by the oil industry. It has four similar faces with equally spaced jacket legs battered at a true slope of 1 to 7. The structure is to be designed for a water depth of 84 feet (MLW). The anchoring of the structure is to be achieved by driving piles through the jacket legs into the seabed. Securing of the jacket is then accomplished by welding of shim plates along the annulus between the jacket leg and piling at the top of the jacket legs. A superstructure consisting of an equipment deck and a top deck will be installed to the piling above the jacket. A boatlanding is to be furnished with the jacket. A set of stairways attached to the superstructure shall be furnished to provide connections among the boatlanding, equipment deck and the top deck.

1.2 DESIGN CRITERIA

Design criteria presented herein serve as guide lines in the conceptual development of the proposed four-pile structure. These criteria are listed as follows:

A. Environmental Criteria

MLW Depth	84 ft
Storm Wave Height	62 ft
Storm Wave Period	12 sec
Maximum Storm Tide	10 ft
Maximum Astronomical Tide	4 ft
Maximum Current (for full depth)	5.4 ft/sec
Wind Velocity	150 Knots @ (+)30 ft

The approach of the storm wind and waves can be from any direction.

B. Foundation Criteria

- (1) The basis for the foundation design is a McClelland report to Cubic Corporation entitled "Foundation Investigation East Coast ACMR Ocean Structures, Volume I". The soil information to be used in this analysis is one boring at Site No. 1 in the aforementioned report.
- (2) Due to the nature of the sea bottom, and sea bottom currents, scouring of 5 feet below mudline will be used in the preliminary piling design to develop the

theoretical soil resistance to laterally applied loads.

C. Live Loads

The live loads shall be as follows:

Equipment Deck 150 psf

Top Deck 100 psf

The loads shall be distributed uniformly over the entire deck areas.

D. Materials

All structural shapes or fabricated tubular goods are assumed to be ASTM A-36 or equal.

E. Corrosion Protection

- (1) All portions of the platform above elevation (-)4' - 0" shall be painted.
- (2) All main structural members located within the splash zone shall have an extra 1/2 inch of sacrificial steel added to their wall thickness. This can be in the form of extra wall thickness or a 1/2 inch steel plate wrap.
- (3) The portion of the platform below elevation (-)4' - 0" will be protected by cathodic protection. This will be provided by sacrificial anodes having a theoretically expected life of ten years.

The criteria employed for determination of structural acceptability are specified by the documents:

- (a) Specification API RP 2A, Recommended Practice for Planning, Designing and Constructing Fixed Offshore Platforms, 7th Edition, American Petroleum Institute, Dallas, Texas, 1976.
- (b) Manual of Steel Construction, 7th Edition, American Institute of Steel Construction, New York, N.Y., 1969.

1.3 DESIGN ASSUMPTIONS

Some assumptions to be used in this analysis are listed as follows:

True batter of piling and jacket leg	1:7
Equipment deck area	500 sq ft
Top deck area	1,200 sq ft
Spacing between decks	15 ft

1.4 PROCEDURES OF ANALYSIS

The grid wave data from the skirt-pile structures design is coded to SEALOAD in Section 2 for loading of the structure. This is again used in Sections 6 and 7 of this report.

In order to find the most economical structural member sizes relating selected jacket base spacings to required piling capacities are shown in Section 3. The graphs and formulas derived are used to produce tables relating jacket base spacing to steel weight in Section 4.

A jacket base spacing of 50 ft was then chosen together with preliminary member sizes in Section 5 and sketches were proposed. Section 6 shows the idealized structure with designated joint and member numbers and also the results of the SEALOAD loadings on the structure. Analyzing the results of the loading together with the penetration required for the piles, it was decided that another SEALOAD run should be made to determine the loading to the pile using a 60 ft jacket base spacing (Section 7). A SEALOAD run was also made on the boatlanding (Section 8) and stairway (Section 9) to provide the additional wind and wave forces to the structure.

The pile sizes and lengths are proposed in Section 10 of this report. The piles are then analyzed to ensure sufficient lateral load carrying capacities. The loading conditions on the structure are detailed in Section 11 and the final STRAN run was made to check the integrity of the entire structure.

The results of the STRAN run are shown in Section 12. Section 13 is the final area and weight take-offs for pricing.

1.5 DESIGN SUMMARY

Some of the more significant results from the analysis are summarized as follows:

Environmental Forces:

Total wind and wave forces (including boatlanding and stairway)	1,350 kips
Total overturning moment (including boatlanding and stairway)	109,627 ft-kips

Pile Axial Loads:

Maximum compressive load	1,712 kips
Maximum tensile load	1,453 kips

Structural Dimensions:

Piling

Outside diameter	36 in.
Penetration below mudline	180 ft

Jacket

Spacing at mudline	50.1 ft
Spacing at work-point level	30.0 ft
Height	101 ft-9 in.

Superstructure

Equipment deck	15 ft x 30 ft
Top deck	35 ft x 35 ft

Structural Steel Weight

Piling	681,620 lbs
--------	-------------

Superstructure 172,000 lbs

Jacket 315,160 lbs

Total weight
(excluding boat-
landing, stairway
and miscellaneous
items) 1,168,780 lbs

1.6 PERSONNEL RESUMES

The personnel whose resumes follow were actively engaged in this project.

CREST OFFSHORE, INC.



James M. Atkinson

Design Engineer

<u>University</u>	<u>Degree</u>	<u>Year</u>
University of Notre Dame	2 yrs. Aeronautical Engineering	1963
Oklahoma State University	Bachelor of Architectural Engineering	1967

Licenses:

Registered Professional Engineer - Oklahoma

Experience:

1975 to Present

Crest Offshore, Inc.

Design Engineer

Designed office buildings and luxury camp for TOTAL Indonesie in Borneo, Indonesia.

1974 to 1975

Atkinson Engineering Co.

Owner

Independent design work with other consulting engineers on Yale Avenue Methodist Church, new Central High School and new Sear's Building.

1972 to 1974

Bloom Van Fossen, Brase Architects & Engineering

Associate Engineer

Responsible for the structural design of Copper Oaks Office Park, Sand Springs Shopping Center, East Vue Shopping Center, United Bank, Falls Office Tower and Shopping Center Denver; The Park, The Port and Falls Phase IV.

CREST OFFSHORE, INC.

James M. Atkinson

Design Engineer

Experience Continued:

1970 to 1972

Sullivan Engineering Co., Tulsa Oklahoma

Senior
Engineer

Responsible for design of Heilder Tower, Hornet Stadium, Sheraton Inn, Oral Robert's University Dormitories, O.R.U. Geodestic Dome, Hathaway Mfg. Co. Plant, U.S. Jaycees Expansion, Union High School, Ponca City Savings & Loan, 32 story bank building in Tampa, Florida; Clinton Jr. High School, Thoreau Jr. High School and Grissom Elementary School.

1969 to 1970

Craig & Keithline Consulting Engineers, Tulsa, Oklahoma

Junior
Engineer

Assisting in the design of bridges, numerous electrical relay stations and river crossing towers. Later became Senior Engineer in charge of designing sewage treatment and water treatment plant structures, schools and commercial buildings.

1967 to 1969

Engineering Pacific, Inc., Portland, Oregon

Junior
Engineer

Responsible for the structural design of concrete tilt-up warehouses, rigid frame steel buildings feed and seed mills, residences, office and commercial buildings.

February 26, 1976

SECTION 2 GRID WAVE DATA

2.1 INTRODUCTION

This structure is designed to the same criteria as those used to design the skirt-pile structure. A Crest Engineering program using Stoke's 5th Order Gravity Wave Theory was used to develop the wave pressure profile for the given wave. This section codes to Synercom Format the pressure profile of that wave. A 24 in. diameter pipe pile extending from the mudline to 145 ft above mudline was used to test the final wave grid. A hand analysis was then computed to test the results from the SEALOAD report. The results were compatible thus indicating that the grid wave data had been coded properly and was ready for use in the loading of the three-pile and four-pile structures.

CREST OFFSHORE, INC.

Sheet 2.02 of 4 FILE

By WV Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 3-31-76 Job No. 27-TL-92 Calculation GRIP WAVE DATA

2.2

TABLE 4P2.1

HORIZONTAL WAVE PRESSURES for 24" ϕ PIPE												
X SPACING FROM CREST												
Z	0			601 (43)		0			(2)		653 (42)	
	PRESSURE	ELEV.		PRESSURE		PRESSURE	ELEV.		PRESSURE		PRESSURE	
1	921	721	142			920					807	
2	845	803	138			845	895	141	140	844	797	
3	777	748	135			778	812	138	130	797	705	
4	710	740	131			718	763	134	133	719	670	
5	661	694	128			663	718	130		670	670	
6	611	624	124			614	639	127	125	602	625	
7	566	566	120			570	570	123		579	579	
8	520	508	117			530	518	120		537	537	
9	489		113			530	518	116	115	488	499	
10	456	456	110			444	469	113	110	442	465	
11	426	417	106			461	469	109		442	434	
12	399	380	103			431	424	106	105	399	405	
13	374		99			404	387	102	100	365	380	
14	352		96			379		99			357	
15	331	318	92			357		95			335	
16	312		89			330	327	92	90	307	316	
17	295	274	85			317		88		307	293	
18	266		78			300	279	85	80	262	282	
19	241	238	71			270		73		262	254	
20	220	210	64			246	243	71	70	229	231	
21	203		57			225	218	63	60	204	211	
22	178	175	43			208		56			195	
23	161	156	28			182	180	42	40	169	171	
24	152		14			165	160	28	20	151	156	
25	150		7			156		14			147	
26	149	149	0			154		7			145	
						153	153	0		144	144	

Same as 0

CREST OFFSHORE, INC.

Sheet 2.02 of 4 FILE

By JW Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 3-31-76 Job No. 27-TL-72 Calculation GRID WAVE DATA

TABLE 4P2-2

HORIZONTAL WAVE PRESSURES for 24" ϕ PIPE												
Z	X SPACING FROM CREST											
	17 (3)			644 (4)			25 (4)		630 (4)			
	PRESSURE		ELEV.	PRESSURE		PRESSURE	ELEV.	PRESSURE				
1	807	844	138	139	138	749	769	778	778	130	416	646
2	799	799	133	130	133	709	709	722	722	133	519	599
3	739	754	130	132	130	609	655	671	671	130	557	557
4	684	702	130	129	130	622	606	625	611	125	506	518
5	635	636		125		502	502	583	554	120	459	483
6	590	570	120	122	120	504	522	544		119	410	451
7	549			118			436	509	498	115	412	421
8	512	512		115		453	453	477	456	110	376	395
9	479	469	110	111	110	414	423	448		109		370
10	448	427	105	108	105	377	396	421	415	106	343	348
11	420			104			371	397	382	102	315	328
12	395	389	100	101	100	344	349	374		99	280	309
13	371			97			323	354		96		292
14	350			94			309	335	323	92	267	276
15	331	327	90	91	90	293	292	317		89	267	262
16	313			87			276	301		85		249
17	297	281	80	84	80	243	262	287	280	80	231	237
18	269			77			237	261	245	70	203	216
19	245	245		70		216	216	239		68		193
20	225	218	60	63	60	193	199	221	219	60	181	183
21	208			50			154	200		55		170
22	184	182	40	42	40	160	162	183	182	40	151	152
23	167	162	20	28	20	144	143	168	163	20	136	139
24	158			14			140	159		14		132
25	156			7			133	157		7		131
26	155	155		0		138	138	156	156	0	130	130

CREST OFFSHORE, INC.

Sheet 204 of 514

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 3-31-70 Job No. 27-TL-92 Calculation GRID WAVE DATA

TABLE 4P2-B

HORIZONTAL WAVE PRESSURES for a 24" ϕ PIPE												
NO. L	X SPACINGS FROM CREST											
	33 (a)		628 (39)		58 (8)		603 (36)					
	PRESSURE	ELEV.	PRESSURE	PRESSURE	ELEV.	PRESSURE	ELEV.	PRESSURE				
1	674	130	133	519	519	397		119	222			
2	630	130	129	494	485	380	374	116	212			
3	590	125	126	444	454	363	374	113	204			
4	553	120	123	406	425	348	348	110	196			
5	519	120	119		399	333	324	107	188			
6	488	115	116	367	375	320	324	104	181			
7	459	110	113	337	352	307	303	101	174			
8	433	110	109		332	295		98	168			
9	408	105	106	307	313	283		95	162			
10	386	100	103	284	296	273	266	92	156			
11	365	100	99		280	263	266	89	151			
12	346		96		266	253		86	146			
13	329		93		252	244		84	142			
14	313	313	90	240	240	236	233	81	137			
15	298		86		229	228	233	80	133			
16	284		83		218	221		75	130			
17	272	272	80	209	209	214	210	72	126			
18	249	241	73	186	192	202	210	66	120			
19	230	241	66		177	191	191	60	114			
20	214	214	60	165	165	182		54	109			
21	200		53		155	174	165	48	105			
22	180	180	45	139	139	161	165	40	99			
23	166	162	27	126	129	152	150	24	94			
24	158		13		123	147	150	12	92			
25	150		7		122	146		6	91			
26	150	150	0	122	122	146	140	0	91			

By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 3-21-76 Job No. 27-TL-92 Calculation GRID WAVE DATA

TABLE 4P2-4

HORIZONTAL WAVE PRESSURES for 24" ϕ PIPE											
S Z	X = 140 FROM COAST										
	91 (12)		570 (32)		330.5 (22)						
L	PRESSURE		ELEV.		PRESSURE		PRESSURE		ELEV.		PRESSURE
1	195	195	105	33	33	-17	-17	80	80		
2	191		103		34	-16			78		
3	187	187	100	35	35	-16			76		
4	183		97		36	-15			74		
5	179		95		37	-14			72		
6	175		92		38	-13	-13	70	70		
7	171	171	90	39	39	-13			68		
8	168		87		39	-12			66		
9	165		84		40	-12			64		
10	161	159	82	41	41	-11			62		
11	158		79		41	-11	-11	60	60		
12	155		76		42	-10			58		
13	153		74		42	-10			56		
14	150	149	71	42	42	-9			54		
15	147		68		43	-9			52		
16	145		66		43	-9			50		
17	142	140	63	44	43	-8			48		
18	138		55		44	-8			44		
19	134		53		44	-7	-7	40	40		
20	131		47		44	-7			36		
21	128	127	42	44	44	-6			32		
22	123		32		44	-6	-6	20	24		
23	119	119	21	45	45	-5			16		
24	117		11		45	-5			8		
25	117		5		45	-5			4		
26	116	116	0	45	45	-5	-5	0	0		

CREST OFFSHORE, INC.

Sheet 2.07 of 4 FILE

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-1-76 Job No. 27-771-92 Calculation GRID WAVE DATA

PAGE 2

SEALOAD=2

LINE NO. 1 2 3 4 5 6 7 8
 1 2 3 4 5 6 7 8
 0 0 0 0 0 0 0 0

43 110
 44 105
 45 100
 46 90
 47 80
 48 70
 49 60
 50 40
 51 20
 52 0

53 SECT COL,LEG TUB 20 1

54 GRP 24C COL,LEG 20 1

55 MEMBER 1 2 24C 11 1

56 JOINT 1 0 0 0

57 JOINT 2 0 0 145

58 111111 FIXED

59 111 FREE END

60 24 1

61 END

62

CREST OFFSHORE, INC.

Sheet 2.08 of 4 PILE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4.1.70 Job No. 27-771-92 Calculation GRID WAVE DATA

*** COEFFICIENT TABLE REPORT ***

DIAMETER IN	NORMAL DRAG COEF	TANG DRAG COEF	MASS COEF
24,000	1,0000	0,0000	1,0000

CREST OFFSHORE, INC.

Sheet 2-09 of 4 PILE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-1-76 Job No. 27-771-92 Calculation GRID WAVE DATA

U.S. NAVY TEMPLET DESIGN = 88 FT MML JOB NO. 27-771-00

100 YEAR STORM WAVE = PRESSURE PROFILES(PSP) 1 PILE THETA = 0

PRELIMINARY DESIGN ATKINSON APRIL 1976

INPUT UNITS
...ENGLISH

OUTPUT UNITS
...ENGLISH

CREST OFFSHORE, INC.

Sheet 2-10 of 4 FILE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-1-76 Job No. 27-771-92 Calculation GRID WAVE DATA

*** WAVE POSITION SUMMARY REPORT ***

LOAD CONDITION 1 WAVE ANGLE θ 0.00

TRIAL NO.	DIST. TO CREST FT	PHASE ANGLE TO-STRUC. (DEG)	X	Y	KIPS	RSLNT	X	Y	FT-KIPS	MUDLINE MOMENT	RSLNT	VERTICAL FORCE KIPS
1	8.0	-4.36	88.9	0.0	0.0	88.9	0.0	0.0	828.0	828.0	828.0	0.0
2	14.0	-8.71	70.3	0.0	0.0	70.3	0.0	0.0	591.7	591.7	591.7	0.0

CREST OFFSHORE, INC.

Sheet 2-11 of 4 P. 1 E

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-1-70 Job No. 27-771-92 Calculation GRID WAVE DATA

*** LOAD-BURNARY-REPDRY ***

WAVE-NUMBER # 1 WAVE-DIRECTION-Y 0.000

X SHEAR FORCE # 88.8864 KIPS

Y SHEAR FORCE # 0.0000 KIPS

RESULTANT SHEAR FORCE # 88.8864 KIPS

X-MIDLINE-MOMENT # 0.0000 FT-KIPS

Y-MIDLINE-MOMENT # 8228.0501 FT-KIPS

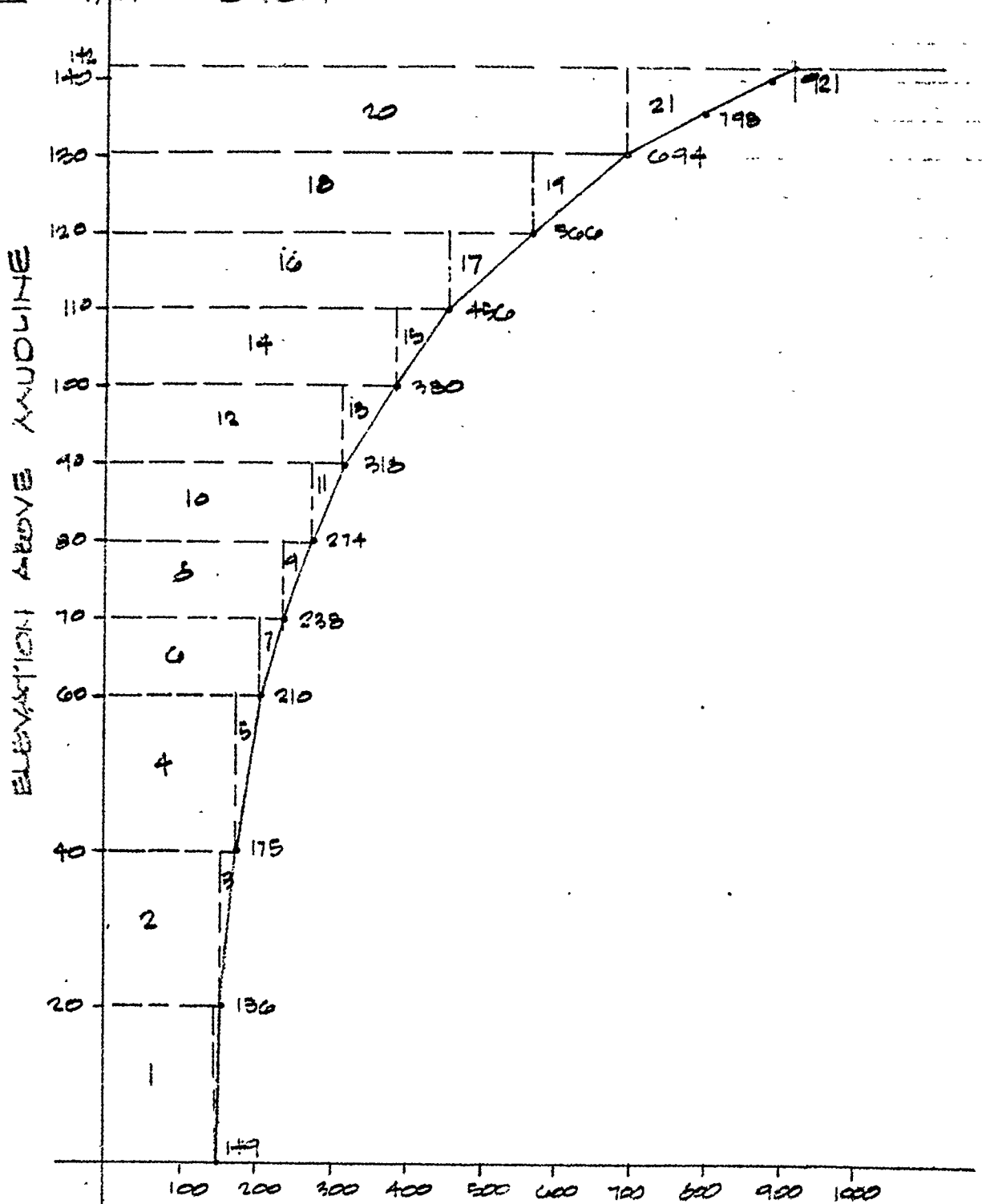
RESULTANT MIDLINE-MOMENT # 8228.0501 FT-KIPS

Z-VERTICAL-FORCE # 0.0000 KIPS

By WMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-2-76 Job No. 27-771-92 Calculation GRID WAVE DATA

APPROX. WAVE PROFILE AT A POINT 3 FT. BEHIND THE WAVE CREST FOR CHECKING WITH SEALOAD RESULTS.

2.4 WAVE CHECK



FOUNDS / SQUARE FOOT

FIGURE 4P2-1

WAVE PROFILE AT -3' FROM CREST OF WAVE

CREST OFFSHORE, INC.

Sheet 2-13 of 4 PLS

By WVA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.2.76 Job No. 31-711-92 Calculation GRID WAVE DATA

TABLE 4P2-5

AREA	FORCE (LBS/SF.) x S.F	V LBS	Mom Arm FT.	MOM FT. KIPS
1	149 x 20 =	2980	10 =	29.8 ^{1K}
2	186 x 20 =	3720	30 =	93.0
3	19 x 20 x .5 =	190	33.3 =	6.33
4	175 x 20 =	3500	60 =	210.0
5	35 x 20 x .5 =	350	53.3 =	18.7
6	210 x 10 =	2100	70 =	147.0
7	28 x 10 x .5 =	140	66.7 =	9.3
8	238 x 10 =	2380	75 =	178.5
9	36 x 10 x .5 =	180	76.7 =	13.8
10	274 x 10 =	2740	85 =	232.9
11	44 x 10 x .5 =	220	86.7 =	19.1
12	318 x 10 =	3180	95 =	302.1
13	62 x 10 x .5 =	310	96.7 =	30.0
14	380 x 10 =	3800	105 =	399
15	76 x 10 x .5 =	380	106.7 =	40.5
16	456 x 10 =	4560	115 =	524.4
17	110 x 10 x .5 =	550	116.7 =	64.2
18	566 x 10 =	5660	125 =	707.5
19	128 x 10 x .5 =	640	126.7 =	81.1
20	699 x 12 =	6740	136 =	913.3
21	227 x 12 x .5 =	1362	138 =	188.0
		45232		
		<u>x 2</u>		4239.0
		90,564	30000	<u>x 2</u>
				8179.2 ^{1K}

CHECK SHEAR

90,564 # vs 88,880 # FROM SEALOAD : O.K.

CHECK MOMENT

8,179.2 FT.K vs 8,218 FT.K FROM SEALOAD : O.K.

ASSUME WAVE GRID IS CODED PROPERLY

SECTION 3

BASE SPACING vs. PILING CAPACITY

3.1 INTRODUCTION

This section uses the forces from the skirt-pile structure to derive two formulas for the length 'L' of the base of the four-pile jacket as a function of the axial load on the piling, either tension or compression. This information is compiled into a graph plotting the base spacing to the required pile compression and tension capacity. The graph is used in Section 4 to determine the weight of the total structure as a function of base spacing. The forces from the skirt-pile structure are:

Shear $V = 1,254.6$ kips

Base Moment $M = 107,302$ ft kips

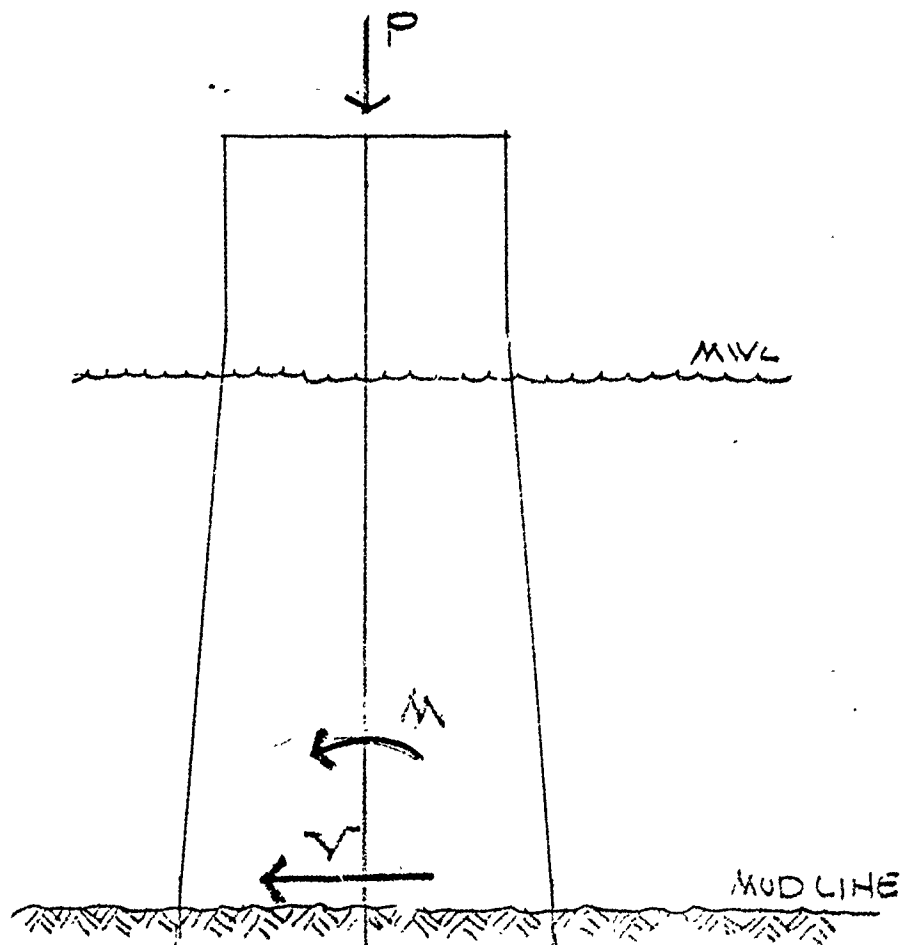
Total Gravity Load = 450 kips

By JWA Client U.S. Navy Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-2-76 Job No. 27-171-92 Calculation BASE SPACING VS. RULING CAPACITY

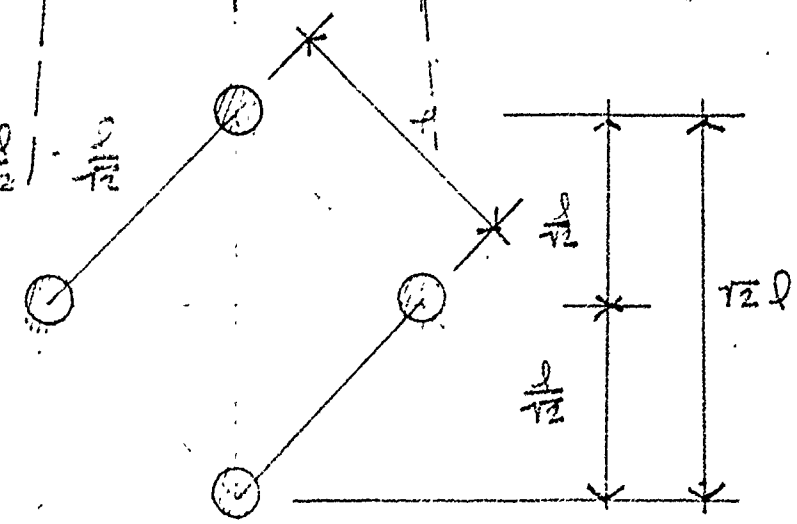
3.2 - LOADING ON STRUCTURE
 TOTAL GRAVITY LOAD $P = 450$ KIPS

BASE SHEAR $V = 1,254.6$ KIPS

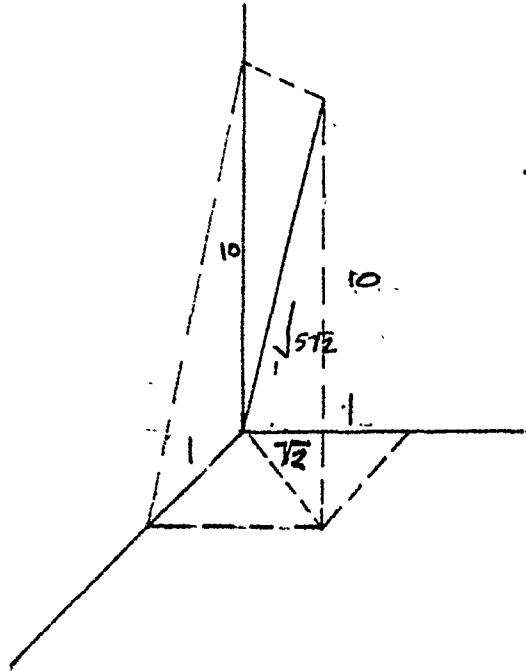
MOMENT $M = 107,302$ FT-KIPS



$$\frac{\sqrt{2} \cdot P}{2} = \frac{\sqrt{2} \cdot \sqrt{2} \cdot l}{\sqrt{2} \cdot 2} = \frac{2 \cdot l}{\sqrt{2} \cdot 2} = \frac{2}{\sqrt{2}} \cdot \frac{l}{2}$$

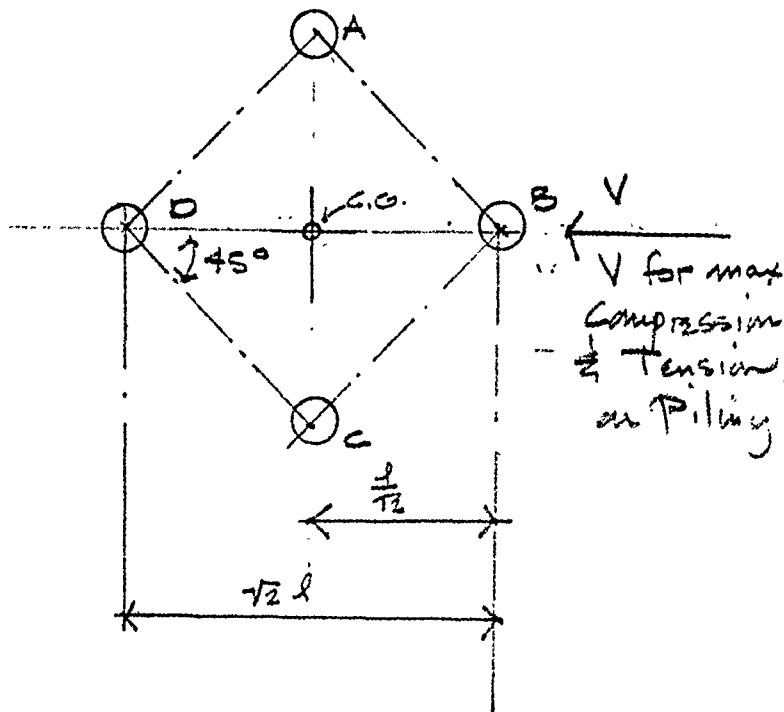


By KAS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.2.76 Job No. 27-711-92 Calculation BASE BRACING VS. PILING CAPACITY



PROJECTED BATTER: 1:10

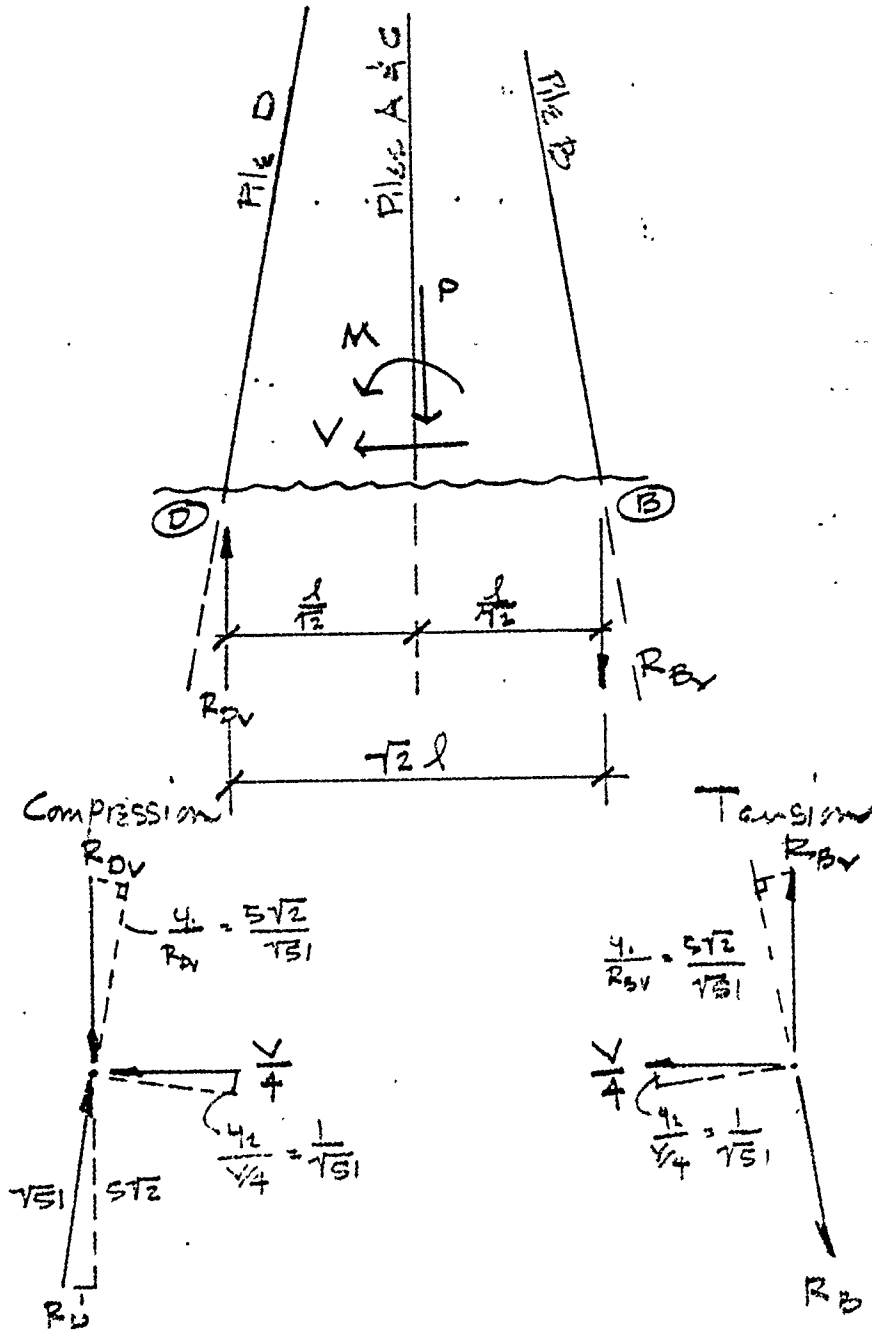
TRUE BATTER
 $\sqrt{2} : 10$
 $1 : \frac{10}{\sqrt{2}} = 1 : 7.071$
 $= \frac{10\sqrt{2}}{2} = 5\sqrt{2}$



V
 V for max
 Compression
 & Tension
 on Piling

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 7.2.74 Job No. 27-171-92 Calculation BASE SPACING VS. PILING CAPACITY

Maximum Compressive & Tensile Forces in Piles



$$R_D = \frac{5\sqrt{2}}{\sqrt{51}} R_{DV} + \frac{1}{\sqrt{51}} \frac{V}{4} \quad (1a) \quad R_B = \frac{5\sqrt{2}}{\sqrt{51}} R_{BV} + \frac{1}{\sqrt{51}} \frac{V}{4} \quad (1b)$$

$$R_{DV} = \frac{\sqrt{51}}{5\sqrt{2}} R_D - \frac{V}{20\sqrt{2}} \quad (2a) \quad R_{BV} = \frac{\sqrt{51}}{5\sqrt{2}} R_B - \frac{V}{20\sqrt{2}} \quad (2b)$$

By JWD Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 7.20.70 Job No. 27-111-92 Calculation BASE SPACING VS. PILING CAPACITY

Summation of moments
 about B axis
 for Compression

about D axis
 for Tension

$$R_{DV} \sqrt{2} l_c = M + \frac{P l_c}{\sqrt{2}} - \frac{P l_c}{2 \sqrt{2}} \quad (3a)$$

$$= M + \frac{P l_c}{2 \sqrt{2}}$$

$$R_{DV} \sqrt{2} l = M - \frac{P l}{\sqrt{2}} + \frac{P l}{2 \sqrt{2}} \quad (3b)$$

$$= M - \frac{P l}{2 \sqrt{2}}$$

$$M = \left(R_{DV} \sqrt{2} - \frac{P}{2 \sqrt{2}} \right) l_c$$

$$\left(R_{DV} \sqrt{2} + \frac{P}{2 \sqrt{2}} \right) l = M$$

$$l_c = \frac{M}{R_{DV} \sqrt{2} - \frac{P}{2 \sqrt{2}}} \quad (4a)$$

$$l_T = \frac{M}{R_{DV} \sqrt{2} + \frac{P}{2 \sqrt{2}}} \quad (4b)$$

Substituting Eq. (2a) into Eq. (4a)

Substituting Eq. (2b) into (4b)

$$l_c = \frac{M}{\frac{\sqrt{51} R_D}{5} - \frac{V}{20} - \frac{P}{2 \sqrt{2}}} \quad (5a)$$

$$l_T = \frac{M}{\frac{\sqrt{51} R_B}{5} - \frac{V}{20} + \frac{P}{2 \sqrt{2}}} \quad (5b)$$

$M = 107,302$ FT. KIPS
 $P = 450$ KIPS
 $V = 12,540$ KIPS

$\frac{\sqrt{51}}{5} = 1.428$
 $\frac{V}{20} = -62.73 \quad -62.73$
 $\frac{P}{2 \sqrt{2}} = -159.10 \quad +159.10$
 $\quad \quad \quad -221.83 \quad +96.37$

$$l_c = \frac{107,302}{1.428 R_D - 221.83} \quad (6a)$$

$$l_T = \frac{107,302}{1.428 R_B + 96.37} \quad (6b)$$

when $l_c = \infty$ $R_D = \text{min}$

when $R_B = 0$ $l_T = \text{max}$

$$R_D = \left[\frac{107,302 + 221.83}{l_c} \right] \frac{1}{1.428}$$

$$= \frac{221.83}{1.428} = 155.3 \text{ K}$$

$$l_T = \frac{107,302}{96.37} = 1113.4'$$

CREST OFFSHORE, INC.

Sheet 3.00 of 4 PILE

By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-26-70 Job No. 27-71-92 Calculation BASE SPACING VS. PILING CAPACITY

TABLE 4P3-1.

PILE DEPTH	COMPRESSION		TENSION	
	R_D (KIPS)	$\lambda = \frac{107,302}{1.428 R_D - 221.33}$ (FT)	R_B (KIPS)	$\lambda = \frac{107,302}{1.428 R_B + 96.37}$ (FT)
1	155.3	∞	0	1113.4
2	250 405.8	793.8 300.0	183 250	300.0 236.7
3	500	218.0	500	132.4
4	750	126.4	750	91.92
5	1000	88.96	1000	70.39
6	1250	68.6	1250	57.03
7	1500	55.9	1500	47.94
8	1750	47.12	1750	41.34
9	2000	40.73	2000	36.34
10	2250	35.87	2250	32.42
11	2500	32.05	2500	29.27
12	2750	28.96	2750	26.67
13	3000	26.41	3000	24.50
	∞	0	∞	0

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4.5.70 Job No. 27-771-92 Calculation BASE SPACING VS. PILING CAPACITY

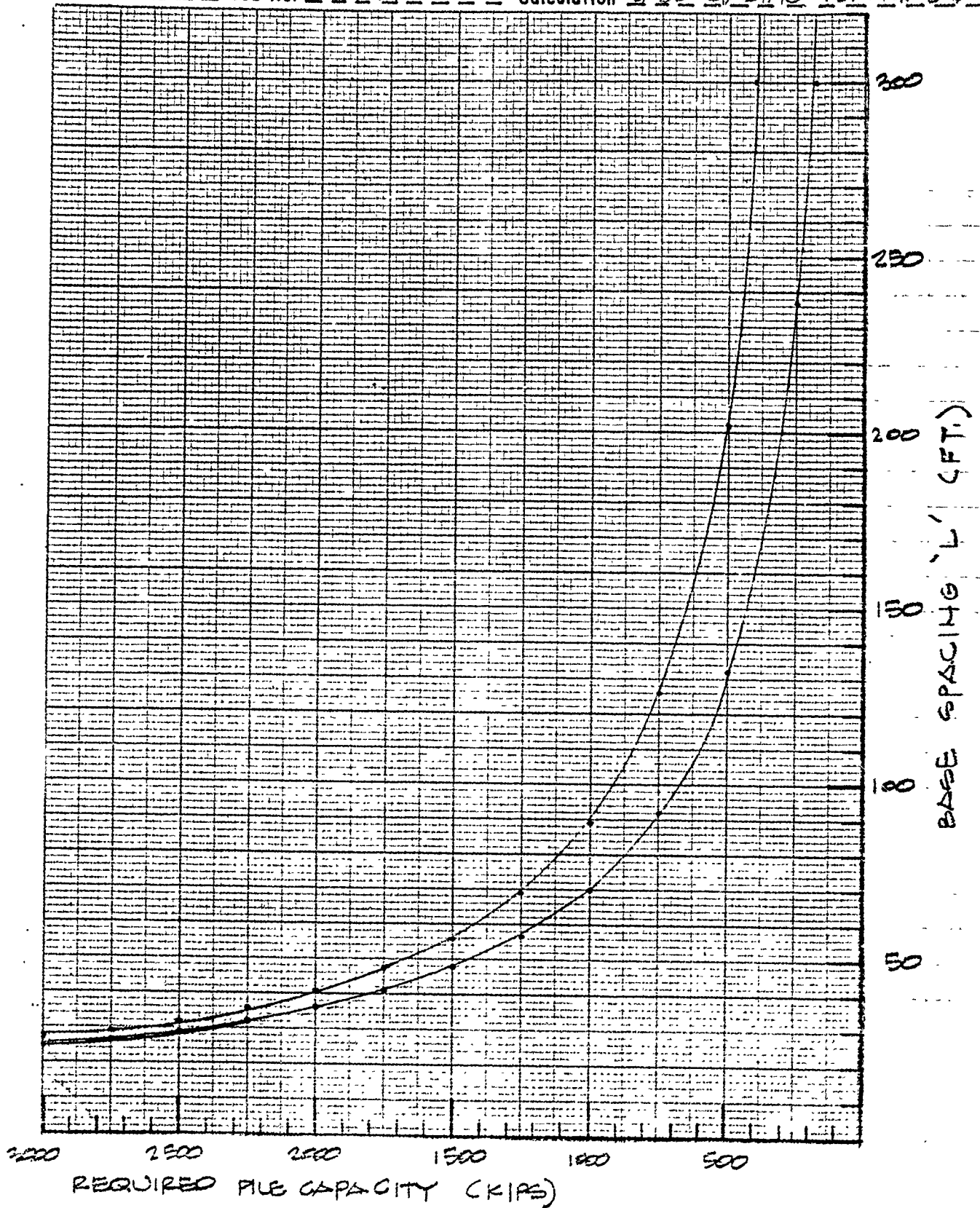


FIGURE 4P3-1

3.3

SECTION 4

STRUCTURAL STEEL WEIGHT VARIATION

4.1 INTRODUCTION

This section derives formulas and then develops tables to determine the least total weight of a four-pile structure for different jacket base widths at mudline from the given soil conditions and the environmental forces.

The template type structure has to resist the overturning moment due to the wind and wave loadings on the structure. The farther apart the jacket legs are spread the lesser the axial load on the piling will be. However the horizontal and diagonal bracing lengths will be increased thus requiring longer and possibly larger members as the slenderness ratio is increased. As the jacket legs get closer together the axial loads on the pilings increase thus requiring longer piles.

Section 4.2 determines the total length of horizontal and diagonal members as a function of the jacket leg spacing 'L' at the mudline. These are shown in Table 4P4-1. These lengths are multiplied by an assumed unit brace member weight to provide Table 4P4-2 which gives total weight of bracing for the different leg spacings chosen.

Section 4.3 computes the total weight of the four jacket legs. This is assumed to be constant for all leg spacings and is shown in Table 4P4-3.

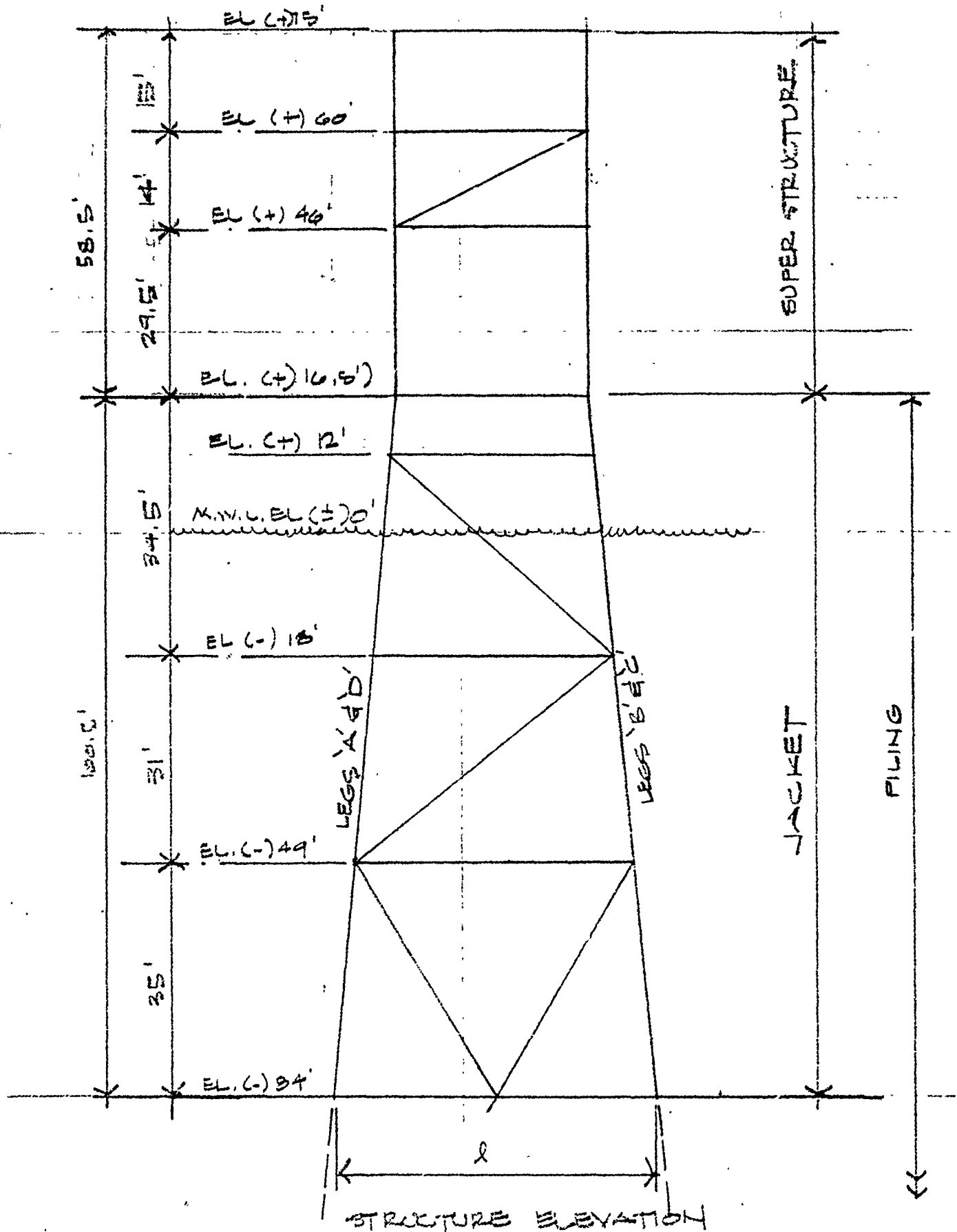
From inspection it was determined the pilings will have to be driven a minimum of 150 ft below mudline. Section 4.4 Table 4P4-4 shows assumed piling sizes from the top of the jacket to 150 ft below mudline. The

additional pile length beyond 150 ft is a function of the required pile axial load capacity. For a given jacket leg spacing 'L' the required pile axial load capacity is found from Figure 4P3-1, Section 3, Page 3.07 and placed in Table 4P4-5. The required pile penetration for the corresponding pile capacity is found from the graph, Figure 4P4-1 (See the report on Three-Pile Structure, Section 2, Page 2.39 for the development of this graph). After determining the required pile length below 150' penetration (H) the total weight of the piles as a function of 'L' is computed in Table 4P4-5.

Table 4P4-6 of Section 4.5 adds the weights of the jacket and piling to get the total weight for each 'L' chosen. The sum of the weights of the superstructure, boat landings, stairways, etc., was assumed to remain constant. From this table a jacket leg base spacing of 50 ft is chosen for the actual structural configuration analysis.

By NWS Client DEA HAY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.5.76 Job No. 27-77-92 Calculation STRUCTURAL STRUCT. VARIATION

4.2 BASE SPACING VS. JACKET MEMBER LENGTHS

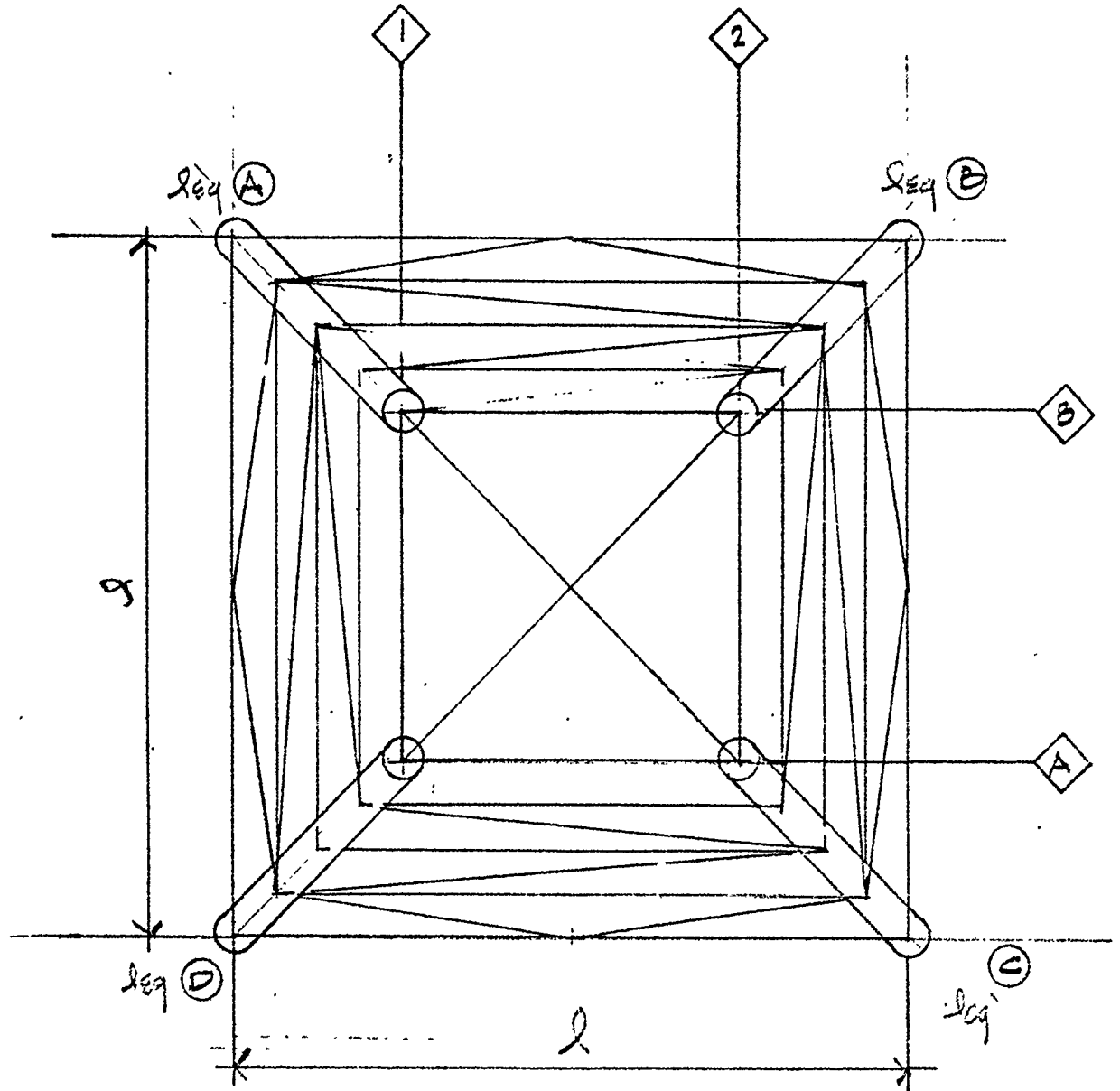


By VVA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.5.76 Job No. 27-771-92 Calculation STRUCTURAL ST. WT. VARIATION

TOP VIEW ON JACKET

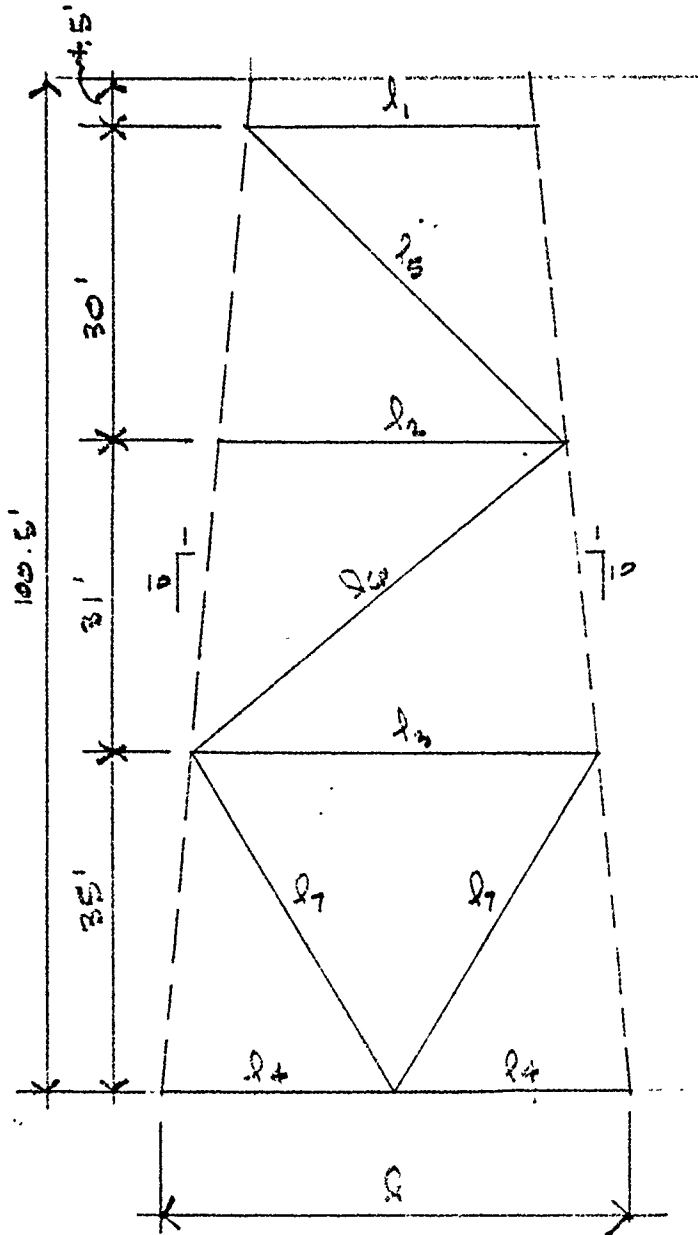
Member Sizes:

- (i) Bracing: See Pg.
- (ii) Leg: $(D+4)" \phi \times 0.50" \text{ wt.}$
- (iii) Leg Joint Cap: $(D+5)" \phi \times 1.0 \text{ wt.}$
 *D: O.D. of Pipe Piles



By W.A. Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 27-171-92 Calculation STRUCTURAL ST. WT. VARIATION

BRACING (4-REQ'D)



$$l_1 = L - 9.6 \times 2 = L - 19.2 \text{ (ft.)}$$

$$l_2 = L - 6.6 \times 2 = L - 13.2 \text{ (ft.)}$$

$$l_3 = L - 3.5 \times 2 = L - 7 \text{ (ft.)}$$

$$l_4 = \frac{L}{2} = 0.5L \text{ (ft.)}$$

$$l_5 = \sqrt{(30)^2 + [L_1 + 3]^2}$$

$$= \sqrt{(30)^2 + [L - 19.2 + 3]^2}$$

$$= \sqrt{(30)^2 + (L - 16.2)^2}$$

$$l_6 = \sqrt{(31)^2 + (L_2 + 3.1)^2}$$

$$= \sqrt{(31)^2 + (L - 13.2 + 3.1)^2}$$

$$= \sqrt{(31)^2 + (L - 10.1)^2}$$

$$l_7 = \sqrt{(35)^2 + (L_4 - 3.5)^2}$$

$$= \sqrt{(35)^2 + (0.5L - 3.5)^2}$$

TOTAL LENGTH $L = l_1 + l_2 + l_3 + 2l_4 + l_5 + l_6 + 2l_7$

CREST OFFSHORE, INC.

Sheet 4.00 of 4 PLE

By WVA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 21-771-92 Calculation STRUCTURAL ST'L WT. VARIATION

TABLE 4P4-1

l	l_1	l_2	l_3	l_4	l_5	l_6	l_7	L (ft.)
	$l - 19.2$	$l - 13.2$	$l - 7$	$0.5l$	$\sqrt{(3.0)^2 + (l - 16.2)^2}$	$\sqrt{(3.1)^2 + (l - 10.1)^2}$	$\sqrt{(3.5)^2 + (5.5 \cdot l - 3.5)^2}$	$l_1 + l_2 + l_3 + 2l_4 + l_5 + l_6 + 2l_7$
30	15.8	21.8	28	17.5	33.4	39.8	37.7	251.2
40	20.8	26.8	33	20	38.3	43.1	38.7	279.4
45	25.8	31.8	38	22.5	41.6	46.7	39.8	308.5
50	30.8	36.8	43	25	45.2	50.5	41.1	338.5
55	35.8	41.8	48	27.5	49.0	54.0	42.4	368.4
60	40.8	46.8	53	30	53.1	58.7	43.9	400.2

ALL UNITS IN FT.
 FOR 1 SIDE

CREST OFFSHORE, INC.

Sheet 4.07 of 4 PLE

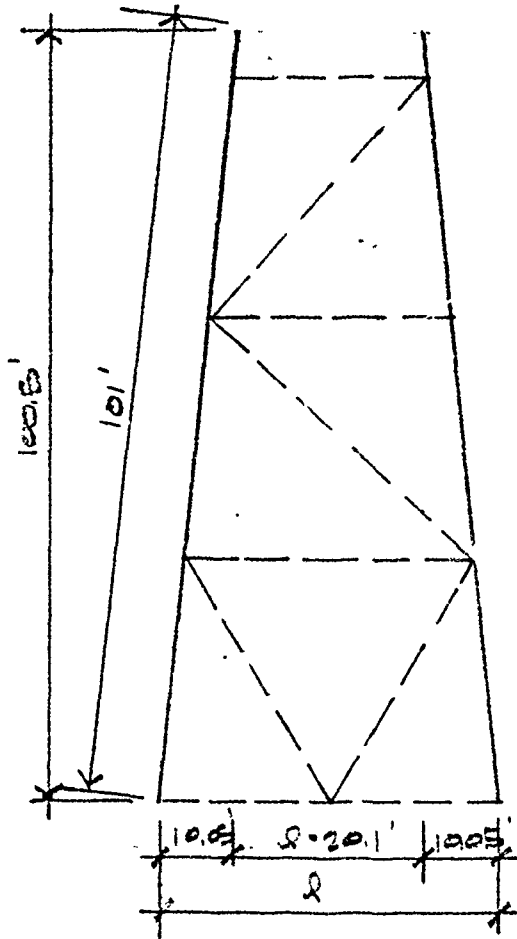
By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.0.76 Job No. 27-771-92 Calculation STRUCTURAL SELF WT VARIATION

TABLE 4P4-2 JACKET BRACING WT./SIDE

BASE SPACING (FT.)	BRACE MEMBER SIZE	TOTAL BRACE MEMBER LENGTH (FT.) (SEE TABLE 4P3-1)	UNIT WT. (LBS/FT.)	TOTAL WT. (LBS.)
35	18" ϕ x 0.50	251.2	93.45	23,475
40	" "	279.4	"	26,110
45	" "	303.5	"	28,329
50	" "	338.5	"	31,033
55	" "	368.4	"	34,427
60	" "	400.2	"	37,399

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 27-111-92 Calculation STRUCTURAL STK WTL VARIATION

4.3 JACKET LEG (4. REQ'D) WEIGHT



BIT ST: 8.0" L₂₀
 1.0" TK.
 4 BIT'S

TOTAL LENGTH = 4 x 8 = 32'

JACKET LEG: 0.50" TK.

TOTAL LENGTH = 101.0' - 32.0' = 69.0'

TABLE 4P-3

FILE Ø. D. (IN.)	SIZE X LENGTH	UNIT WT. (LBS. / FT.)	TOTAL WT. (LBS.)
JACKET LEG			
30	40" Ø x .5 x 69'	210.93	14,954
JACKET CAN			
30	41" Ø x 1.0 x 32'	427.21	13,671
TOTAL WT.			28,625

By VWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 27-171-92 Calculation STRUCTURAL STK WT VARIATION

4.4 PILING (4-REQ'D)

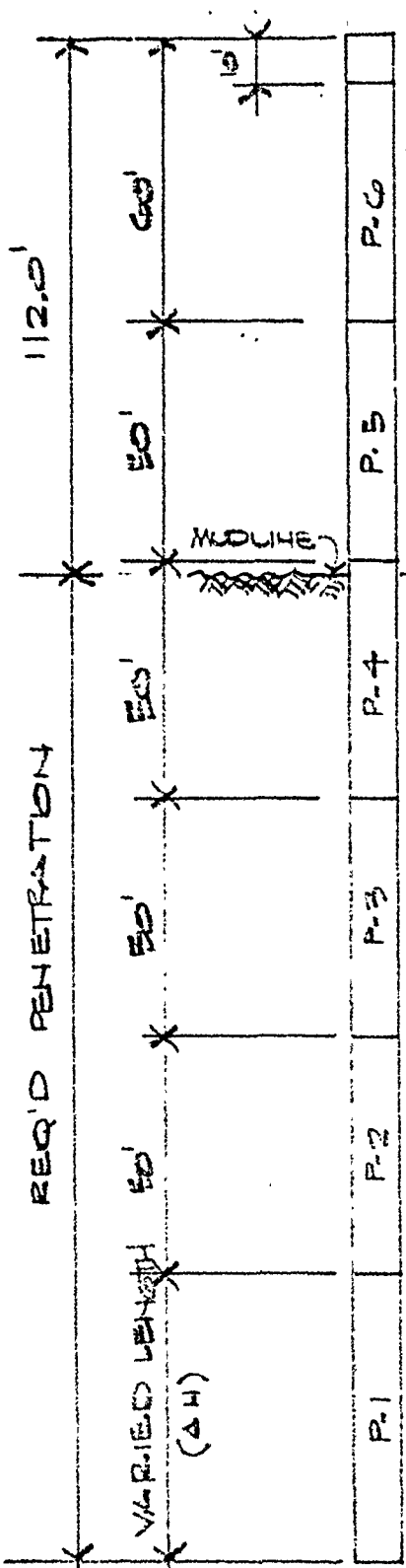


TABLE 4P4-4

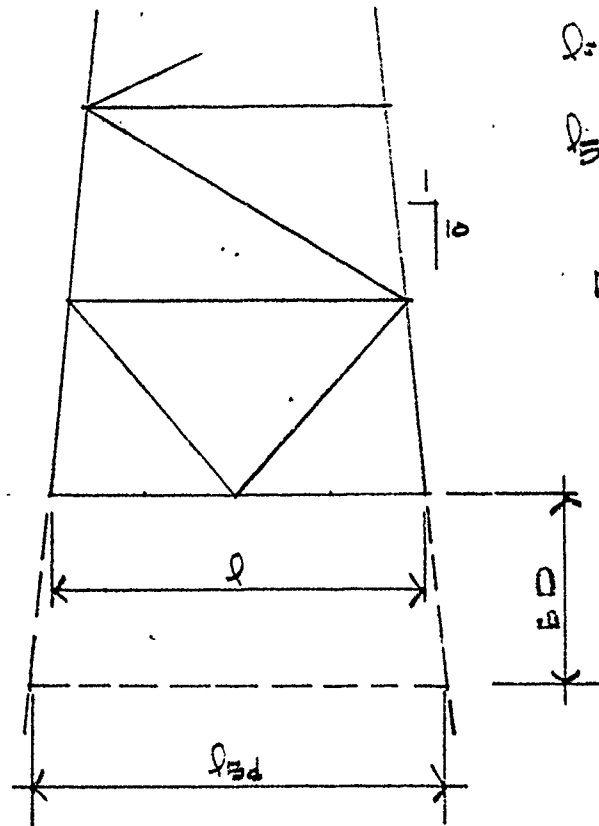
PIPE PILE	WT. (LBS/FT)	LENGTH (FT)	TOTAL WT. (LBS)
P-6 36" φ x 1.25"	463.92	60	27,835
P-5 36" φ x 1.25	463.92	50	23,196
P-4 36" φ x 1.50	552.7	50	27,635
P-3 36" φ x 1.50	552.7	50	27,635
P-2 36" φ x 1.25	463.92	50	23,196
P-1 36" φ x 0.75	282.36	ΔH	282.36 ΔH
TOTAL WT = 129,497#			+ 282.36 ΔH

$$\Delta H = [Req'd Penetration + 112] \cdot 260$$

$$- Req'd Penetration \cdot 140$$

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 21-711-92 Calculation STRUCTURAL S/L OUT. VARIATION

RELATIONSHIP BETWEEN l AND l_{ED}



l = base spacing @ mudline

l_{ED} = base spacing @ 5D below mudline

D = pile outside diameter = 36"

$\therefore 5D = 5 \times 36 = 180$

$$l_{ED} = l + \frac{180}{10} \times 2$$

$$= l + 36$$

By C. Chern Client NAVY
Date 4-1-76 Job No. 27-721-92

Subject STRUCTURAL CONCEPT ANALYSIS
Calculation STRUCTURAL S/L OF VARIATION

Pile Capacity (kips)

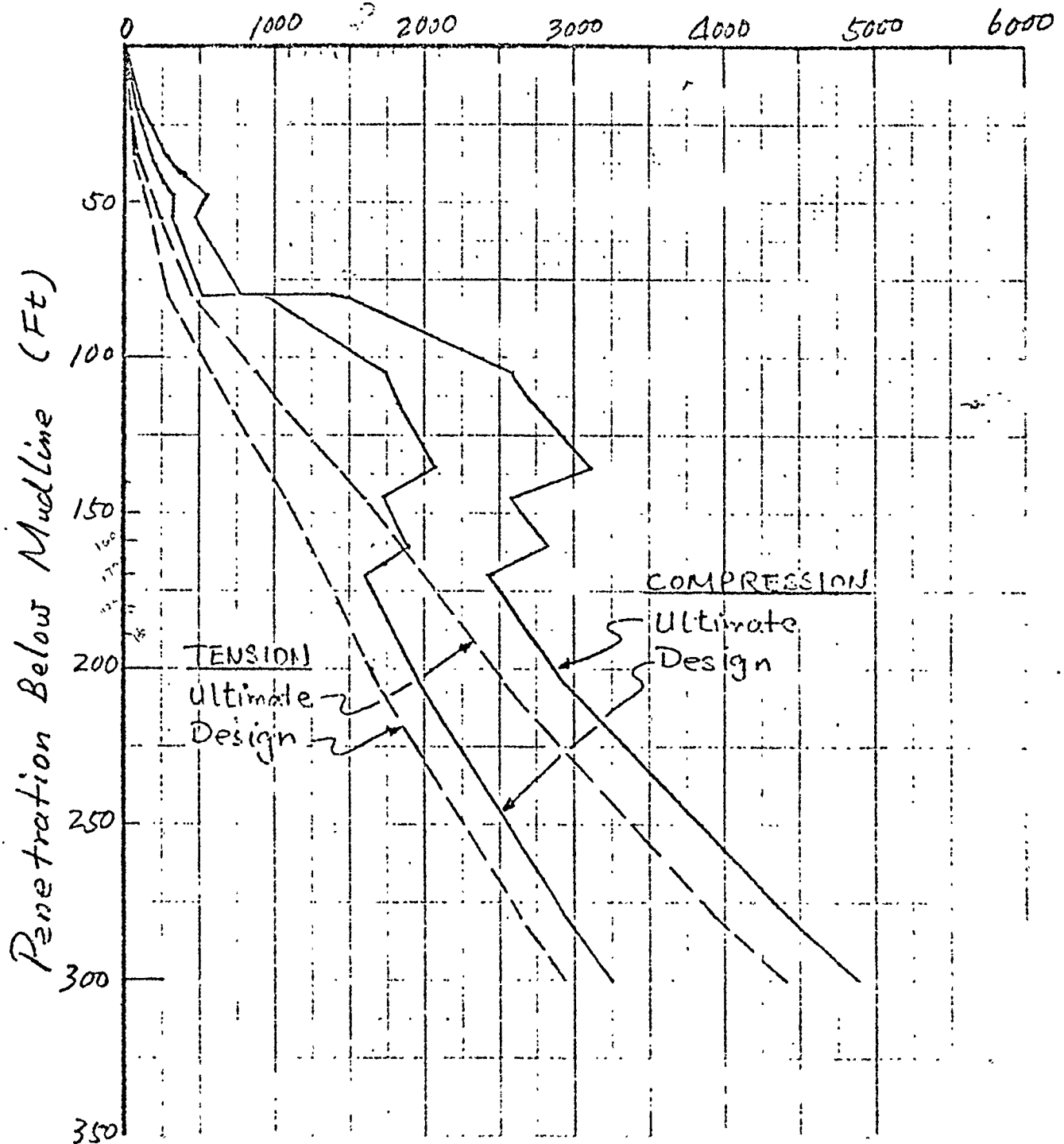


FIGURE 4P4-1

36-in. Diameter Pipe Piles

CREST OFFSHORE, INC.

Sheet 4.12 of 4.15

By VWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 21-171-92 Calculation STRUCTURAL FILE WT VARIATION

TABLE 4P4-5 300-114, DIAMETER PIPE FILES - WT / LEG									
FILES SPACING	REAR FILE CAPACITY		REQ'D FILE PENETRATION	FILE SEGMENT #1		TOTAL WT = 129,497 # + 282,360 ΔH (10 FILES)			
	FT.	FT.		Δ H	WT.				
FT.	FT.	KIPS	FT.	FT.	WT.	WT.			
35	35	2125	1875	200*	77	21,742	151,239		
40	40	1875	1650	200*	52	14,683	144,180		
45	45	1720	1500	185*	37	10,447	139,944		
50	50	1550	1350	172*	24	6,777	136,274		
55	55	1450	1225	160*	12	3,385	132,885		
60	60	1350	1125	150*	2	565	139,062		

NOTES: Δ = base spacing C mud line
 Req'd = base spacing @ 50 below mud line
 = Δ + 25
 Δ H = File segmented length = Penetration Req'd - 148
 * denotes transition intervals.

By KWS Client U.S. Navy Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5.6.70 Job No. 31-771-00 Calculation STRUCTURAL STILT VARIATION

4.5 TOTAL WEIGHT OF STRUCTURE

$$W_T = W_P + W_J + W_S + W_M \quad (1)$$

where $W_T =$ TOTAL WEIGHT of STRUCTURE

$W_P =$ WEIGHT of PILING

$W_J =$ WEIGHT of JACKET

$W_S =$ WEIGHT of SUPER STRUCTURE

$W_M =$ WEIGHT of MISCELLANEOUS ITEMS
 such as boat landing, walkway, etc...

Assume: superstructure & miscellaneous remain constant
 $W_S + W_M = \text{CONSTANT} = K$

therefore

$$W = W_P + W_J + K \quad (2)$$

neglecting K

$$W = W_P + W_J \quad (3)$$

$W = 4 W_P + 4 (\text{wt. jacket legs} + \text{wt. jacket bracing})$

$$W_T = 4 (W_P + W_J) \quad (4)$$

CREST OFFSHORE, INC.

Sheet 4-14 of 4 FILE

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 27-71-92 Calculation STRUCTURAL WT'S AT VARIATION

TABLE 4P4-CO TOTAL WT'S FOR 4 FILE w/ 36" φ PILES					
BASE SPACING AT MUDLINE	WEIGHT				
	JACKET		PIPING TABLE 4P3-5	TOTAL for 1-SIDE	TOTAL for STRUCTURE T x 4 / 2000
	LEG TABLE 4P3-3	BRACES TABLE 4P3-2			
FT.	LBS	LBS	LBS	LBS	TONS
35	28,225	23,475	151,239	202,939	405.9 ^T
40	"	26,110	144,130	198,515	397.0 ^T
45	"	28,329	139,444	196,998	394.0 ^T
50	"	31,635	136,274	196,132	392.3 ^T
55	"	34,427	132,335	195,537	391.1 ^T
60	"	37,399	130,062	195,686	391.4 ^T

SECTION 5
STRUCTURAL CONFIGURATION

5.1 INTRODUCTION

With a jacket leg base spacing of 50' chosen from Section 4, the structure is drawn showing dimensions and assumed member sizes.

Section 5.2 shows the four (4) faces of the jacket and the jacket bracing plan at various elevations. The lengths of members are computed and their KL/r ratios established in Table 4P5-1.

Section 5.3 shows a typical elevation of the superstructure.

Section 5.4 computes the preliminary deck beam sizes for both upper and lower decks.

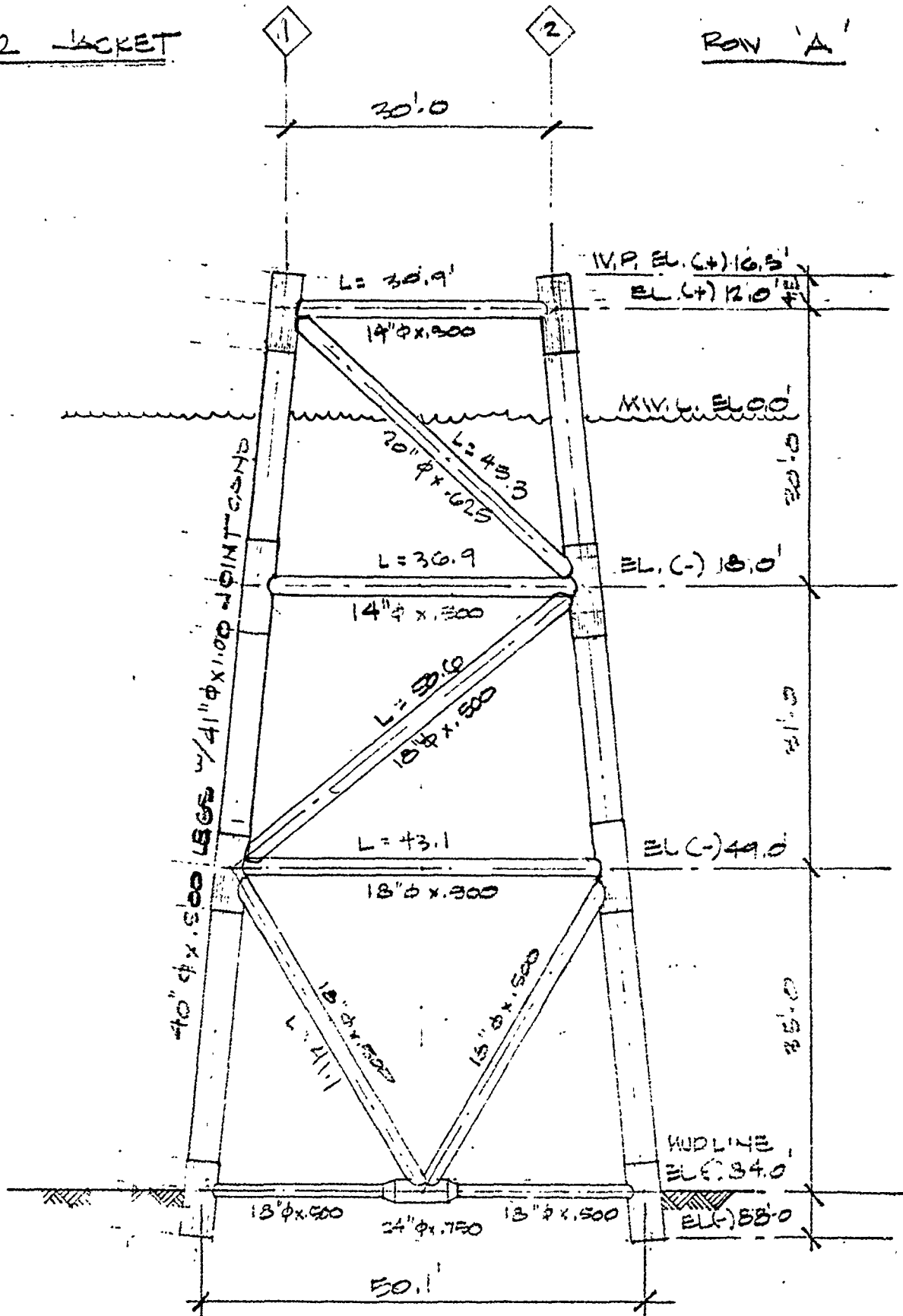
CREST OFFSHORE, INC.

Sheet 5.02 of 4 PILE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.10.70 Job No 21-77-72 Calculation STRUCTURAL CONFIGURATION

5.2 JACKET

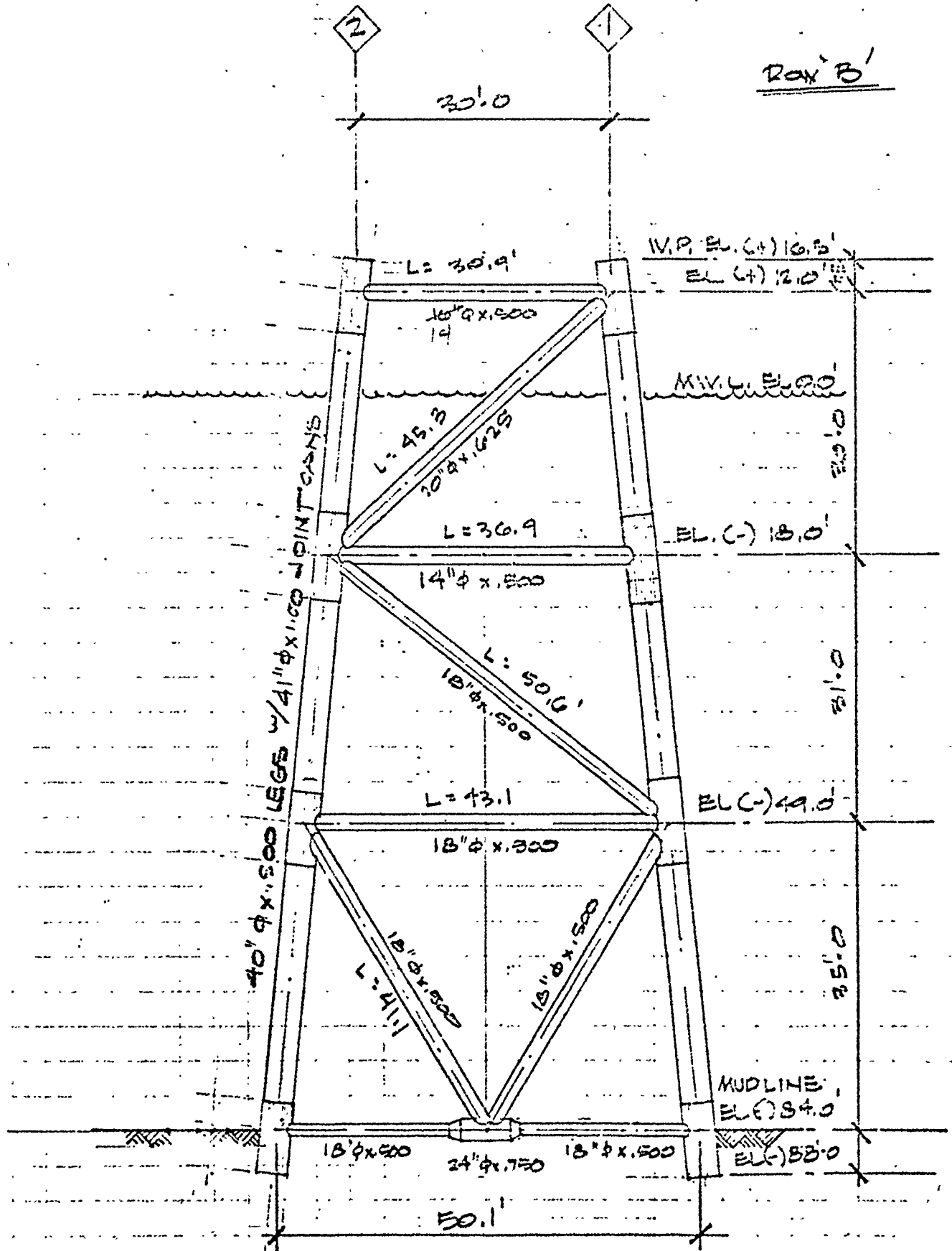
ROW 'A'



CREST OFFSHORE, INC.

Sheet 5.03 of 4 PILE

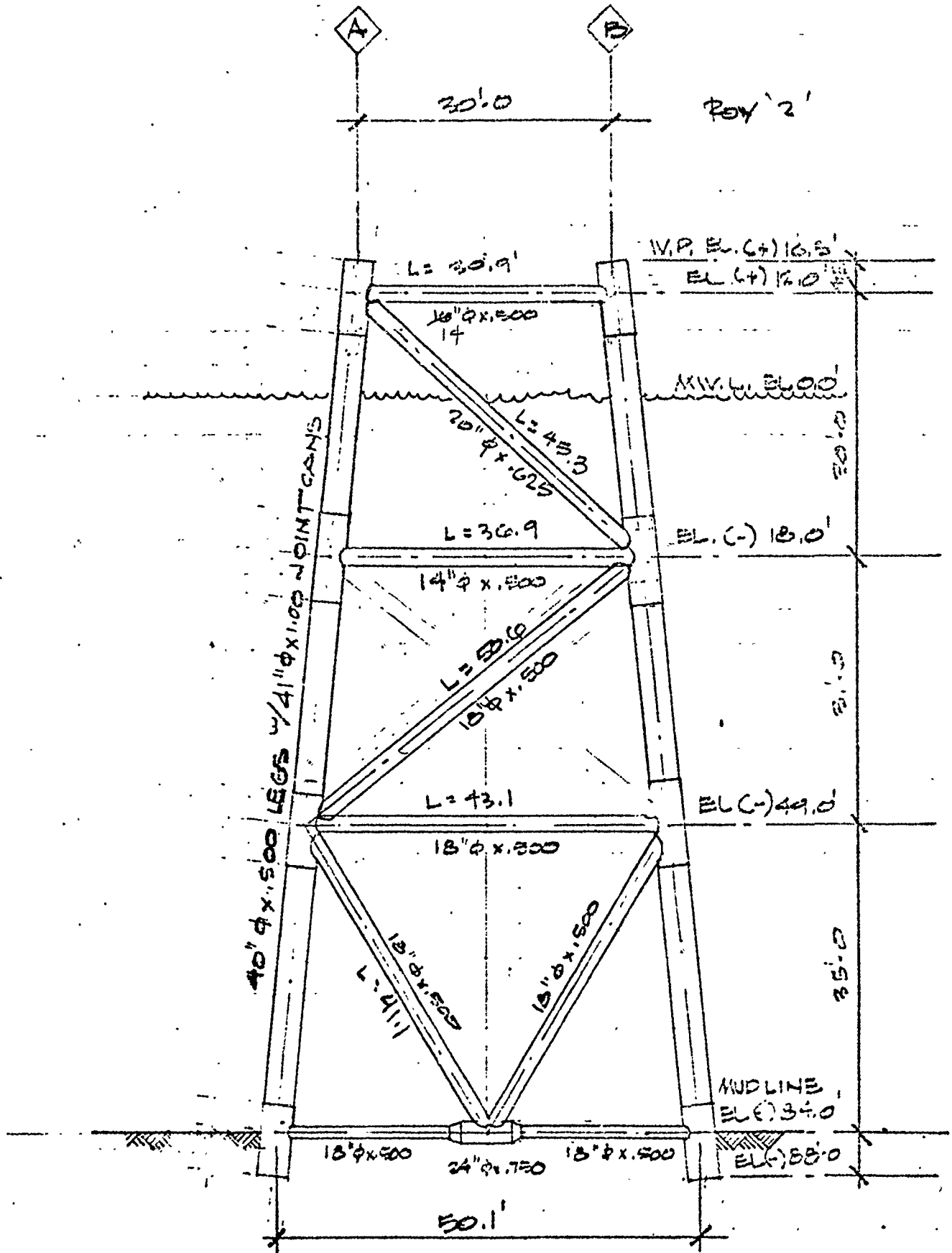
By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 10.7.70 Job No. 37-771-92 Calculation STRUCTURAL CONCEPTION



CREST OFFSHORE, INC.

Sheet E.04 of 4 E.02

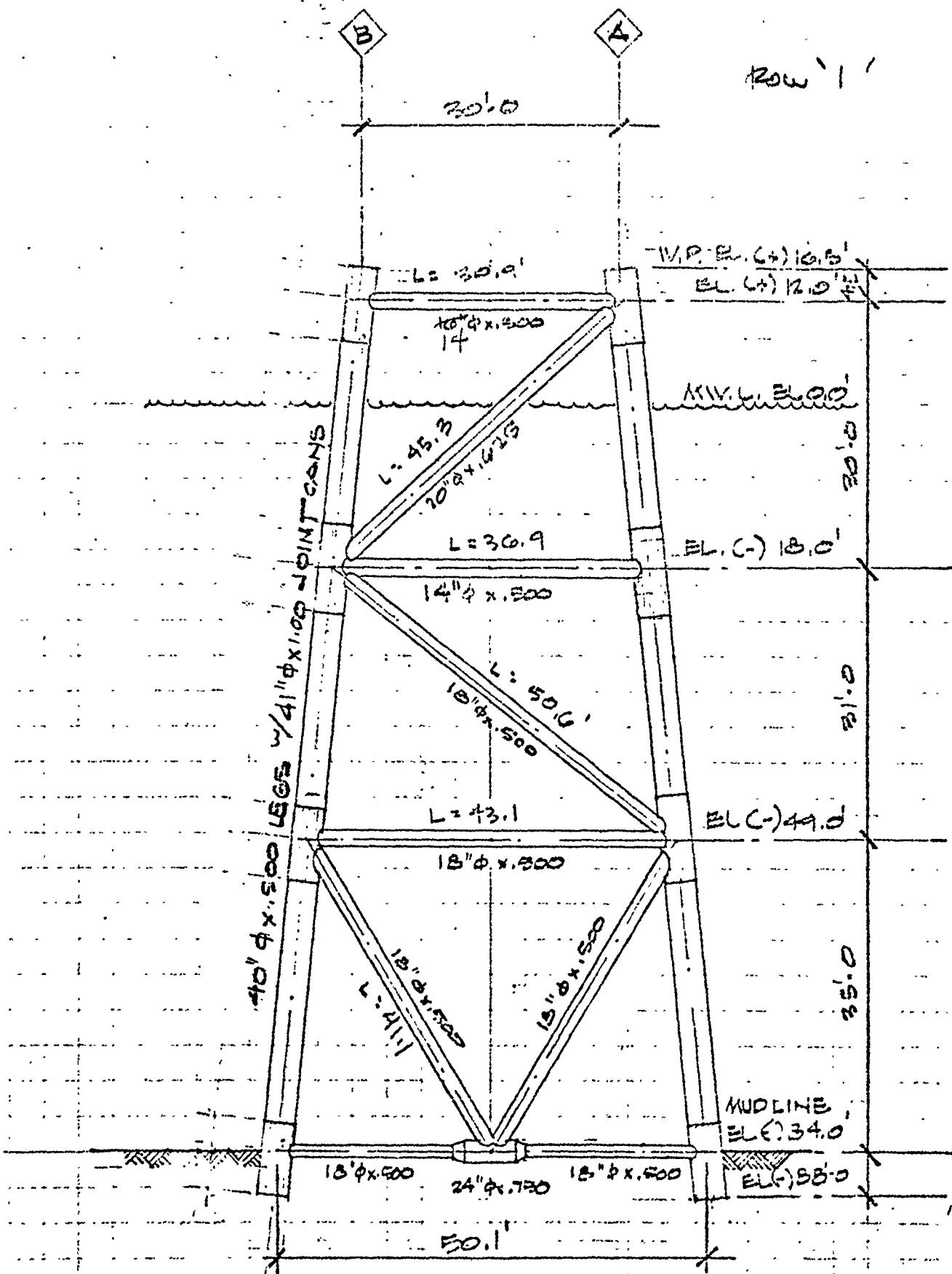
By JWS Client U.S. Navy Subject STRUCTURAL CONCEPT ANALYSIS
 Date APR 20 1970 Job No. 201-771-92 Calculation STRUCTURAL CONFIGURATION



CREST OFFSHORE, INC.

Sheet EL-05 of 4 PAGES

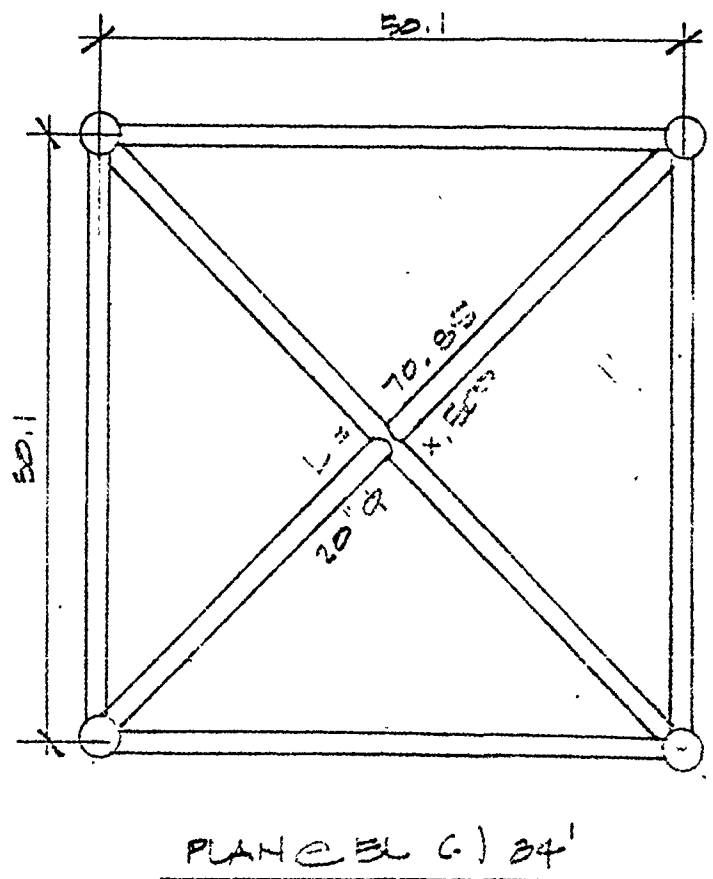
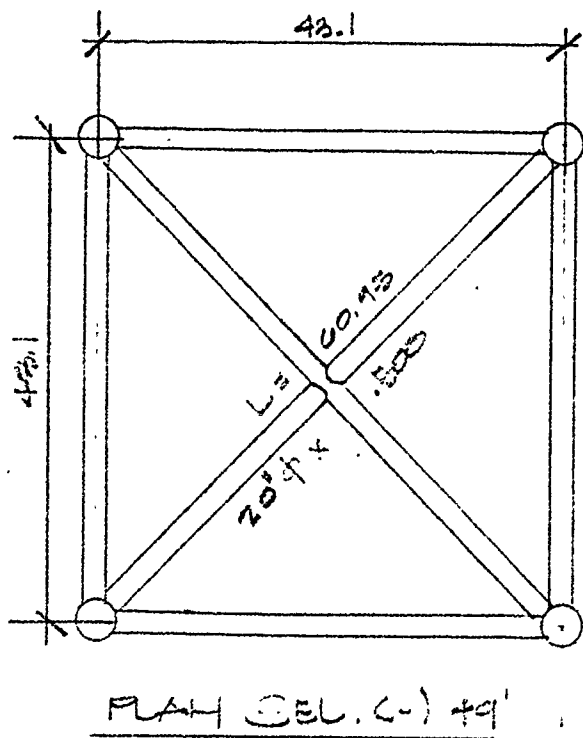
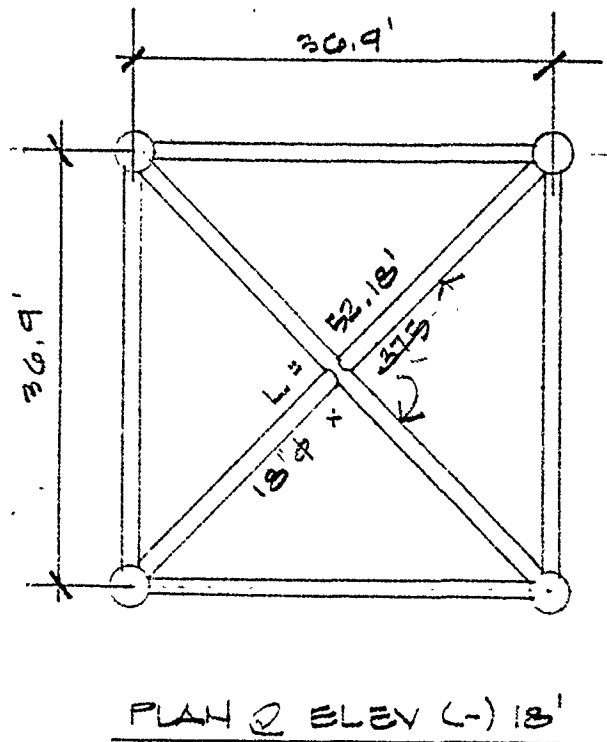
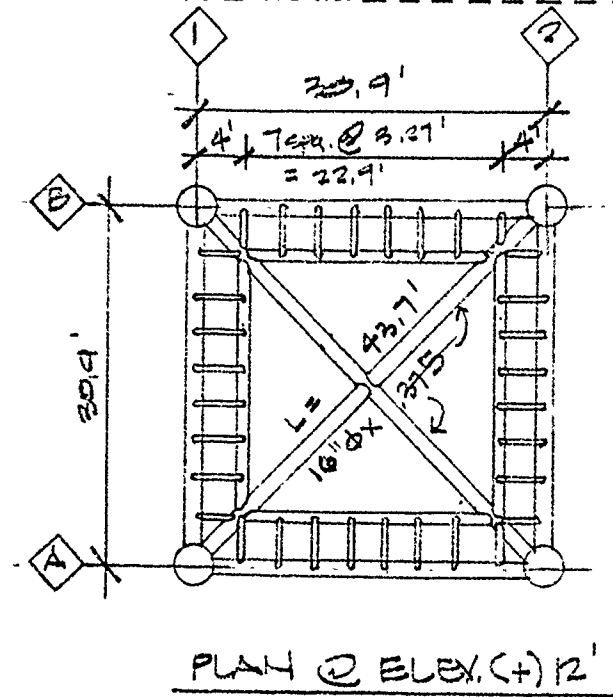
By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.70 Job No. 47-771-121 Calculation STRUCTURAL CONFIGURATION



CREST OFFSHORE, INC.

Sheet 5.00 of 4 PILE

By WAS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-7-70 Job No. 21-771-92 Calculation STRUCTURAL CONFIGURATION



CREST OFFSHORE, INC.

Sheet 5.07 of 4 PAGES

By MA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-7-70 Job No. 27-771-92 Calculation STRUCTURAL CONFIGURATION

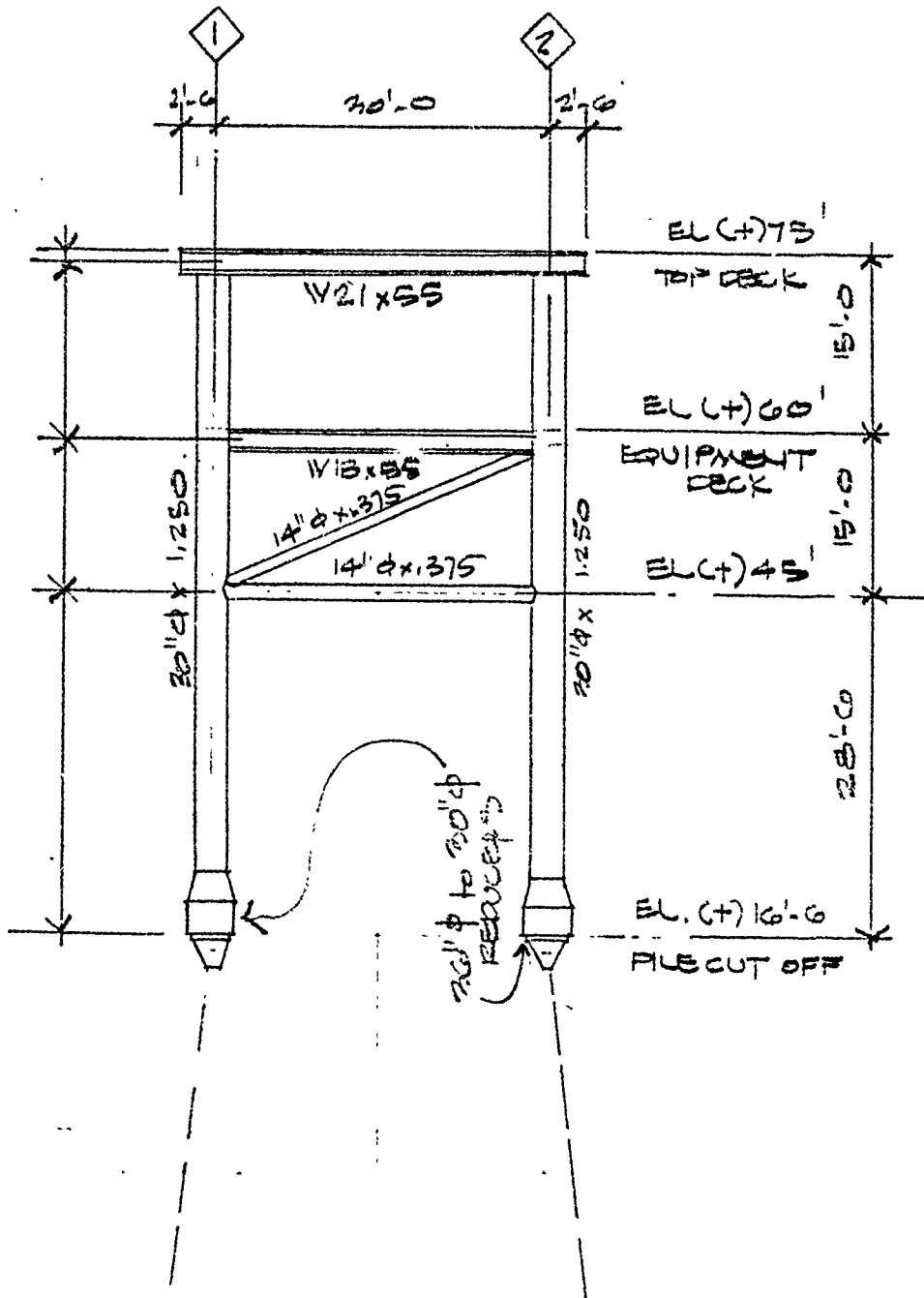
TABLE 4PS-1 $\frac{KL}{r}$ RATIO'S FOR BRACING MEMBERS					
LOCATION	MEMBER SIZE	K	L	r	$\frac{KL}{r}$
HOR. BRACING ↓	14" ϕ x .500	0.7	30.9	4.82	54 ✓
	16" ϕ x .500	0.7	30.9	5.484	47
	12 3/4" ϕ x .500	"	30.9	4.335	72 ✓
	12" ϕ x .500	"	43.1	6.191	58 ✓
	18" ϕ x .500	"	25	6.191	34 ✓
DIAG. BRACING ↓	20" ϕ x .500	0.3	45.3	6.855	63 ✓
	18" ϕ x .500	"	50.6	6.191	73 ✓
	18" ϕ x .500	"	41.1	6.191	64 ✓
INT. X-BRACING	14" ϕ x .375	0.3	43.7	4.820	87
	14" ϕ x .375	"	52.18	4.820	104
	18" ϕ x .500	"	60.95	6.191	95
	13" ϕ x .500	"	70.65	6.191	110
	draw. #12 16" ϕ x .500	"	43.7	5.484	76 ✓
	16" ϕ x .500	"	52.18	5.484	91
	draw -49 20" ϕ x .500	"	60.95	6.897	85 ✓
	20" ϕ x .500	"	70.65	6.897	99
	draw -18 18" ϕ x .500	"	52.18	6.191	81 ✓
	draw -84 20" ϕ x .500	"	36	6.897	50 ✓

CREST OFFSHORE, INC.

Sheet 5.02 of 4 PILE

By WMA Client U.S. Navy Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-7-70 Job No. 27-171-00 Calculation STRUCTURAL CONFIGURATION

B.3



SUPER STRUCTURE ELEVATION

CREST OFFSHORE, INC.

Sheet 5.09 of 4 PLS

By VAA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.6.76 Job No. 27-11-92 Calculation STRUCTURAL CONFIGURATION

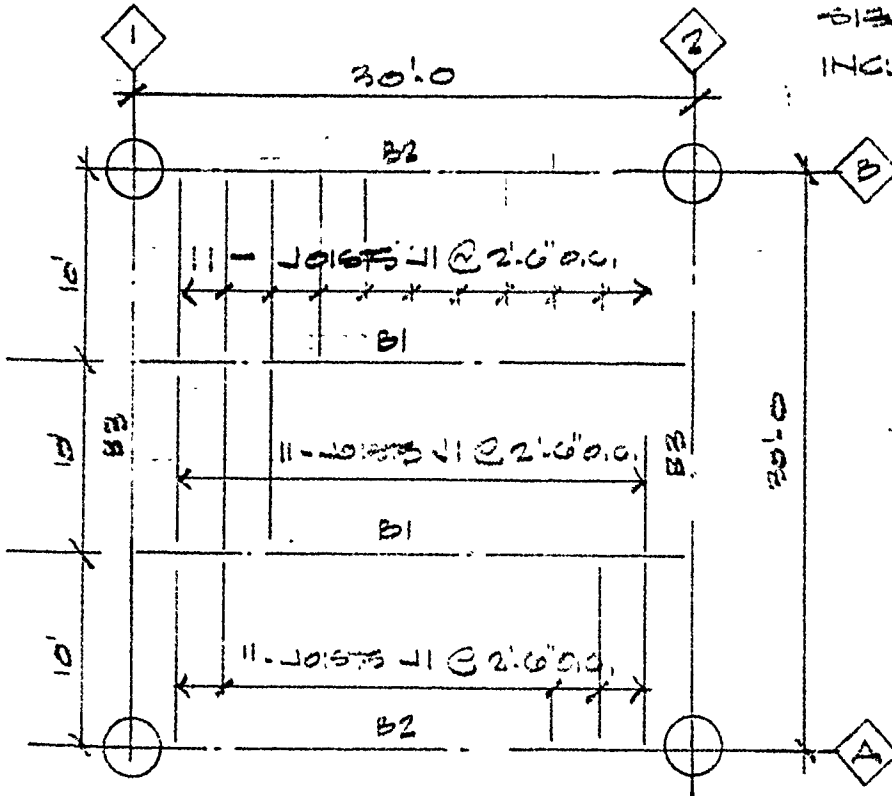
5.4 DECK LAYOUTS

LOADINGS

UPPER DECK 100 PSF
 LOWER DECK 150 PSF

USE 30'x30' BAY SPACING
 30" # COLUMNS

PRELIMINARY BEAM
 SIZING - DOES NOT
 INCLUDE WIND LOADING

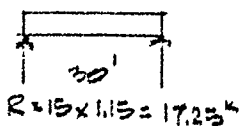


DEAD LOAD
 4" CR'D PL = 10.2 PSF
 JOISTS = 1.8 "
 MISC. = 3.0 "
 15 "
 LIVE LOAD = 100 PSF
 TOTAL LOAD = 115 PSF

UPPER DECK FRAMING

BEAM B1

$w = 10' \times 0.115 + 0.04 = 1.19$



$M = 1.19 (30)^2 / 8 = 133.9 \text{ k}$
 $S_R = 133.9 \times 12 / 24 = 66.9 \text{ in}^3$

Allowable = $\frac{L}{240} = \frac{30 \times 12}{240} = 1.5"$

$I_R = \frac{5(1.19)(30)^4}{384 \times 29 \times 1.5} = 498.6 \text{ in}^4$

try W18 x 40 w/ $S_x = 68.4 \text{ in}^3$ $L_c = 6.4$
 $I_x = 612 \text{ in}^4$

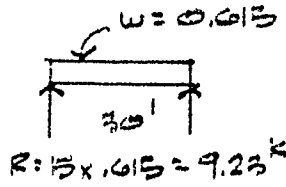
CREST OFFSHORE, INC.

Sheet 2-10 of 4 FILE

By MM Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-6-92 Job No. 27-171-92 Calculation STRUCTURAL CONFIGURATION

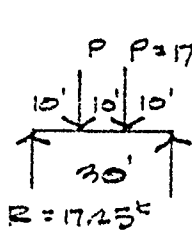
BEAM B2

$w = 115 \text{ PSF} \times 5' + 40 = 615 \text{ #/ft}$



$M = 0.615 (30)^2 / 8 = 69.2 \text{ k}$
 $S_p = 69.2 \times 12 / 24 = 34.6 \text{ in}^3$
 $I_p = \frac{5 (0.615) (30)^4}{384 \times 29 \times 1.5} = 257.7 \text{ in}^4$
 try $W 16 \times 26$ w/ $S_x = 35.3$ $L_c = 5.6'$
 $I_x = 2.23$

BEAM B3

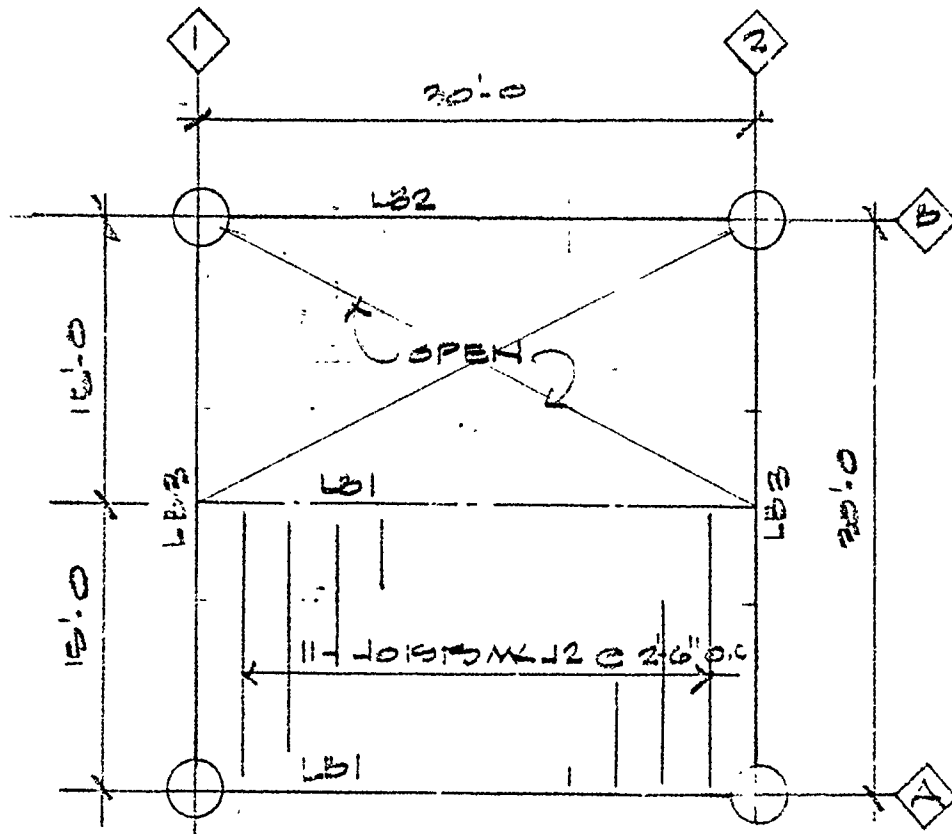


$w = 125 \times 115 \text{ PSF} + 40 = 153.75 \text{ #/ft}$

$M = 17.25 \text{ k} \times 10' + 0.184 (30)^2 / 8 = 172.5 \text{ k} + 20.7 = 193.2 \text{ k}$
 $S_p = 193.2 \times 12 / 24 = 96.6 \text{ in}^3$
 $FEM = -\frac{2Pb}{9} = -\frac{2(17.25)(30)}{9} = 115 \text{ k}$ $+M = 193.2$
 $-\frac{123.3}{9}$
 $+ \frac{64.4}{9} \text{ k}$
 $+ wL^2 / 12 = 0.184 (30)^2 / 12 = 13.3$
 128.8 k

USE $W 21 \times 55$ w/ $S_x = 98.4 \text{ in}^3$
 $L_c = 8.7$
 $L_u = 9.5$

By JVA Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-6-76 Job No. 27-711-93 Calculation STRUCTURAL CONFIGURATION



DEAD LOAD
 4" CKR'D R: 10.2 PSF
 JOISTS = 1.8 "
 MISC = 3.0 "
15.0 PSF
 LIVE LOAD = 150 PSF
 TOTAL LOAD = 165 PSF

LOWER DECK FRAMING PLAN

BEAM LB1

$w = 165 \text{ PSF} \times 7.5' + 40 = 1,277.5 \text{ \#}'$
 $w = 1,278 \text{ \#}'$
 $M = 1278 (30)^2 / 8 = 143.8 \text{ \#}'^2$
 $S_p = 143.8 \times 12 / 24 = 71.9 \text{ \#}'^3$
 $FEM = 1,278 (30)^2 / 12 = 95.85$
 $R = 1,278 \times 3 = 19.17$
 $S_p = 47.93 \text{ \#}'^3$

BEAM LB3

$w = 165 \times 165 + 40 = 240$
 $M = 19.17 (30) / 4 + 0.240 (30)^2 / 8 = 171.4 \text{ \#}'^2$ (simple)
 $S_p = 83.7 \text{ \#}'^3$
 $FEM = -P L / 3 - w L^2 / 12 = -19.17 (30) / 3 - 240 (30)^2 / 12 = -90.3$
 $+M = 171.4 - 90.3 = 81.1$
 $S_p = 90.3 \times 12 / 4 = 45.15$

USE W 13 x 55 w/ $S_x = 93.4$ $L_c = 8.0$
 for LB1, LB2, LB3 $L_u = 12.1$

SECTION 6

50 FT JACKET BASE SPACING

6.1 INTRODUCTION

The idealized structural model is shown in Section 6.2 with the joint numbers, member numbers and directions. In order to simulate the behavior of the platform structure, fictitious wishbone members are introduced to connect the jacket leg joints and the piles. These members are coded not to take any wave forces. They will be ignored by SEALOAD but are necessary for STRAN. The dead load report from SEALOAD will, however, include weights for the wishbones which will have to be subtracted out for the computations of the laterally loaded pile.

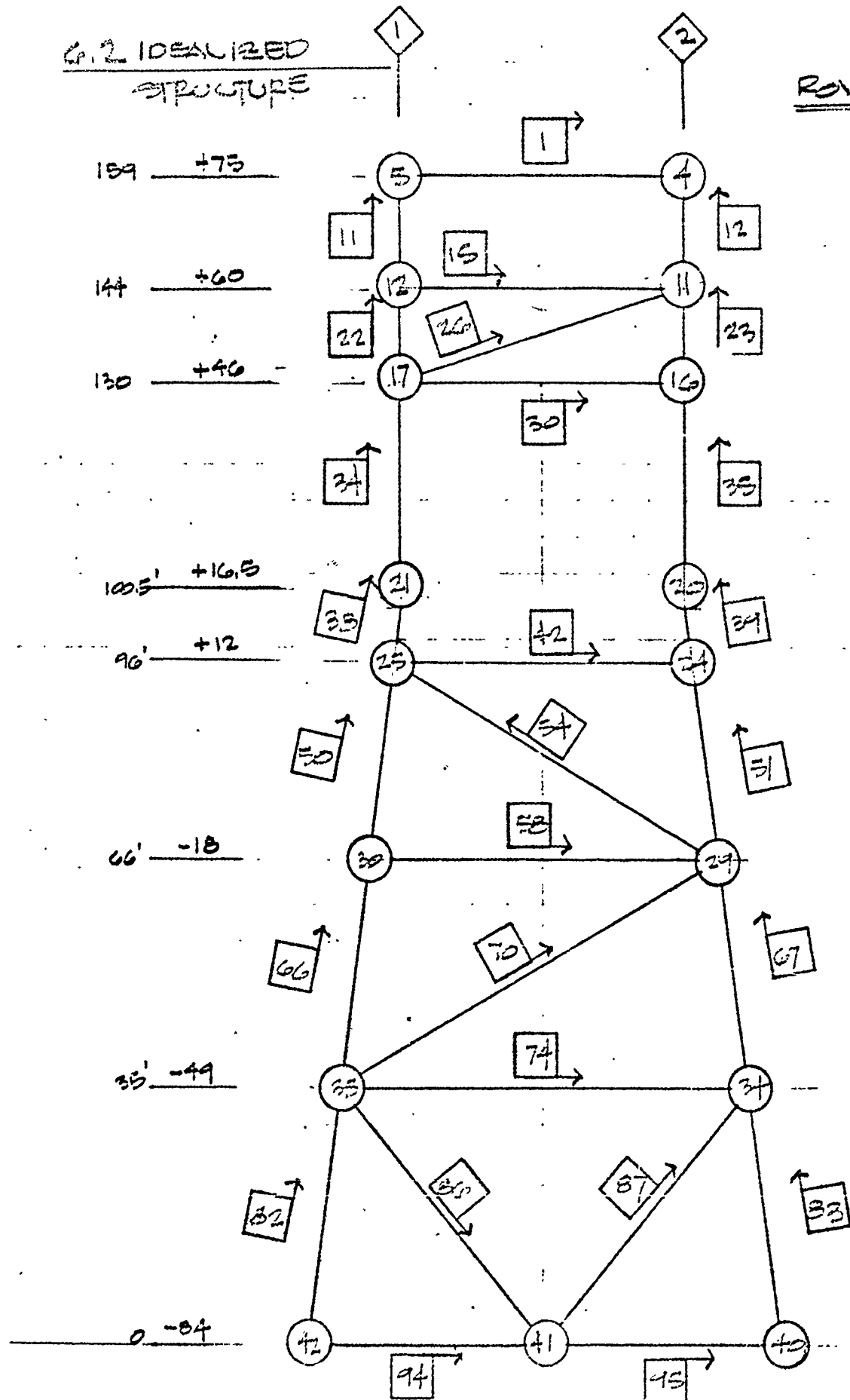
Section 6.3 gives the direction of wave and wind loading data.

All information was coded on keypunch forms, cards were punched and assembled with the cards from Section 2 'Grid Wave Data' to form a complete SEALOAD deck. A computer run was made and the results saved on a tape. Section 6.4 shows the input information and the load summary reports of that run.

By VVA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 1-6-73 Job No. 27-77-92 Calculation STRUCTURE W/L = 50 FT.

6.2 IDEALIZED STRUCTURE

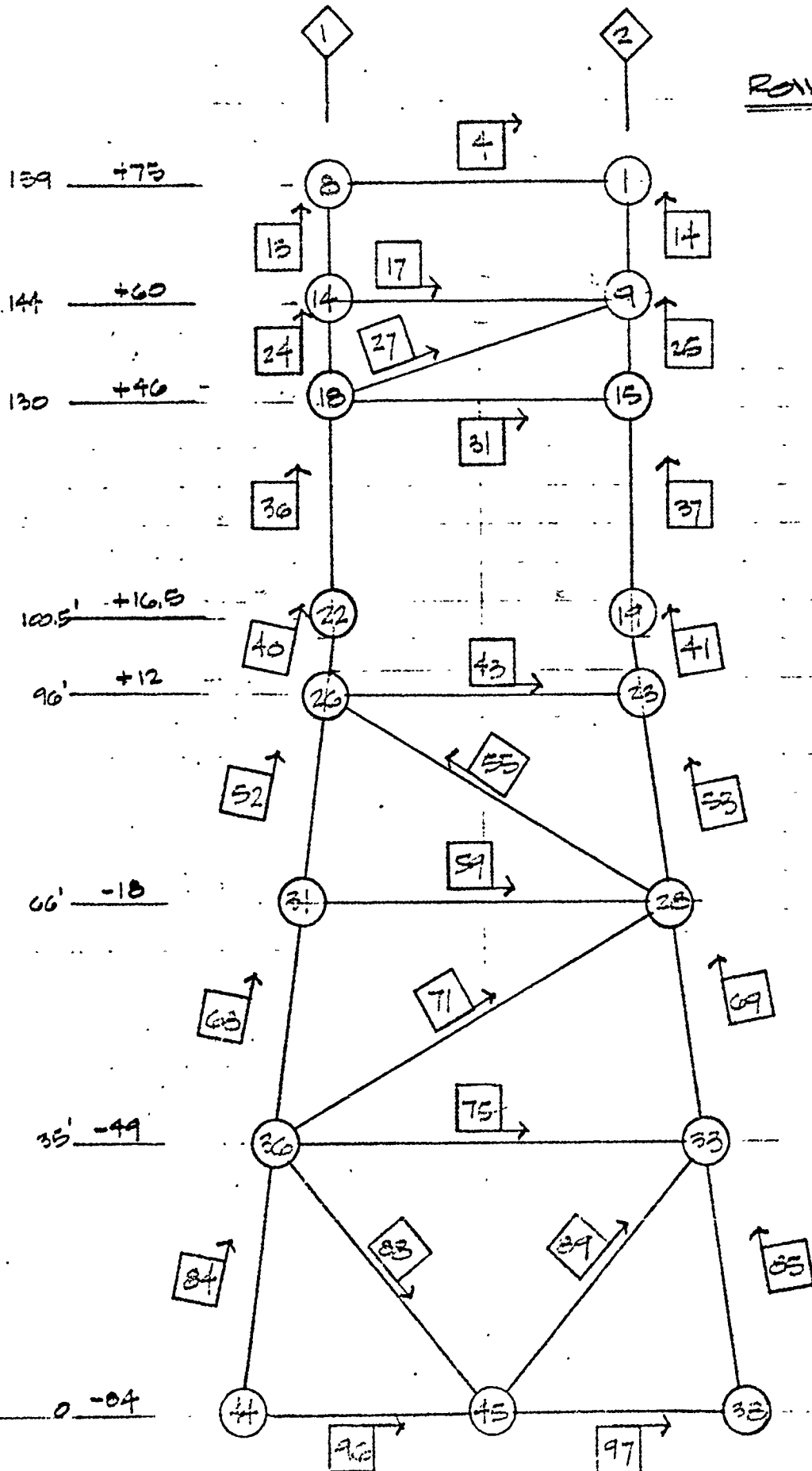
ROW A



CREST OFFSHORE, INC.

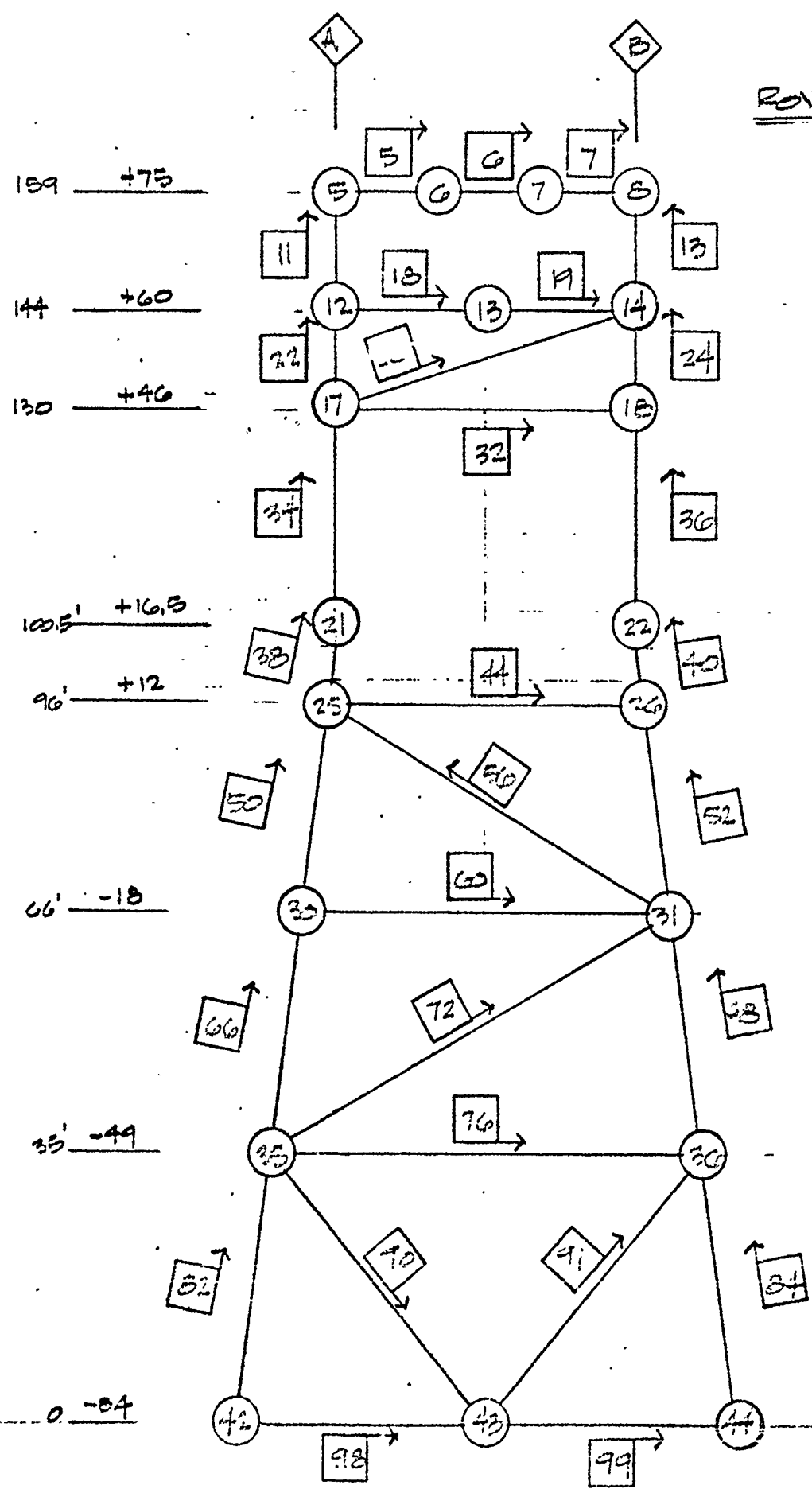
Sheet 6.03 of 7 PILE

By VSA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-6-76 Job No. 27-771-92 Calculation STRUCTURE w/L = 50 FT



By VWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4.6.79 Job No. 27-171-92 Calculation STRUCTURE W/ L=50'

ROW 1

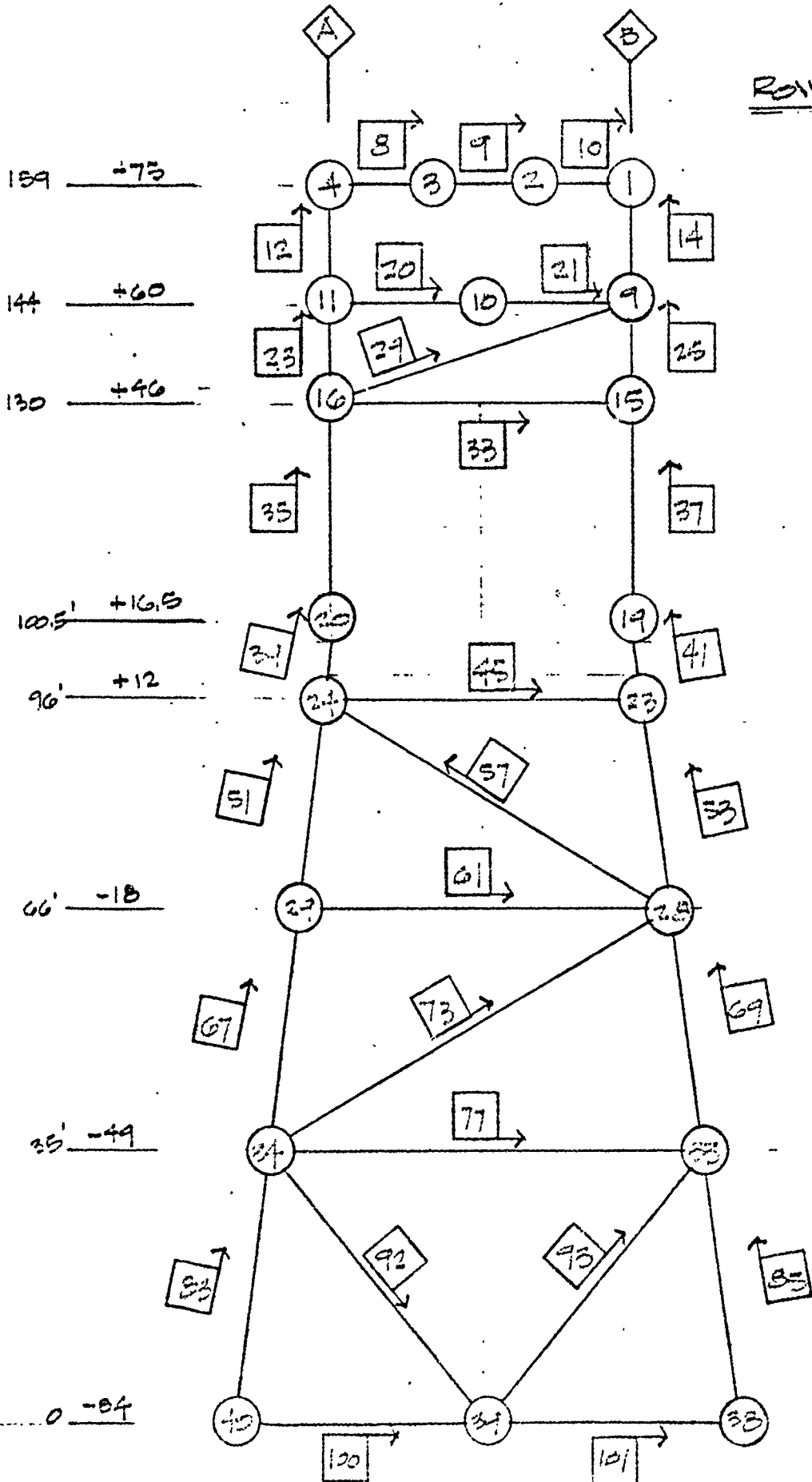


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Sheet 052 of 2 PILE

By VVA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 10-70 Job No. 27-771-92 Calculation STRUCTURE W/L=50'

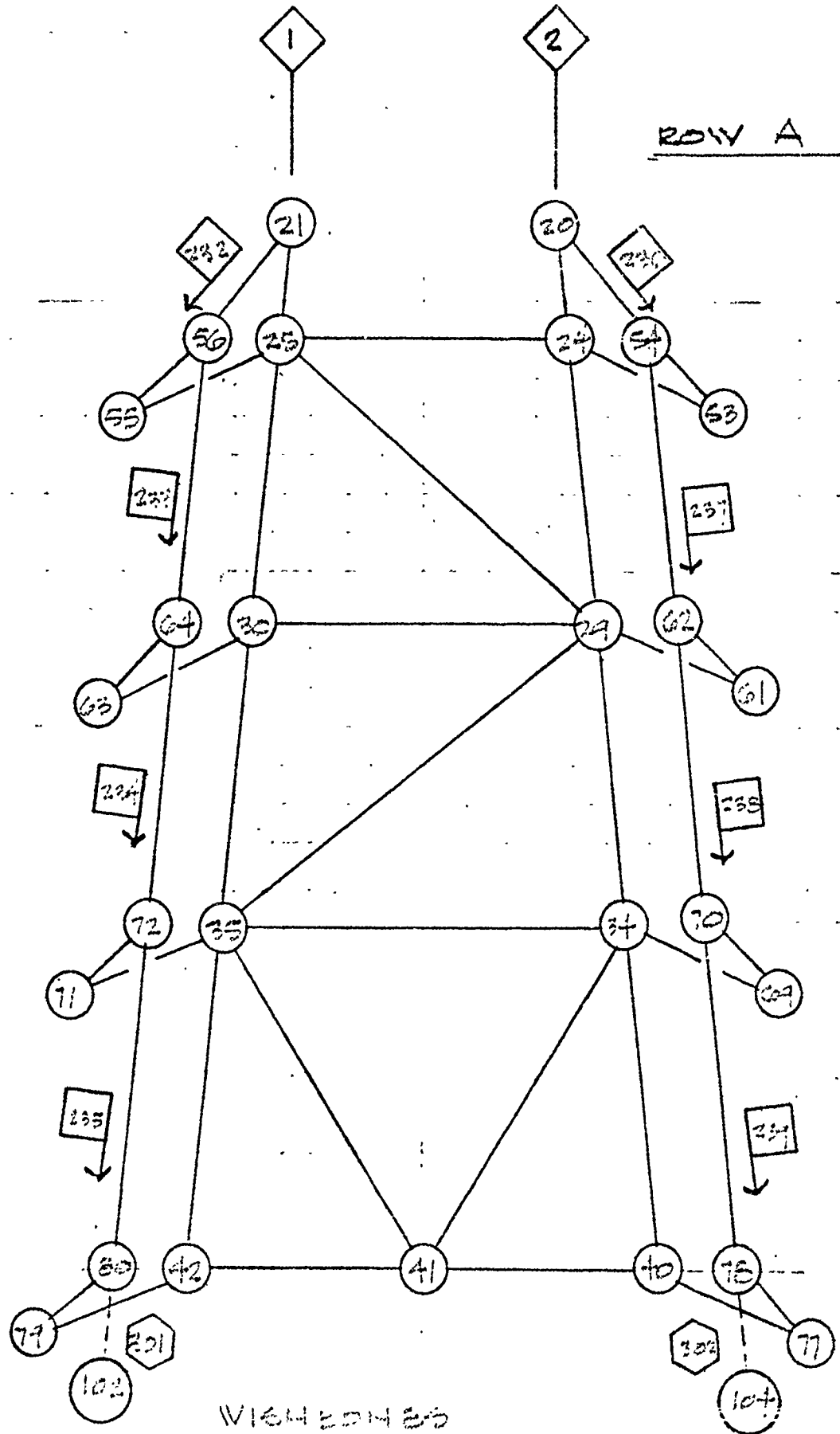
ROW 2



CREST OFFSHORE, INC.

Sheet 6.56 of 4 PLS

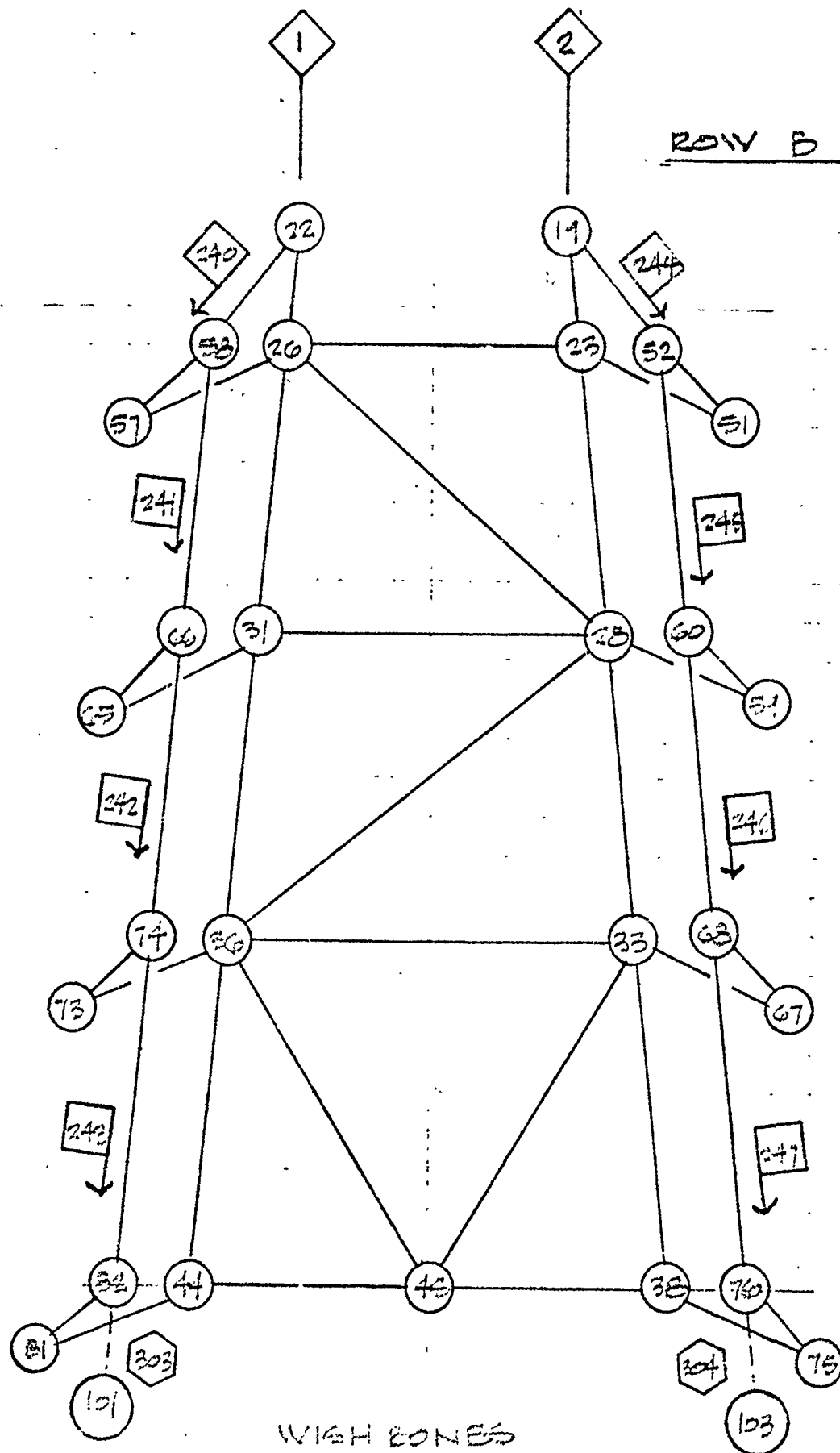
By W.A. Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 7-6-76 Job No. 37-771-92 Calculation STRUCTURE w/ L=50'



CREST OFFSHORE, INC.

Sheet 607 of 4 PILE

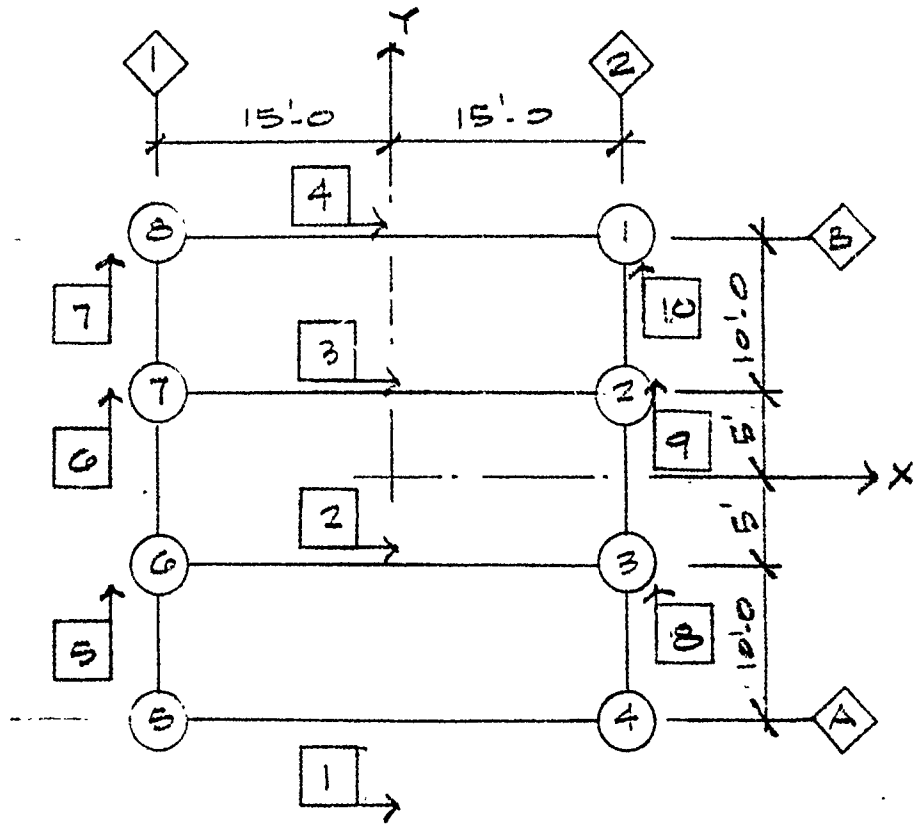
By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-6-76 Job No. 27-771-92 Calculation STRUCTURE W/ L=50'



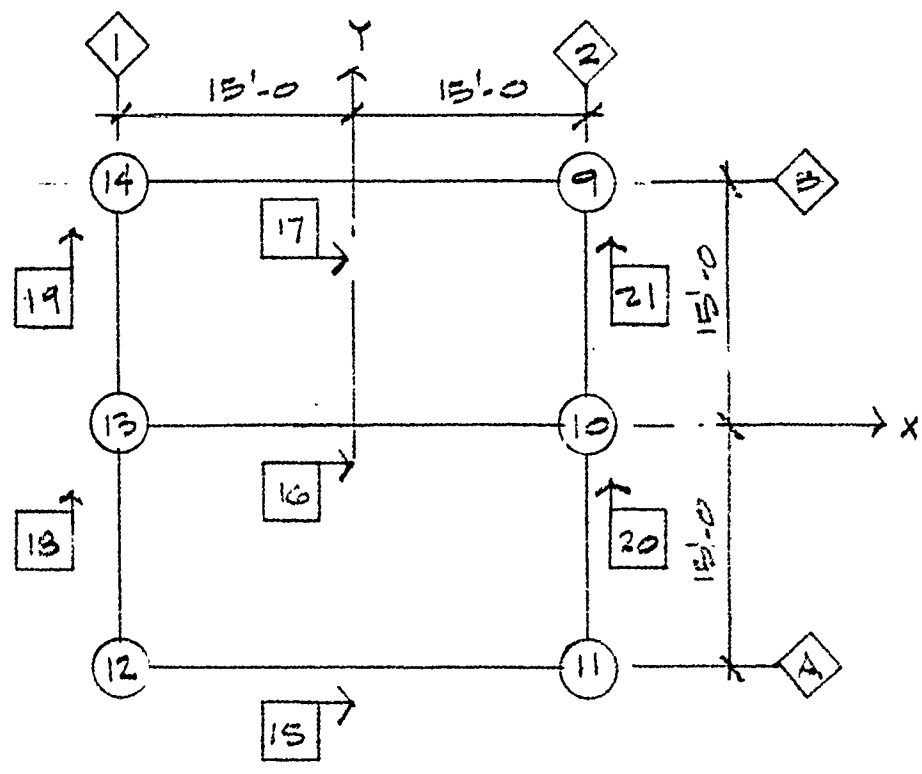
CREST OFFSHORE, INC.

Sheet 60B of 4 FILE

By JWA Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-7-70 Job No. 21-771-92 Calculation STRUCTURE W/L = 50'



ELEV. (+) 75'-0 (139)

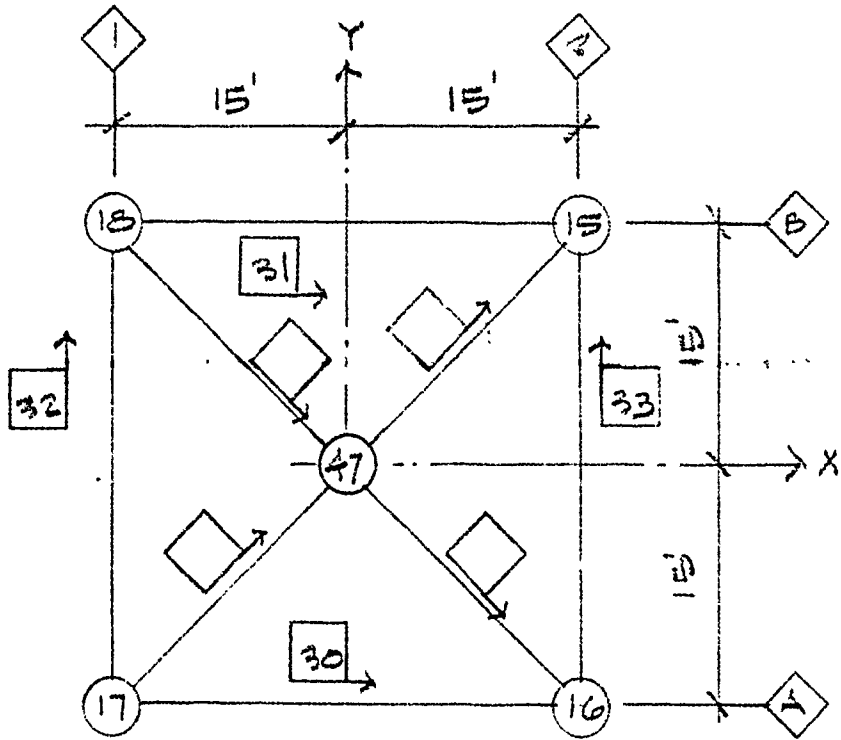


ELEV. (+) 60 (144)

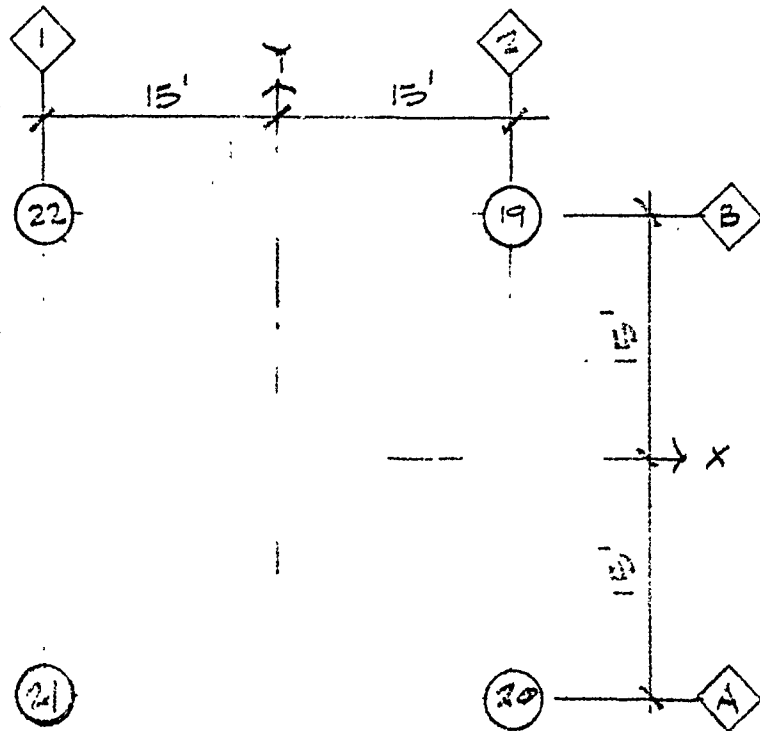
CREST OFFSHORE, INC.

Sheet 6.09 of 4 FILE

By JWA Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-7-76 Job No. 21-111-92 Calculation STRUCTURE w/ L=50'

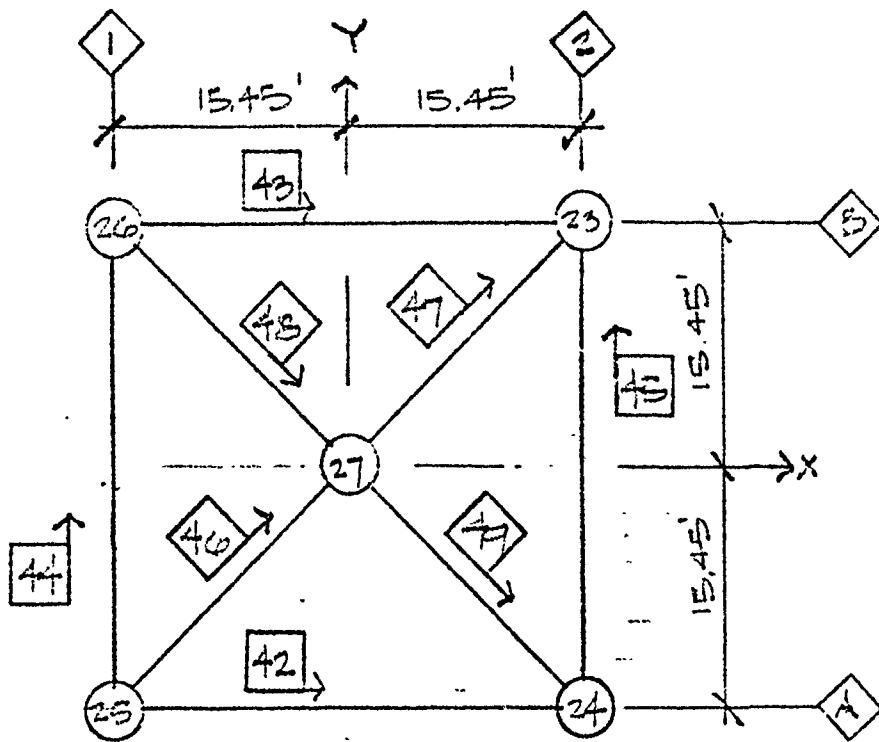


ELEV. (+) 4.5' (130)

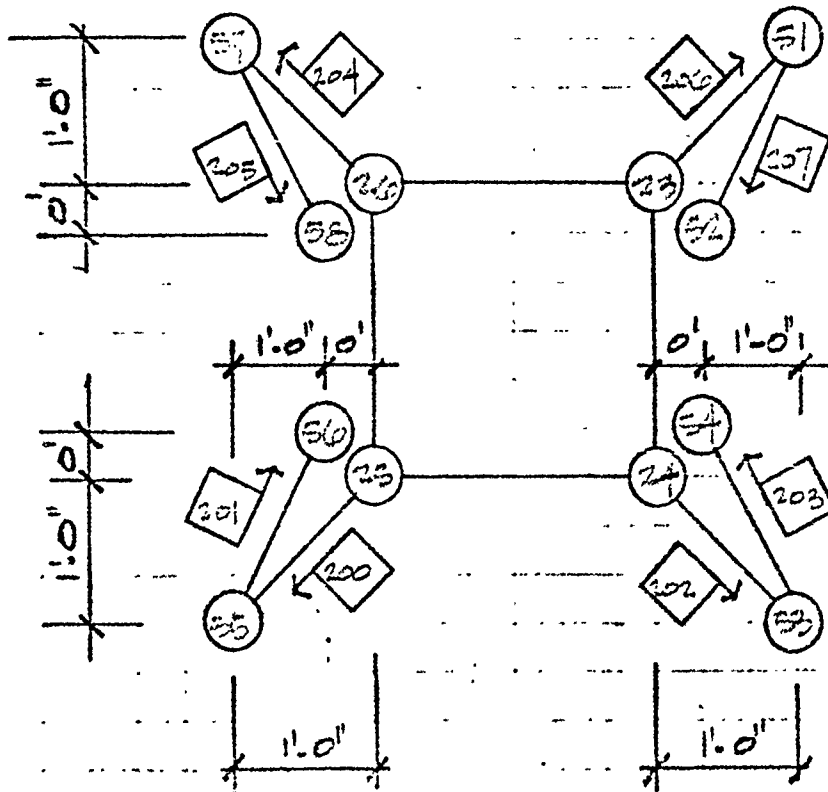


ELEV. (+) 6.5' (100.5)

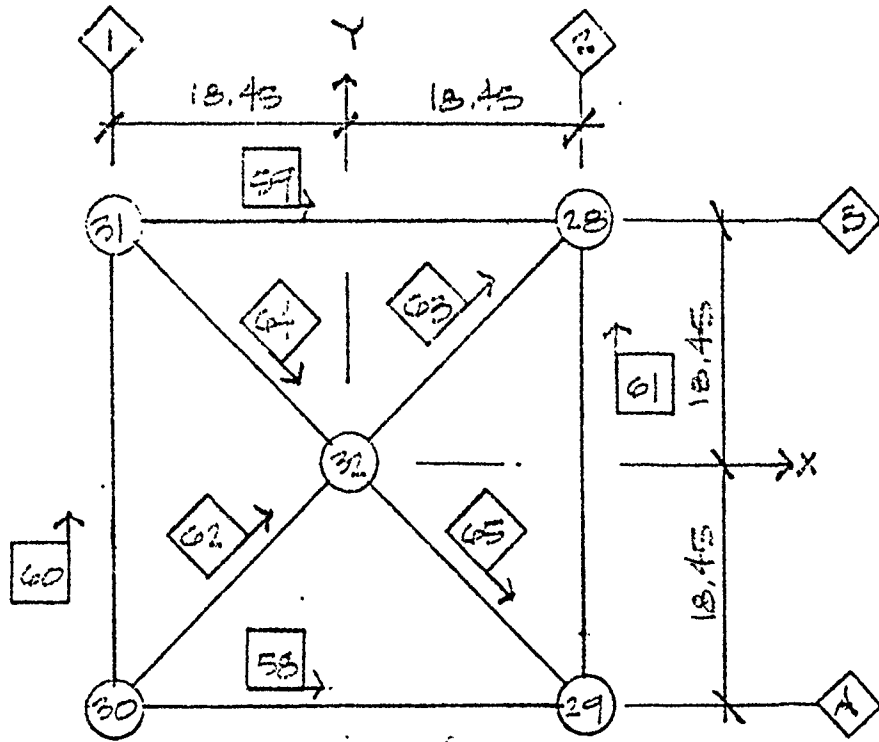
By W.S. Client US. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-7-76 Job No. 21-771-92 Calculation STRUCTURE 1/2 = 50'



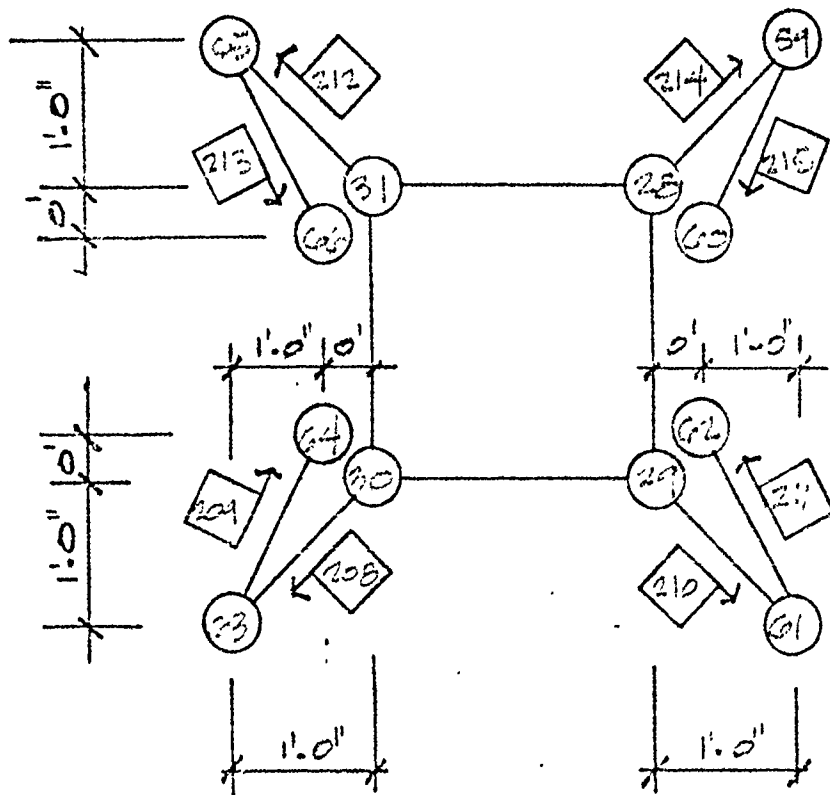
ELEV. (+) 12 (-) 90



By WMS Client US. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.1.92 Job No. 21-771-92 Calculation STRUCTURE 4 / L = 50'



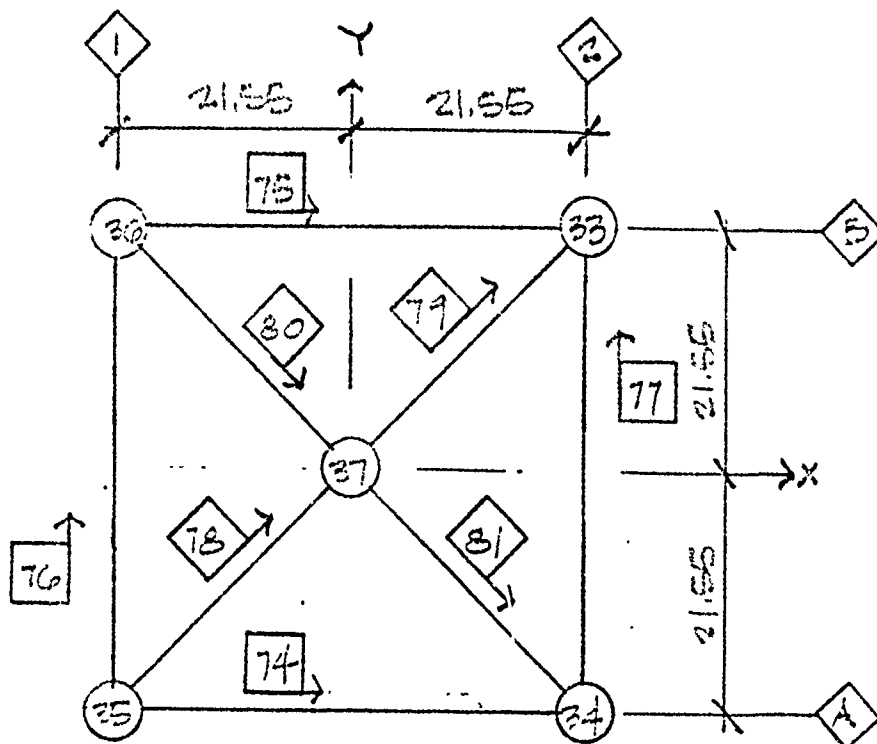
ELEV. (-) 13 (66)



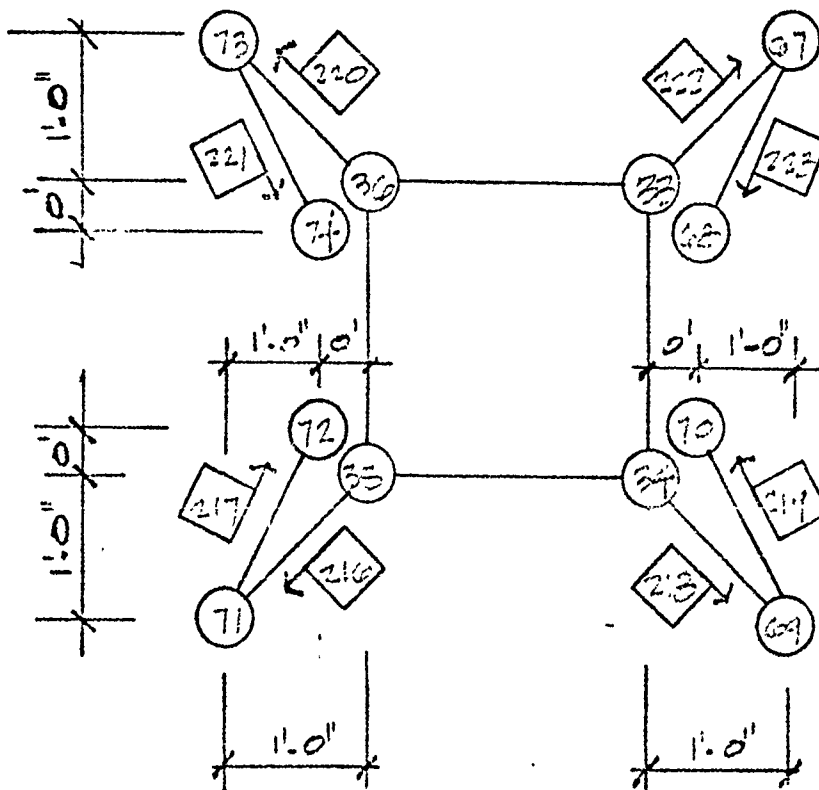
CREST OFFSHORE, INC.

Sheet 0.12 of 4 P. 2

By JMS Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date Feb 7-76 Job No. 21-111-92 Calculation STRUCTURE Y/L = 50'



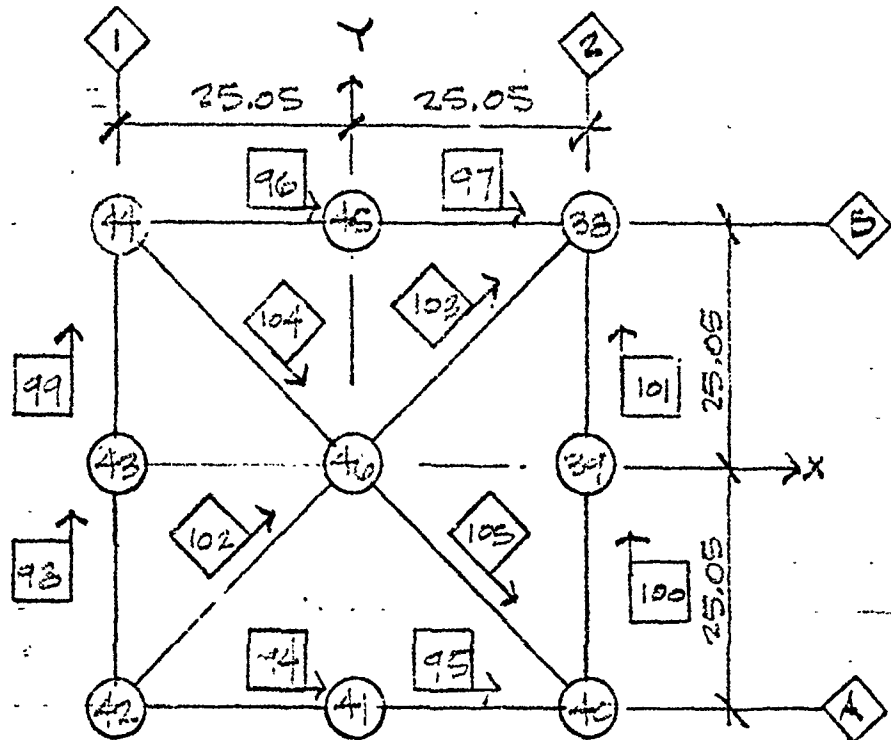
ELEV. (-) 49 (35)



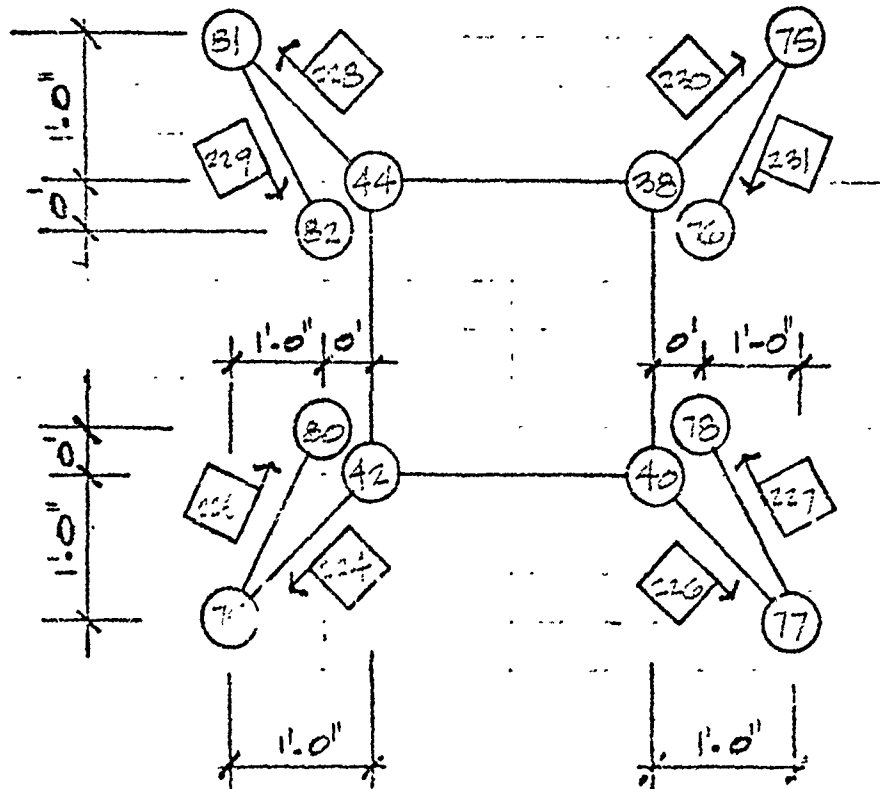
CREST OFFSHORE, INC.

Sheet 613 of 413

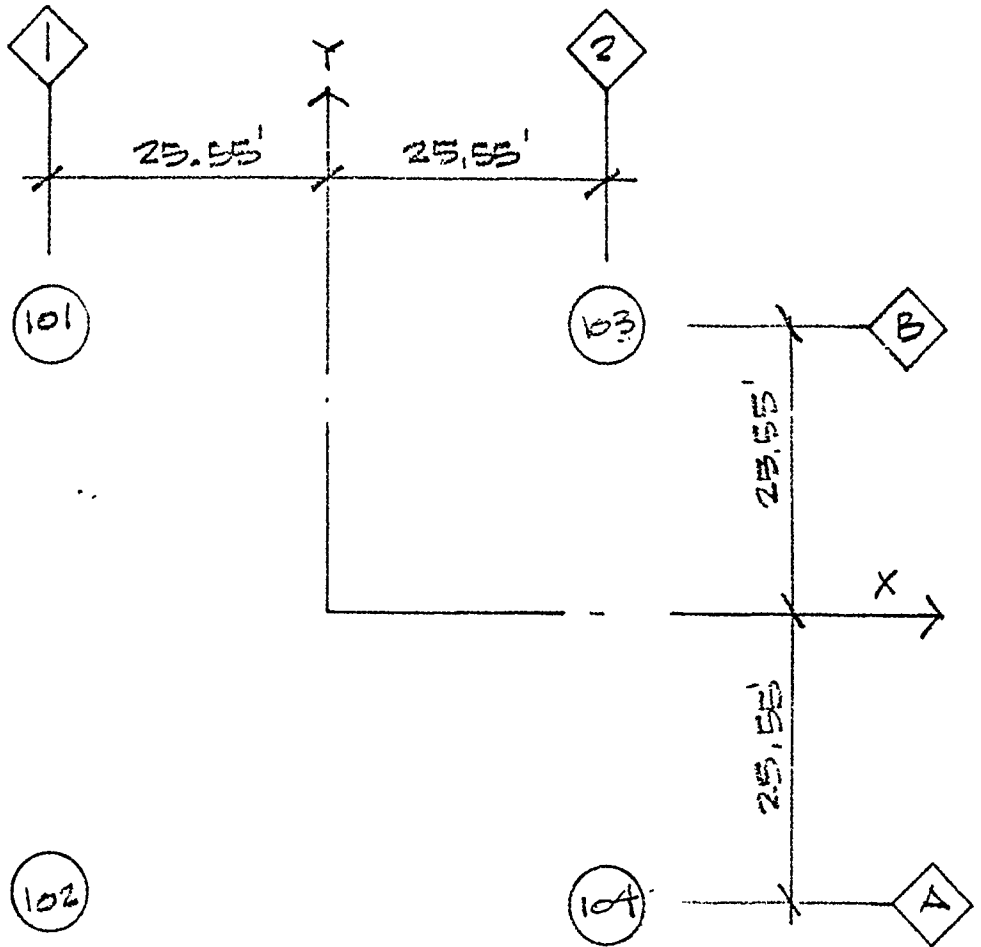
By KWS Client US NAVY Subject STRUCTURAL CONCEPT ANSWERS
 Date 7-76 Job No. 21-71-92 Calculation STRUCTURE 4/LE 50'



ELEV. (-) 84 (0)



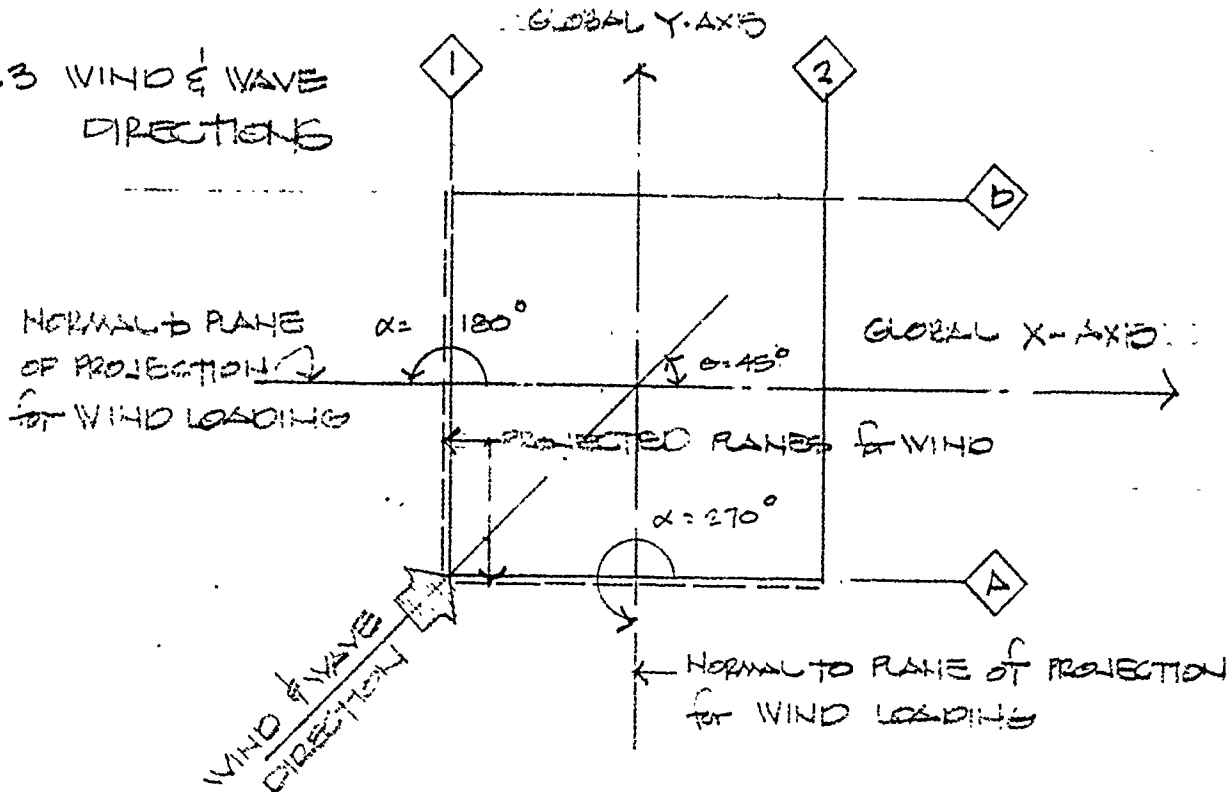
By VWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-7-76 Job No. 21-771-92 Calculation STRUCTURE w/ L=50'



ELEV. (...) 89 (-5)

By JWD Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-7-76 Job No. 27-111-92 Calculation STRUCTURE W/L=50'

2.3 WIND & WAVE DIRECTIONS



WIND LOADING

AREAS TO CONSIDER

FENCE or SAFETY NET = $2' \times 40' = 80$ S.F.T.

AREA BETWEEN DECKS = $15' \times 40' \times 0.50 = 300$ S.F.T.

WIND VELOCITY = 173 MPH

CREST OFFSHORE, INC.

Sheet 017 of 4 PILE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-13-76 Job No. 27-771-00 Calculation L=50'

SEALOAD*2

LINE NO. 1 1.5 2 3 4 5 6 7 8
 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5

43	WIND	1	80	270	177	1005	173	15	ALL											
44		2	300	270	158	1005	173	15	ALL											
45		3	80	180	177	1005	173	15	ALL											
46		4	300	180	168	1005	173	15	ALL											
47		5																		
48		6																		
49		7																		
50		8																		
51																				
52																				
53																				
54																				
55	WIND	1	80	270	177	1005	173	15	ALL											
56		2	300	270	158	1005	173	15	ALL											
57		3	80	180	177	1005	173	15	ALL											
58		4	300	180	168	1005	173	15	ALL											
59		5																		
60	SECT	W21X55	WF	16.2		1.24	1140		48.5	8.215	522	20.8	.375	.563						
61	SECT	W18X55	WF	16.2		1.66	891		45	7.532	630	18.12	.390	.438						
62	SECT	W18X40	WF	11.8		0.81	612		19.1	6.018	524	17.9	.316	.438						
63	SECT	W18X40	TUR	21.21		968	484		14	5.00										
64	SECT	W18X40	TUR	24.35		1464	732		16	5.00										
65	SECT	W18X40	TUR	27.49		2106	1053		18	5.00										
66	SECT	W18X40	TUR	30.63		2914	1457		20	5.00										
67	SECT	W18X40	TUR	33.77		3822	1961		22	5.00										
68	SECT	W18X40	TUR	36.91		4730	2565		24	5.00										
69	SECT	W18X40	TUR	40.05		5638	3169		26	5.00										
70	SECT	W18X40	TUR	43.19		6546	3773		28	5.00										
71	SECT	W18X40	TUR	46.33		7454	4377		30	5.00										
72	SECT	W18X40	TUR	49.47		8362	4981		32	5.00										
73	SECT	W18X40	TUR	52.61		9270	5585		34	5.00										
74	SECT	W18X40	TUR	55.75		10178	6189		36	5.00										
75	SECT	W18X40	TUR	58.89		11086	6793		38	5.00										
76	SECT	W18X40	TUR	62.03		12094	7397		40	5.00										
77	SECT	W18X40	TUR	65.17		13102	8001		42	5.00										
78	SECT	W18X40	TUR	68.31		14110	8605		44	5.00										
79	SECT	W18X40	TUR	71.45		15118	9209		46	5.00										
80	SECT	W18X40	TUR	74.59		16126	9813		48	5.00										
81	SECT	W18X40	TUR	77.73		17134	10417		50	5.00										
82	SECT	W18X40	TUR	80.87		18142	11021		52	5.00										
83	SECT	W18X40	TUR	84.01		19150	11625		54	5.00										
84	SECT	W18X40	TUR	87.15		20158	12229		56	5.00										
85	SECT	W18X40	TUR	90.29		21166	12833		58	5.00										
86	SECT	W18X40	TUR	93.43		22174	13437		60	5.00										
87	SECT	W18X40	TUR	96.57		23182	14041		62	5.00										
88	SECT	W18X40	TUR	99.71		24190	14645		64	5.00										
89	SECT	W18X40	TUR	102.85		25198	15249		66	5.00										
90	SECT	W18X40	TUR	106.00		26206	15853		68	5.00										
91	SECT	W18X40	TUR	109.14		27214	16457		70	5.00										
92	SECT	W18X40	TUR	112.28		28222	17061		72	5.00										
93	SECT	W18X40	TUR	115.42		29230	17665		74	5.00										
94	SECT	W18X40	TUR	118.56		30238	18269		76	5.00										
95	SECT	W18X40	TUR	121.70		31246	18873		78	5.00										
96	SECT	W18X40	TUR	124.84		32254	19477		80	5.00										
97	SECT	W18X40	TUR	127.98		33262	20081		82	5.00										
98	SECT	W18X40	TUR	131.12		34270	20685		84	5.00										
99	SECT	W18X40	TUR	134.26		35278	21289		86	5.00										
100	SECT	W18X40	TUR	137.40		36286	21893		88	5.00										
101	SECT	W18X40	TUR	140.54		37294	22497		90	5.00										
102	SECT	W18X40	TUR	143.68		38302	23101		92	5.00										
103	SECT	W18X40	TUR	146.82		39310	23705		94	5.00										
104	SECT	W18X40	TUR	149.96		40318	24309		96	5.00										
105	SECT	W18X40	TUR	153.10		41326	24913		98	5.00										
106	SECT	W18X40	TUR	156.24		42334	25517		100	5.00										
107	SECT	W18X40	TUR	159.38		43342	26121		102	5.00										
108	SECT	W18X40	TUR	162.52		44350	26725		104	5.00										
109	SECT	W18X40	TUR	165.66		45358	27329		106	5.00										
110	SECT	W18X40	TUR	168.80		46366	27933		108	5.00										
111	SECT	W18X40	TUR	171.94		47374	28537		110	5.00										
112	SECT	W18X40	TUR	175.08		48382	29141		112	5.00										
113	SECT	W18X40	TUR	178.22		49390	29745		114	5.00										
114	SECT	W18X40	TUR	181.36		50398	30349		116	5.00										
115	SECT	W18X40	TUR	184.50		51406	30953		118	5.00										
116	SECT	W18X40	TUR	187.64		52414	31557		120	5.00										
117	SECT	W18X40	TUR	190.78		53422	32161		122	5.00										
118	SECT	W18X40	TUR	193.92		54430	32765		124	5.00										
119	SECT	W18X40	TUR	197.06		55438	33369		126	5.00										
120	SECT	W18X40	TUR	200.20		56446	33973		128	5.00										
121	SECT	W18X40	TUR	203.34		57454	34577		130	5.00										
122	SECT	W18X40	TUR	206.48		58462	35181		132	5.00										
123	SECT	W18X40	TUR	209.62		59470	35785													

CREST OFFSHORE, INC.

Sheet 61B of 4 FILE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-13-76 Job No. 27-711-92 Calculation L-50

SEALOAD=2

LINE NO. 1 2 3 4 5 6 7 8

93 GRP WBN WISHRN 14 .375
 94 GRP DB3 BRACE3 14 .375
 95 GRP WBS BRACE3 14 .375
 96 MEMBER 5 4 X21
 97 MEMBER 6 3 X17
 98 MEMBER 7 2 X17
 100 MEMBER 8 1 X21
 101 MEMBER 5 6 X21
 102 MEMBER 6 7 X21
 103 MEMBER 7 5 X21
 104 MEMBER 4 3 X21
 105 MEMBER 3 2 X21
 106 MEMBER 2 1 X21
 107 MEMBER 12 5 SSC
 108 MEMBER 11 4 SSC
 109 MEMBER 14 8 SSC
 110 MEMBER 9 1 SSC
 111 MEMBER 12 11 X18
 112 MEMBER 13 10 X18
 113 MEMBER 14 9 X18
 114 MEMBER 12 13 X18
 115 MEMBER 13 14 X18
 116 MEMBER 11 10 X18
 117 MEMBER 10 9 X18
 118 MEMBER 17 12 SSC
 119 MEMBER 16 11 SSC
 120 MEMBER 18 11 SSC
 121 MEMBER 15 9 SSC
 122 MEMBER 17 11 DR3
 123 MEMBER 18 9 DR3
 124 MEMBER 17 14 DR3
 125 MEMBER 16 9 DR3
 126 MEMBER 12 15 HB3
 127 MEMBER 18 15 HB3
 128 MEMBER 17 18 HB3
 129 MEMBER 16 15 HB3
 130 MEMBER 21 17 SSC
 131 MEMBER 20 16 SSC
 132 MEMBER 22 18 SSC
 133 MEMBER 19 15 SSC
 134 MEMBER 25 21 JLI
 135 MEMBER 24 20 JLI
 136 MEMBER 26 22 JLI
 137 MEMBER 23 19 JLI
 138 MEMBER 25 24 HBI
 139 MEMBER 28 23 HBI
 140 MEMBER 25 26 HBI
 141 MEMBER 24 23 HBI
 142 MEMBER 25 27 HBI

30
30
30
30

30
30
30
14
14
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14
14
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30
30
F 40
F 40
F 40
F 40
14
14
14
14

CREST OFFSHORE, INC.

Sheet 619 of 4 FILE

By KWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-15-76 Job No. 29-711-00 Calculation L-FC

PAGE

SEALOAD#2

NO.	1	2	3	4	5	6	7	8
183	MEMBER	27	23	XBI				16
184	MEMBER	26	24	XBI				16
185	MEMBER	30	25	JLI			F	40
186	MEMBER	29	24	JLI			F	40
187	MEMBER	31	26	JLI			F	40
188	MEMBER	28	23	JLI				20
189	MEMBER	29	25	DRI				20
190	MEMBER	28	26	DRI				20
191	MEMBER	31	25	DRI				20
192	MEMBER	29	24	DRI				20
193	MEMBER	31	28	HBI				14
194	MEMBER	30	31	HBI				14
195	MEMBER	29	28	HBI				14
196	MEMBER	30	28	HBI				14
197	MEMBER	32	32	XB2				18
198	MEMBER	32	20	XB2				18
199	MEMBER	31	32	XB2				18
200	MEMBER	32	29	XB2				18
201	MEMBER	35	30	JLI			F	40
202	MEMBER	34	29	JLI			F	40
203	MEMBER	36	31	JLI			F	40
204	MEMBER	33	28	JLI			F	40
205	MEMBER	33	29	DB2				18
206	MEMBER	36	28	DB2				18
207	MEMBER	35	31	DB2				18
208	MEMBER	34	29	DB2				18
209	MEMBER	35	34	DB2				18
210	MEMBER	36	33	DB2				18
211	MEMBER	35	36	DB2				18
212	MEMBER	34	33	DB2				18
213	MEMBER	37	37	XB3				20
214	MEMBER	37	33	XB3				20
215	MEMBER	37	33	XB3				20
216	MEMBER	36	37	XB3				20
217	MEMBER	37	34	XB3				20
218	MEMBER	42	35	JLI			F	40
219	MEMBER	40	30	JLI			F	40
220	MEMBER	44	36	JLI			F	40
221	MEMBER	38	35	JLI				18
222	MEMBER	35	41	DB2				18
223	MEMBER	41	34	DB2				18
224	MEMBER	36	45	DB2				18
225	MEMBER	45	33	DB2				18
226	MEMBER	35	43	DB2				18
227	MEMBER	43	36	DB2				18
228	MEMBER	43	36	DB2				18
229	MEMBER	34	39	DB2				18
230	MEMBER	39	33	DB2				18
231	MEMBER	42	41	DB2				18
232	MEMBER	41	40	DB2				18
233	MEMBER	44	45	DB2				18

CREST OFFSHORE, INC.

Sheet 020 of 4 PLS

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-15-76 Job No. 27-771-22 Calculation L=30'

LINE NO. 1 2 3 4 5 6 7 8
 SEALOAD=2
 1.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5
 45 42 40 39 38 42 46 46
 44 40 35 55 55 53 54 57
 57 58 51 52 30 63 64 61
 62 31 65 60 28 59 55 71
 72 50 60 60 70 71 73 74
 73 74 67 67 62 70 80 77
 78 40 77 40 81 82 38 75
 75 21 56 56 64 72 72 80
 80 25 50 54 62 62 70 78
 78 22 58 58

LINE NO.	1	2	3	4	5	6	7	8
193	MEMBER	45	38	MR2				18
194	MEMBER	42	43	MR2				18
195	MEMBER	43	44	MR2				18
196	MEMBER	40	39	MR2				18
197	MEMBER	39	38	MR2				18
198	MEMBER	42	46	MR3				20
199	MEMBER	46	38	MR3				20
200	MEMBER	40	46	MR3				20
201	MEMBER	44	40	MR3				20
202	MEMBER	25	55	MR7				
203	MEMBER	50	55	MR7				
204	MEMBER	24	53	MR7				
205	MEMBER	53	54	MR7				
206	MEMBER	24	57	MR7				
207	MEMBER	57	58	MR7				
208	MEMBER	23	51	MR7				
209	MEMBER	51	52	MR7				
210	MEMBER	30	63	MR7				
211	MEMBER	63	64	MR7				
212	MEMBER	25	61	MR7				
213	MEMBER	61	62	MR7				
214	MEMBER	31	65	MR7				
215	MEMBER	65	60	MR7				
216	MEMBER	28	59	MR7				
217	MEMBER	59	60	MR7				
218	MEMBER	55	71	MR7				
219	MEMBER	71	72	MR7				
220	MEMBER	50	60	MR7				
221	MEMBER	60	70	MR7				
222	MEMBER	71	73	MR7				
223	MEMBER	73	74	MR7				
224	MEMBER	33	67	MR7				
225	MEMBER	67	68	MR7				
226	MEMBER	62	70	MR7				
227	MEMBER	79	80	MR7				
228	MEMBER	40	77	MR7				
229	MEMBER	77	78	MR7				
230	MEMBER	40	81	MR7				
231	MEMBER	81	82	MR7				
232	MEMBER	38	75	MR7				
233	MEMBER	75	75	MR7				
234	MEMBER	21	56	PI				F 36 2
235	MEMBER	56	64	PI				F 36 2
236	MEMBER	64	72	PI				F 36 2
237	MEMBER	72	80	PI				F 36 2
238	MEMBER	25	50	PI				F 36 4
239	MEMBER	54	62	PI				F 36 4
240	MEMBER	62	70	PI				F 36 4
241	MEMBER	70	78	PI				F 36 4
242	MEMBER	22	58	PI				F 36 1

CREST OFFSHORE, INC.

Sheet 621 of 7

By KWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-15-72 Job No. 17-77-12 Calculation K-172

BEALOAD#2

PAGE 6

LINE NO.	1	2	3	4	5	6	7	8	MEMBER	TYPE	LOAD	MEMBER	TYPE	LOAD
243	58	66	PI						UPERDECK	F	36	1		
244	66	74	PI						UPERDECK	F	36	1		
245	74	82	P2						UPERDECK	F	36	1		
246	19	52	PI						UPERDECK	F	36	3		
247	52	60	PI						UPERDECK	F	36	3		
248	60	68	PI						UPERDECK	F	36	3		
249	66	76	P2						UPERDECK	F	36	3		
250	80	102	P2						UPERDECK	F	36	2		
251	78	104	P2						UPERDECK	F	36	4		
252	82	101	P2						UPERDECK	F	36	1		
253	76	103	P2						UPERDECK	F	36	3		
254														
255	1	15						159	UPERDECK					
256	2	15						159	UPERDECK					
257	3	15						159	UPERDECK					
258	4	15						159	UPERDECK					
259	5	15						159	UPERDECK					
260	6	15						150	UPERDECK					
261	7	15						159	UPERDECK					
262	8	15						159	UPERDECK					
263	9	15						144	LOERDECK					
264	10	15						144	LOERDECK					
265	11	15						144	LOERDECK					
266	12	15						144	LOERDECK					
267	13	15						144	LOERDECK					
268	14	15						144	LOERDECK					
269	15	15						130	SUPERSTR					
270	16	15						130	SUPERSTR					
271	17	15						130	SUPERSTR					
272	18	15						130	SUPERSTR					
273	19	15						1005	SUPERSTR					
274	20	15						1005	SUPERSTR					
275	21	15						1005	SUPERSTR					
276	22	15						1005	SUPERSTR					
277	23	15.45	15.45					96	ELEV.+12					
278	24	15.45	15.45					96	ELEV.+12					
279	25	15.45	15.45					96	ELEV.+12					
280	26	15.45	15.45					96	ELEV.+12					
281	27	0	0					66	ELEV.+12					
282	28	10.45	10.45					66	ELEV.+18					
283	29	18.45	18.45					66	ELEV.+18					
284	30	18.45	18.45					66	ELEV.+18					
285	31	18.45	18.45					66	ELEV.+18					
286	32	0	0					66	ELEV.+18					
287	33	21.55	21.55					35	ELEV.+49					
288	34	21.55	21.55					35	ELEV.+49					
289	35	21.55	21.55					35	ELEV.+49					
290	36	21.55	21.55					35	ELEV.+49					
291	37	0	0					35	ELEV.+49					
292	38	25.05	25.05					0	ELEV.+84					

CREST OFFSHORE, INC.

Sheet 6.23 of FILE

By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-15-76 Job No. 21-701-92 Calculation L=50'

JOB NO. 27-771-00

U.S. NAVY TEMPLET DESIGN - 84.EI.PWL

100 YEAR STORM WAVE - PRESSURE PROFILES (PSP) - 4 PILE - THETA = 45 DEG

PRELIMINARY DESIGN - ATKINSON - APRIL 1976

JOB NO. 27-771-00

STRUCTURE

INPUT UNITS
...ENGLISH

OUTPUT UNITS
...ENGLISH

CREST OFFSHORE, INC.

Sheet 6.4 of 4 FILE

By WVA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-15-92 Job No. 27-771-92 Calculation LE 50

*** COEFFICIENT TABLE REPORT ***

DIAMETER IN	NORMAL DRAG COEF	TANG DRAG COEF	MASS COEF
14,000	1.0000	0.0000	1.0000
40,000	1.0000	0.0000	1.0000

CREST OFFSHORE, INC.

Sheet 6.2 of 5 PLE

By KWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-15-76 Job No. 27-771-92 Calculation L-50

**** DEAD LOAD REPORT ****

MEAN WATER DEPTH = 123,500 FT

LOAD CONDITION 1

STRUCTURE DEAD LOAD = 535,075 KIPS
- 15,125 (WISDOMS)
520,950
522.5 K

CREST OFFSHORE, INC.

Sheet 6 of 1 FILE

By JWA Client U.S. NAVY Subject STRUCTURE CONCEPT ANALYSIS
 Date 4-18-72 Job No. 27-771-72 Calculation L=50'

*** WAVE POSITION SUMMARY REPORT ***

LOAD CONDITION: 1 WAVE ANGLE = 45.00

TRIAL NO.	DIST. TO CREST FT.	PHASE ANGLE TO SYSTRUC. (DEG)	HEAVE			MUDLINE MOMENT			VERTICAL FORCE		
			X	Y	Z	FT-KIPS	Y	Z	KIPS	Y	Z
1	24.0	15.07	445.6	845.6	1195.9	63649.	63649.	90013.	90013.	5.3	5.3
2	20.0	10.89	803.2	803.2	1220.7	65948.	65948.	93264.	93264.	8.7	8.7
3	16.0	8.71	871.1	671.1	1231.9	67112.	67112.	94911.	94911.	7.9	7.9
4	12.0	6.54	883.8	683.8	1249.9	68964.	68964.	97529.	97529.	8.7	8.7
5	8.0	4.36	885.2	685.2	1251.9	69404.	69404.	98151.	98151.	8.9	8.9
6	4.0	2.18	882.5	682.5	1248.0	69594.	69594.	98421.	98421.	9.6	9.6
7	0.0	0.00	874.0	674.0	1236.8	69181.	69181.	97837.	97837.	9.7	9.7
8	4.0	2.18	858.1	658.1	1213.5	67789.	67789.	95869.	95869.	7.5	7.5
9	8.0	4.36	838.7	638.7	1166.2	66091.	66091.	93467.	93467.	5.9	5.9
10	12.0	6.54	814.8	614.8	1152.4	63943.	63943.	90429.	90429.	2.9	2.9
11	16.0	8.71	780.7	580.7	1104.0	60536.	60536.	85610.	85610.	1.3	1.3

CREST OFFSHORE, INC.

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-15-72 Job No. 27-771-92 Calculation L = 50'

*** LOAD SUMMARY REPORT ***

WAVE NUMBER = 1 WAVE DIRECTION = 45.000

X SHEAR FORCE = 832,4620 KIPS
Y SHEAR FORCE = 882,4620 KIPS

RESULTANT SHEAR FORCE = 1247,9897 KIPS

X HUDLINE MOMENT = 69594,1543 FT-KIPS
Y HUDLINE MOMENT = 69594,1543 FT-KIPS

RESULTANT HUDLINE MOMENT = 98470,9969 FT-KIPS

Z VERTICAL FORCE = 7,5592 KIPS

SECTION 7

60 FT JACKET BASE SPACING

7.1 INTRODUCTION

The moments and horizontal forces from Section 6 (50 FT JACKET BASE) indicated that the piling would be too long for the pile driving hammer we originally chose. A SEALOAD run was made by quickly changing the joint coordinates to model a structure with a jacket base spring of 60 ft. With the results of that run it was determined the 60 ft jacket base would not appreciably reduce the piling lengths and the decision was made to keep the 50 ft jacket base structure and increase the pile driving hammer size. See report of the three-pile structure (Appendix C of 27-771-92 Report) for calculations and discussion of the pile driving hammer.

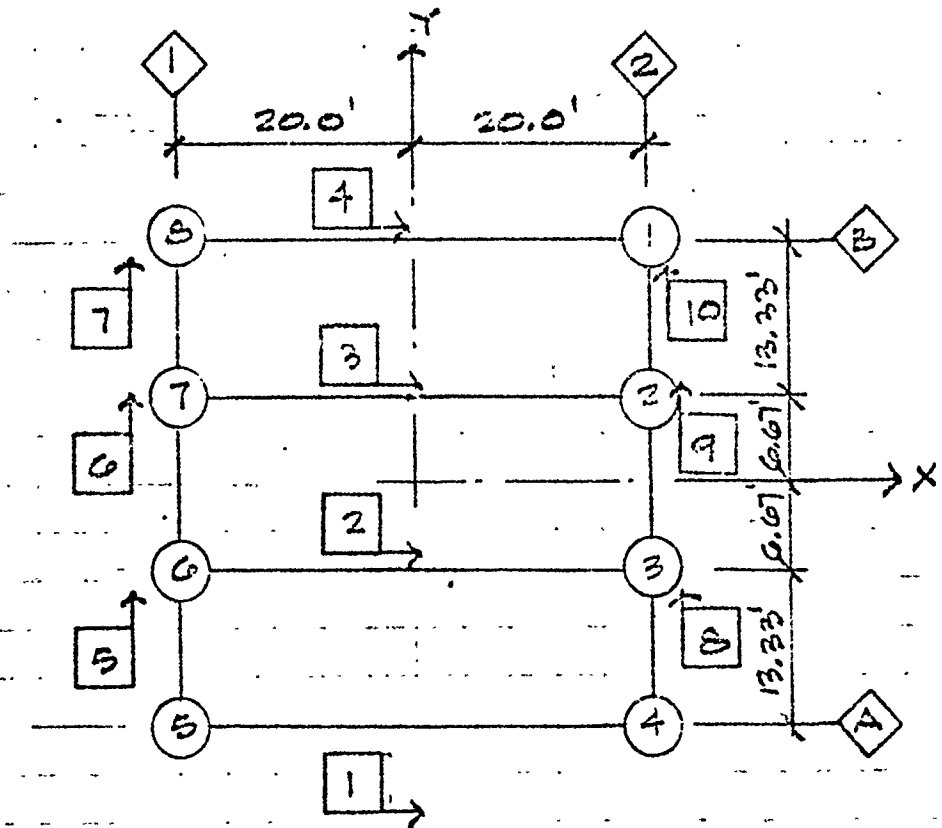
Section 7.2 shows the plan view only of the computer model as the elevations will remain the same as the Section 6, 50 ft Jacket Base model.

Section 7.3 shows wave direction and wind loading.

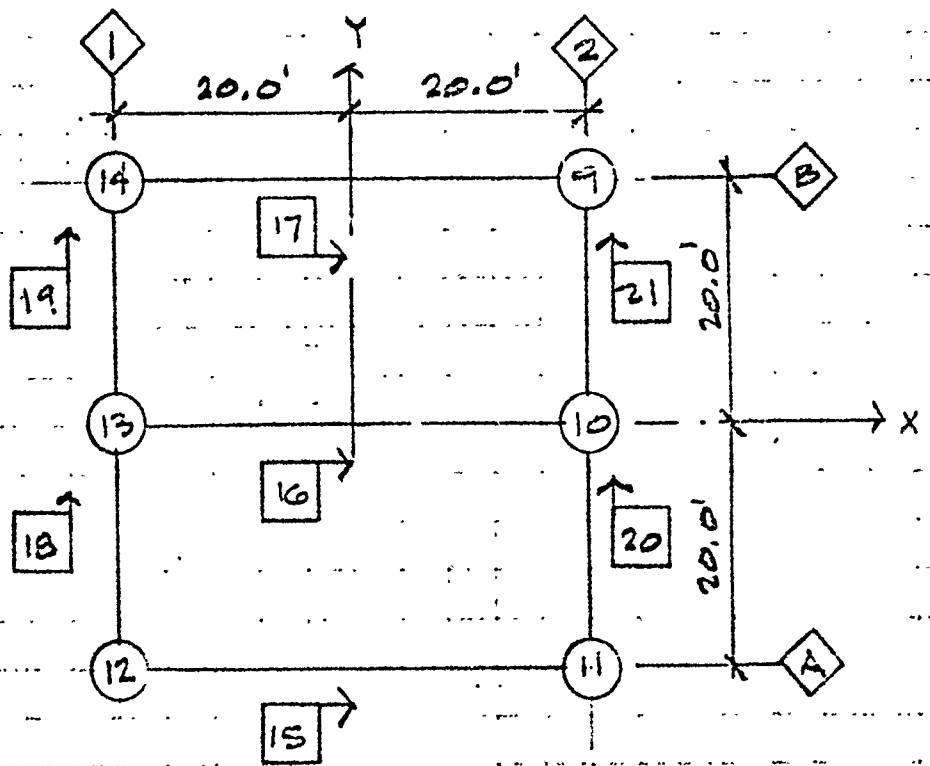
Section 7.4 shows the results of the SEALOAD run for this structure.

By VVA Client CFR BY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-20-70 Job No. 27-1-92 Calculation 60' JACKET BASE

7.2



ELEV. (+) 75.0 (+130)

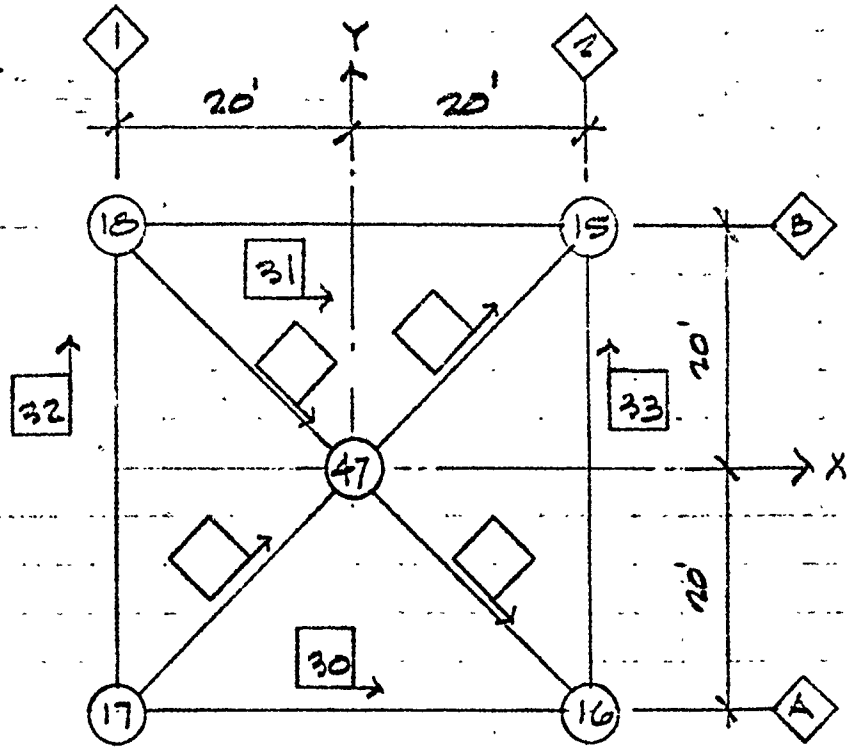


ELEV. (+) 60 (+144)

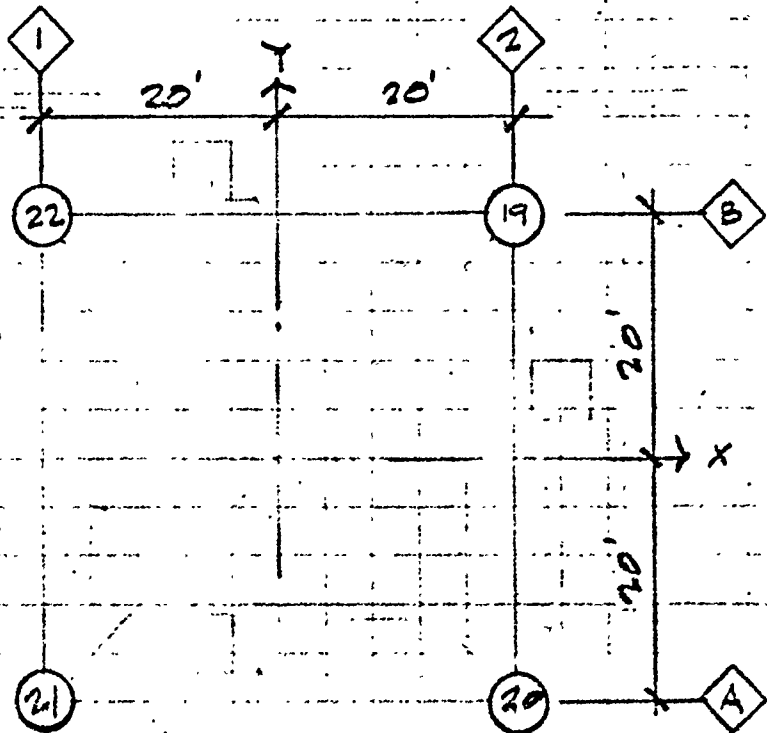
CREST OFFSHORE, INC.

Sheet 7.03 of 4 NLE

By JMA Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 11-20-92 Job No. 21-111-92 Calculation 60' JACKET BASE

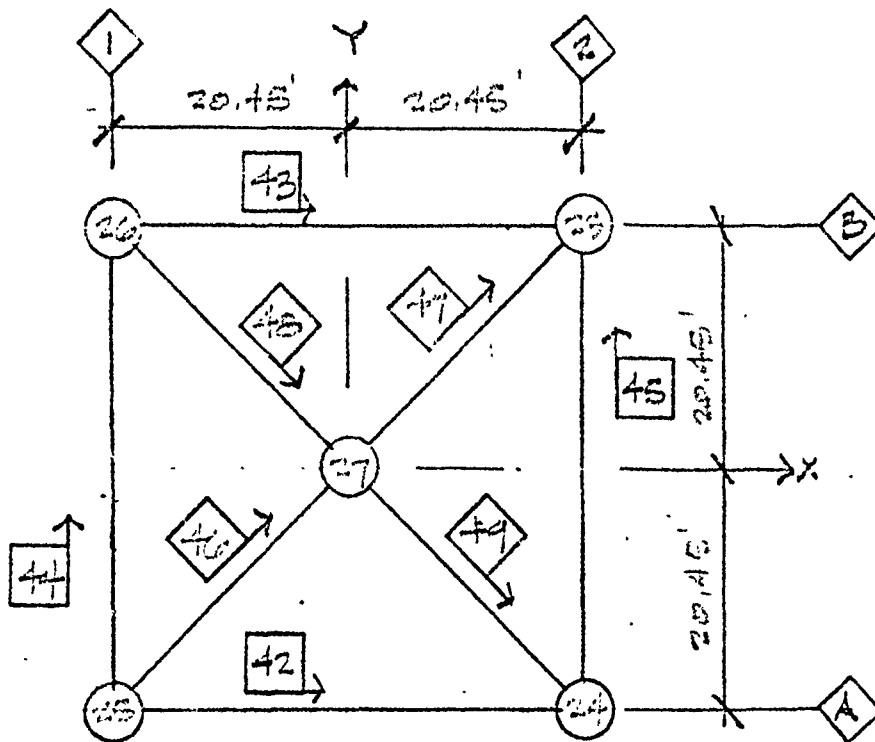


ELEV. (+) 46' (130)

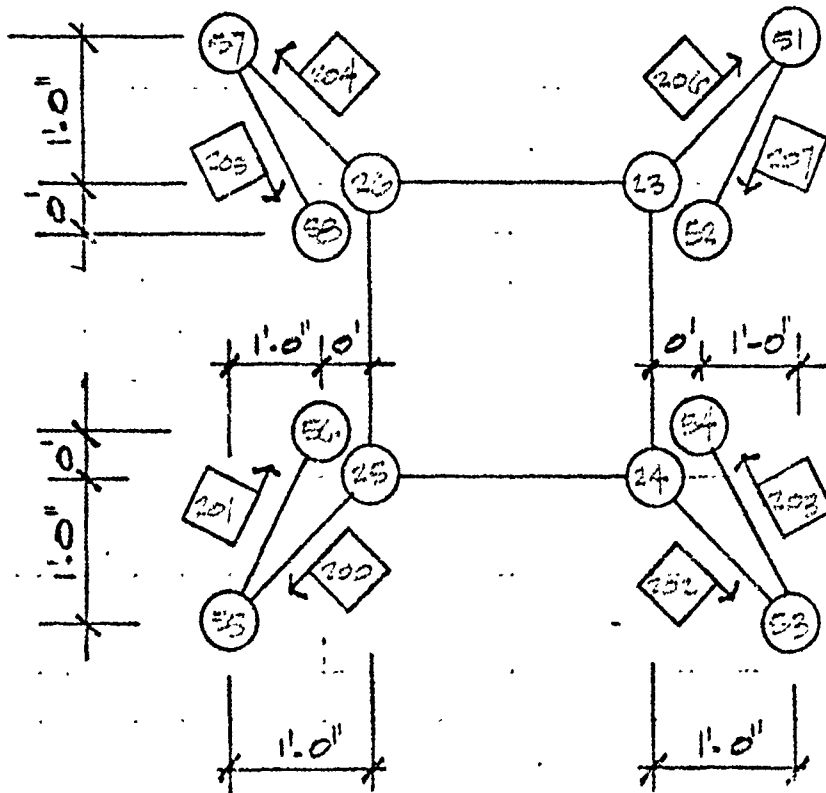


ELEV. (+) 6.5' (100.5)

By JMS Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-10-92 Job No. 21-711-92 Calculation STRUCTURE 4/L=50'



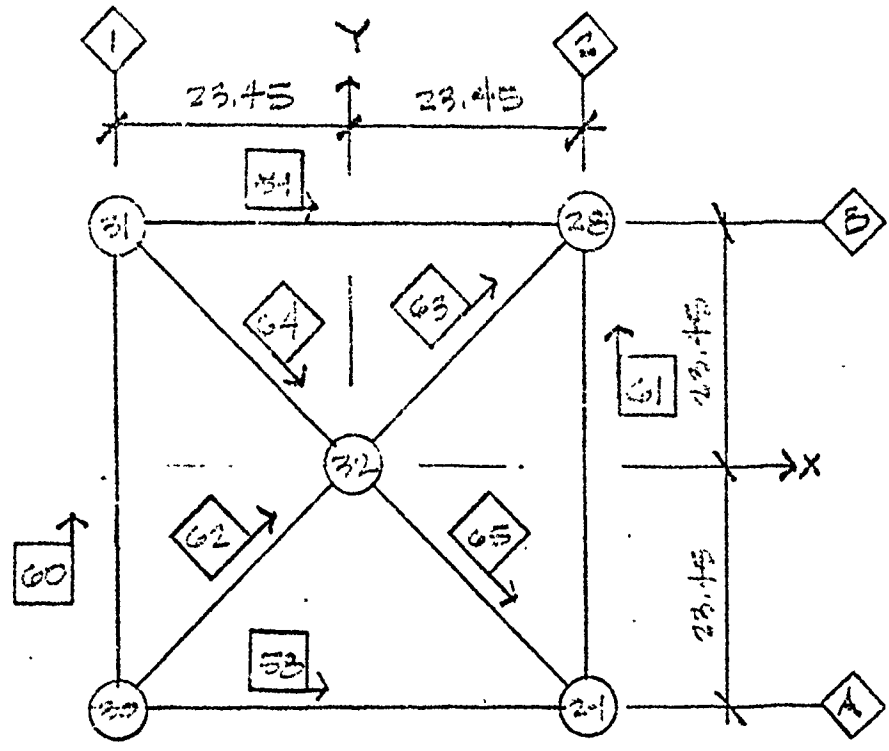
ELEV. (+) 12 (96)



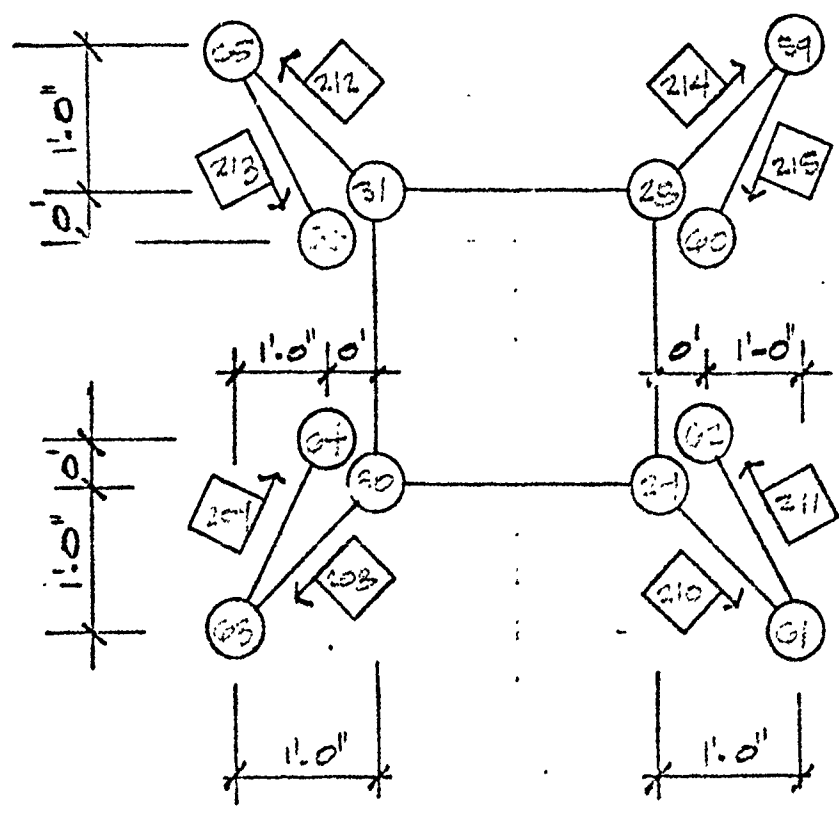
CREST OFFSHORE, INC.

Sheet 7.02 of 4 PLS

By JMS Client US. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.25.76 Job No. 21-771-92 Calculation STRUCTURE Y/L=60



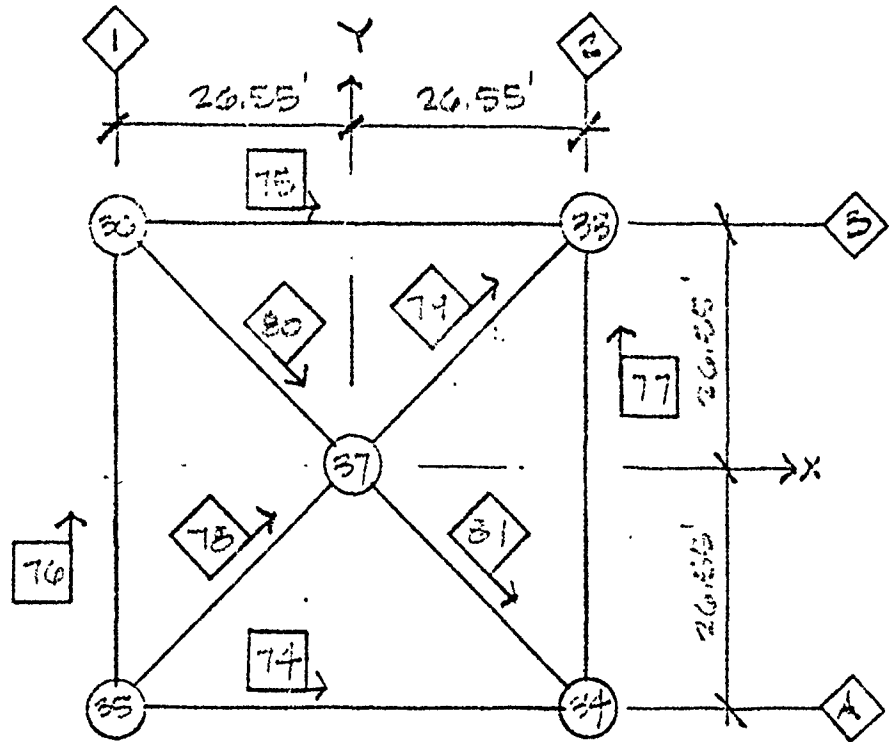
ELEV. (-) B (60)



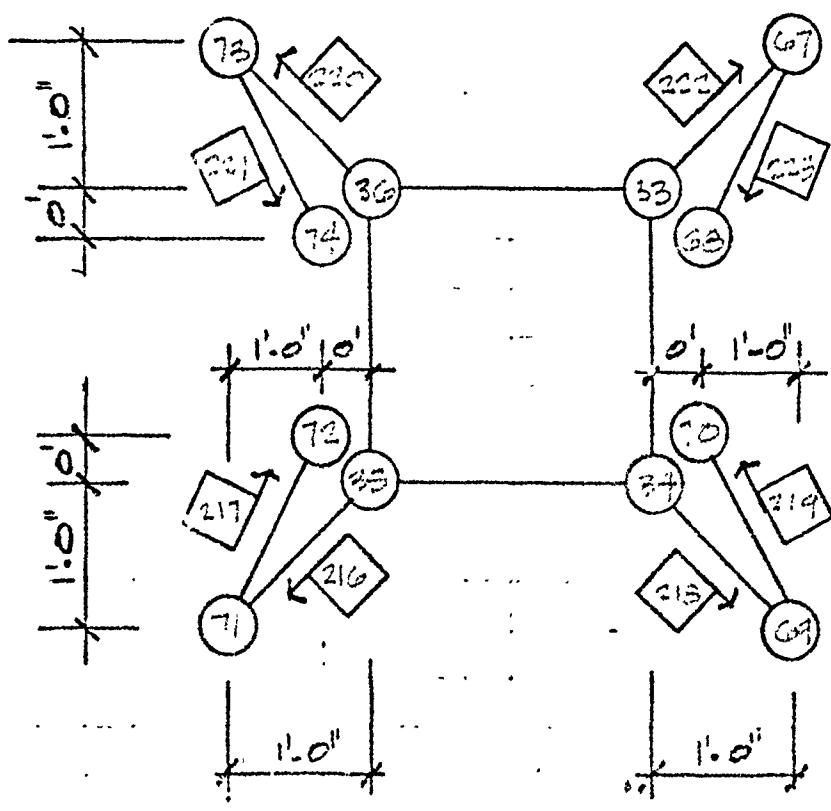
CREST OFFSHORE, INC.

Sheet 7.00 of 4 P1.2

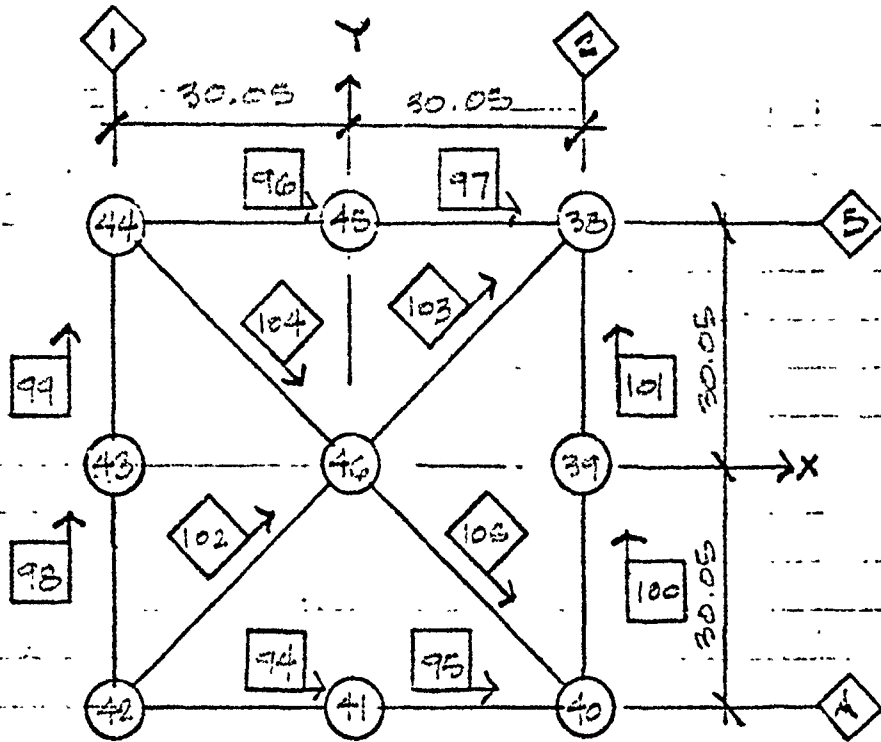
By JMS Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-28-78 Job No. 21-711-92 Calculation STRUCTURE W/ L=60'



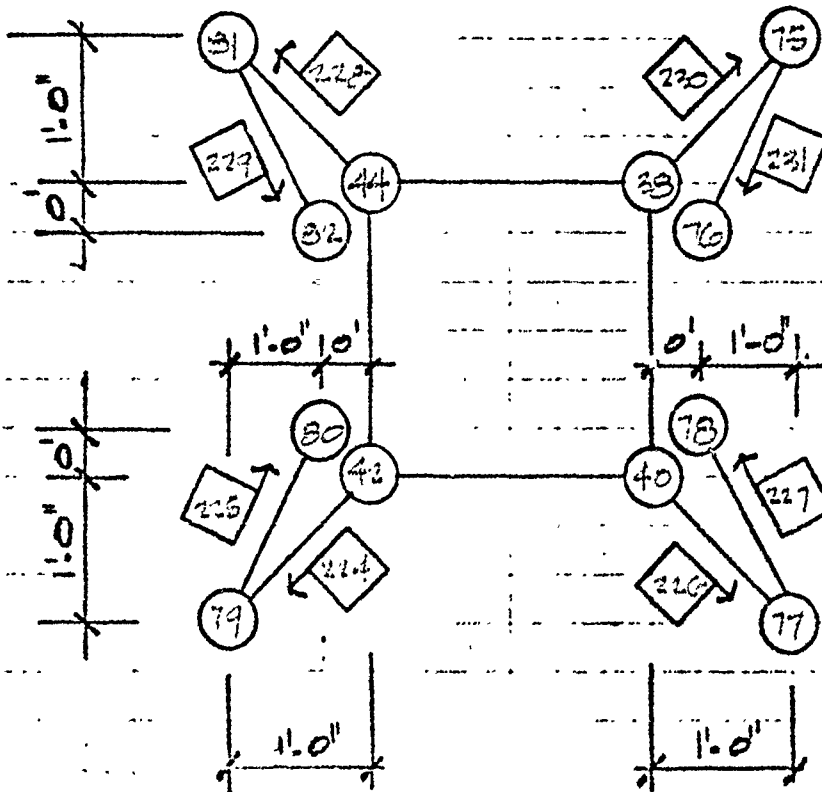
ELEV. (-) 49 (35)



By KMS Client US. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 12-22-92 Job No. 21-771-92 Calculation STRUCTURE 4/L = 60'



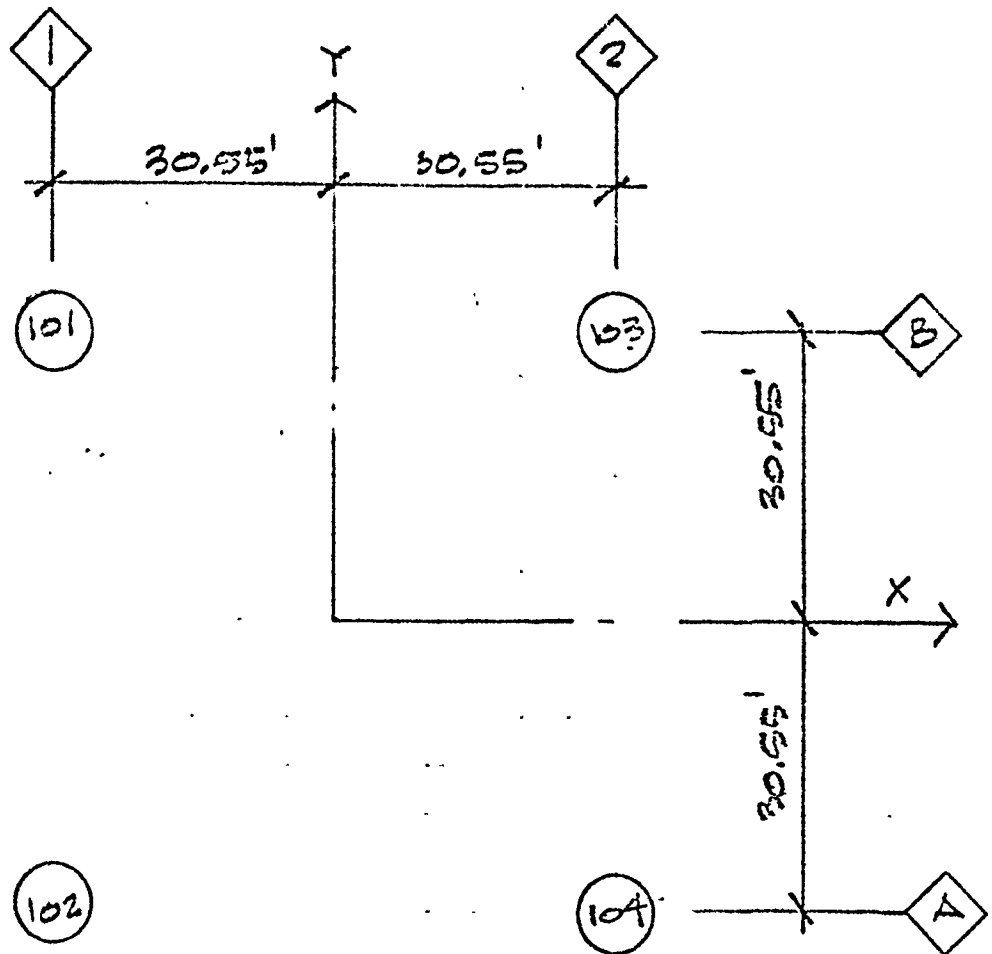
ELEV. (-) 84 (0)



CREST OFFSHORE, INC.

Sheet 103 of 4 PLS

By JWS Client U.S. Navy Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-20-70 Job No. 27-771-92 Calculation STRUCTURE/L100'

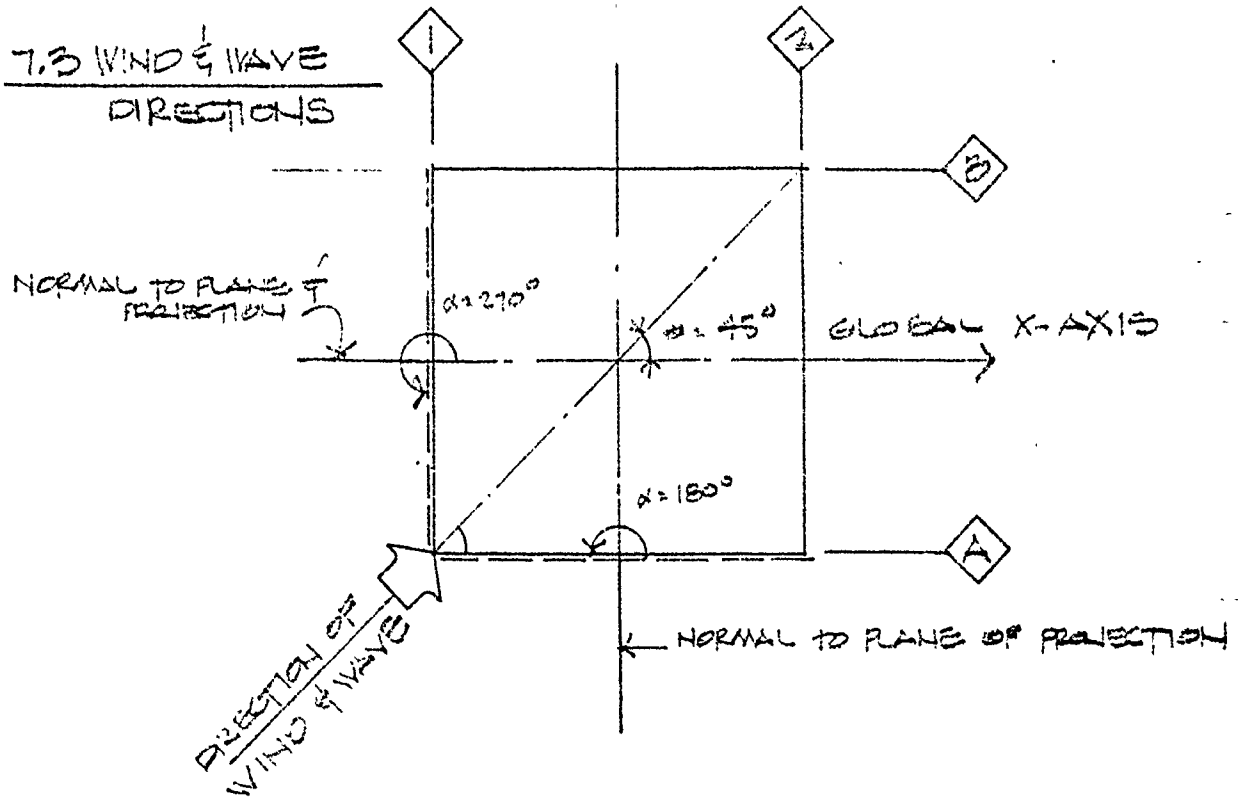


ELEV. (-) 89 (-5)

CREST OFFSHORE, INC.

Sheet 1.09 of 4.00

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-20-76 Job No. 21-771-92 Calculation LOGO



WIND AREAS

FENCE = $50' \times 2' = 100 \text{ S.F.}$

AREA BETWEEN DECKS = $50' \times 15' \times 0.5 = 375 \text{ S.F.}$

CREST OFFSHORE, INC.

Sheet 7.10 of 4 PILE

By WKS Client U.S. NAVY Subject SPURIAL CONCEPT ANALYSIS
Date 4-21-76 Job No. 27-771-92 Calculation L = 60

U.S. NAVY TEMPLET DESIGN - 84 FT MxL JOB NO. 27-771-00
100 YEAR STORM WAVE - PRESSURE PROFILES (PSF) 4 PILE THETA = 45 DEG
PRELIMINARY DESIGN ATKINSON APRIL 1976
STRUCTURE JOB NO. 27-771-00

INPUT UNITS
....ENGLISH

OUTPUT UNITS
....ENGLISH

CREST OFFSHORE, INC.

Sheet 7.11 of 4 PILE

By WYS Client CREST Subject STRUCTURAL CONCEPT ANALYSIS
Date 10/1/92 Job No. 27-71-92 Calculation L=00

PILE COEFFICIENT TABLE REPORT

MASS COEF

1.0000
1.0000

TANG DRAG COEF

0.0000
0.0000

NORMAL DRAG COEF

1.0000
1.0000

DIAMETER
IN

14.000
40.000

CREST OFFSHORE, INC.

Sheet 712 of 4-PILE

By KWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-21-76 Job No. 2J-771-92 Calculation L-60

*** DEAD LOAD REPORT ***

LOAD CONDITION: MEAN WATER DEPTH 123,500 FT

STRUCTURE DEAD LOAD: 573,848 KIPS
+ 13,402
560,449
less with base loads = 242 x 2.83 x 148 #/1

CREST OFFSHORE, INC.

Sheet 103 of 105

By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 1-21-76 Job No. 27-711-92 Calculation L=00'

*** WAVE POSITION SUMMARY REPORT ***

WAVE ANGLE = 45.00

LOAD CONDITION 1

TRIAL NO.	DIST. TO CREST FT	PHASE ANG. WAVE TO STRUC. (DEG)	SHEAR FORCE KIPS		BENDING MOMENT FT-KIPS		VERTICAL FORCE KIPS	
			X	Y	X	Y	X	Y
1	-24.0	13.07	891.0	891.0	-68227.	68227.	96488.	3.6
2	-20.0	10.89	899.7	899.7	-69402.	69402.	98149.	-4.6
3	-16.0	8.71	911.5	911.5	-71037.	71037.	100462.	5.3
4	-12.0	6.54	921.4	921.4	-72556.	72556.	102609.	-5.6
5	-8.0	4.36	926.7	926.7	-73556.	73556.	104025.	6.1
6	-4.0	2.18	921.4	921.4	-73418.	73418.	103829.	-5.7
7	0.0	0.00	914.0	914.0	-73093.	73093.	103369.	5.2
8	4.0	-2.18	894.8	894.8	-71325.	71325.	100869.	-2.2
9	8.0	-4.36	871.9	871.9	-69126.	69126.	97759.	2.2
10	12.0	-6.54	844.2	844.2	-66536.	66536.	94097.	5.3
11	16.0	-8.71	818.3	818.3	-64243.	64243.	90854.	5.3

CREST OFFSHORE, INC.

Sheet 7 of 11

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 1-21-70 Job No. 27-771-72 Calculation L-001

APPENDIX A - SUMMARY REPORT

WAVE DIRECTION = 45.000

WAVE NUMBER = 1

X SHEAR FORCE = 926.6732 KIPS

Y SHEAR FORCE = 926.6732 KIPS

RESULTANT SHEAR FORCE = 1310.5139 KIPS

X MUDLINE MOMENT = 73556.4324 FT-KIPS

Y MUDLINE MOMENT = 73556.4324 FT-KIPS

RESULTANT MUDLINE MOMENT = 104024.5043 FT-KIPS

Z VERTICAL FORCE = 6.0790 KIPS

SECTION 8
BOATLANDING

8.1 INTRODUCTION

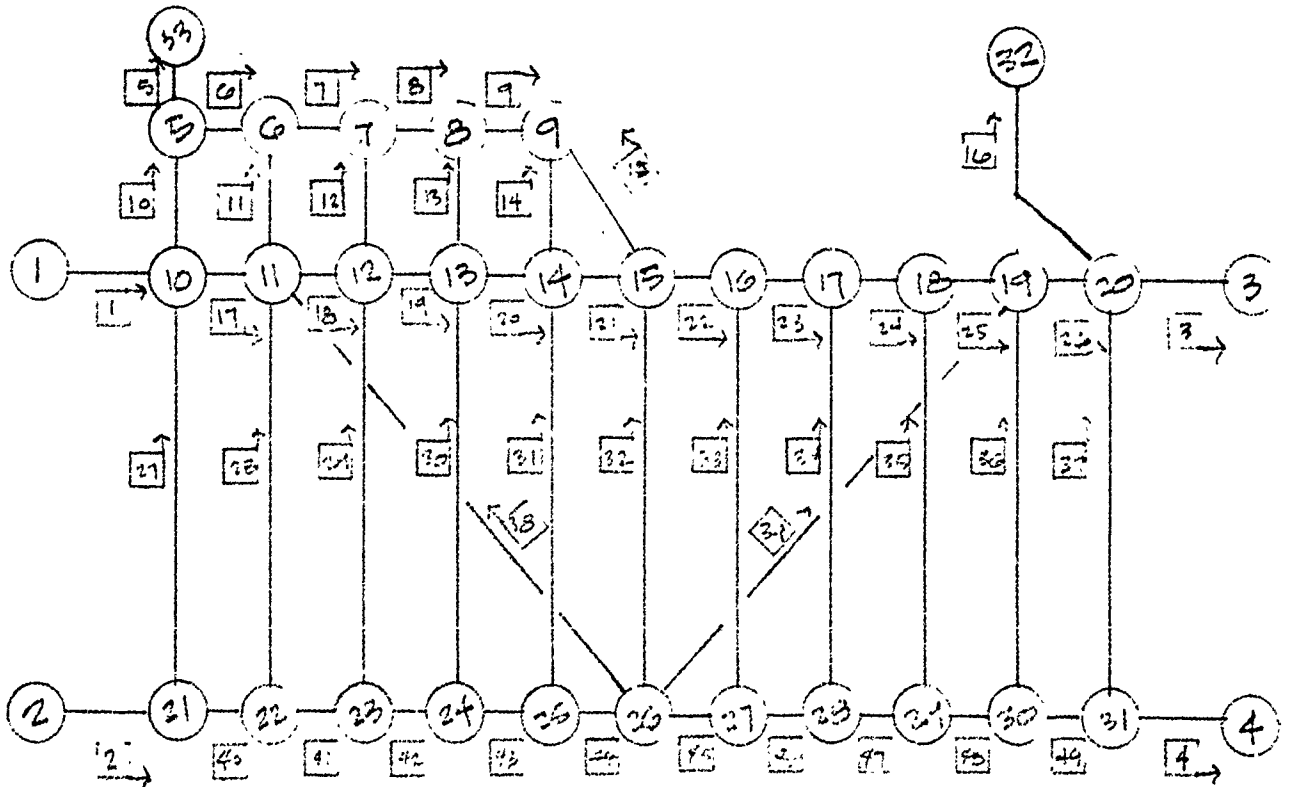
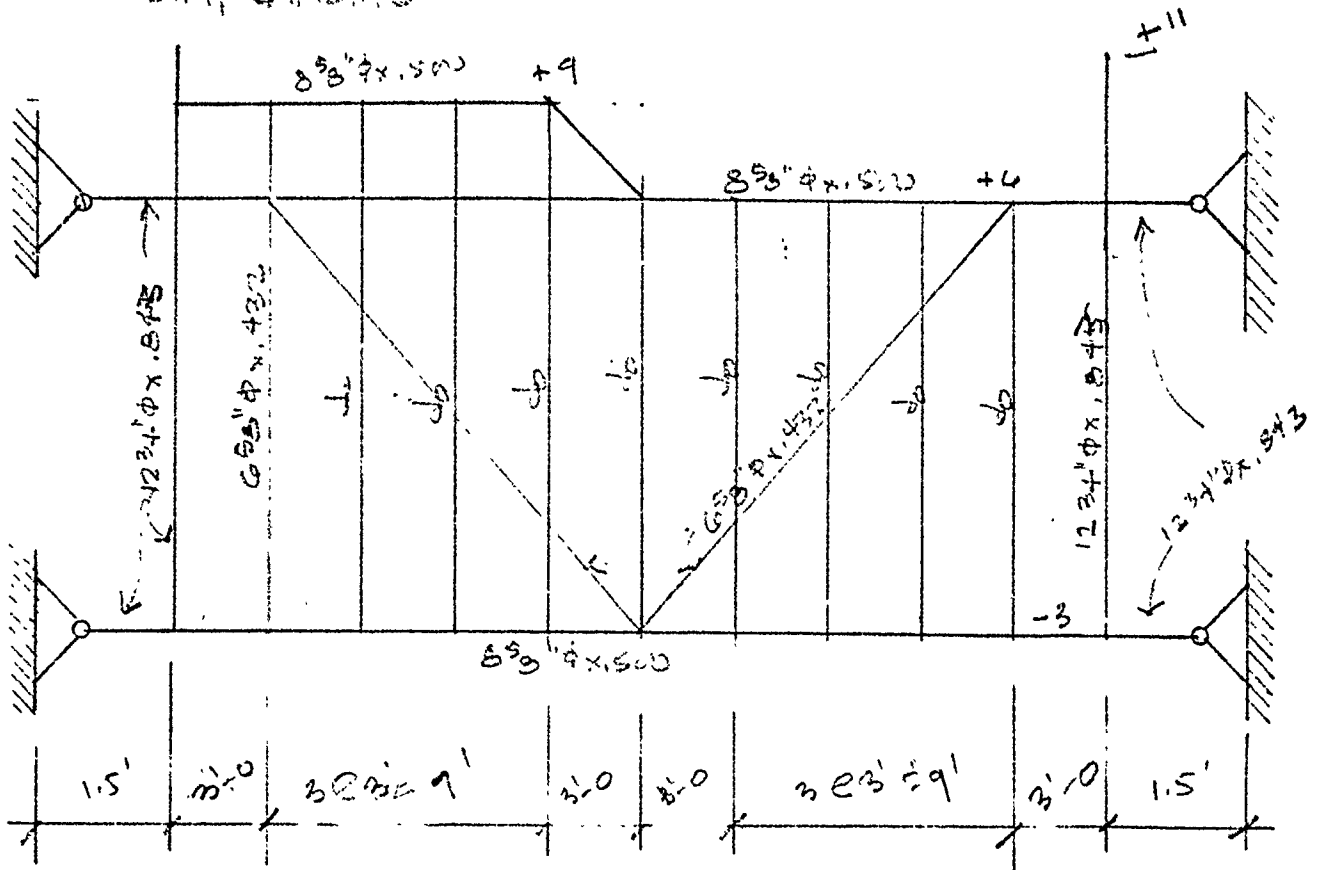
The boat landing was modeled as a separate structure and a SEALOAD run was made using the same wave and wind characteristics as those specified previously. The boat landing was attached to the trestle between Rows '1' and '2' at elevations +11 ft and -3 ft from MLW. This section shows the idealized model for the boat landing structure and the results of the SEALOAD program run. For simplicity loading conditions for the three-pile structure (see Report 27-771-92 Appendix C) were also made with this run. Loading #1 is for wind and wave at a 45° angle to the boat landing to be used with the four-pile structure. Loading #2 is for wind and wave normal to the boatlanding and #3 in 180° from #2, both to be used with the three-pile structure.

CREST OFFSHORE, INC.

Sheet B.02 of 4 PLE

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.13.76 Job No. 21-771-92 Calculation BOAT LANDING

BOAT LANDING



CREST OFFSHORE, INC.

Sheet 3.03 of 4 FILE

By KM Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date _____ Job No. 27-771-92 Calculation POSTLANDING

U.S. NAVY TEMPLET DESIGN - 64 FT MWL JOB NO. 27-771-00
100 YEAR STORM WAVE - PRESSURE PROFILES (PSP) & PILE THETA - 45DEG
PRELIMINARY DESIGN ATKINSON APRIL 1976

BOAT LANDING

INPUT UNITS, ENGLISH

OUTPUT UNITS, ENGLISH

CREST OFFSHORE, INC.

Sheet 3 of 4 FILE

By WWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date _____ Job No. 27-771-92 Calculation POSTLANSING

*** WAVE POSITION SUMMARY REPORT ***

LOAD CONDITION 1

WAVE ANGLE = 45.00

TRIAL NO.	DIST. TO CREST FT	PHASE ANGLE WAVE TO STRUC. (DEG)	HEAVE		HEAVE RATE		HEAVE ACCELERATION		HEADLINE MOMENT		VERTICAL FORCE	
			X	Y	X	Y	X	Y	FT-KIPS	Y	KIPS	Z
1	24.0	13.07	19.1	34.4	39.3	-2968.	1657.	3416.	1637.	3373.	3	3
2	16.0	8.71	16.8	33.9	38.8	-2950.	1637.	3373.	1637.	3373.	3	3
3	8.0	4.36	14.3	33.0	37.8	-2869.	1594.	3282.	1525.	3199.	2	2
4	0.0	-0.00	17.5	31.6	36.1	-2744.	1525.	3199.	1525.	3199.	2	2
5	0.0	-4.36	16.5	29.7	34.0	-2580.	1435.	2952.	1435.	2952.	2	2
6	16.0	-8.71	15.2	27.4	31.3	-2377.	1323.	2729.	1323.	2729.	1	1

CREST OFFSHORE, INC.

Sheet 005 of 4 FILE

By VWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date _____ Job No. 21-711-72 Calculation POSTLANDING

*** LOAD SUMMARY REPORT ***

WAVE NUMBER # 1 WAVE DIRECTION # 45.000

X SHEAR FORCE # 10.0904 KIPS

Y SHEAR FORCE # 34.3700 KIPS

RESULTANT SHEAR FORCE # 39.2965 KIPS

X MUDLINE MOMENT # -2987.5663 FT-KIPS

Y MUDLINE MOMENT # 1656.9426 FT-KIPS

RESULTANT MUDLINE MOMENT # 3416.2862 FT-KIPS

Z VERTICAL FORCE # 2770 KIPS

CREST OFFSHORE, INC.

Sheet 2 of 4 FILE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-15-72 Job No. 27-771-92 Calculation BOAT LANDING

*** WAVE POSITION SUMMARY REPORT ***

LOAD CONDITION 2		WAVE ANGLE = 90.00		VERTICAL FORCE KIPS		
TRIAL NO.	DIST. TO CREST FT	PHASE ANGLE TO STRUC. (DEG)	H X Y KIPS	MOMENT X Y FT-KIPS	RSLNT	
1	20.0	10.89	0.0	48.4	48.4	4206.
2	15.0	6.17	0.0	47.6	47.6	4141.
3	10.0	5.45	0.0	46.5	46.5	4042.
4	5.0	2.72	0.0	45.2	45.2	3932.
5	0.0	-0.00	0.0	43.7	43.7	3800.
6	5.0	-2.72	0.0	41.9	41.9	3646.
7	10.0	-5.45	0.0	40.0	40.0	3476.

CREST OFFSHORE, INC.

Sheet 907 of 4 PAGES

By JWS Client US NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 1-15-70 Job No. 27-71-92 Calculation POST LANDING

**** LOAD SUMMARY REPORT ****

WAVE NUMBER = 2

WAVE DIRECTION = 70.000

X SHEAR FORCE = 0.0000 KIPS
Y SHEAR FORCE = 48.3778 KIPS
RESULTANT SHEAR FORCE = 48.3778 KIPS

X MUDLINE MOMENT = 4205.5219 FT-KIPS
Y MUDLINE MOMENT = 0.0000 FT-KIPS
RESULTANT MUDLINE MOMENT = 4205.5219 FT-KIPS

Z VERTICAL FORCE = 0.0000 KIPS

CREST OFFSHORE, INC.

Sheet 8.08 of 4 PLE

By JMS Client USNAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-15-76 Job No. 27-771-92 Calculation POSTLANDING

WAVE POSITION SUMMARY REPORT

WAVE ANGLE = 270.00

LOAD CONDITION 3

TRIAL NO.	DIST. TO CREST FT	PHASE ANG. WAVE TO STRUC. (DEG)	H E A R		MUDLINE MOMENT		RSLNT		VERTICAL FORCE KIPS
			X	Y	X	Y	X	Y	
1	20.0	10.89	0.0	46.4	4033.	0.	4033.	0.0	
2	15.0	8.17	0.0	47.5	4132.	0.	4132.	0.0	
3	10.0	5.45	0.0	48.4	4208.	0.	4208.	0.0	
4	5.0	2.72	0.0	48.7	4236.	0.	4236.	0.0	
5	0.0	-0.00	0.0	48.9	4249.	0.	4249.	0.0	
6	5.0	-2.72	0.0	48.8	4245.	0.	4245.	0.0	
7	10.0	-5.45	0.0	48.4	4206.	0.	4206.	0.0	

CREST OFFSHORE, INC.

Sheet 809 of 4 PLE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-15-72 Job No. 27-77L-92 Calculation POSTLANDING

LOAD SUMMARY REPORT ***

WAVE DIRECTION = 270.000

WAVE NUMBER = 3

X SHEAR FORCE # 0.0000 KIPS

Y SHEAR FORCE # 48.8782 KIPS

RESULTANT SHEAR FORCE # 48.8782 KIPS

X MUDLINE MOMENT # 4248.9737 FT-KIPS

Y MUDLINE MOMENT # 0.0000 FT-KIPS

RESULTANT MUDLINE MOMENT # 4248.9737 FT-KIPS

Z VERTICAL FORCE # 0.0000 KIPS

SECTION 9

STAIRWAY

9.1 INTRODUCTION

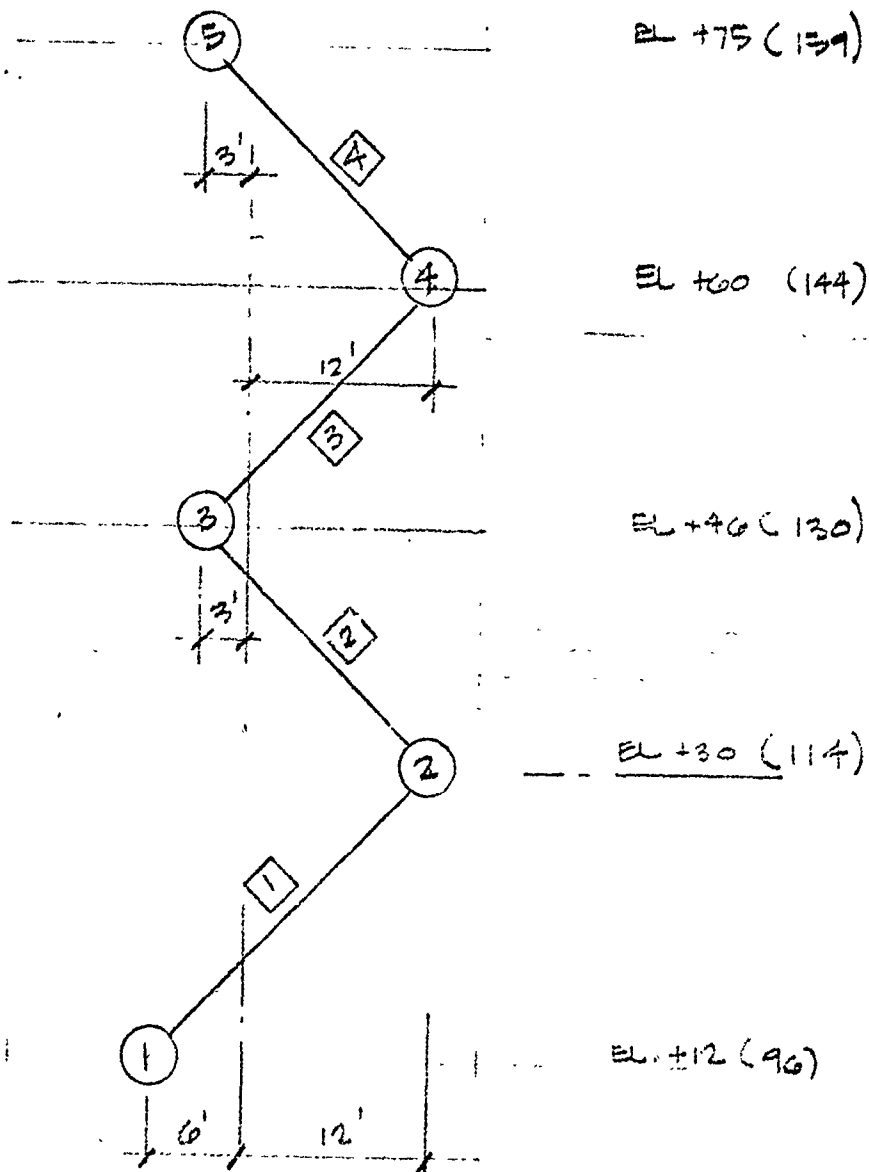
The stairway was modeled as a separate structure also. The stairway was attached to the superstructure legs between Rows '1' and '2' at elevations +11 ft and +75 ft from MLW.

This section shows the idealized model for the stairway structure and the results of the SEALOAD run. Loading #1 is for the four-pile structures while loadings #2 and #3 are to be used with the three-pile structure.

CREST OFFSHORE, INC.

Sheet 902 of 4 FILE

By WAS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-19-76 Job No. 27-77-92 Calculation STAIRWAY



CREST OFFSHORE, INC.

Sheet 9.03 of 4 FILE

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-19-76 Job No. 21-776-92 Calculation STAIRWAY

U.S. NAVY TEMPLET DESIGN - 64 FT MWL JOB NO. 27-771-00

100 YEAR STORM WAVE + PRESSURE PROFILES (SP) 4 PILE THETA = 45 DEG

PRELIMINARY DESIGN ATKINSON APRIL 1976

STAIRWAY

INPUT UNITS
.....ENGLISH

OUTPUT UNITS
.....ENGLISH

5 0 3 5 3

CREST OFFSHORE, INC.

Sheet 9 of 4 PLE

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-19-76 Job No. 27-77-92 Calculation STEELWAY

***** WAVE POSITION SUMMARY REPORT *****

WAVE ANGLE = 45.00

LOAD CONDITION 1

TRIAL NO.	DIST. TO CREST PT	PHASE ANG. WAVE TO STRUC. (DEG)	HEADING AREA		MUDLINE MOMENT		VERTICAL FORCE	
			X	Y	FT-KIPS	RSQNT	X	Y
1	24.0	13.07	26.2	32.3	3217.	7187.	4.1	
2	16.0	8.71	27.6	55.2	3406.	7634.	5.6	
3	8.0	4.36	28.0	56.1	3467.	7790.	6.3	
4	0.0	-0.99	27.0	54.2	3351.	7531.	6.0	
5	8.0	-4.36	23.9	47.8	2946.	6590.	3.9	
6	16.0	-8.71	20.3	40.4	2485.	5530.	1.7	

CREST OFFSHORE, INC.

Sheet 105 of 4 PLS

By JMS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-19-76 Job No. 27-771-92 Calculation STAIRWAY

WAVE LOAD SUMMARY REPORT ****

WAVE DIRECTION = 45.000

WAVE NUMBER = 1

X SHEAR FORCE # 27,9532 KIPS

Y SHEAR FORCE # 56,0893 KIPS

RESULTANT SHEAR FORCE # 62,6889 KIPS

X MUDLINE MOMENT # -6979,5942 FT-KIPS

Y MUDLINE MOMENT # 3467,1854 FT-KIPS

RESULTANT MUDLINE MOMENT # 7789,7554 FT-KIPS

Z VERTICAL FORCE # -6,2904 KIPS

CREST OFFSHORE, INC.

Sheet 1.06 of 4 PLE

By JWD Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-15-76 Job No. 27-771-92 Calculation SAIRWAY

***** WAVE POSITION SUMMARY REPORT *****

LOAD CONDITION 2 WAVE ANGLE B 90.00

TRIAL NO.	DIST. TO CREST FT	PHASE ANGLE (DEG)	B HEAVE		MUDLINE MOMENT		VERTICAL FORCE	
			X KIPS	Y KIPS	X FT-KIPS	Y FT-KIPS	Z KIPS	Z KIPS
1	20.0	10.89	0.0	80.7	10025.	0.	10025.	0.0
2	15.0	6.17	0.0	81.2	10128.	0.	10128.	0.0
3	10.0	5.45	0.0	77.9	9694.	0.	9694.	0.0
4	5.0	2.72	0.0	74.0	9182.	0.	9182.	0.0
5	0.0	-0.00	0.0	68.1	8403.	0.	8403.	0.0
6	9.0	-2.72	0.0	62.4	7661.	0.	7661.	0.0
7	10.0	-5.45	0.0	56.8	6952.	0.	6952.	0.0

CREST OFFSHORE, INC.

Sheet 907 of 4 PLU

By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-15-70 Job No. 23-771-92 Calculation STABILITY

*** LOAD SUMMARY REPORT ***

WAVE NUMBER # 2

HAVE DIRECTION = 90.000

X SHEAR FORCE # 0.0000 KIPS
Y SHEAR FORCE # 81.2343 KIPS
RESULTANT SHEAR FORCE # 81.2343 KIPS

X HUDLINE MOMENT # -10128.1059 FT-KIPS
Y HUDLINE MOMENT # 0.0000 FT-KIPS
RESULTANT HUDLINE MOMENT # 10128.1059 FT-KIPS

Z VERTICAL FORCE # 0.0000 KIPS

CREST OFFSHORE, INC.

Sheet 9.08 of 4 PLE

By WSS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-13-70 Job No. 27-71-92 Calculation STAIRWAY

*** WAVE POSITION SUMMARY REPORT ***

WAVE ANGLE = 270.00

LOAD CONDITION 3

VERTICAL FORCE KIPS

MUDLINE MOMENT FT-KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

REACT. KIPS

TRIAL NO.	DIST. TO CREST FT	PHASE ANG. WAVES TO STRUC. (DEG)	X	Y	Z	REACT. X	REACT. Y	REACT. Z	MUDLINE MOMENT	VERTICAL FORCE
1	20.0	10.89	0.0	48.2	0.0	5772.	48.2	0.0	5772.	0.0
2	15.0	6.17	0.0	61.9	0.0	7502.	61.9	0.0	7502.	0.0
3	10.0	5.45	0.0	67.0	0.0	8162.	67.0	0.0	8162.	0.0
4	5.0	2.72	0.0	71.1	0.0	8703.	71.1	0.0	8703.	0.0
5	0.0	0.00	0.0	75.1	0.0	9249.	75.1	0.0	9249.	0.0
6	5.0	-2.72	0.0	79.2	0.0	9810.	79.2	0.0	9810.	0.0
7	10.0	-5.45	0.0	80.7	0.0	10028.	80.7	0.0	10028.	0.0

CREST OFFSHORE, INC.

Sheet 109 of 5 PILE

By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-15-76 Job No. 27-77692 Calculation SWAYWAY

*** LOAD SUMMARY REPORT ***

WAVE DIRECTION = 270.000

WAVE NUMBER = 3

X SHEAR FORCE = 0.0000 KIPS

Y SHEAR FORCE = 80.6303 KIPS

RESULTANT SHEAR FORCE = 80.6303 KIPS

X MUDLINE MOMENT = 10027.6899 FT-KIPS

Y MUDLINE MOMENT = 0.0000 FT-KIPS

RESULTANT MUDLINE MOMENT = 10027.6899 FT-KIPS

Z VERTICAL FORCE = 0.0000 KIPS

SECTION 10

LATERALLY LOADED PILE CAPACITY

10.1 INTRODUCTION

Some additional data input into the STRAN Program is "Equivalent Pile" length, area, and moment of inertia of the piling sections below mudline to simulate the supports in a more realistic manner. This is done with the results of the "LATERALLY LOADED PILE" Program* which also checks the stress in the piles.

Section 10.2 computes the pile top restraint and the shear and axial loadings to the pile top from the SEALOAD results. Both the 50 ft and 60 ft jacket base spacings were checked for required pile penetration and it was decided to use the 50 ft jacket base spacing for the final analysis.

Section 10.3 shows soil data, minimum wall thickness required for pile driving hammer in accordance with API Recommendations, pile driving resistance curve, and final thicknesses and lengths of piles chosen.

All information was coded and a LATERALLY LOADED PILE Program run was made. The whole run is shown in Exhibit A with only the results of that run being shown in Section 10.4.

Section 10.5 computes the "Equivalent Pile Properties" to be used in the STRAN run.

* This program is available at Crest Engineering, Inc., Tulsa.

CREST OFFSHORE, INC.

Sheet 10.02 of 4 PLS

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4.22.76 Job No. 21-171-92 Calculation LATERALLY LOADED PILE

10.2 DATA PREPARATION

1. PILE TOP RESTRAINT

$$K = \frac{3.5 EI}{L} \quad \left(= \frac{M}{\theta} \right)$$

WHERE $E = 29,000 \text{ ksi}$

$$I = 24,240.424 \text{ in}^4$$

(for a 36" ϕ x 115" WT PILE)

$$L = 33' = 420 \text{ in}$$

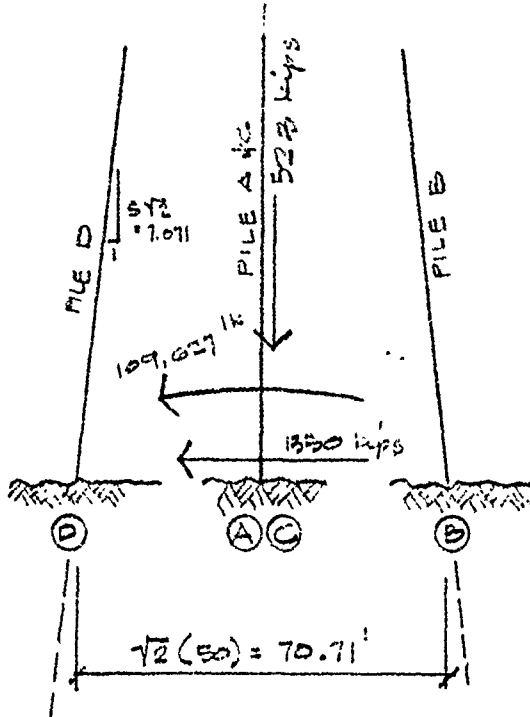
$$\therefore K = \frac{3.5 (29,000) (24,240.424)}{420} \quad \left(\frac{\text{kip} \cdot \text{in}}{\text{in}} = \text{in} \cdot \text{kip} \right)$$

$$= 5,858,102.407 \text{ } 5,858,102 \text{ in} \cdot \text{kip} / \text{rad.}$$

By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-20-76 Job No. 27-171-92 Calculation LATERAL LOADED PILE

LOADINGS AT PILE TOP

$l = 50'$



PERFORMED FROM COMPUTER PROGRAMS

LEAD LOAD = 522.5 k

	V	M
STRUCTURE	1248 k	98421 k
BOAT LANDING	39.3 k	3416.3 k
STAIRS	62.7 k	7789.8 k

TOTALS 1350 k 109,627.1 k

CHECK COMPRESSION!

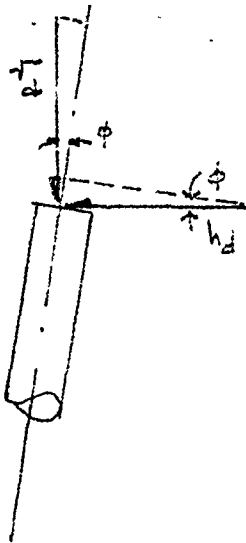
Moment about B axis

$$M = 0 = M_{WUTURN} + M_p - M_{AC} - M_D$$

$$= 109,627 + \frac{522 \times 10.71}{2} - \frac{522 \times 70.71}{2} - 70.71(72)$$

$$V = \left[109,627 + \frac{522 \times 70.71}{2} \right] \frac{1}{70.71} = 1681.1$$

$$h_d = 1350 \times \frac{1}{5} = 270 \text{ k}$$



Axial Compression

$$A_d = V \cos \phi + h_d \sin \phi$$

$$= 1681.1 \times \frac{572}{751} + 270 \times \frac{1}{751} = 1264.5 + 47.3$$

$$= 1711.8 \text{ k compression}$$

Shear

$$S_d = h_d \cos \phi - V \sin \phi$$

$$= 270 \times \frac{572}{751} - 1681.1 \times \frac{1}{751} = 204.17 - 225.7$$

$$= 98.3 \text{ k}$$

By VAD Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 1-22-76 Job No. 31-111-92 Calculation LATERALLY LOADED PILE

CHECK TENSION

$l = 50'$

Moment about D axis

$$M = 0 = M_{u+uW} - M_p + M_{ac} - M_B$$

$$= 109,427 - 523 \times \frac{70.71}{2} + \frac{523}{2} \times \frac{70.71}{2} - 70.71 v_b$$

$$\therefore v_b = \left[109,427 - \frac{523 \times 70.71}{2} \right] \frac{1}{70.71} = 1419.6 \text{ k}$$

$$h_b = 1350 / 4 = 337.5 \text{ k}$$

Axial Tension

$$A_b = v_b \cos \phi + h_b \sin \phi$$

$$= 1419.6 \times \frac{5\sqrt{2}}{751} + 337.5 \times \frac{1}{751} = 1405.0 + 47.3$$

$$= 1452.9 \text{ k tension}$$

Shear

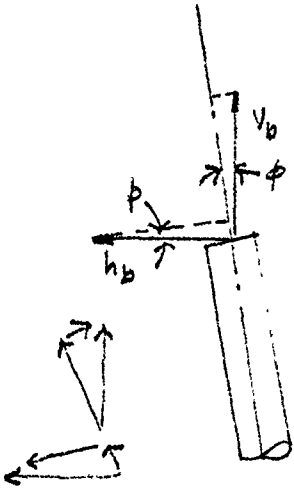
$$S_b = h_b \cos \phi - v_b \sin \phi$$

$$= 337.5 \frac{5\sqrt{2}}{751} - 1419.6 \frac{1}{751} = 324.2 - 198.8$$

$$= 135.4 \text{ k}$$

with 1711.8 k compression
 1452.9 k tension

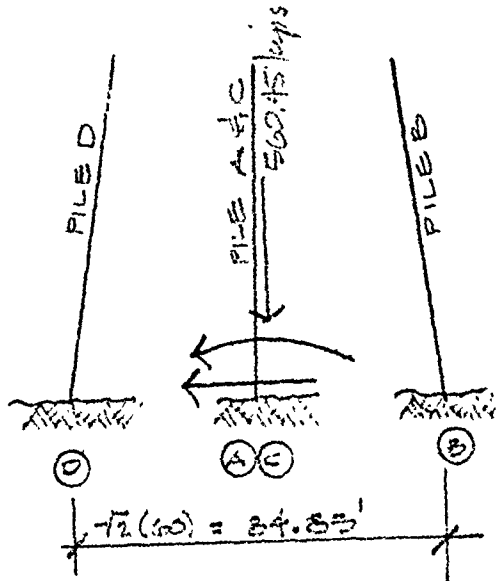
Required Penetration = 180'



By JWA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.21.76 Job No. 21-711-92 Calculation ITERALLY LAPPED PILE

LOADINGS AT PILE TOP

$l = 60'$



REACTS FROM COMPUTER PROGRAMS

	DEAD LOAD = 560.45 k	
	V	M
STRUCTURE	1310.5 k	104,024.5 k'
BOAT LANDING	39.3 k	3416.3 k'
STAIRS	62.7 k	7789.8 k'

TOTALS $1412.5 k$ $115,230.6 k'$
COMPRESSION
 Moment about B axis

$$M_c = 0 = M_{LV} + M_{WH} + M_P - M_{AC} - M_D$$

$$= 115,230.6 + \frac{560.45 \times 24.85}{2} - \frac{560.45 \times 24.85}{2 \times 2} - 34.85(V_D)$$

$$\therefore V_D = \left[115,230.6 + \frac{560.45 \times 24.85}{2 \times 2} \right] \frac{1}{34.85} = 1498.2 k$$

$$h_d = 1412.5 / 4 = 353.125 k$$

Axial Compression

$$A_d = V_D \cos \phi + h_d \sin \phi$$

$$= 1498.2 \times \frac{5\sqrt{2}}{751} + 353.125 \times \frac{1}{751}$$

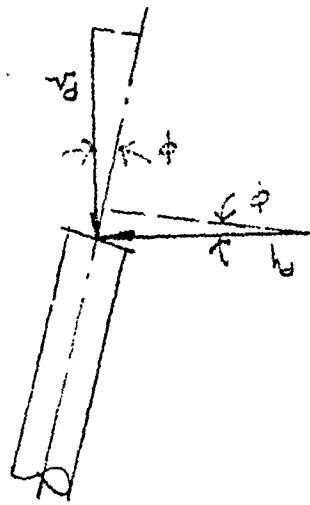
$$= 1532.7 k \text{ Compression}$$

Shear

$$S_d = h_d \cos \phi - V_D \sin \phi$$

$$= 353.125 \times \frac{5\sqrt{2}}{751} - 1498.2 \times \frac{1}{751}$$

$$= 139.9 k$$

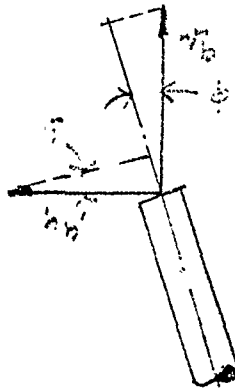


By JWS Client U.S. NAVY Subject SPRINTAL CONCEPT ANALYSIS
 Date 4-21-76 Job No. 21-711-00 Calculation LATERALLY LOADED PILE

CHECK TENSION

$\theta = 60^\circ$

Moment about D axis



$$M_b = 0 = M_{DWH} - M_p + M_{AC} - M_D$$

$$= 115,230.6 - \frac{500.4 \times 50 + 85}{2} + \frac{500.4 \times 50 + 85}{2 \times 2} - 35.95 \theta$$

$$\therefore V_b = \left[115,230.6 - \frac{500.4 \times 50 + 85}{2 \times 2} \right] \frac{1}{8 + 85} = 1217.94 \text{ k}$$

$$h_b = 1412.5, \quad \therefore T = 353.125 \text{ k}$$

Axial Tension

$$A_t = V_b \cos \theta + h_b \sin \theta$$

$$= 1217.94 \times \frac{5\sqrt{2}}{51} + 353.125 \times \frac{1}{\sqrt{51}} = 1255.4 \text{ k tension}$$

Shear

$$S_D = h_b \sin \theta - V_b \cos \theta$$

$$= 353.125 \times \frac{5\sqrt{2}}{51} - 1217.94 \times \frac{1}{\sqrt{51}}$$

$$= 179.1 \text{ k}$$

Required Pile Penetration

with $C = 1532.9 \text{ k}$
 $T = 1255.4 \text{ k}$

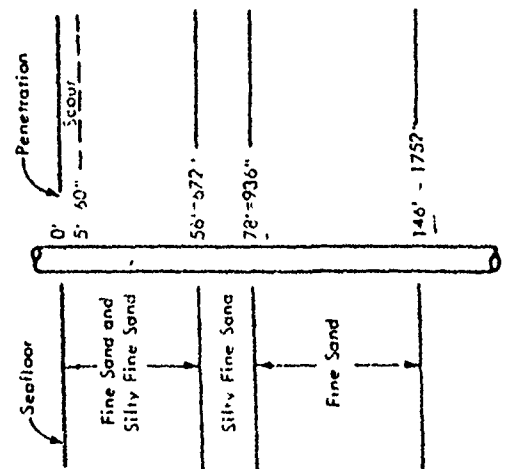
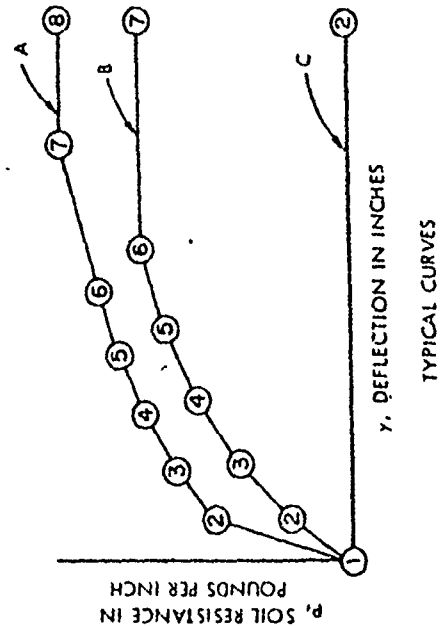
Required Pile Penetration = 162.5'

By C. CHEW Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-15-74 Job No. 27-771-92 Calculation LATERALLY LOADED PILE

SECTION 10.3
 PILE DESIGN

36-IN. DIAMETER PIPE PILES
 Coordinates of Curve Points

Perimeter inches	Typical Curve	Coordinates of Curve Points															
		Y ₁	P ₁	Y ₂	P ₂	Y ₃	F ₁	Y ₄	F ₂	Y ₅	F ₃	Y ₆	F ₄	Y ₇	F ₅	Y ₈	F ₆
0.060	C	0	0	0.16	85	0.070	120	0.18	145	0.30	168	0.50	190	1.13	244	20.0	244
96	A	0	0	0.089	222	0.063	574	0.17	926	0.29	1194	0.50	1544	1.13	247	20.0	247
165	A	0	0	0.057	637	0.071	1267	0.18	1975	0.30	2521	0.50	3244	1.13	5190	20.0	5190
232	A	0	0	0.081	320	0.063	857	0.17	1390	0.29	1792	0.50	2320	1.13	3710	20.0	3710
264	A	0	0	0.012	606	0.066	1374	0.17	2189	0.29	2809	0.50	3626	1.13	5803	20.0	5803
336	A	0	0	0.012	756	0.066	1716	0.17	2734	0.29	3509	0.50	4529	1.13	7247	20.0	7247
420	A	0	0	0.038	2492	0.090	3743	0.19	5396	0.30	6733	0.50	8537	1.13	13660	20.0	13660
432	A	0	0	0.038	3777	0.090	5224	0.19	8395	0.30	10475	0.50	13280	1.13	21248	20.0	21248
672	A	0	0	0.054	664	0.13	900	0.34	1222	0.84	1658	2.10	2250	20.0	2250		
673	B	0	0	0.041	664	0.13	900	0.34	1222	0.84	1658	2.10	2250	20.0	2250		
936	E	0	0	0.038	15395	0.090	8107	0.19	11692	0.30	14588	0.50	18497	1.13	29596	20.0	29596
937	A	0	0	0.038	10109	0.090	15181	0.19	21887	0.30	27307	0.50	34624	1.13	55398	20.0	55398
1752	A	0	0														

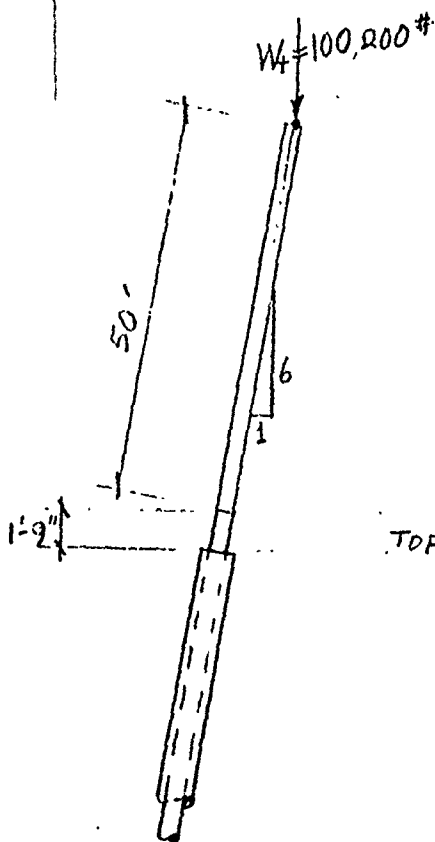


P-Y DATA
 Boring 1

STRATIGRAPHY ASSUMED FOR P-Y DATA

By 210824 Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-1-72 Job No. 27-72142 Calculation LATERAL LOAD PILE

MINIMUM WALL THICKNESS REQUIREMENTS



Use Vulcan 060 Hammer

Wt. of Hammer = 60,000 LBS

Wt. of Pile Cap = 40,200 LBS

Total Wt. = 100,200 LBS

Bending moment at the top of jacket leg is

$$M = 100,200 \times \left[\frac{1}{6} (51.75) \right] = 864,225 \text{ ft-lbs}$$

Axial Compression at the top of jacket leg (Piling)

$$P = 100,200 \times \frac{6}{\sqrt{37}} = 98,837 \text{ LBS}$$

36" O.D. x 1.25" WT $I = 20,630 \text{ in}^4$

$A = 136.5 \text{ in}^2$

Combined Stress $\sigma_c = \frac{M d}{I} + \frac{P}{A}$

$$\sigma_c = \frac{(864,225 \times 12) \times 18}{20,630} + \frac{98,837}{136.5}$$

CREST OFFSHORE, INC.

Sheet 6.09 of 4 PILE

By C. SHEEN Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-23-79 Job No. 27-771-912 Calculation LATERALLY LOADED PILE

$$\begin{aligned}\sigma_c &= 9,773 \text{ psi} \\ &= 9.8 \text{ ksi}\end{aligned}$$

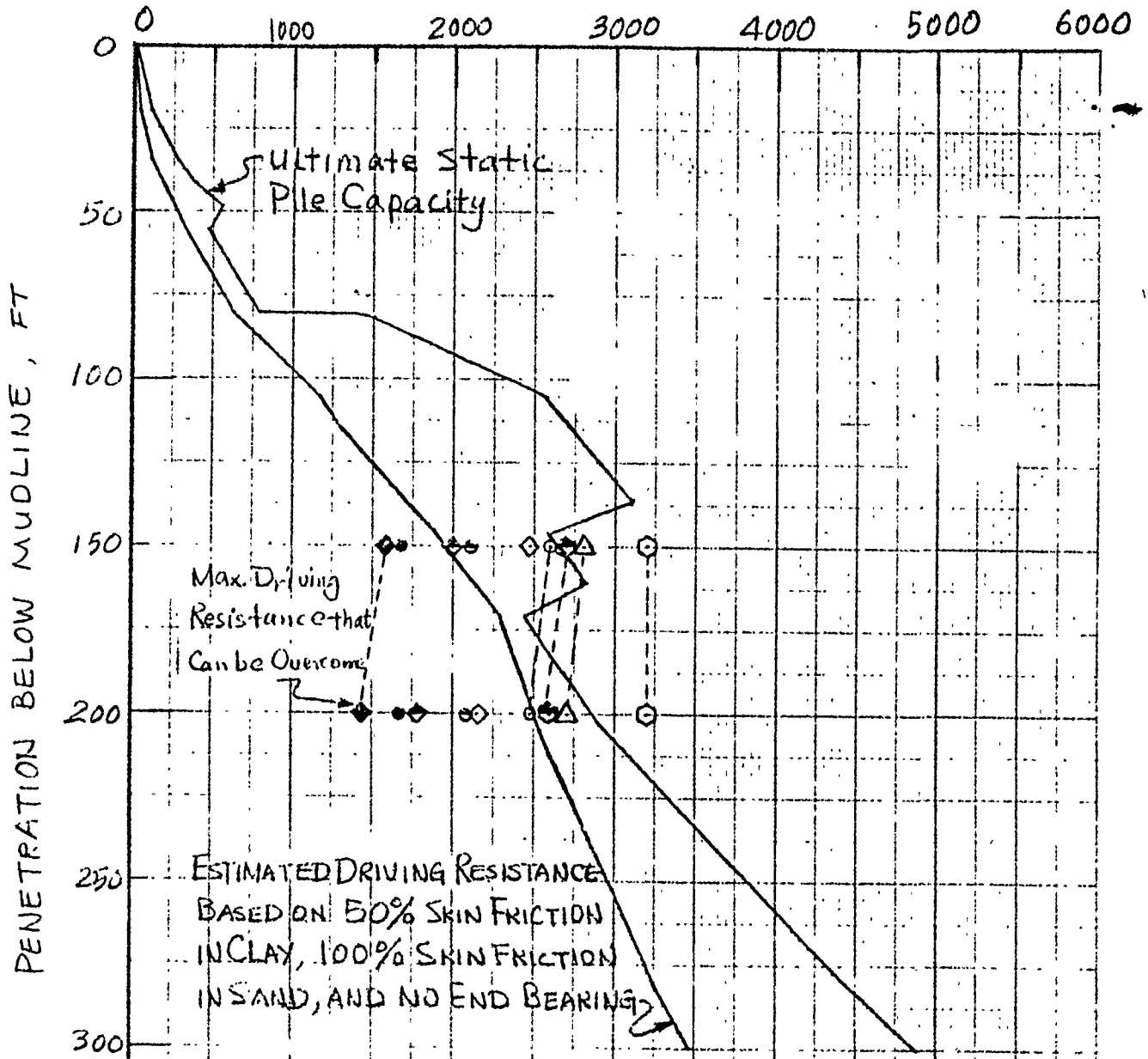
Use Impact factor of 2 for the dynamic effect on piles during driving:

$$\sigma_{cd} = 2 \times \sigma_c = 19.6 \text{ ksi} < 22 \text{ ksi}$$

Say O.K.

By S. CHEEN Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-14-76 Job No. 27-77-92 Calculation LATERALLY LOADED PILE

ULTIMATE STATIC PILE CAPACITY, KIPS
 ESTIMATED DRIVING RESISTANCE, KIPS

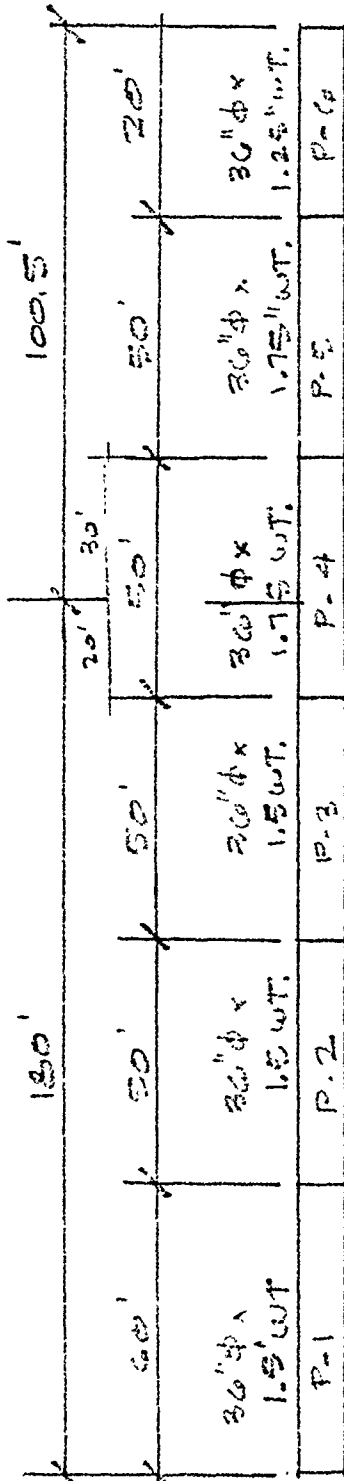


SYMBOL			R _u at Pile Tip, % Penetration		Q at Pile Hammer
7.5" WT Min.	1.25" WT Min.	1.5" WT Min.	150'	200'	Tip, in.
◆	●		50	50	3 Vulcan 940
◇	○		35	35	1
		△	14	14	.025
		●	35	35	1 Vulcan
		○	14	14	.025 060

36-IN. DIAMETER PIPE PILES

By WV Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.21.76 Job No. 27-771-42 Calculation LATERALLY LOADED PILE

ESTIMATED PENETRATION - 180 FT. BELOW MUDLINE



36" φ x 1.25" WT. $I = 20,025.14 \text{ in}^4$
 $A = 130.46 \text{ in}^2$

36" φ x 1.75" WT. $I = 27,690.04 \text{ in}^4$
 $A = 188.30 \text{ in}^2$

36" φ x 1.5" WT. $I = 24,234.25 \text{ in}^4$
 $A = 162.58 \text{ in}^2$

CREST OFFSHORE, INC.

Sheet 12 of 4 Pile

By MS Client U.S. NAVY Subject MINIMUM CONCEPT ANALYSIS
 Date 4-22-70 Job No. 27-771-92 Calculation LATERALLY LOADED PILE

SECTION 10.4

RESULTS

STA.	DIST. ALONG PILE	DEFL.	SLOPE	MOMENT	BEAM COLUMN SPWAR	APPLIED MIN LATERAL LOADING	COORINFD STRESS(MAX.)
0	0.0	0.534	-0.16710-02	-0.9790 04		0.0	-15.060
1	36.000	0.466	-0.20160-02	-0.6120 04	102.398	0.0	-12.028
2	72.000	0.340	-0.27010-02	-0.2940 04	102.460	0.0	-10.828
3	108.000	0.504	-0.22270-02	0.1260 04	97.409	-6.083	-9.534
4	144.000	0.224	-0.20970-02	0.4720 04	85.209	-21.506	-11.004
5	180.000	0.157	-0.18350-02	0.7380 04	57.949	-31.771	-13.272
6	216.000	0.097	-0.10020-02	0.8880 04	22.416	-38.104	-14.125
7	252.000	0.050	-0.10415-02	0.2000 04	-15.761	-36.022	-15.051
8	288.000	0.014	-0.50070-05	0.7760 04	-44.958	-24.422	-15.111
9	324.000	0.001	-0.30000-03	0.9600 04	-60.675	-1.983	-13.370
10	360.000	-0.006	-0.12510-03	0.3400 04	-55.564	12.281	-11.573
11	396.000	-0.008	-0.72510-06	0.1620 04	-80.677	17.261	-10.157
12	432.000	-0.006	0.51080-01	0.6670 03	-74.453	15.337	-9.103
13	468.000	-0.004	0.59230-03	-0.1340 03	-11.390	71.803	-6.127
14	504.000	-0.002	0.47660-04	-0.3530 03	-2.816	6.108	-8.133
15	540.000	-0.001	0.29680-04	-0.3480 03	1.409	2.034	-8.503
16	576.000	-0.000	0.14820-04	-0.2520 03	2.824	0.286	-8.788
17	612.000	0.000	0.09920-05	-0.1440 03	2.633	-0.704	-8.104
18	648.000	0.000	-0.13230-05	-0.6220 02	1.827	-0.921	-7.910
19	684.000	0.000	-0.10950-05	-0.1200 02	0.954	-0.740	-7.740
20	720.000	0.000	-0.20430-05	0.7580 01	0.385	-0.668	-7.588
21	756.000	0.000	-0.14820-05	0.1460 02	0.040	-0.221	-7.434
22	792.000	0.000	-0.01090-06	0.1230 02	-0.101	-0.060	-7.279
23	828.000	-0.000	-0.32820-06	0.7550 01	-0.122	0.019	-7.122
24	864.000	-0.000	-0.07710-07	0.3460 01	-0.092	0.043	-6.966
25	900.000	-0.000	0.61080-07	0.9260 00	-0.052	0.038	-6.813
26	936.000	-0.000	0.77770-07	-0.2530 00	-0.021	0.024	-6.657
27	972.000	-0.000	0.57370-07	-0.5700 00	-0.003	0.011	-6.503
28	1008.000	-0.000	0.31200-07	-0.4850 00	0.004	0.005	-6.350
29	1044.000	0.000	0.11930-08	-0.2920 00	0.005	-0.001	-6.197
30	1080.000	0.000	0.15260-08	-0.1270 00	0.004	-0.002	-6.043
31	1116.000	0.000	-0.23310-08	-0.2920-01	0.002	-0.002	-5.889
32	1152.000	0.000	-0.27370-08	0.1280-01	0.001	-0.001	-5.736
33	1188.000	0.000	-0.93440-08	0.2160-01	-0.000	-0.000	-5.583
34	1224.000	-0.000	-0.29870-09	0.1660-01	-0.000	-0.000	-5.429
35	1260.000	-0.000	0.66030-11	0.9000-02	-0.000	0.000	-5.275
36	1296.000	-0.000	0.96950-10	0.3300-03	-0.000	0.000	-5.121
37	1332.000	-0.000	0.47670-10	-0.7640-03	-0.000	0.000	-4.967
38	1368.000	-0.000	0.51490-10	-0.7510-03	0.000	0.000	-4.813
39	1404.000	0.000	0.21120-10	-0.4780-03	0.000	-0.000	-4.659
40	1440.000	0.000	0.39970-11	-0.2150-03	0.000	-0.000	-4.505
41	1476.000	0.000	-0.25570-11	-0.5170-04	0.000	-0.000	-4.351
42	1512.000	0.000	-0.34510-11	0.1560-04	0.000	-0.000	-4.197
43	1548.000	0.000	-0.25440-11	0.2900-04	0.000	-0.000	-4.043
44	1584.000	0.000	-0.10960-11	0.2130-04	-0.000	-0.000	-3.889
45	1620.000	-0.000	-0.30430-12	0.1660-04	-0.000	-0.000	-3.735
46	1656.000	-0.000	0.48200-11	0.1050-05	-0.000	0.000	-3.581
47	1692.000	-0.000			-0.000	0.000	-3.427

By MA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-22-76 Job No 21-171-92 Calculation LATERALLY LOADED PILE

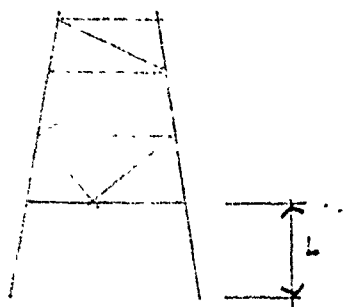
SECTION 10.5
EQUIVALENT PILE PROPERTIES

$l = 50'$
 from CD measurement plan

$L =$ equivalent pile length

$$= \frac{\text{Maximum Moment}}{\text{Maximum Shear}} = \frac{M_m}{P_m}$$

$$L = \frac{9790^{11k}}{93.3^{12k} \cdot 12k} = \frac{99.1''}{12k} = 8.26'$$



$A =$ cross sectional area of "equivalent pile"

$$= \frac{PL}{\Delta E}$$

where $P = 1712$ KIP comp.

$L = 8.26'$ (use above)

$E = 29 \times 10^3$ KSI

$$\Delta = \sum \frac{P_i L_i}{A_i E}$$

$$\Delta = \sum \frac{P_i L_i}{A_i E} = \frac{1}{E} \left[\frac{P_1 L_1}{A_1} + \frac{P_2 L_2}{A_2} + \frac{P_3 L_3}{A_3} + \frac{P_4 L_4}{A_4} + \frac{P_5 L_5}{A_5} \right]$$

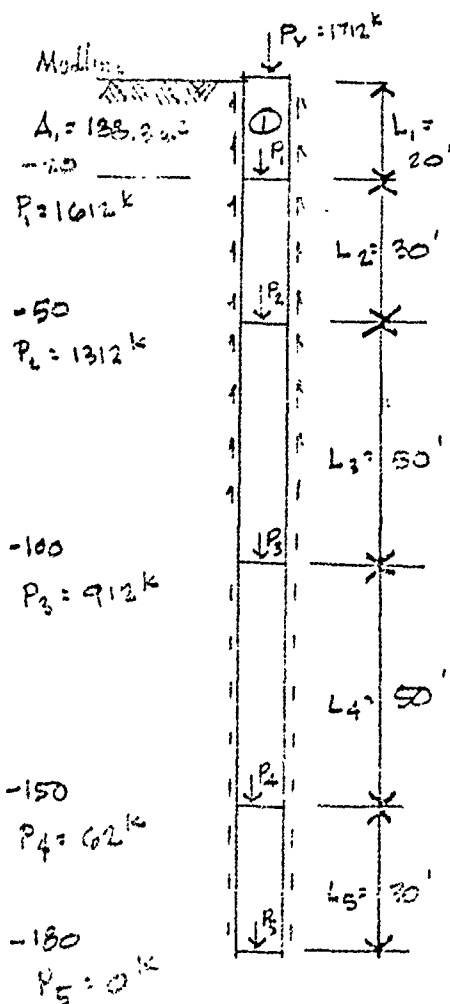
$$= \frac{1}{E} \left[\frac{1712(20)}{188.3} + \frac{1612(30)}{162.6} + \frac{1312(50)}{162.6} + \frac{912(50)}{162.6} + \frac{62(30)}{162.6} \right]$$

$$= \frac{1174.6}{E}$$

$$\therefore A = \frac{PL}{\Delta E} = \frac{1712^k \times 8.26'}{\frac{1174.6}{E} \times E}$$

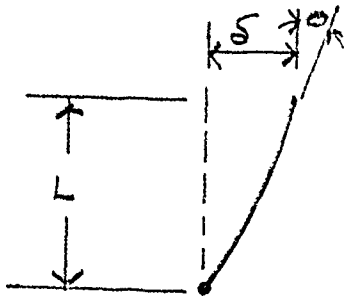
$$\left(\frac{\text{K-FT}}{\text{K-FT}/\text{in}^2} = \text{in}^2 \right)$$

$$\text{Area } \bar{A} = 12.04 \text{ m}^2$$



By WMA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-23-76 Job No. 21-771-92 Calculation LATERALLY LOADED PILE

$l = 50'$



$I =$ equivalent pile moment of inertia

$$= \frac{V L^3}{3E(\delta - \theta L)}$$

where $V = 18.3^k$

$E = 29 \times 10^3 \text{ ksi}$

$L = 8.26'$

$\delta = 0.534 \text{ in}$

$\theta = 0.001671$

$$\therefore I = \frac{18.3^k \times (8.26 \text{ FT})^3 \times 1728 \frac{\text{in}^3}{\text{FT}^3}}{3(29 \times 10^3 \text{ ksi})(0.534 - 0.001671 \times 8.26 \times 12)}$$

$= 2002.18 \text{ in}^4$

$\frac{\text{in}^k \times \text{FT}^3 \times \text{in}^3}{\text{K} \times \text{K} - \text{FT}^3} = \text{in}^4$

USE $L = 8.26'$

$A = 12.04 \text{ in}^2$

$I = 3002 \text{ in}^4$

SECTION 11

LOADINGS

11.1 INTRODUCTION

At this point the SEALOAD data update file was used to input the loadings from the stairway, boatlanding, and decks on the same tape with the structure loading from Section 5. The combined loading conditions were also input to this tape.

When the final tape had been checked and edited the STRAN space frame analysis was run. The results of that run can be found in Exhibit 'B' of this report.

By JWA Client U.S. NAVY Subject STRUCTURAL CONCRETE ANALYSIS
Date 5-26-76 Job No. 27-771-92 Calculation LOADINGS

DATA PREPARATION

1. SEALOAD Program will calculate wind and wave, dead weight of structure adjusted by the buoyancy effect.
2. Additional wind and wave forces due to boat landing and stairways --- input as joint loads
3. Additional dry weight of the superstructure:
 - a. Secondary deck beams (joists)
 - b. Floor plates
 - c. Handrails, kickplates, safety nets, etc.,
-- input as joint loads

By WKS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.20.76 Job No. 27-771-92 Calculation LOADINGS

11.3 LOADINGS

- #1 {
 - 1a - WIND & WAVE ON JACKET & SUPERSTRUCTURE
BOTH APPROACH AT 45° ANGLE
 - 1b - DEAD LOAD ON JACKET & SUPERSTRUCTURE
DRY WT. + BUOYANCY

- #2 {
 - 2a - WIND & WAVE ON DECK LANDING & STAIRWAY
BOTH APPROACH AT 45° ANGLE
 - 2b - ADDITIONAL DEAD LOAD ON SUPERSTRUCTURE
EQUIP. DECK & TOP DECK

- #3 - LIVE LOADS ON SUPER STRUCTURE
EQUIP DECK & TOP DECK

- #4 - COMBINED LOADS #1 + #2

- #5 - COMBINED LOADS #1 + #2 + #3

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.22.76 Job No. 27-77-92 Calculation LOADING

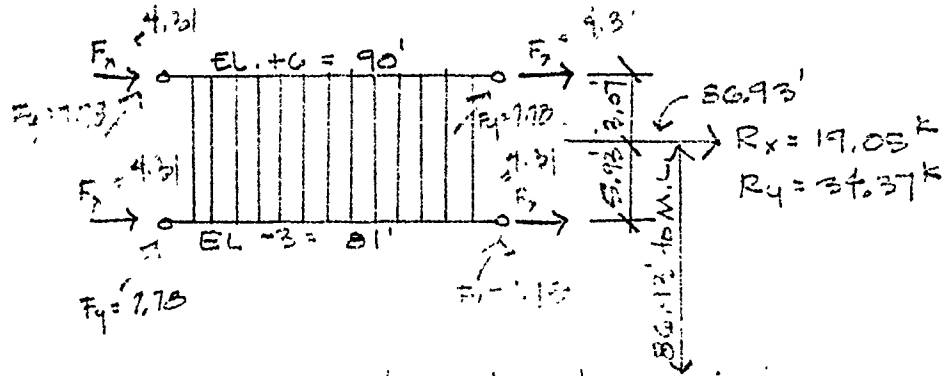
STRAN INPUT DATA LOAD # 2a

Boat Landing Forces

$$h = \frac{M}{V} = \frac{3416.23 \text{ k}}{39.30 \text{ k}} = 86.93'$$

$$h_x = \frac{M_y}{V_x} = \frac{1656.9426 \text{ k}}{19.050 \text{ k}} = 86.9768'$$

$$h_y = \frac{M_x}{V_y} = \frac{-2937.5663 \text{ k}}{34.3700 \text{ k}} = 86.9237'$$



Notes: joints are at elev. +12 (96' elev. including) and at elev. -12 (60' elev. including)

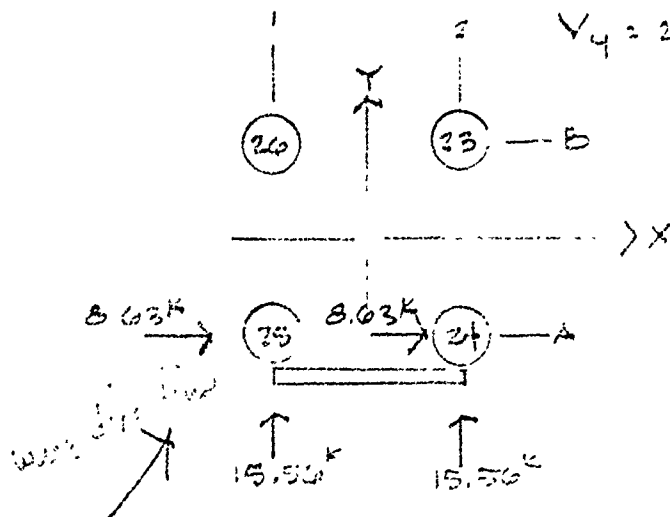
∴ put moments into joints @ elev. +12

equivalent $V_x = \frac{1656.94 \text{ k}}{96} = 17.26 \text{ k}$

$V_y = \frac{2937.57}{96} = 31.12 \text{ k}$

$R_x = \frac{V_x}{2} = \frac{17.26}{2} = 8.63 \text{ k}$

$R_y = \frac{V_y}{2} = \frac{31.12}{2} = 15.56 \text{ k}$



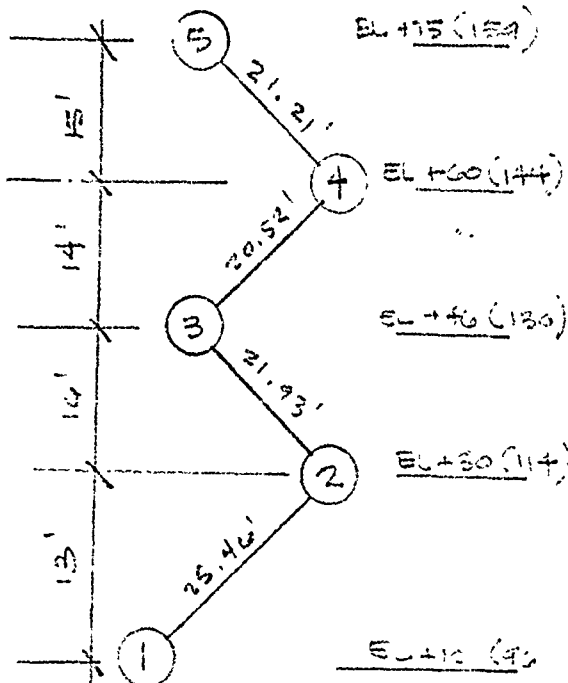
CREST OFFSHORE, INC.

Sheet 11.05 of 11

By JVS Client U.S. Navy Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-22-76 Job No. 27-171-42 Calculation LOAD DIRECTION

STRAIN INPUT DATA LOAD #2a

Stairways



$$h = \frac{M_y}{V} = \frac{7789.7554^k}{62.6489^k} = 124.300'$$

$$h_x = \frac{M_y}{V_x} = \frac{3467.1854}{27.9532} = 124.033'$$

$$h_y = \frac{M_x}{V_y} = \frac{2975.5442}{56.0893} = 124.366'$$

Use of joints at 144 & 130

NEGLECT JOINTS 1 & 5 AS THEY TAKE ONLY A SMALL AMOUNT OF THE LOAD
 ASSUME JOINTS 2, 3, & 4 TAKE EQUAL AMOUNTS OF LOAD,

FORCES IN X-DIRECTION

$$V_{2x} = V_{3x} = V_{4x} = V_x$$

V_x is distributed between joints 1 & 3

$$\therefore V_{3x} = V_x + 2V_x = 1.5V_x$$

$$V_{1x} = 2V_{2x} = 0.5V_x$$

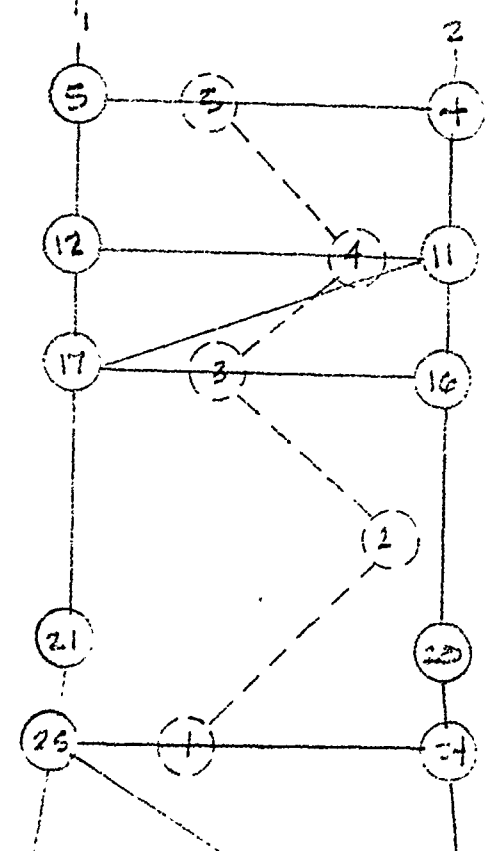
$$M_y = 3467.2 = 144 \times V_x + 130 \times 1.5 V_x + 90 \times 0.5 V_x$$

$$V_x = \frac{3467.2}{387} = 8.96^k$$

$$V_{4x} = 8.96^k$$

$$V_{3x} = 1.5 \times 8.96^k = 13.44^k$$

$$V_{1x} = 0.5 \times 8.96^k = 4.48^k$$



STAIRS SUPERIMPOSED ON STRUCTURE

CREST OFFSHORE, INC.

Sheet 11.06 of 4 PILES

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-26-72 Job No. 27-771-92 Calculation LOADING

TRY LOADING FROM WAVE PROFILE

LOAD ON WAVE'S	MBL from	to wave face	avg st.	face in X	length Y	moment in dist. X	Y
1-2	96	to 102	359	99	$x .140 - x 3.49 \times 2'$	= 202	-
				"	$x -.240 \times 3.49 \times 2$	=	
	102	- 108	399	105	$x .133 - x 3.49 \times 2$	= 237	
				"	$x -.260 \times 3.49 \times 2$	=	
	108	- 114	451	111	$x .150 - x 3.49 \times 2$	= 283	
				"	$x -.300 \times 3.49 \times 2$	=	
2-3	114	- 117.3	505	116.7	$x .108 - x 7.31 \times 2$	= 287	
				"	$x -.337 \times 7.31 \times 2$	=	
	117.3	- 124.7	503	122	$x .193 - x 7.31 \times 2$	= 335	
				"	$x -.375 \times 7.31 \times 2$	=	
	124.7	- 130	638	127.4	$x .213 - x 7.31 \times 2$	= 397	
				"	$x -.425 \times 7.31 \times 2$	=	
3-4	130	- 134	705	132	$x .235 - x 5.86 \times 2$	= 304	
				"	$x -.470 \times 5.86 \times 2$	=	
	134	- 138	765	136	$x .255 - x 5.34 \times 2$	= 400	
				"	$x -.510 \times 5.34 \times 2$	=	
	138	- 142	844	140	$x .281 - x 5.86 \times 2$	= 401	
				"	$x -.503 \times 5.86 \times 2$	=	
	142	- 144	180	143	$x .060 - x 2.93 \times 2$	= 50	
				"	$x -.130 \times 2.93 \times 2$	=	
4-5	144	- 159	133	151.5	$x .061 - x 21.21 \times 2$	= 392	
				"	$x -.122 \times 21.21 \times 2$	=	

3413.6 6827.3
 3467 6975
 Resultant Resultant

FORCES TO DIRECTION

DIR	FORCES	RESULTANT
1	X $(.14 \times 8.49 + .133 \times \frac{3.49}{2}) \times 2 = 3.107$ Y $(.133 \times \frac{3.49}{2}) \times 2 = 0.922$	7.031 resultant
2	X $(.133 \times \frac{3.49}{2} + .15 \times 3.49 + .163 \times 7.31 + .183 \times \frac{7.31}{2}) \times 2 = 1.507$ Y $(.15 \times 3.49 + .163 \times 7.31 + .183 \times \frac{7.31}{2}) \times 2 = 15.013$	16.735 resultant
3	X $(.183 \times \frac{7.31}{2} + .213 \times 7.31 + .235 \times 5.86 + .255 \times \frac{5.86}{2}) \times 2 = 8.74$ Y $(.213 \times 7.31 + .235 \times 5.86 + .255 \times \frac{5.86}{2}) \times 2 = 17.474$	19.54 resultant
4	X $(.255 \times \frac{5.86}{2} + .281 \times 5.34 + .06 \times 2.93 + .061 \times \frac{21.21}{2}) \times 2 = 6.433$ Y $(.281 \times 5.34 + .06 \times 2.93 + .061 \times \frac{21.21}{2}) \times 2 = 12.860$	14.38 resultant
5	X $(.061 \times \frac{21.21}{2}) \times 2 = 1.274$ Y $(.061 \times \frac{21.21}{2}) \times 2 = 2.588$	2.89 resultant

By WWS Client U.S. Navy Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.26.76 Job No. 21-111-12 Calculation LOADING

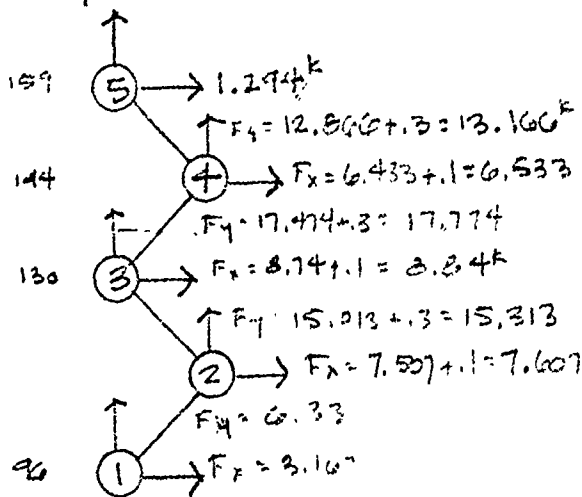
CHECK OVERTURNING MOMENT

JOINT	ELEV.	Result	X	Max. R.	Max. y	Max. z
1	96	7.081		619.3		
			3.167		304.0	
2	114	16.795		1913.5		
			7.507		855.3	
3	130	19.54		2540.2		
			8.74		1136.2	
4	144	14.38		2070.7		
			6.433		926.4	
5	159	2.89		459.5		
			1.214			
				<u>7063.7</u>	<u>3428.1</u>	<u>6356</u>
				<u>7789.3</u>	<u>25467.2</u>	<u>6775.6</u>
					<u>39.1</u>	<u>119.3</u>

DIFFERENCE =

IF DIFF. IS APPLIED EQUALLY TO JOINTS 2, 3, 4
 then $V_x = \frac{39.1}{114+130+144} = 0.1^k$ ADDED TO EA JOINT

$F_y = 2.583^k$ $V_y = 119.3/383 = 0.3^k$ ADDED TO EA JOINT

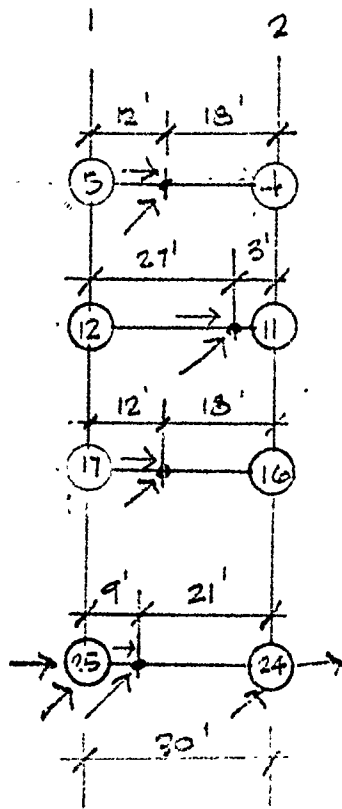


JOINT LOADS FROM STAIRWAY

IN ORDER TO APPLY LOADS TO STRUCTURE JOINT 2 LOADS WILL HAVE TO BE SPLIT BTWN JOINT 1 & JOINT 3
 \therefore JOINT 1: $F_x = 3.167 + \frac{7.507 \times 16}{34} = 6.747$
 $F_y = 6.33 + 15.313 \times \frac{16}{34} = 13.536$
 JOINT 3: $F_x = 8.84 + \frac{7.507 \times 18}{34} = 12.867$
 $F_y = 17.774 + 15.313 \times \frac{18}{34} = 25.830$

$\therefore M_x = 3466.928 = 3467$ from computer
 $M_y = 6971.378 \approx 6975$ " "

By JWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.20.76 Job No. 21-771-92 Calculation LOADING



LOADS FROM STAIRWAY JOINT 3 ON MBR (5-4)

$$F_x = 1.294^k$$

$$F_y = 2.588^k$$

LOADS FROM STAIRWAY JOINT 4 ON MBR (12-11)

$$F_x = 6.583^k$$

$$F_y = 13.166^k$$

LOADS FROM STAIRWAY JOINT 3 & 2 ON MBR (17-16)

$$F_x = 12.867^k$$

$$F_y = 25.880^k$$

LOADS FROM STAIRWAY JOINT 1 & 2 ON MBR (25-24)

$$F_x = 6.747^k$$

$$F_y = 13.580^k$$

LOADS FROM EAST LANDING ON MBR (25-24)

$$F_x = 8.03^k \text{ EA, END}$$

$$F_y = 15.50^k \text{ EA, END}$$

CREST OFFSHORE, INC.

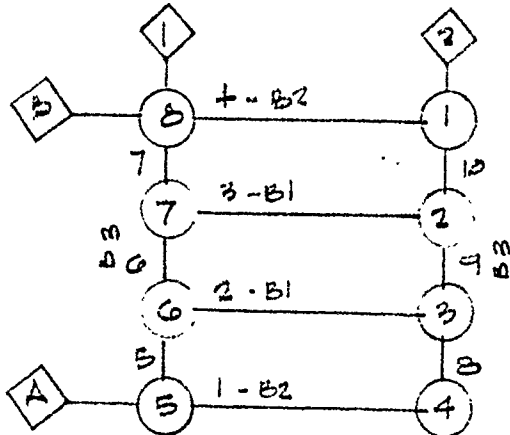
Sheet 11.09 of 4 PILE

By JMS Client UFG NEXY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-26-76 Job No. 27-711-92 Calculation LOADINGS

STRAN INPUT DATA - LOAD #26

DEAD LOADS FROM BEAMS

UPPER DECK



DEAD LOAD

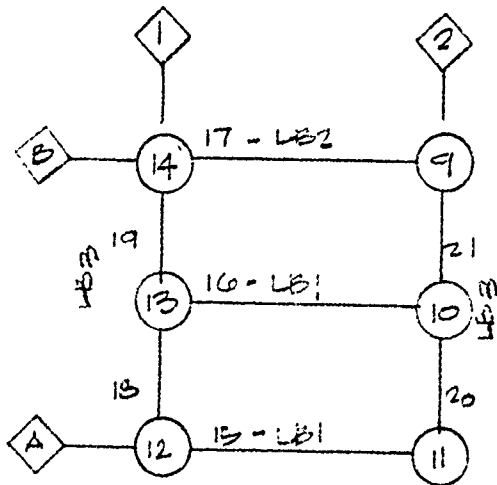
$$\begin{aligned} 4 \text{ L-FLOOR} &= 10.2 \text{ PSF} \\ \text{DECK} &= 1.8 \\ \text{MISC} &= 3.0 \\ \text{CHANNELS, WALKERS} &= 15.0 \text{ PSF} \end{aligned}$$

$$W_{B1} = (0.015 \times 10) = 0.150 \text{ K/1}$$

$$W_{B2} = (0.015 \times 5) = 0.075 \text{ K/1}$$

$$W_{B3} = 0$$

EQUIPMENT DECK



DEAD LOAD = same as above = 15.0 PSF

$$W_{LB1} = (0.015 \times 7.5) = 0.113 \text{ K/1}$$

(13-10) & (12-11)

$$W_{LB2} = 0$$

(14-9)

$$W_{LB3} = 0$$

(12-13)

(13-14)

(11-10)

(10-9)

CREST OFFSHORE, INC.

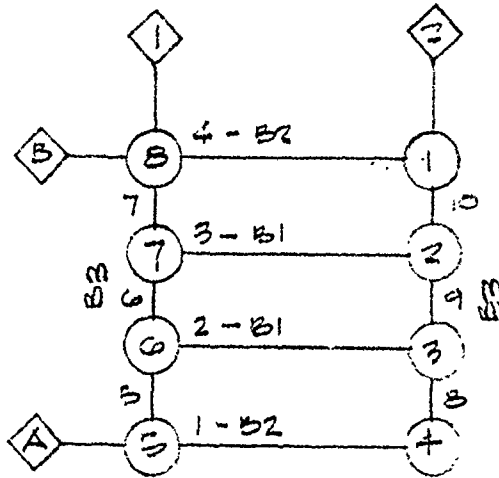
Sheet 11.10 of 4115

By KKS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4-26-76 Job No. 27-371-92 Calculation LOADING

STRAN INPUT DATA - LOAD #3

LIVE LOADS from DECKS

UPPER DECK



LIVE LOAD = 100 PSF

$$w_{B1} = 0.1 \times 15 = 1.5 \text{ K}$$

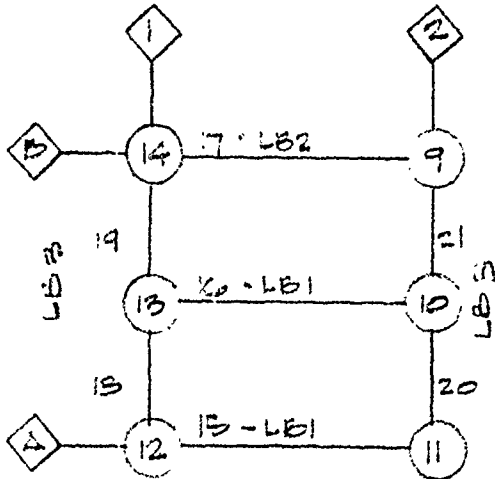
∴ 0.3 × 7.2

$$w_{B2} = 0.1 \times 7.5 = 0.75 \text{ K}$$

∴ 5.4 × 3.1

$$w_{B3} = 0$$

EQUIPMENT DECK



LIVE LOAD = 150 PSF

$$w_{LB1} = 0.150 \times 7.5 = 1.125 \text{ K}$$

∴ 13-10
12-11

$$w_{LB2} = 0$$

$$w_{LB3} = 0$$

SECTION 12

SPACE FRAME ANALYSIS

12.1 INTRODUCTION

The results of the STRAN space frame analysis, found in Exhibit 'B' of this report show the four-pile structure with a jacket base spacing of 50 ft to work, with a few exceptions, for member and pile sizes chosen.

Section 12.2 discusses the differences between the deflection at mudline of the STRAN run and that of the LATERALLY LOADED PILE run.

Section 12.3 shows all members that have a unity check greater than 1.0. Since the storm forces which produce these stresses are temporary, all unity checks less than 1.33 are assumed acceptable. Members with unity checks greater than 1.33 are discussed in 12.4.

Section 12.5 shows a typical elevation of the structure with final selected member sizes.

By WMA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 4.30.70 Job No. 27-111-92 Calculation ANALYSIS OF STRAIN RUN

12.2 DEFLECTIONS

DEFLECTION AT MODULUS
 $\Delta = \sqrt{(0.520)^2 + (0.540)^2} = 0.754''$

this compares favorably to the 0.534" from the pile

HORIZONTAL FORCES AT SUPPORTS

LOADING CONDITION #

JOINT	F _X	F _Y	Resultant	F _Z	
101	-99.93 ^k	-147.40 ^k	-178.15 ^k	206.31 ^k	
102	-243.82 ^k	-243.21 ^k	-344.44 ^k	-1284.44 ^k	TENSION
103	-203.23 ^k	-275.30 ^k	-331.37 ^k	1515.58 ^k	COMPRESSION
104	-135.75 ^k	-130.52 ^k	-183.73 ^k	105.73 ^k	

total: -745.35^k -737.44^k 1037.44^k 543.23^k

COMPRESSION 1515.58^k \approx 1711.5^k used for 1/6 of laterally loaded pile
 TENSION 1284.44^k \approx 1452.9^k " " " " " "
 SHEAR 331.37 \approx 337.5 used for 1/6 of laterally loaded pile.

CONCLUSION - THE RESULTS OF THE STRAIN COMPUTER
 OUTPUT COMPARE FAVORABLY WITH THE LOADS
 INPUT TO THE LATERALLY LOADED PILE PROGRAM.
 THESE PILES ASSUME THE PILING IS AS GOOD
 AS DESIGNED IN THE LATERALLY LOADED
 PILING SECTION.

CREST OFFSHORE, INC.

Sheet 12.03 of 4214E

By VMA Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5.2.70 Job No. 21-711-92 Calculation ANALYSIS OF STRAIN RATIO

12.3 SUMMARY OF MEMBERS WITH UNITY CHECK > 1.0

MEMBER	MEMBER NO.	MAX. U.D.	FORCE	MOMENT	AXIAL	MOM
I.D.		U.D.	F _x	M _x	A _x	M _y
			LIPS	IN-LIPS	IN-LIPS	
V1B	12-11	173.21	5	-35.45	0	569.22 347.99
V1B	14-9	1.33	1	-31.44	0	4.35 -23.47
DB3	17-14	1.07	5	} < 1.33 ∴ ACCEPTABLE		
HB3	17-16	1.75	4		-25.44	-5.44
JL1	25-21	2.06	5	-1230.93	626.87	14933.39 .02.12
JL1	23-19	1.92	5	1247.33	312.09	-12,416.47 -592.93
JL1	28-23	1.71	5	1301.3	144.2	-9,667.22 -529.57
- JL1	27-20	1.05	5	} LESS THAN 1.33 ∴ ACCEPTABLE		
- JL1	26-21	1.04	4			
- JL1	29-24	1.04	5			
- JL1	30-25	1.21	5			
- JL1	31-26	1.01	5			
PI	19-22	1.15	5	} LESS THAN 1.33 ∴ ACCEPTABLE		
PI	32-30	1.15	5			
PI	55-54	1.10	4			
- PI	21-56	1.10	4			
HB1	25-24	1.07	4			
DB1	31-27	1.31	5	} APPROX = 1.0 ∴ ACCEPTABLE		
DB1	29-25	1.20	5			
DB1	23-24	1.05	5			
P2	63-76	1.01	5			
P3	76-103	6.61	5	} THESE ARE EQUIVALENT MEMBERS THEREFORE NEGLECT		
P3	80-102	5.04	4			
P3	82-101	1.49	5			
- P3	78-104	1.15	1			

By WWS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5-2-76 Job No. 32-771-92 Calculation ANALYSIS OF STEAM RUN

12.4 OVERSTRESSED MEMBERS

GROUP VIII

MEMBERS 12-11 AND 14-9 ARE SPANDREL BEAMS AT THE EQUIPMENT DECK LEVEL. THESE BEAMS WERE SIZED FOR THE LIVE AND DEAD LOADS ONLY FROM THE EQUIPMENT DECK. WHEN THE STEAM RUN WAS MADE THE REACTION FROM THE WAVE FORCE ON THE STAIRWAY WAS INPUT AT ABOUT THE CENTER LINE OF THE BEAM 12-11. NO PROVISION WAS MADE FOR THE K/F OF THIS BEAM IN THE HORIZONTAL DIRECTION.

THE WAVE AND WIND FORCES PRODUCE TWISTING MOMENTS ABOUT THE STRUCTURE WHICH HAVE PRODUCED AXIAL LOADS IN THESE BEAMS ALSO.

THESE BEAMS WERE SIZED TO TAKE MOMENT IN THE GLOBAL Z DIRECTION ONLY. THEY MUST BE INCREASED IN SIZE TO TAKE MOMENT IN BOTH DIRECTIONS PLUS AXIAL LOAD.

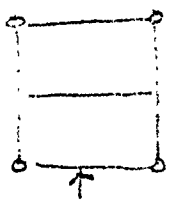
with $P = 63.45^k$ $M_Y = 560.22^k$ $M_Z = 347.99^k$

try W18x105

$A = 30.9 \text{ m}^2$ $S_x = 202 \text{ m}^3$ $S_y = 42.3 \text{ m}^3$ $r = 2.84$

$\frac{KL}{r}$ in the member y axis = $1.0 \times 30' \times 12'' / 2.84 = 126$

$\therefore F_y = 9.91$



UNITY CE = $\frac{P}{F_y} + \frac{M_Y}{F_y S_y} + \frac{M_Z}{F_y S_x} = \frac{63.45}{9.91} + \frac{560.22}{9.91 \times 42.3} + \frac{347.99}{9.91 \times 202}$

= $0.218 + 0.60 + 0.08 = 0.8985 < 1.0$

ok.

USE W18x105 for all SPANDREL BEAMS AT THE EQUIPMENT DECK.

NOTE: WE CAN RELOCATE STAIRWAYS TO THE SUPERSTRUCTURE LEGS TO AVOID HEAVY BEAMS.

By WLD Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5-2-76 Job No. 27-11-72 Calculation ANALYSIS OF STRAIN RUN

GROUP H33
NUMBER 17-16

LOADS; $P = 25.44^k$ $M_1 = 442.61^k$ $M_2 = 1347.23^k$
 UNITY CHECK = 1.75
 SIZE = 14" ϕ x 0.375
 TRY 14" ϕ x 0.500 $A_x = 21.200$ $S = 69.126$ $r = 4.78$
 LENGTH = 27.5'
 $\frac{Kl}{r} = \frac{1.0 \times 27.5 \times 12}{4.78} = 69.0 \therefore F_c = 16.53 \text{ ksi}$

$$\text{UNITY CHECK} = \frac{\frac{25.44^k}{21.200 \text{ in}^2}}{16.53 \text{ ksi}} + \frac{\frac{442.61}{69.126}}{22} + \frac{\frac{1347.23}{69.126}}{22}$$

$$= 0.073 + 0.29 + 1.2 = 1.56 > 1.33$$

TRY 16" ϕ x 0.500 $A = 24.347$ $S = 91.510$ $r = 5.5$
 $\frac{Kl}{r} = \frac{1.0 \times 27.5 \times 12}{5.5} = 60 \therefore F_c = 17.43$

$$\text{UNITY CHECK} = \frac{\frac{25.44^k}{24.347 \text{ in}^2}}{17.43 \text{ ksi}} + \frac{\frac{442.61}{91.510}}{22} + \frac{\frac{1347.23}{91.510}}{22}$$

$$= 0.06 + 0.22 + 0.92 = 1.20 < 1.33 \therefore \text{OK}$$

USE 16" ϕ x 0.500' for all HORIZ. BRACING
 AT ELEV. +45.

By MM Client O.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5.2.76 Job No. 27-11-92 Calculation ANALYSIS OF STEEL TUB

GROUP JLI

THE JACKET LEGS HAVE 41" ϕ x 1.00" JOINT CANS WHICH WERE NOT CONSIDERED IN THE STRAIN RUN FOR SAKE OF SIMPLICITY.

MEMBERS 25-21
 23-19

ARE 41" ϕ x 1.00" FOR THEIR ENTIRE LENGTH
 $w/A = 125.000$ $S = 1226.754$ $r = 14.15$

Max forces at mbr 23-19

$F_x = 1297$ $M_T = 12,416$ $M_z = 592.93$

LENGTH = 4.54'

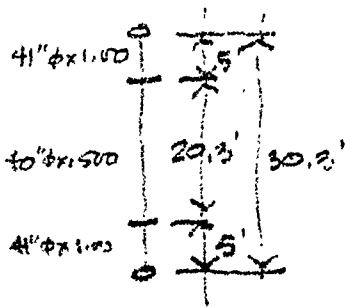
$\therefore KL/r = 1.0 \times 4.54 \times 12 / 14.15 = 4$

$F_c = 21.44$

UNITY CHECK = $\frac{1297^2}{125.000^2} + \frac{12416}{1226.754} + \frac{592.93}{1226.754}$

U.C. = 0.48 + 0.10 + 0.22 = 1.16 < 1.53 \therefore o.k.

MEMBER 25-23



$F_x = 1301.3^k$ $M_T = 9007.22^{in-k}$ $M_z = 509.57^{in-k}$

Stress at ends = $F_y = 1.67^k$ $F_z = 43.91^k$

\therefore Max S' from top: $M_T = 9007.2 - 5' \times 43.91^k = 7032.6$

$M_z = 509.57 - 5' \times 1.67^k = 401.37$

41" ϕ x 1.00 in joint cans are used at each end of the member, from above it can be seen they will be ok, \therefore check stress in portion of member that is 40" ϕ x 0.50

$w/A = 62.046$ $S = 675.3$ $r = 13.903$

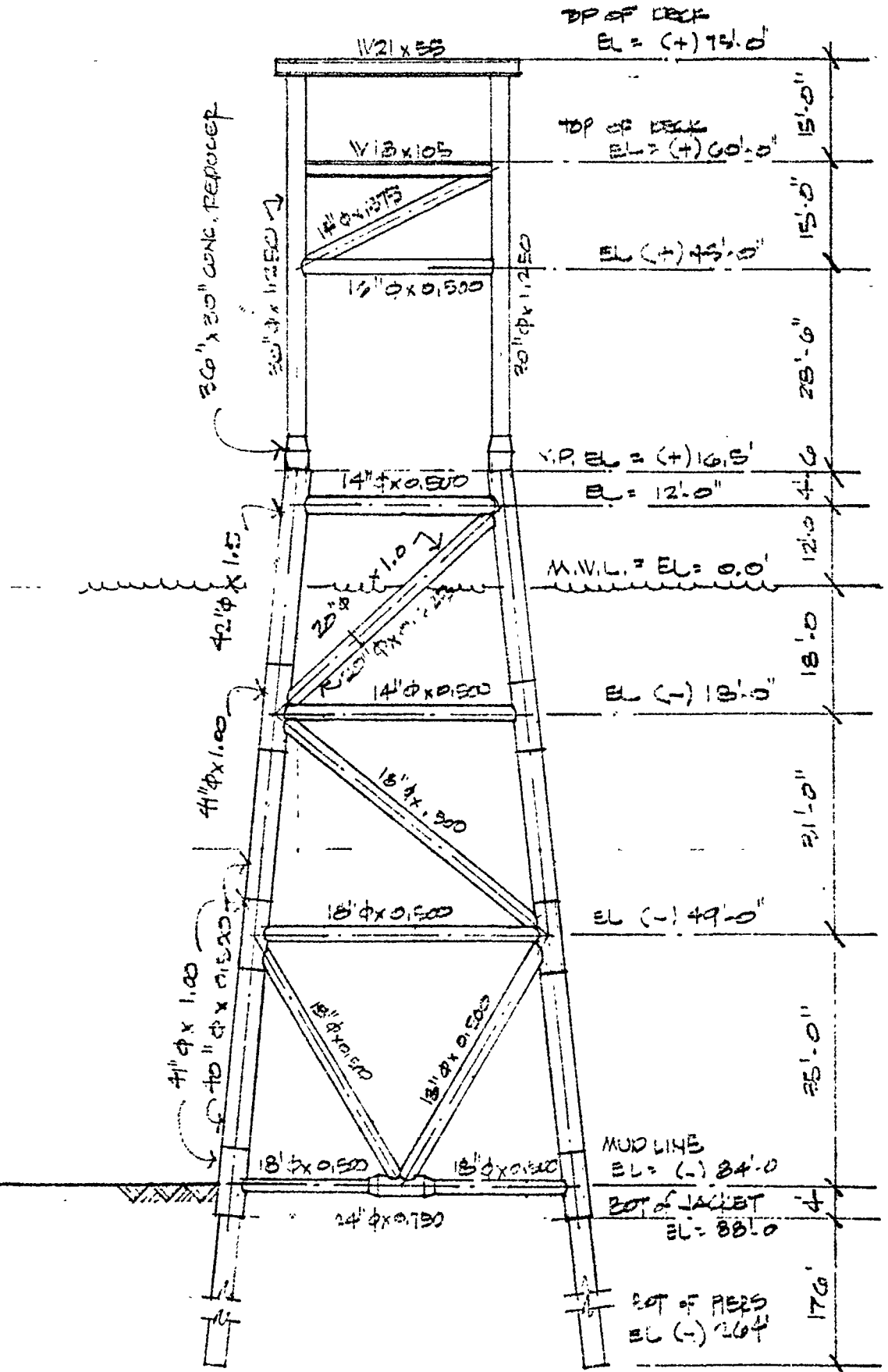
$KL/r = 20.3 \times 12 / 13.903 = 17.4$ $F_c = 20.73$

unity check = $\frac{1301.3}{62.046} + \frac{7032.6}{675.3} + \frac{401}{675.3} = 1.57 > 1.33$

USE 41" ϕ x 1.00" FOR ENTIRE LENGTH OF MBR.

By KAN Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5-2-70 Job No. 27-771-92 Calculation ANALYSIS OF STRAIN RUN

12.5 FINAL STRUCTURE CONFIGURATION



SECTION 13

CORROSION PROTECTION AND STRUCTURAL STEEL WEIGHT

13.1 INTRODUCTION

In order to determine prices for the four-pile structure, area and weight take-offs are made. Section 13.2 shows areas to consider for painting and corrosion protection.

Section 13.3 is a copy of the SEALOAD report which shows member lengths. These lengths are used in the tables of Section 13.4. Each table gives the weight and area of different sections of the structure. Section 13.5 gives the totals of all the areas and weights.

By JSS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5-7-76 Job No. 27-171-12 Calculation IDEA & WEIGHT TAKE-OFFS

13.2 AREAS TO CONSIDER

A. FOR PAINTING

FROM NAVY SPECIFICATIONS, PAINTING IS REQUIRED FROM -4' BELOW MEAN WATER LEVEL TO THE TOP OF THE PILES

B. FOR CORROSION PROTECTION

1. FROM API REPAIR CATHODIC PROTECTION IS REQ'D BELOW THE SPLASH ZONE WHICH IS 4 FT. BELOW LOW LUNAR TIDE.

MEAN WATER LEVEL = 34'-0"
 LOW LUNAR TIDE = 0'-0"
 -4' = 4'-0"
 EL. TOP OF CATHODIC PROTECTION = 80'-0"

2. CURRENT REQUIREMENTS FOR CATHODIC PROTECTION

CURRENT DENSITY = $CO \text{ mA/A} / \# \text{ OF SURFACE AREA IN WATER}$
 $1 \text{ mA/A} / \# \text{ OF SURFACE AREA IN MUD ZONE}$

5. DESIGN LIFE

N = 10 YEARS

CREST OFFSHORE, INC.

Sheet 1305 of 1015

By WLS Client W.D. YAW Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5-1-72 Job No. 21-27-92 Calculation UNIT 215

MISC. MEMBER LENGTHS

Member	Length	Unit	DL
359 LOAD Z	45	38.511 1.50	DL 0 1
370 LOAD Z	42	43.478 1.50	DL 0 1
371 LOAD Z	43	44.411 1.50	DL 0 1
372 LOAD Z	40	39.100 1.50	DL 0 1
373 LOAD Z	39	34.101 1.50	DL 0 1
374 LOAD Z	42	46.102 1.25	DL 0 1
375 LOAD Z	46	38.103 1.25	DL 0 1
376 LOAD Z	44	46.104 1.25	DL 0 1
377 LOAD Z	46	40.105 1.25	DL 0 1
378 LOAD Z	25	55	DL 0 1
379 LOAD Z	55	56	DL 0 1
380 LOAD Z	24	53	DL 0 1
381 LOAD Z	53	54	DL 0 1
382 LOAD Z	26	57	DL 0 1
383 LOAD Z	57	58	DL 0 1
384 LOAD Z	23	51	DL 0 1
385 LOAD Z	51	52	DL 0 1
386 LOAD Z	30	63	DL 0 1
387 LOAD Z	63	64	DL 0 1
388 LOAD Z	29	61	DL 0 1
389 LOAD Z	61	62	DL 0 1
390 LOAD Z	31	65	DL 0 1
391 LOAD Z	65	66	DL 0 1
392 LOAD Z	28	59	DL 0 1
393 LOAD Z	59	60	DL 0 1
394 LOAD Z	35	71	DL 0 1
395 LOAD Z	71	72	DL 0 1
396 LOAD Z	34	69	DL 0 1
397 LOAD Z	69	70	DL 0 1
398 LOAD Z	36	73	DL 0 1
399 LOAD Z	73	74	DL 0 1
400 LOAD Z	35	67	DL 0 1
401 LOAD Z	67	68	DL 0 1
402 LOAD Z	42	74	DL 0 1
403 LOAD Z	79	80	DL 0 1
404 LOAD Z	40	77	DL 0 1
405 LOAD Z	77	78	DL 0 1
406 LOAD Z	44	61	DL 0 1
407 LOAD Z	81	82	DL 0 1
408 LOAD Z	38	75	DL 0 1
409 LOAD Z	75	76	DL 0 1
410 LOAD Z	21	56	DL 0 1
411 LOAD Z	56	64	DL 0 1
412 LOAD Z	64	72	DL 0 1
413 LOAD Z	72	60	DL 0 1
414 LOAD Z	20	54	DL 0 1
415 LOAD Z	54	62	DL 0 1
416 LOAD Z	62	70	DL 0 1
417 LOAD Z	70	78	DL 0 1
418 LOAD Z	22	58	DL 0 1
419 LOAD Z	58	66	DL 0 1
420 LOAD Z	66	74	DL 0 1
421 LOAD Z	74	82	DL 0 1
422 LOAD Z	19	52	DL 0 1
423 LOAD Z	52	60	DL 0 1
424 LOAD Z	60	68	DL 0 1

1106-104

1.25 X P.F. UNIT
 -B+
 X 2.2
 10.83

CREST OFFSHORE, INC.

Sheet 13.002 of 4 PILE

By JMS Client U.S. NAVY Subject PROPOSED CONCEPT ANALYSIS
 Date 3-4-76 Job No. 37-71-92 Calculation LOADING

MBD
 BEARING
 DEFLECTIONS

425	LOAD Z	6A	76	0.00	-.481	35.35	-.481	GLOR UNIF	DL 0 1
426	LOAD Z	20	102	0.00	-.481	15.16	-.481	GLOR UNIF	DL 0 1
427	LOAD Z	78	104	0.00	-.481	15.16	-.481	GLOR UNIF	DL 0 1
428	LOAD Z	82	101	0.00	-.481	15.16	-.481	GLOR UNIF	DL 0 1
429	LOAD Z	76	103	0.00	-.481	15.16	-.481	GLOR UNIF	DL 0 1
430	LOAD X	21	56	0.00	115			GLOR CONC	WN 1 1
431	LOAD Y	21	56	0.00	115			GLOR MOMT	WN 1 1
432	LOAD X	21	56	0.00	115			GLOR CONC	WN 1 1
433	LOAD Y	21	56	0.00	115			GLOR MOMT	WN 1 1
434	LOAD X	21	56	0.00	115			GLOR CONC	WN 1 1
435	LOAD Y	21	56	0.00	115			GLOR MOMT	WN 1 1
436	LOAD X	20	54	0.00	115			GLOR CONC	WN 1 1
437	LOAD Y	20	54	0.00	115			GLOR MOMT	WN 1 1
438	LOAD X	20	54	0.00	115			GLOR CONC	WN 1 1
439	LOAD Y	22	58	0.00	115			GLOR MOMT	WN 1 1
440	LOAD X	22	58	0.00	115			GLOR CONC	WN 1 1
441	LOAD Y	22	58	0.00	115			GLOR MOMT	WN 1 1
442	LOAD X	22	58	0.00	115			GLOR CONC	WN 1 1
443	LOAD Y	19	52	0.00	115			GLOR MOMT	WN 1 1
444	LOAD X	19	52	0.00	115			GLOR CONC	WN 1 1
445	LOAD Y	19	52	0.00	115			GLOR MOMT	WN 1 1
446	LOAD X	19	52	0.00	115			GLOR CONC	WN 1 1
447	LOAD Y	21	56	0.00	431			GLOR MOMT	WN 2 1
448	LOAD X	21	56	0.00	431			GLOR CONC	WN 2 1
449	LOAD Y	21	56	0.00	431			GLOR MOMT	WN 2 1
450	LOAD X	21	56	0.00	431			GLOR CONC	WN 2 1
451	LOAD Y	20	54	0.00	431			GLOR MOMT	WN 2 1
452	LOAD X	20	54	0.00	431			GLOR CONC	WN 2 1
453	LOAD Y	20	54	0.00	431			GLOR MOMT	WN 2 1
454	LOAD X	20	54	0.00	431			GLOR CONC	WN 2 1
455	LOAD Y	22	58	0.00	431			GLOR MOMT	WN 2 1
456	LOAD X	22	58	0.00	431			GLOR CONC	WN 2 1
457	LOAD Y	22	58	0.00	431			GLOR MOMT	WN 2 1
458	LOAD X	22	58	0.00	431			GLOR CONC	WN 2 1
459	LOAD Y	19	52	0.00	431			GLOR MOMT	WN 2 1
460	LOAD X	19	52	0.00	431			GLOR CONC	WN 2 1
461	LOAD Y	19	52	0.00	431			GLOR MOMT	WN 2 1
462	LOAD X	19	52	0.00	431			GLOR CONC	WN 2 1
463	LOAD Y	21	56	0.00	115			GLOR MOMT	WN 3 1
464	LOAD X	21	56	0.00	115			GLOR CONC	WN 3 1
465	LOAD Y	21	56	0.00	115			GLOR MOMT	WN 3 1
466	LOAD X	21	56	0.00	115			GLOR CONC	WN 3 1
467	LOAD Y	20	54	0.00	115			GLOR MOMT	WN 3 1
468	LOAD X	20	54	0.00	115			GLOR CONC	WN 3 1
469	LOAD Y	20	54	0.00	115			GLOR MOMT	WN 3 1
470	LOAD X	20	54	0.00	115			GLOR CONC	WN 3 1
471	LOAD Y	22	58	0.00	115			GLOR MOMT	WN 3 1
472	LOAD X	22	58	0.00	115			GLOR CONC	WN 3 1
473	LOAD Y	22	58	0.00	115			GLOR MOMT	WN 3 1
474	LOAD X	22	58	0.00	115			GLOR CONC	WN 3 1
475	LOAD Y	19	52	0.00	115			GLOR MOMT	WN 3 1
476	LOAD X	19	52	0.00	115			GLOR CONC	WN 3 1
477	LOAD Y	19	52	0.00	115			GLOR MOMT	WN 3 1
478	LOAD X	19	52	0.00	115			GLOR CONC	WN 3 1
479	LOAD Y	21	56	0.00	431			GLOR MOMT	WN 4 1
480	LOAD X	21	56	0.00	431			GLOR CONC	WN 4 1

CREST OFFSHORE, INC.

Sheet 13.07 of 4 Pile

By WJ Client U.S. NAVY Subject SPROCK CONCEPT ANALYSIS
 Date 5.20.70 Job No. 27-11-72 Calculation WEIGHT TAKE-OFFS

13.4 AREA AND WEIGHT TAKE-OFFS

SEE SHEET 5.4 P10 PILING

for cathodic protection

ITEM	SIZE	UNIT WT. %	UNIT LENGTH FT.	NO. MEMB.	TOTAL WT. K	UNIT AREA $\frac{sq}{ft}$	TOTAL SURFACE AREA $\frac{sq}{ft}$
P.6	30" ϕ x 1.25	0.464	20'	1 x 4	37.12 ^K	/	/
P.5 P.4	30" ϕ x 1.75	0.610	50'	2 x 4	250.0 ^K	$\frac{2}{10} \times 9.425$	754 ^{sq}
P.3 P.2	30" ϕ x 1.50	0.553	50'	2 x 4	221.2 ^K	9.425	3770 ^{sq}
P.1	30" ϕ x 1.50	0.553	60'	1 x 4	132.72 ^K	9.425	2262 ^{sq}
SPICE POINT	33" ϕ x 0.625	0.210	3'	5 x 4	34.50 ^K	/	/
PILING TOTALS					631.60 ^K		6786 ^{sq}
					340.8 ^T		

CREST OFFSHORE, INC.

Sheet 13.02 of 4 PILES

By JW Client U.S. NAVY Subject STANDARD CONCEPT ANALYSIS
 Date 5-2-76 Job No. 27-111-92 Calculation WEIGHT TAKE-OFFS

PILING EXCESS TO BE CUT OFF IN FIELD

PILE	SIZE	UNIT WT. #	UNIT LENGTH FT	NO. PILES	TOTAL WT. #	UNIT AREA #	TOTAL SURFACE AREA #
P.6	30"φ x 1.75	0.407	10'	1 x 4	15.50 ^k		
P.5	30"φ x 1.75	0.407	2'	2 x 4	10.24 ^k		
P.4							
P.3	30"φ x 1.50	0.533	2'	3 x 4	13.27 ^k		
P.2							
P.1							
PILING EXCESS TOTALS					42.07 ^k		
					21.07		

CREST OFFSHORE, INC.

Sheet 13.09 of 4 PLS

By WV Client W. J. WY Subject PERIODIC CORROSION ANALYSIS
 Date 5-2-76 Job No. 27-11-92 Calculation WEIGHT TAKE OFF

JACKET

for cathodic protection

ITEM	SIZE	UNIT WT. %	UNIT LENGTH FT	NO. UNITS	TOTAL WT. %	UNIT AREA #/ft	TOTAL SURFACE AREA #
HBI ③ +12	4" φ x 0.500	0.072	27.90	4	5.04 ^k		
XB1 +12	16" φ x 0.5	0.083	40.4	2	6.71 ^k		
W1 ① +23	41" φ x 1.00	0.4272	43.45	4	74.25 ^k	$\frac{23}{37.5} \times 10.73$	1080 ^φ
BHT ② +23	41" φ x 1.00	0.4272	10.0	2x4	34.18 ^k	10.73	429 ^φ
W1 +12	40" φ x 0.500	0.211	51.5	4	43.47	10.472	2157 ^φ
DB1 ① +12	20" φ x 0.625	0.1293	42.37	4	21.91 ^k	5.230	837.4 ^φ
HBI +12	14" φ x 0.500	0.072	33.70	4	9.72 ^k	5.665	497.0 ^φ
XB2 +12	18" φ x 0.500	0.0935	40.9	2	9.14 ^k	4.712	420.6 ^φ
DB2 +12 +49	18" φ x 0.500	0.0935	47.7	4	17.84 ^k	4.712	871.0 ^φ
HBI +49	18" φ x 0.500	0.0935	40.1	4	15.20 ^k	4.712	735.8 ^φ
XB3 +49	20" φ x 0.500	0.104	57.6	2	11.93 ^k	5.230	603.2 ^φ
DB2 +49 +24	18" φ x 0.500	0.0935	33.25	8	28.61	4.712	1441.9 ^φ
HBI +24	18" φ x 0.500	0.0935	22.05	8	16.49 ^k	4.712	531.2 ^φ
XB3 +24	20" φ x 0.500	0.104	67.6	2	14.00 ^k	5.230	707.9 ^φ
BHT +24	24" φ x 0.150	0.1332	5.0	4	7.22 ^k	6.233	125.7 ^φ
JACKET TOTALS					315.16 ^k		10832 ^φ
					153 ^T		

CREST OFFSHORE, INC.

Sheet 1610 of 4 PLE

By WJ Client U.S. NAVY Subject PERIODIC CONCEPT ANALYSIS
 Date 5.8.70 Job No. 57-111-92 Calculation WEIGHT TAKE OFFS

JACKET WALKWAY

Areas for Painting

PER	SIZE	UNIT WT. %	UNIT LENGTH	NO. UNITS	TOTAL WT. %	UNIT AREA %	TOTAL AREA #
	6" ϕ x 0.473	0.0385	25'	4	1.54%	2.258	208 #
	2" ϕ x 0.270	0.003	4'	3x4	1.0%	0.753	90 #
	1/4" x 2" x 2"	0.0102	4' x 20' / 4' x 22'	2	4.2%	A x 2	832 #
JACKET WALKWAY TOTALS					88%	4.4%	
JACKET WALKWAY TOTALS					4' x 10'	AREAS for PAINTING	
HBI 12	14" ϕ x 0.510	/	27.90	4	/	4.189	407 #
HBI 4.12	1/2" ϕ x 0.510	/	10.9'	2	/	15.005	290 #
	4" ϕ x 1.10	/	13.45	4	/	16.5 / 39.75 x 10.75	779 #
JACKET WALKWAY TOTALS					9.3%	4.4%	for painting 2073 #

CREST OFFSHORE, INC.

Sheet 13.11 of 4 PLS

By VLL Client W. H. HAY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5-2-76 Job No. 22-111-92 Calculation WALKOFF TRUSS

BOAT LANDING

TYPE	SIZE	UNIT WT. #/L	UNIT LENGTH FT	NO. MEMB.	TOTAL WT. #	UNIT AREA #/L	TOTAL SURFACE AREA #/L
HCP.	3 3/4" x 0.500	0.0734	15'	1	0.65 ^k	2.258	34 #
HCP.	3 3/4" x 0.500	0.0734	30'	4	5.21 ^k	2.258	271
HCP.	12 3/4" x 0.343	0.1073	3'	4	0.93 ^k	3.34	27
VEPT.	12 3/4" x 0.343	0.1073	10'	2	3.43 ^k	3.34	107
VEPT.	6 3/4" x 0.432	0.0230	13'	4	1.40 ^k	1.734	90
VEPT.	6 3/4" x 0.432	0.0230	10'	5	1.43 ^k	1.734	87
DIAG.	6 3/4" x 0.432	0.0230	15'	2	0.83 ^k	1.734	52
STAYS	6 3/4" x 0.432	0.0230	4'	14	1.60 ^k	1.734	97
DIAG. STAYS	6 3/4" x 0.432	0.0230	5.7'	12	1.26 ^k	1.734	119
	1 1/2" CHFD P	0.0102	4' x 24'	1	0.95 ^k	2	192
BOAT LANDING TOTALS					13.47 ^k	total area painting	1276 # 400 # 610 #

CREST OFFSHORE, INC.

Sheet B J 2 of 4 PLS

By JJA Client U.S. NAVY Subject PROPOSED CONCEPT ANALYSIS
 Date 5-2-76 Job No. 22-171-92 Calculation WEIGHT TAKE OFFS

SUPER STRUCTURE

MFR	SIZE	UNIT WT. %	UNIT LENGTH FT.	NO. KEYS	TOTAL WT. K	UNIT AREA #	TOTAL SURFACE AREA #
W21	W21x55	0.055	30'	2	3.3 ^K	0.52	31.2 #
W17	W13x40	0.040	30'	2	2.4 ^K	0.42	25.2 #
W21	W21x55	0.055	30'	2	3.3 ^K	0.52	31.2 #
W13	W13x55	0.055	30'	1	1.65	0.46	13.8 #
W13	W13x103	0.105	30'	4	12.00 ^K	0.53	69.0 #
D83 +45	14" φ x 0.375	0.055	30.61	4	6.73 ^K	3.665	449.0 #
H83 +45	16" φ x 0.500	0.083	27.50	4	9.13	4.19	460.9 #
X81 +45	16" φ x 0.500	0.083	40.0	2	6.64 ^K	4.19	335.2
580	30" φ x 1.350	0.384	58.5'	4	89.86 ^K	7.854	1837.5 #
580	27" φ x 0.625	0.170	5'	4	5.63 ^K	7.07	220.2
D83	1/2" x 3/8" x 2	0.015 [#]	35x35	1	18.4	A x 2	2450
D83	1/2" x 3/8" x 2	0.015	15x30	1	6.75	A x 2	900
W10	W10x11.5	0.0115	10'	33	3.8 ^K	3.0	990
W10	W10x11.5	0.0115	15'	11	1.9 ^K	3.0	495
SUPER STRUCTURE TOTALS					172.1 86 ^T	Proportion	8315 #

CREST OFFSHORE, INC.

Sheet 13 of 4 PLE

By W.A. Client U.S. OFFSHORE Subject STAIRWAY CONCEPT ANALYSIS
 Date 5-2-76 Job No. 22-771-92 Calculation WEIGHT TAKE-OFFS

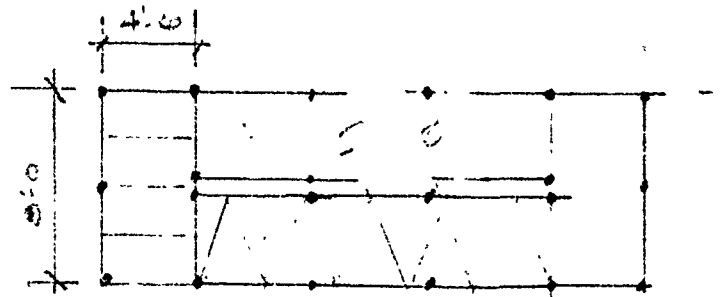
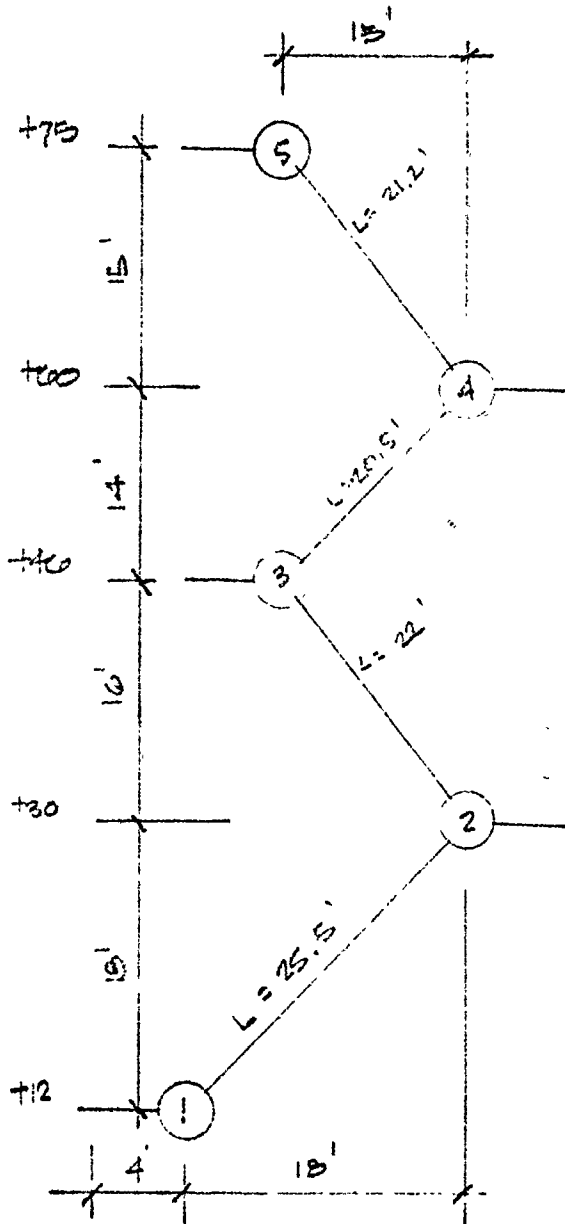
STAIRWAY

ITEM	SIZE	UNIT WT. #	UNIT LENGTH FT	NO. REQD.	TOTAL WT. #	UNIT AREA #/1	TOTAL SURFACE SQ. FT.
SPRINGER	2 1/2 x 20.7	0.0207	89.2	2	3.7 ^k	3	55.5 #
HORIZ. WALKWAY RAIL	1.9" φ x 0.145	0.00272	89.2	4	1.0 ^k	0.5	173.4 #
VERT. HAND-RAIL	1.9" φ x 0.145	0.00272	3.5	47	0.4 ^k	0.5	82.3 #
TREADS	1" x 30" x 10 7/8" GRATING	0.00730	2.5	10	1.3 ^k	2.0	350 #
HICK P.	4" x 4" P	0.001	89.2	2	0.2 ^k	0.71	120.7 #
LANDING	14.6 x 15.5	0.0155	25	5	1.94 ^k	3	375 #
LANDING	1" x 30" GRATING	0.00730	5' x 4.5'	5	1.32 ^k	2	300 #
LANDING BRACE	6 3/4" φ x 0.25	0.017	6.4	5	0.01 ^k	1.73	61.8 #
LANDING BRACE	4 1/2" φ x 0.237	0.0103	25	5	1.35 ^k	1.18	147.5 #
STAIR BRACE	2 3/8 x 4 1/4	0.0232	3.3	10 x 5	0.53 ^k	0.7	115.5 #
LANDING JOIST	3 1/2" x 4" x 0.30	0.0102	4.5	3 x 5	0.7 ^k	0.92	62.1 #
LANDING HAND-RAIL	1.9" φ x 0.145	0.00272	18	2 x 5	0.49 ^k	0.5	90 #
					13.54 ^k		
	+ 15% MISCELLANEOUS			=	2.06		
STAIRWAY TOTALS					15.0 ^k 7.8 ^T	1	2434 #

CREST OFFSHORE, INC.

Sheet 13.14 of 4 PAGES

By JWS Client U.S. NAVY Subject STRUCTURAL ANALYSIS
 Date 5.13.76 Job No. 27-271-76 Calculation VT. TAKE OFF



VERT HANDRAILS 3/STAIR X 4 STAIRS = 32
 3/LANDINGS = 15
47

TRADES @ 8" RES
 5-4 - 23.5' @ 3.8" = 21 TRADES
 4-3 21 3.8 = 20 "
 3-2 24 2.8 = 23 "
 2-1 27 2.8 = 26
70

CREST OFFSHORE, INC.

Sheet 13.15 of 40

By JAS Client U.S. NAVY Subject STRUCTURAL CONCEPT ANALYSIS
 Date 5-1-78 Job No. 22-71-1 Calculation AREA TAKE-OFF

13.5 PAINTING AND ANODES
AREA TAKE-OFF DATA

A. AREAS ABOVE SPLASH ZONE TO BE PAINTED

JACKET WALKWAY	=	2678	#
SUPER-STRUCTURE	=	8315	#
BOAT LANDING	=	610	#
STAIRWAY	=	2,484	#

TOTAL AREA = 14 087 #

B. AREAS BELOW SPLASH ZONE REQUIRING ANODES

FILING	=	6756	#	BELOW MUD LINE
JACKET	=	12,332	#	
PORT LANDING	=	466	#	

TOTAL AREA = 11,343 # ABOVE MUDLINE

C. TOTAL CURRENT REQUIREMENTS

$$I = 6 \text{ amp/#} \times 11,343 \# + 2 \text{ amp/#} \times 6756 \#$$

$$= 81,600 \text{ amp} = 81.60 \text{ amp}$$

D. CAPACITY OF ALLOY

$$\text{ALLOY CAP.} = \text{SINO} = \frac{1250 \text{ amp-hrs}}{16}$$

E. TOTAL WT. OF ANODES REQ'D

$$W_a = \frac{81.60 \text{ amp} \times 1250 \text{ amp-hrs} \times 24 \text{ hrs/day}}{1250 \text{ amp-hrs/#}}$$

$$= 5722.7 \text{ lbs}$$

F. NO. OF ANODES REQ'D

$$N = \frac{5722.7 \text{ lbs}}{323 \text{ lbs/#}} = 17.61 \text{ or } 18 \text{ ANODES}$$

CREST OFFSHORE, INC.

Sheet 13.0 of 10

By JWS Client USIT Subject STRUCTURAL CONCEPT ANALYSIS
Date 4-28-76 Job No. 27-77-92 Calculation WEIGHT TAKE-OFF

13.0 WEIGHT TAKE-OFF DATA

PIPING	=	340.8 ^T
PIPING EXCESS	=	21.0 ^T
JACKET	=	158.0 ^T
JACKET WALKWAY	=	4.4 ^T
BOAT LANDING	=	9.2 ^T
SUPER STRUCTURE	=	86.0 ^T
STAIRWAY	=	7.8 ^T
		<hr/>
TOTAL WEIGHT	=	627.2 ^T

EXHIBIT A

Laterally Loaded Pile Capacity

TSU40 JOB (M) IN FROM GROUP=PM027 , DSPECR , DEVICE=PM027R01, 0A3
 //LFC901 JOB (00442705002777)007(LFC27011),J.ATKINSON,PRTY=4,CLASS=A,C
 // TIME=(005),REGION=254
 //MAIN LINE=(005,*,*),CARD=100,C
 //JOBLINE DO DS=ENCPRD,LIR037,NISP=SHR
 // EXEC PGM=TKC997
 //FT06F001 DD SYSOUT=A
 //FT05F001 DD *

LFC901 IEF4031 LFC9011 STARTED TIME=13.52.52
 LFC901 IEF234E D 6AC,ASPHAC FT06F001
 LFC901 *37 IFCASPO PAC IS LFC901 A
 LFC901 *38 IFCASPO PAC IS LFC901 ASP10001
 LFC901 IEC202E K 61E,017345,NL,LFC9011,
 LFC901 IFLFC9011 CC=00442705 PM=2777100 Y J=LFC27011 M=J.ATKINSON A=71345
 LFC901 IEF200J LFC9011 F00F0 TIME=13.53.12
 //LFC901 JOB (00442705002777100)LECP27011),J.ATKINSON,PRTY=4,CLASS=A,*
 // TIME=(005),REGION=254
 //JOBLINE DO DS=ENCPRD,LIR037,NISP=SHR
 // EXEC PGM=TKC997
 //FT06F001 DD SYSOUT=A
 //FT05F001 DD *

//FT05F001 DD UNITS=(TFC,OFFFRY,DSNAME=KASPI0001,
 // DISP=(M,D,DELETE),VOL=SER=017345,DCB=(LRECL=80,HLKSIZE=80.RECFM=F)
 //
 IEF230I ALLCG. FOR LFC901
 IEF237I 197 ALLOCATED TO JOBLIR
 IEF237I 6AC ALLOCATED TO FT06F001
 IEF237I 6AC ALLOCATED TO FT05F001
 IEF142I - STEP WAS EXECUTED - COND CODE 0000 PASSED
 IEF245I DCP400.LIR03
 IEF245I VOL SER NOS= DC507P.
 IEF245I SYS76113,113J009,RV001,LFC9011,ASPDAD001 DELETED
 IEF245I VOL SER NOS= ASPHAC.
 IEF245I SYS76113,113J409,RV001,LFC9011,ASPT0001 DELETED
 IEF245I VOL SER NOS= 017345.
 IEF373I STEP / / START 76113.1352
 IEF374I STEP / / STOP 76113.1353 CPU 0M1N 01.30SEC STMR V1RY 100K

PERFE DATA ACQUISITION SYSTEM

STEP NAME	START TIME	13.52.00	MAIN CORE REFD	256 K	LCS CORE REFD	0 K	STEP CPU	00.00.01.39 *
PGM NAME	STOP TIME	13.53.11.52	MAIN CORE USED	100 K	LCS CORE USED	0 K	JOB CPU	00.00.01.39 *
DISPATCH	PRTY	1	MAIN CORE RORRD	0 K	LCS CORE RORRD	0 K	CONDITION CODE	0000 *
EXCP STATISTICS								
EXCP COUNT	UNIT	EXCP COUNT	UNIT	EXCP COUNT	UNIT	EXCP COUNT	UNIT	EXCP COUNT
197	0	6AC	453	6AE	43			
EXCP TOTAL 476								
STEP011 STEP/								
STEP051 DCP400.LIR03								
STEP051 VOL SER NOS= DC500P.								
STEP051 JOB /LFC901 / START 76113.1352								
STEP051 JOB /LFC901 / STOP 76113.1353 CPU 0M1N 01.30SEC								

FE3701 JOB / LEC9011 / START 76113.1352
FE3701 JOB / LEC9011 / STMP 76113.1553 CPU 0414 01.39SEC

PACES DATA ACQUISITION SYSTEM

JOB LOG NUMBER = LEC9011 76113 13.49.09.35

PROGRAMMER J. ATKINSON

DATE 04/22/76 76.113 INITIATION TIME 13.52.52.00

ACCTG DATA 00442705002777100YLEC27011

CPU TIME 00.00.01.39 TERMINATION TIME 13.53.11.89

PROGRAM LEC9011

PRIORITY 02 ELAPSED TIME 00.00.19.89

SYSTEM TO 58 = 50

CLASS A COMPLETION STATUS 00000

LATERALLY LOADED PILE PROGRAM

HEAD COLUMN ANALYSIS USING RIGID LINK FINITE ELEMENT TECHNIQUES

NUMBER OF PROBLEMS IN SET = 1

GENERAL PROBLEM TITLE = 4-PILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) J. ATKINS

PROBLEM DESCRIPTION = HUDLINE CONDITIONS V=98.8KIP R=5858102 P= 1712 KIP COMP.

TABLE 1 = PROGRAM CONTROLS DATA FOR PROJ NO. 1

PRIOR DATA OPTIONS (HOLD = 1)	DEFLS.	TABLE NUMBER				
		2	3	4A	4B	5
	0	0	0	0	0	0

NUMBER OF CARDS INPUT THIS PROBLEM

TABLE NUMBER		TABLE NUMBER	
3	4A	4B	5
1	0	3	30

LIST OF POSTER STATIONS = 5 10 15 20 30

TABLE 2 = CONSTANTS AND ITERATION CONTROL DATA

NUMBER OF INCREMENTS = 60
CALCULATE STRESSES (YES = 1) (NO = 0) = 1
INCREMENT LENGTH = 0.3600 02
MAXIMUM ALLOWABLE DEFLECTION = 0.1000 03
DEFLECTION CONTROL TOLERANCE = 0.1000 02
MODULUS OF ELASTICITY = 0.3000 05
PILE RADIUS = 0.1600 02
STA. AT WHICH FORCES AND DEFLECTIONS SPECIFIED FOR RETENTION = 1

DEFLECTION CLOSURE TOLERANCE LESS THAN 0.001 SUGGESTED

TABLE 3 - SPECIFIED DEFLECTIONS AND SLOPES

STATION	CASE	DEFLECTION	SLOPE
			NONE SPECIFIED

TABLE GA - PILE PROPERTIES

PROGRAM AUTOMATICALLY AVERAGES MOMENT OF INERTIA AND AREA VALUES AT CHANGES IN A AND I THUS, I VALUES FOR TOP AND BOTTOM ELEMENTS ARE HALVED.

FROM STA.	TO STA.	I
0	7	0.277C 05 0.1880 03
7	40	0.242D 05 0.163D 03
40	64	0.242D 05 0.163D 03

TABLE 4B - PILE LOADINGS

FROM	TO	CONT.	Q	S	T	R	P
0	0	0	0.988D 02	0.0	0.0	0.586D 07	0.0
0	1	1	0.0	0.0	0.0	0.0	-0.171D 04
17	1	1	0.0	0.0	0.0	0.0	-0.131D 04
33	1	1	0.0	0.0	0.0	0.0	-0.910D 03
50	0	0	0.0	0.0	0.0	0.0	-0.600D 02

PROGRAM ASSUMES LOADING OF TABLE 4B AND 5 IS APPLIED AS
 CONCENTRATED LOADING AT INCREMENT POINTS. THEORETICAL SPRINGS
 CONNECTING RIGID FINITE ELEMENTS ARE LOCATED AT INCREMENT POINTS.

TABLE 5 - NONLINEAR LOAD AND SUPPORT CURVES

FROM TO CONT. MULTIPLIER MULTIPLIER TOTAL NO. OF POINTS SYMMETRY

0 1 -0.3000-01 0.1000-02 2 1

ACTUAL Q-W CURVE POINTS

Q 0.3000 04 0.0 0.0 0.3000 04
 W -0.9900 02 -0.9900 02 0.9800 02 0.9900 02

FROM TO CONT. MULTIPLIER MULTIPLIER TOTAL NO. OF POINTS SYMMETRY

2 1 -0.3000-01 0.1000-02 2 1

ACTUAL Q-W CURVE POINTS

Q 0.3000 04 0.0 0.0 -0.3000 04
 W -0.9900 02 -0.9900 02 0.9800 02 0.9900 02

FROM TO CONT. MULTIPLIER MULTIPLIER TOTAL NO. OF POINTS SYMMETRY

3 1 -0.3000-01 0.1000-02 3 1

ACTUAL Q-W CURVE POINTS

Q 0.8790 01 0.8790 01 0.6660 01 0.6060 01 0.5220 01 0.4320 01 0.3060 01
 W -0.9990 02 -0.2000 02 -0.1130 01 -0.5000 00 -0.3000 00 -0.1800 00 -0.7000 01 -0.1600 01

Q -0.3000 01 -0.4320 01 -0.5220 01 -0.6060 01 -0.6660 01 -0.8790 01 -0.8790 01
 W 0.1600 01 0.7000 01 0.1800 00 0.3000 00 0.5000 00 0.1130 01 0.2000 02 0.9990 02

FROM TO CONT. MULTIPLIER MULTIPLIER TOTAL NO. OF POINTS SYMMETRY

5 1 -0.3000-01 0.1000-02 5 1

ACTUAL Q-W CURVE POINTS

Q	0.8900 02	0.8900 02	0.5560 02	0.4300 02	0.3330 02	0.2070 02	0.7940 01
W	-0.9990 02	-0.2000 02	-0.1130 01	-0.5000 00	-0.1700 00	-0.6300 01	-0.9000 02
Q	-0.7980 01	-0.2070 02	-0.5550 02	-0.4300 02	-0.8900 02	-0.8900 02	-0.3900 02
W	0.9000 02	0.6300 01	0.1700 00	0.2900 00	0.5000 00	0.1130 01	0.2000 02

FROM TO CONT. QMULTIPLIER QMULTIPLIER TOTAL NO. OF POINTS SYMMETRY

7 1 -0.3000 01 0.1000 02 8 1

ACTUAL Q-W CURVE POINTS

Q	0.1870 03	0.1870 03	0.1170 03	0.9080 02	0.7110 02	0.4560 02	0.2260 02
W	-0.9990 02	-0.2000 02	-0.1130 01	-0.5000 00	-0.1800 00	-0.7100 01	-0.1700 01
Q	-0.2290 02	-0.4560 02	-0.7110 02	-0.9080 02	-0.1170 03	-0.1870 03	-0.1870 03
W	0.1700 01	0.7100 01	0.1800 00	0.5000 00	0.1130 01	0.2000 02	0.9990 02

FROM TO CONT. QMULTIPLIER QMULTIPLIER TOTAL NO. OF POINTS SYMMETRY

9 1 -0.3000 01 0.1000 02 8 1

ACTUAL Q-W CURVE POINTS

Q	0.2090 03	0.2090 03	0.1310 03	0.1010 03	0.7880 02	0.4950 02	0.2180 02
W	-0.9990 02	-0.2000 02	-0.1130 01	-0.5000 00	-0.2900 00	-0.1700 00	-0.6600 01
Q	-0.2180 02	-0.4950 02	-0.7880 02	-0.1010 03	-0.2090 03	-0.2090 03	-0.2090 03
W	0.1200 01	0.6600 01	0.1700 00	0.2900 00	0.1130 01	0.2000 02	0.9990 02

FROM TO CONT. QMULTIPLIER QMULTIPLIER TOTAL NO. OF POINTS SYMMETRY

12 1 -0.3000 01 0.1000 02 8 1

ACTUAL Q-W CURVE POINTS

Q	0.4920 03	0.4920 03	0.4920 03	0.3070 03	0.2420 03	0.1540 03	0.1500 03
W	-0.9990 02	-0.2000 02	-0.1130 01	-0.5000 00	-0.3000 00	-0.1900 00	-0.9000 01

0.3400=01 0.9000=01 0.1900 00 0.5000 00 0.1130 01 0.2000 02 0.9990 02

FROM TO CONT. MULTIPLIER MULTIPLIER TOTAL NO. OF POINTS SYMMETRY

19 1 -0.3000=01 0.1000=02 6 1

ACTUAL Q-W CURVE POINTS

H	0.7650 03	0.7650 03	0.4780 03	0.3770 03	0.3020 03	0.2100 03	0.1400 03
W	-0.9990 02	-0.2000 02	-0.1130 01	-0.5000 00	-0.3000 00	-0.9000=01	-0.3800=01
Q	-0.1400 03	-0.2100 03	-0.3020 03	-0.3770 03	-0.4780 03	-0.7650 03	-0.7650 03
W	0.3800=01	0.9000=01	0.1900 00	0.5000 00	0.1130 01	0.2000 02	0.9990 02

FROM TO CONT. MULTIPLIER MULTIPLIER TOTAL NO. OF POINTS SYMMETRY

26 1 -0.3000=01 0.1000=02 6 1

ACTUAL Q-W CURVE POINTS

G	0.1070 04	0.1070 04	0.6660 03	0.5250 03	0.4210 03	0.2920 03	0.1940 0.
H	-0.9990 02	-0.2000 02	-0.1130 01	-0.5000 00	-0.3000 00	-0.9000=01	-0.3800=01
Q	-0.1940 03	-0.2920 03	-0.4210 03	-0.5250 03	-0.6660 03	-0.1070 04	-0.1070 04
W	0.3800=01	0.9000=01	0.1900 00	0.5000 00	0.1130 01	0.2000 02	0.9990 02

FROM TO CONT. MULTIPLIER MULTIPLIER TOTAL NO. OF POINTS SYMMETRY

49 1 -0.3000=01 0.1000=02 6 1

ACTUAL Q-W CURVE POINTS

Q	0.1990 04	0.1990 04	0.1250 04	0.9830 03	0.7880 03	0.5470 03	0.3660 03
W	-0.9990 02	-0.2000 02	-0.1130 01	-0.5000 00	-0.3000 00	-0.9000=01	-0.3800=01
Q	-0.3660 03	-0.5470 03	-0.7880 03	-0.9830 03	-0.1250 04	-0.1990 04	-0.1990 04
W	0.3800=01	0.9000=01	0.1900 00	0.5000 00	0.1130 01	0.2000 02	0.9990 02

PROBLEM NUMBER = 1

PROBLEM DESCRIPTION = MUDLINE CONDITIONS VERR, AKIP R25858102 Ps 1712 KIP COMP.

ITERATION HISTORY DATA

ITER. NUP.	OFF CURVES	NO. STAS. NOT CLOSED	DEFLS. AT MONITOR STAS.					
			5	10	15	20	30	
1	NO	14	0.496D-01	-0.638D-02	0.609D-04	0.312D-04	0.198D-06	
2	NO	14	0.133D 00	-0.819D-02	-0.523D-03	0.999D-04	0.331D-06	
3	NO	9	7.155D 00	-0.687D-02	-0.815D-03	0.120D-03	0.324D-06	
4	NO	9	0.157D 00	-0.645D-02	-0.853D-03	0.121D-03	0.314D-06	
5	NO	0	0.157D 00	-0.645D-02	-0.853D-03	0.121D-03	0.314D-06	

RESULTS

STA.	DIST. ALONG PILE	DEFL.	SLOPE	MOMENT	BEAM COLUMN SMFAR	APPLIED NON LATERAL LOADING	COMBINED STRESS(MAX.)
0	0.0	0.534	-0.16710-02	-0.9790 04	0.0	0.0	-15.460
1	36.000	0.486	-0.20160-02	-0.6120 04	102.198	0.0	-12.908
2	72.000	0.349	-0.22010-02	-0.2440 04	102.460	0.0	-10.428
3	108.000	0.308	-0.22270-02	0.1260 04	99.409	-6.083	-9.536
4	144.000	0.229	-0.20970-02	0.4720 04	85.204	-21.806	-11.864
5	180.000	0.157	-0.18350-02	0.7390 04	57.949	-31.771	-13.272
6	216.000	0.097	-0.14820-02	0.8890 04	22.416	-38.104	-14.125
7	252.000	0.050	-0.10810-02	0.9000 04	-14.701	-56.822	-15.851
8	288.000	0.019	-0.06070-03	0.7760 04	-46.958	-74.422	-15.111
9	324.000	0.001	-0.34880-03	0.5620 04	-60.675	-91.983	-13.374
10	360.000	-0.006	-0.12510-03	0.3400 04	-55.564	-12.881	-11.573
11	396.000	-0.008	-0.72510-05	0.1620 04	-40.677	17.261	-10.110
12	432.000	-0.006	0.51080-04	0.4670 03	-24.453	15.337	-8.106
13	468.000	-0.004	0.59240-04	-0.1380 03	-11.390	10.808	-8.717
14	504.000	-0.002	0.47660-04	-0.3530 03	-2.916	6.106	-8.733
15	540.000	-0.011	0.29680-04	-0.3480 03	1.409	2.494	-8.504
16	576.000	-0.000	0.14820-04	-0.2520 03	2.824	6.296	-8.368
17	612.000	0.000	0.49920-05	-0.1480 03	2.633	-0.704	-8.148
18	648.000	0.000	-0.13230-05	-0.6220 02	1.827	-0.921	-7.930
19	684.000	0.000	-0.10950-05	-0.1290 02	0.994	-0.749	-7.740
20	720.000	0.000	-0.20530-05	0.9380 01	0.385	-0.468	-7.584
21	756.000	0.000	-0.14820-05	0.1480 02	0.040	-0.221	-7.834
22	792.000	0.000	-0.61090-06	0.1230 02	-0.101	-0.060	-7.279
23	828.000	-0.000	-0.32020-06	0.7530 01	-0.122	0.019	-7.122
24	864.000	-0.000	-0.47710-07	0.3860 01	-0.092	0.043	-6.966
25	900.000	-0.000	0.61680-07	0.9260 00	-0.052	0.038	-6.811
26	936.000	-0.000	0.77770-07	-0.2530 00	-0.021	0.024	-6.657
27	972.000	-0.000	0.57370-07	-0.5700 00	-0.003	0.011	-6.503
28	1008.000	-0.000	0.31200-07	-0.4850 00	0.004	0.003	-6.350
29	1044.000	0.000	0.11930-07	-0.2920 00	0.005	-0.001	-6.197
30	1080.000	0.000	0.15460-08	-0.1270 00	0.004	-0.002	-6.043
31	1116.000	0.000	-0.23310-08	-0.2920-01	0.002	-0.002	-5.890
32	1152.000	0.000	-0.27370-08	0.1280-01	0.001	-0.001	-5.736
33	1188.000	0.000	-0.14830-08	0.2180-01	0.000	-0.000	-5.583
34	1224.000	0.000	-0.93440-09	0.1660-01	-0.000	-0.000	-5.430
35	1260.000	0.000	-0.29870-09	0.9000-02	-0.000	0.000	-5.276
36	1296.000	0.000	0.66030-11	0.3310-02	-0.000	0.000	-5.123
37	1332.000	-0.000	0.96950-10	0.3300-03	-0.000	0.000	-4.969
38	1368.000	-0.000	0.87670-10	-0.7040-03	-0.000	0.000	-4.816
39	1404.000	-0.000	0.51490-10	-0.7510-03	0.000	0.000	-4.663
40	1440.000	0.000	0.21120-10	-0.4760-03	0.000	0.000	-4.510
41	1476.000	0.000	-0.39970-11	-0.2130-03	0.000	0.000	-4.356
42	1512.000	0.000	-0.25570-11	-0.5170-04	0.000	0.000	-4.203
43	1548.000	0.000	-0.34510-11	0.1560-04	0.000	0.000	-4.049
44	1584.000	0.000	-0.25440-11	0.2900-04	0.000	0.000	-3.896
45	1620.000	-0.000	-0.10960-11	0.2130-04	-0.000	0.000	-3.742
46	1656.000	-0.000	-0.30430-12	0.1660-04	-0.000	0.000	-3.588
47	1692.000	-0.000	0.44200-12	0.4450-05	-0.000	0.000	-3.435
48	1728.000	-0.000			-0.000	0.000	-3.281
49	1764.000	-0.000			-0.000	0.000	-3.128
50	1800.000	-0.000			-0.000	0.000	-2.974
51	1836.000	-0.000			-0.000	0.000	-2.821
52	1872.000	-0.000			-0.000	0.000	-2.667
53	1908.000	-0.000			-0.000	0.000	-2.514
54	1944.000	-0.000			-0.000	0.000	-2.360
55	1980.000	-0.000			-0.000	0.000	-2.207
56	2016.000	-0.000			-0.000	0.000	-2.053
57	2052.000	-0.000			-0.000	0.000	-1.900
58	2088.000	-0.000			-0.000	0.000	-1.746
59	2124.000	-0.000			-0.000	0.000	-1.593
60	2160.000	-0.000			-0.000	0.000	-1.439
61	2196.000	-0.000			-0.000	0.000	-1.286
62	2232.000	-0.000			-0.000	0.000	-1.132
63	2268.000	-0.000			-0.000	0.000	-0.979
64	2304.000	-0.000			-0.000	0.000	-0.825
65	2340.000	-0.000			-0.000	0.000	-0.672
66	2376.000	-0.000			-0.000	0.000	-0.518
67	2412.000	-0.000			-0.000	0.000	-0.365
68	2448.000	-0.000			-0.000	0.000	-0.211
69	2484.000	-0.000			-0.000	0.000	-0.058
70	2520.000	-0.000			-0.000	0.000	0.095
71	2556.000	-0.000			-0.000	0.000	0.248
72	2592.000	-0.000			-0.000	0.000	0.401
73	2628.000	-0.000			-0.000	0.000	0.554
74	2664.000	-0.000			-0.000	0.000	0.707
75	2700.000	-0.000			-0.000	0.000	0.860
76	2736.000	-0.000			-0.000	0.000	1.013
77	2772.000	-0.000			-0.000	0.000	1.166
78	2808.000	-0.000			-0.000	0.000	1.319
79	2844.000	-0.000			-0.000	0.000	1.472
80	2880.000	-0.000			-0.000	0.000	1.625
81	2916.000	-0.000			-0.000	0.000	1.778
82	2952.000	-0.000			-0.000	0.000	1.931
83	2988.000	-0.000			-0.000	0.000	2.084
84	3024.000	-0.000			-0.000	0.000	2.237
85	3060.000	-0.000			-0.000	0.000	2.390
86	3096.000	-0.000			-0.000	0.000	2.543
87	3132.000	-0.000			-0.000	0.000	2.696
88	3168.000	-0.000			-0.000	0.000	2.849
89	3204.000	-0.000			-0.000	0.000	3.002
90	3240.000	-0.000			-0.000	0.000	3.155
91	3276.000	-0.000			-0.000	0.000	3.308
92	3312.000	-0.000			-0.000	0.000	3.461
93	3348.000	-0.000			-0.000	0.000	3.614
94	3384.000	-0.000			-0.000	0.000	3.767
95	3420.000	-0.000			-0.000	0.000	3.920
96	3456.000	-0.000			-0.000	0.000	4.073
97	3492.000	-0.000			-0.000	0.000	4.226
98	3528.000	-0.000			-0.000	0.000	4.379
99	3564.000	-0.000			-0.000	0.000	4.532
100	3600.000	-0.000			-0.000	0.000	4.685

MOVEMENTS AND REACTIONS STORED FOR STIFFNESS COEFFICIENT DEVELOPMENT USE

DEFLECTIONS	ROTATIONS	MOMENTS	LATERAL SHEAR	AT STATION
0.866326 00	-0.201500-02	-0.612140 00	0.987750 02	1

0.866326 00

-0.201500-02

-0.612140 00

0.987750 02

1

USER SHOULD NOTE LATERAL SHEAR ABOVE IS TRUE SHEAR AT STATION
 SHEAR GIVEN IS PERPENDICULAR TO ORIGINAL UNDEFLECTED MEMBER AXIS.

ASP JOB NO. = 7345

DATE = 76.113

//LECP011 JOB (0004270500277100TLEC27011),J,ATKINSON',PRTY=4,CLASS=A,C7345

ELAPSED TIME OK MAIN = 8 * 000.39, START TIME = 13.52.53

DDNAME = SYSMSG
DDNAME = FIC06F001
LINES OUTPUT FOR THIS JOB = 000510

PRINTED ON RM027PRI, LINES = 000077

PRINTED ON RM027PRI, LINES = 000453

CARDS FROM MAIN FOR THIS JOB = NONE

EXHIBIT B

STRAN - Space Fram Analysis

SYNERCOM TECHNOLOGY USER BULLETIN

MARCS BULLETIN NO. 4
*** STRAN 6 ***
04/05/76

STRAN RELEASE MOD 12

ALL ERROR MESSAGES WERE CORRECTED FOR EASY REFERENCE. A FEW INSTANCES
OF PRINTING THE WRONG ERROR MESSAGE WERE CORRECTED.

THE DAYFILE MESSAGE OF UNIT NO. FOLLOWED BY FOUR INTEGERS WAS
REPLACED BY A MORE EXPLANATORY MESSAGE.

NEW FEATURE: USER SELECTION OF METHOD FOR CALCULATING ALLOWABLE
MEMBER STRESSES AND UNITY CHECKS:

1. ON THE OPTIONS CARD COLUMNS P5=26 TWO NEW SELECTIONS NA AND UC.
AP WERE ADDED TO PREVIOUSLY ALLOWED SELECTIONS BLANKS AND UC.
BLANKS-- SUPPRESSES CALCULATIONS OF ALLOWABLE MEMBER STRESSES
AND UNITY CHECKS.

UC -- REQUESTS CALCULATION IN ACCORDANCE WITH AMERICAN
INSTITUTE OF STEEL CONSTRUCTION (AISC) 1949 CODE.

INVESTIGATION OF LOCAL HUCKLING FOLLOWS THE RECOM-
MENDATION OF THE COLUMN RESEARCH COUNCIL. THE UC
OPTION HAS NOT CHANGED FROM PRIME STRAN RELEASES.

NA -- 3-DIGIT AS UC, BUT IN COMPUTING COMBINED UNITY CHECK
ALSO FORMULA 1.6-2 IS ALWAYS USED; FORMULAS 1.6-1A
AND 1.6-1B ARE NEVER USED.

AP -- REQUESTS CALCULATION IN ACCORDANCE WITH AFRICAN
PETROLEUM INSTITUTE (API) REPORT API-RP-2A.

2. THE SELECTION P5 ON THE OPTIONS CARD COLUMNS 27-28 WAS DE-
LETED. IF SELECTED, IT WILL BE IGNORED AND A WARNING MESSAGE
WILL BE PRINTED.

3. GROUP CARD COLUMN 701 IF THE LETTER N APPEARS, LOCAL HUCKLING
EFFECTS WILL NOT BE INVESTIGATED FOR ALL TUBULAR MEMBERS
HAVING THIS GROUP CODE.

NOTE - IF YOU DO NOT HAVE ALL OF THE BULLETINS, YOU
MAY OBTAIN A COPY BY SUBMITTING A RUN
WITH THE FOLLOWING CONTROL CARDS:

GET, MARCUL.
COPYSHF, MARCSUL.

 * STRAN
 * A SYNERCOM TECHNOLOGY, INC. DEVELOPMENT *
 * RELEASE 6, NOV 12
 * MARCH 1976
 *

DATE 08/30/76

4-PILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

PROGRAM OPTIONS

THE FOLLOWING OPTIONS HAVE BEEN REQUESTED FOR THIS ANALYSIS

INPUT *****CARD PLUS DATA FILE INPUT

INPUT UNITS *****ENGLISH

OUTPUT UNITS *****ENGLISH

EXECUTION *****UNITY CHECK
 ..UNITY CHECKS COMPUTED BY AMERICAN INSTITUTE OF
 STEEL CONSTRUCTION 1969 CODE, SECTIONS 1.5, 1.6,
 LOCAL BUCKLING STRESS OF TUBULAR WE BERS INVES-
 TIGATED BY COLUMN RESEARCH COUNCIL METHOD.
 RESULTS INVALID FOR A514 STEEL.
 ***** NO. OF SEGMENTS 1
 ***** VARBL MEMB, SEGMENTS/SECT 1

LOAD ***** NO. BASIC LOAD COMDS. 3
 ***** NO. COMBINED LOAD COMDS. 2

REPORT ***** INPUT ECHO AND GROUP PROP PRINT
 ***** JOINT DEFLECTIONS PRINT
 ***** GROUP AND UN CHK SUMMARY PRINT
 ***** MEMBER STRESS REPORT NO. 1 PRINT
 ***** MEMBER STRESS REPORT NO. 2 PRINT
 ***** MEMBER STRESS REPORT NO. 3 PRINT
 ***** REACTION FORCES AND MOMTS PRINT
 ***** EQUILIBRIUM CHECK PRINT

EQUILIBRIUM CHECK EDIT VALUES
 ***** FORCES 100.00 LB
 ***** MOMENTS 100.00 IN-LB

STRAN - GROUP PROPERTIES REPORT

PAGE 1
DATE 04/30/76

4-PILE AC/R STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON
TUBULAR MEMBER PROPERTIES

GRP	M/S	JOINT THICK FT.	WT IN.	OD IN.	AX IN2	IX IN2	IY IN2	IZ IN2	FY KSI	KY	KZ	SHEAR AREA IN2	INPUT SEC LEN FT.
*** E = 29000000.0 PSI, G = 11600000.0 PSI ***													
MB1	2	3.00	.500	14.00	21.21	967.51	483.76	483.76	36.0	1.0	1.0	21.21	-0.00
HR2	2	3.00	.500	14.00	27.49	2100.54	1053.17	1053.17	36.0	1.0	1.0	27.49	-0.00
DR1	2	3.00	.625	20.00	58.04	3573.94	1786.97	1786.97	36.0	.8	.8	38.04	-0.00
DR2	2	3.00	.500	14.00	27.49	2100.54	1053.17	1053.17	36.0	.8	.8	27.49	-0.00
XN1	2	2.00	.500	14.00	24.35	1463.88	731.94	731.94	36.0	.7	.7	24.35	-0.00
XR2	2	2.50	.500	18.00	27.49	2100.54	1053.17	1053.17	36.0	.7	.7	27.49	-0.00
XR3	2	2.50	.500	20.00	30.63	2913.73	1456.86	1456.86	36.0	.7	.7	30.63	-0.00
S3C	1	-0.00	1.250	30.00	112.90	23374.03	11687.02	11687.02	36.0	1.0	1.0	112.90	-0.00
JL1	1	-0.00	.500	40.00	62.05	24205.87	12102.94	12102.94	36.0	1.0	1.0	62.05	-0.00
P1	1	-0.00	1.250	36.00	136.46	41250.18	20625.09	20625.09	36.0	1.0	1.0	36.46	-0.00
P2	1	-0.00	1.500	36.00	162.58	48468.39	24234.20	24234.20	36.0	1.0	1.0	162.58	-0.00
DA3	2	2.50	.375	14.00	16.05	745.52	372.76	372.76	36.0	.8	.8	16.05	-0.00
MB3	2	2.50	.375	14.00	16.05	745.52	372.76	372.76	36.0	1.0	1.0	16.05	-0.00

STRAN - GROUP PROPERTIES REPORT

PAGE 3
DATE 04/30/76

4-PLY ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON
PRISMATIC SECTION MEMBERS

GRP	M/S	THICK FT.	Z-DPTH IN.	V-DPTH IN.	AX IN2	IX IN2	IY IN2	IZ IN2	SY KSI	KY	KZ	INPUT SEC LEN FT.
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*** E = 29000000.0 PSI, G = 11600000.0 PSI ***

MEM	2	-0.00	10.00	5.00	50.00	30000.00	30000.00	30000.00	36.0	1.0	1.0	-0.00
P3	1	-0.00	36.00	36.00	12.04	48480.00	24240.00	24240.00	36.0	1.0	1.0	-0.00

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STRAN PUT DATA

4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- ,ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
50	MEMBER	18	14	SSC				30
51	MEMBER	15	9	SSC				30
52	MEMBER	17	11	DA3				14
53	MEMBER	18	9	DA3				14
54	MEMBER	17	14	DA3				14
55	MEMBER	16	9	DA3				14
56	MEMBER	17	16	HA3				14
57	MEMBER	18	15	HA3				14
58	MEMBER	17	18	HA3				14
59	MEMBER	16	15	HA3				14
60	MEMBER	21	17	SSC				30
61	MEMBER	20	16	SSC				30
62	MEMBER	22	18	SSC				30
63	MEMBER	19	15	SSC				30
64	MEMBER	25	21	JLI				F 40
65	MEMBER	24	20	JLI				F 40
66	MEMBER	20	22	JLI				F 40
67	MEMBER	23	19	JLI				F 40
68	MEMBER	25	24	HAI				14
69	MEMBER	26	23	HAI				14
70	MEMBER	25	26	HAI				14
71	MEMBER	24	23	HAI				14
72	MEMBER	25	27	XAI				16
73	MEMBER	27	23	XAI				16
74	MEMBER	26	27	XAI				16
75	MEMBER	27	24	XAI				16
76	MEMBER	30	25	JLI				F 40
77	MEMBER	29	24	JLI				F 40
78	MEMBER	31	26	JLI				F 40
79	MEMBER	24	23	JLI				F 40
80	MEMBER	29	25	DHI				20
81	MEMBER	28	26	DHI				20
82	MEMBER	31	25	DHI				20
83	MEMBER	26	24	DHI				20
84	MEMBER	30	29	HAI				14
85	MEMBER	31	28	HAI				14
86	MEMBER	30	31	HAI				14
87	MEMBER	29	28	HAI				14
88	MEMBER	30	32	XH2				18
89	MEMBER	32	28	XH2				18
90	MEMBER	31	32	CR2				18
91	MEMBER	32	29	CR2				18
92	MEMBER	35	30	JLI				F 40
93	MEMBER	34	29	JLI				F 40
94	MEMBER	30	31	JLI				F 40
95	MEMBER	33	28	JLI				F 40
96	MEMBER	35	29	DA2				18
97	MEMBER	36	28	DA2				18
98	MEMBER	35	31	DA2				18



STRAN IN UT DATA

4-PILE AGY OFFICE (MHC BY U.S. NAVY (36-IN. DIAMETER PILING)) - J. ATKINSON

LINE NO. 1 2 3 4 5 6 7 8

99	MEMBER	34	28	DR2						18
100	MEMBER	35	34	HR2						18
101	MEMBER	36	33	HR2						18
102	MEMBER	35	36	HR2						18
103	MEMBER	34	33	HR2						18
104	MEMBER	35	37	HR3						20
105	MEMBER	37	33	HR3						20
106	MEMBER	36	37	HR3						20
107	MEMBER	37	34	HR3						20
108	MEMBER	42	35	JL1				F	40	40
109	MEMBER	40	34	JL1				F	40	40
110	MEMBER	40	36	JL1				F	40	40
111	MEMBER	38	33	JL1				F	40	40
112	MEMBER	35	41	DR2						18
113	MEMBER	41	34	DR2						18
114	MEMBER	36	45	DR2						18
115	MEMBER	45	33	DR2						18
116	MEMBER	35	43	DR2						18
117	MEMBER	43	36	DR2						18
118	MEMBER	34	39	DR2						18
119	MEMBER	39	33	DR2						18
120	MEMBER	42	41	HR2						18
121	MEMBER	41	40	HR2						18
122	MEMBER	44	45	HR2						18
123	MEMBER	45	38	HR2						18
124	MEMBER	42	43	HR2						18
125	MEMBER	43	44	HR2						18
126	MEMBER	40	39	HR2						18
127	MEMBER	39	38	HR2						18
128	MEMBER	42	46	HR3						20
129	MEMBER	46	38	HR3						20
130	MEMBER	44	46	HR3						20
131	MEMBER	46	40	HR3						20
132	MEMBER	25	55	HRN						18
133	MEMBER	55	56	HRN						18
134	MEMBER	24	53	HR1						18
135	MEMBER	53	54	HRN						18
136	MEMBER	26	57	HRN						18
137	MEMBER	57	58	HRN						18
138	MEMBER	23	51	HRN						18
139	MEMBER	51	52	HRN						18
140	MEMBER	30	63	HRN						18
141	MEMBER	63	64	HRN						18
142	MEMBER	29	61	HRN						18
143	MEMBER	61	62	HRN						18
144	MEMBER	31	65	HRN						18
145	MEMBER	65	66	HRN						18
146	MEMBER	28	59	HRN						18
147	MEMBER	59	60	HRN						18

STRAN INPUT DATA

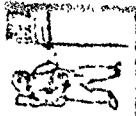
4-PILE AREA STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
295	LOAD Z	17	12	0.00	-.347	14.00	-.347	GL08 UNIF DL 0 1
296	LOAD Z	16	11	0.00	-.347	14.00	-.347	GL08 UNIF DL 0 1
297	LOAD Z	18	14	0.00	-.347	14.00	-.347	GL08 UNIF DL 0 1
298	LOAD Z	15	9	0.00	-.347	14.00	-.347	GL08 UNIF DL 0 1
299	LOAD Z	17	11	1.25	-.055	30.61	-.055	GL08 UNIF DL 0 1
300	LOAD Z	18	9	1.25	-.055	30.61	-.055	GL08 UNIF DL 0 1
301	LOAD Z	17	14	1.25	-.055	30.61	-.055	GL08 UNIF DL 0 1
302	LOAD Z	16	9	1.25	-.055	30.61	-.055	GL08 UNIF DL 0 1
303	LOAD Z	17	16	1.25	-.055	27.50	-.055	GL08 UNIF DL 0 1
304	LOAD Z	18	15	1.25	-.055	27.50	-.055	GL08 UNIF DL 0 1
305	LOAD Z	17	16	1.25	-.055	27.50	-.055	GL08 UNIF DL 0 1
306	LOAD Z	16	15	1.25	-.055	27.50	-.055	GL08 UNIF DL 0 1
307	LOAD Z	21	17	0.00	-.347	23.00	-.347	GL08 UNIF DL 0 1
308	LOAD Z	21	17	23.00	-.347	6.50	-.347	GL08 UNIF DL 0 1
309	LOAD Z	20	16	0.00	-.347	23.00	-.347	GL08 UNIF DL 0 1
310	LOAD Z	20	16	23.00	-.347	6.50	-.347	GL08 UNIF DL 0 1
311	LOAD Z	22	18	0.00	-.347	23.00	-.347	GL08 UNIF DL 0 1
312	LOAD Z	22	18	23.00	-.347	6.50	-.347	GL08 UNIF DL 0 1
313	LOAD Z	19	15	0.00	-.347	23.00	-.347	GL08 UNIF DL 0 1
314	LOAD Z	19	15	23.00	-.347	6.50	-.347	GL08 UNIF DL 0 1
315	LOAD Z	25	21	0.00	-.183	4.54	-.183	GL08 UNIF DL 0 1
316	LOAD Z	24	20	0.00	-.183	4.54	-.183	GL08 UNIF DL 0 1
317	LOAD Z	26	22	0.00	-.183	4.54	-.183	GL08 UNIF DL 0 1
318	LOAD Z	23	19	0.00	-.183	4.54	-.183	GL08 UNIF DL 0 1
319	LOAD Z	25	24	1.50	-.063	27.90	-.063	GL08 UNIF DL 0 1
320	LOAD Z	26	23	1.50	-.063	27.90	-.063	GL08 UNIF DL 0 1
321	LOAD Z	25	26	1.50	-.063	27.90	-.063	GL08 UNIF DL 0 1
322	LOAD Z	23	23	1.50	-.063	27.90	-.063	GL08 UNIF DL 0 1
323	LOAD Z	25	25	1.00	-.072	19.85	-.072	GL08 UNIF DL 0 1
324	LOAD Z	27	23	1.00	-.072	19.85	-.072	GL08 UNIF DL 0 1
325	LOAD Z	26	27	1.00	-.072	19.85	-.072	GL08 UNIF DL 0 1
326	LOAD Z	27	24	1.00	-.072	19.85	-.072	GL08 UNIF DL 0 1
327	LOAD Z	30	25	0.00	-.183	30.30	-.183	GL08 UNIF DL 0 1
328	LOAD Z	29	24	0.00	-.183	30.30	-.183	GL08 UNIF DL 0 1
329	LOAD Z	31	26	0.00	-.183	30.30	-.183	GL08 UNIF DL 0 1
330	LOAD Z	24	23	0.00	-.183	30.30	-.183	GL08 UNIF DL 0 1
331	LOAD Z	29	25	1.50	-.112	42.37	-.112	GL08 UNIF DL 0 1
332	LOAD Z	28	26	1.50	-.112	42.37	-.112	GL08 UNIF DL 0 1
333	LOAD Z	31	25	1.50	-.112	42.37	-.112	GL08 UNIF DL 0 1
334	LOAD Z	24	24	1.50	-.112	42.37	-.112	GL08 UNIF DL 0 1
335	LOAD Z	30	29	1.50	-.063	33.90	-.063	GL08 UNIF DL 0 1
336	LOAD Z	31	28	1.50	-.063	33.90	-.063	GL08 UNIF DL 0 1
337	LOAD Z	30	31	1.50	-.063	33.90	-.063	GL08 UNIF DL 0 1
338	LOAD Z	29	28	1.50	-.063	33.90	-.063	GL08 UNIF DL 0 1
339	LOAD Z	30	32	1.15	-.081	23.79	-.081	GL08 UNIF DL 0 1
340	LOAD Z	32	28	1.15	-.081	23.79	-.081	GL08 UNIF DL 0 1
341	LOAD Z	31	32	1.15	-.081	23.79	-.081	GL08 UNIF DL 0 1
342	LOAD Z	32	29	1.15	-.081	23.79	-.081	GL08 UNIF DL 0 1
343	LOAD Z	35	30	0.00	-.183	31.31	-.183	GL08 UNIF DL 0 1

STRAN INPUT DATA

#-PILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8	
344	LOAD Z	34	29	0.00	-.183	31.31	-.183	GLOR UNIF	DL 0 1
345	LOAD Z	36	31	0.00	-.183	31.31	-.183	GLOR UNIF	DL 0 1
346	LOAD Z	33	28	0.00	-.153	31.31	-.183	GLOR UNIF	DL 0 1
347	LOAD Z	35	29	1.50	-.081	47.70	-.081	GLOR UNIF	DL 0 1
348	LOAD Z	36	28	1.50	-.081	47.70	-.081	GLOR UNIF	DL 0 1
349	LOAD Z	35	31	1.50	-.081	47.70	-.081	GLOR UNIF	DL 0 1
350	LOAD Z	34	28	1.50	-.081	47.70	-.081	GLOR UNIF	DL 0 1
351	LOAD Z	35	34	1.50	-.081	40.10	-.081	GLOR UNIF	DL 0 1
352	LOAD Z	36	33	1.50	-.081	40.10	-.081	GLOR UNIF	DL 0 1
353	LOAD Z	35	36	1.50	-.081	40.10	-.081	GLOR UNIF	DL 0 1
354	LOAD Z	34	33	1.50	-.081	40.10	-.081	GLOR UNIF	DL 0 1
355	LOAD Z	35	37	1.25	-.091	27.98	-.091	GLOR UNIF	DL 0 1
356	LOAD Z	37	33	1.25	-.091	27.98	-.091	GLOR UNIF	DL 0 1
357	LOAD Z	36	37	1.25	-.091	27.98	-.091	GLOR UNIF	DL 0 1
358	LOAD Z	37	34	1.25	-.091	27.98	-.091	GLOR UNIF	DL 0 1
359	LOAD Z	42	35	0.00	-.183	35.35	-.183	GLOR UNIF	DL 0 1
360	LOAD Z	40	34	0.00	-.183	35.35	-.183	GLOR UNIF	DL 0 1
361	LOAD Z	44	36	0.00	-.183	35.35	-.183	GLOR UNIF	DL 0 1
362	LOAD Z	44	33	0.00	-.183	35.35	-.183	GLOR UNIF	DL 0 1
363	LOAD Z	35	41	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
364	LOAD Z	41	34	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
365	LOAD Z	36	45	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
366	LOAD Z	45	33	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
367	LOAD Z	35	43	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
368	LOAD Z	43	36	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
369	LOAD Z	34	39	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
370	LOAD Z	39	33	1.50	-.081	38.25	-.081	GLOR UNIF	DL 0 1
371	LOAD Z	42	41	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
372	LOAD Z	41	40	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
373	LOAD Z	44	45	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
374	LOAD Z	45	38	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
375	LOAD Z	42	43	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
376	LOAD Z	43	44	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
377	LOAD Z	40	39	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
378	LOAD Z	39	38	1.50	-.081	22.05	-.081	GLOR UNIF	DL 0 1
379	LOAD Z	42	46	1.25	-.091	32.93	-.091	GLOR UNIF	DL 0 1
380	LOAD Z	46	38	1.25	-.091	32.93	-.091	GLOR UNIF	DL 0 1
381	LOAD Z	44	46	1.25	-.091	32.93	-.091	GLOR UNIF	DL 0 1
382	LOAD Z	46	40	1.25	-.091	32.93	-.091	GLOR UNIF	DL 0 1
383	LOAD Z	25	55	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
384	LOAD Z	55	56	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
385	LOAD Z	24	53	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
386	LOAD Z	53	53	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
387	LOAD Z	26	47	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
388	LOAD Z	57	58	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
389	LOAD Z	23	51	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
390	LOAD Z	51	52	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
391	LOAD Z	30	63	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1
392	LOAD Z	63	64	0.00	-.148	2.83	-.148	GLOR UNIF	DL 0 1



STRAN PUT DATA

4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIA.FEYER PILING) -- J.ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
393	LOAD Z	29	61	0.00	0.148	2.83	0.148	DL 0 1
394	LOAD Z	61	62	0.00	0.148	2.83	0.148	DL 0 1
395	LOAD Z	31	65	0.00	0.148	2.83	0.148	DL 0 1
396	LOAD Z	65	66	0.00	0.148	2.83	0.148	DL 0 1
397	LOAD Z	28	59	0.00	0.148	2.83	0.148	DL 0 1
398	LOAD Z	59	60	0.00	0.148	2.83	0.148	DL 0 1
399	LOAD Z	35	71	0.00	0.148	2.83	0.148	DL 0 1
400	LOAD Z	71	72	0.00	0.148	2.83	0.148	DL 0 1
401	LOAD Z	34	69	0.00	0.148	2.83	0.148	DL 0 1
402	LOAD Z	69	70	0.00	0.148	2.83	0.148	DL 0 1
403	LOAD Z	36	73	0.00	0.148	2.83	0.148	DL 0 1
404	LOAD Z	73	74	0.00	0.148	2.83	0.148	DL 0 1
405	LOAD Z	33	67	0.00	0.148	2.83	0.148	DL 0 1
406	LOAD Z	67	68	0.00	0.148	2.83	0.148	DL 0 1
407	LOAD Z	42	79	0.00	0.148	2.83	0.148	DL 0 1
408	LOAD Z	79	80	0.00	0.148	2.83	0.148	DL 0 1
409	LOAD Z	40	77	0.00	0.148	2.83	0.148	DL 0 1
410	LOAD Z	77	78	0.00	0.148	2.83	0.148	DL 0 1
411	LOAD Z	46	81	0.00	0.148	2.83	0.148	DL 0 1
412	LOAD Z	81	82	0.00	0.148	2.83	0.148	DL 0 1
413	LOAD Z	38	75	0.00	0.148	2.83	0.148	DL 0 1
414	LOAD Z	75	76	0.00	0.148	2.83	0.148	DL 0 1
415	LOAD Z	21	56	0.00	0.403	4.53	0.403	DL 0 1
416	LOAD Z	56	64	0.00	0.403	30.30	0.403	DL 0 1
417	LOAD Z	64	72	0.00	0.403	31.31	0.403	DL 0 1
418	LOAD Z	72	80	0.00	0.481	35.35	0.481	DL 0 1
419	LOAD Z	20	54	0.00	0.403	4.53	0.403	DL 0 1
420	LOAD Z	54	62	0.00	0.403	30.30	0.403	DL 0 1
421	LOAD Z	62	70	0.00	0.403	31.31	0.403	DL 0 1
422	LOAD Z	70	78	0.00	0.481	35.35	0.481	DL 0 1
423	LOAD Z	22	58	0.00	0.403	4.53	0.403	DL 0 1
424	LOAD Z	58	66	0.00	0.403	30.30	0.403	DL 0 1
425	LOAD Z	66	74	0.00	0.403	31.31	0.403	DL 0 1
426	LOAD Z	74	82	0.00	0.481	35.35	0.481	DL 0 1
427	LOAD Z	19	52	0.00	0.403	4.53	0.403	DL 0 1
428	LOAD Z	52	60	0.00	0.403	30.30	0.403	DL 0 1
429	LOAD Z	60	68	0.00	0.403	31.31	0.403	DL 0 1
430	LOAD Z	68	76	0.00	0.481	35.35	0.481	DL 0 1
431	LOAD Z	80	102	0.00	0.481	15.16	0.481	DL 0 1
432	LOAD Z	78	104	0.00	0.481	15.16	0.481	DL 0 1
433	LOAD Z	82	101	0.00	0.481	15.16	0.481	DL 0 1
434	LOAD Z	76	103	0.00	0.481	15.16	0.481	DL 0 1
435	LOAD X	21	56	0.00	115			WN 1 1
436	LOAD Y	21	56	0.00	115			WN 1 1
437	LOAD Y	21	56	0.00	115			WN 1 1
438	LOAD X	21	56	0.00	115			WN 1 1
439	LOAD X	20	54	0.00	115			WN 1 1
440	LOAD Y	20	54	0.00	115			WN 1 1
441	LOAD Y	20	54	0.00	115			WN 1 1

0.00 105503 GLOB MOMT
0.00 105503 GLOB CONC
0.00 105503 GLOB MOMT
0.00 105503 GLOB CONC



STRAN INPUT DATA

PILE ACNR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
442	LOAD X	20	54		0.00-105503	GLOR	MOMT	WN 1 1
443	LOAD Y	22	58		0.00	GLOR	CONC	WN 1 1
444	LOAD Y	22	54		0.00	105503	GLOR	MOMT
445	LOAD Y	22	58		0.00	105503	GLOR	CONC
446	LOAD X	22	58		0.00-105503	GLOR	MOMT	WN 1 1
447	LOAD X	19	52		0.00	GLOR	CONC	WN 1 1
448	LOAD Y	19	52		0.00	105503	GLOR	MOMT
449	LOAD Y	19	52		0.00	GLOR	CONC	WN 1 1
450	LOAD X	19	52		0.00-105503	GLOR	MOMT	WN 1 1
451	LOAD X	21	56		0.00	GLOR	CONC	WN 2 1
452	LOAD Y	21	56		0.00	349092	GLOR	MOMT
453	LOAD Y	21	56		0.00	GLOR	CONC	WN 2 1
454	LOAD X	21	56		0.00-349092	GLOR	MOMT	WN 2 1
455	LOAD X	20	54		0.00	GLOR	CONC	WN 2 1
456	LOAD Y	20	54		0.00	349092	GLOR	MOMT
457	LOAD Y	20	54		0.00	GLOR	CONC	WN 2 1
458	LOAD X	20	54		0.00-349092	GLOR	MOMT	WN 2 1
459	LOAD X	22	58		0.00	GLOR	CONC	WN 2 1
460	LOAD Y	22	58		0.00	349092	GLOR	MOMT
461	LOAD Y	22	58		0.00	GLOR	CONC	WN 2 1
462	LOAD X	22	58		0.00-349092	GLOR	MOMT	WN 2 1
463	LOAD X	19	52		0.00	GLOR	CONC	WN 2 1
464	LOAD Y	19	52		0.00	349092	GLOR	MOMT
465	LOAD Y	19	52		0.00	GLOR	CONC	WN 2 1
466	LOAD X	19	52		0.00-349092	GLOR	MOMT	WN 2 1
467	LOAD X	21	56		0.00	GLOR	CONC	WN 3 1
468	LOAD Y	21	56		0.00	105503	GLOR	MOMT
469	LOAD Y	21	56		0.00	GLOR	CONC	WN 3 1
470	LOAD X	21	56		0.00-105503	GLOR	MOMT	WN 3 1
471	LOAD X	20	54		0.00	GLOR	CONC	WN 3 1
472	LOAD Y	20	54		0.00	105503	GLOR	MOMT
473	LOAD Y	20	54		0.00	GLOR	CONC	WN 3 1
474	LOAD X	20	54		0.00-105503	GLOR	MOMT	WN 3 1
475	LOAD X	22	58		0.00	GLOR	CONC	WN 3 1
476	LOAD Y	22	58		0.00	105503	GLOR	MOMT
477	LOAD Y	22	58		0.00	GLOR	CONC	WN 3 1
478	LOAD X	22	58		0.00-105503	GLOR	MOMT	WN 3 1
479	LOAD X	19	52		0.00	GLOR	CONC	WN 3 1
480	LOAD Y	19	52		0.00	105503	GLOR	MOMT
481	LOAD Y	19	52		0.00	GLOR	CONC	WN 3 1
482	LOAD X	19	52		0.00-105503	GLOR	MOMT	WN 3 1
483	LOAD X	21	56		0.00	GLOR	CONC	WN 4 1
484	LOAD Y	21	56		0.00	349092	GLOR	MOMT
485	LOAD Y	21	56		0.00	GLOR	CONC	WN 4 1
486	LOAD X	21	56		0.00-349092	GLOR	MOMT	WN 4 1
487	LOAD X	20	54		0.00	GLOR	CONC	WN 4 1
488	LOAD Y	20	54		0.00	349092	GLOR	MOMT
489	LOAD Y	20	54		0.00	GLOR	CONC	WN 4 1
490	LOAD X	20	54		0.00-349092	GLOR	MOMT	WN 4 1

4-PILE ACRB STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
491	LOAD X	22	58					WN 4 1
492	LOAD Y	22	58					WN 4 1
493	LOAD Y	22	58		0.00	349092		GLOR MOMT
494	LOAD X	22	58					GLOR CONC
495	LOAD X	19	52		0.00	349092		GLOR MOMT
496	LOAD Y	19	52					GLOR CONC
497	LOAD Y	19	52		0.00	349092		GLOR MOMT
498	LOAD X	19	52					GLOR CONC
499	LOAD X	17	12		0.00		118	GLOR MOMT
500	LOAD Y	17	12		0.00		118	GLOR UNIF
501	LOAD X	17	12		2.97		125	GLOR UNIF
502	LOAD Y	17	12		2.97		125	GLOR UNIF
503	LOAD X	17	12		5.95		135	GLOR UNIF
504	LOAD Y	17	12		5.95		135	GLOR UNIF
505	LOAD X	16	11		0.00		136	GLOR UNIF
506	LOAD Y	16	11		0.00		136	GLOR UNIF
507	LOAD X	16	11		3.83		148	GLOR UNIF
508	LOAD Y	16	11		3.83		148	GLOR UNIF
509	LOAD X	16	11		7.67		163	GLOR UNIF
510	LOAD Y	16	11		7.67		163	GLOR UNIF
511	LOAD X	18	14		0.00		136	GLOR UNIF
512	LOAD Y	18	14		0.00		136	GLOR UNIF
513	LOAD X	18	14		3.83		148	GLOR UNIF
514	LOAD Y	18	14		3.83		148	GLOR UNIF
515	LOAD X	18	14		7.67		163	GLOR UNIF
516	LOAD Y	18	14		7.67		163	GLOR UNIF
517	LOAD X	15	9		0.00		124	GLOR UNIF
518	LOAD Y	15	9		0.00		124	GLOR UNIF
519	LOAD X	15	9		1.97		130	GLOR UNIF
520	LOAD Y	15	9		1.97		130	GLOR UNIF
521	LOAD X	15	9		3.95		137	GLOR UNIF
522	LOAD Y	15	9		3.95		137	GLOR UNIF
523	LOAD X	17	11		0.00		10	GLOR UNIF
524	LOAD Y	17	11		0.00		57	GLOR UNIF
525	LOAD Z	17	11		0.00		20	GLOR UNIF
526	LOAD X	17	11		6.46		11	GLOR UNIF
527	LOAD Y	17	11		6.46		62	GLOR UNIF
528	LOAD Z	17	11		6.46		24	GLOR UNIF
529	LOAD X	17	11		12.93		12	GLOR UNIF
530	LOAD Y	17	11		12.93		68	GLOR UNIF
531	LOAD Z	17	11		12.93		26	GLOR UNIF
532	LOAD X	17	11		19.39		13	GLOR UNIF
533	LOAD Y	17	11		19.39		74	GLOR UNIF
534	LOAD Z	17	11		19.39		28	GLOR UNIF
535	LOAD X	18	9		0.00		11	GLOR UNIF
536	LOAD Y	18	9		0.00		63	GLOR UNIF
537	LOAD Z	18	9		0.00		24	GLOR UNIF
538	LOAD X	18	9		6.48		12	GLOR UNIF
539	LOAD Y	18	9		6.48		63	GLOR UNIF
540	LOAD Z	18	9		6.48		24	GLOR UNIF

STRAN INPUT DATA

4-PILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

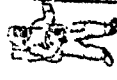
LINE NO.	1	2	3	4	5	6	7	8	
540	LOAD Z	18	9	6.48	25			GLOR UNIF	MV 0 1
541	LOAD X	18	9	12.96	12	6.48		GLOR UNIF	MV 0 1
542	LOAD Y	18	9	12.96	6	6.48	70	GLOR UNIF	MV 0 1
543	LOAD Z	18	9	12.96	25	6.48		GLOR UNIF	MV 0 1
544	LOAD X	17	14	0.00	51	6.46	57	GLOR UNIF	MV 0 1
545	LOAD Y	17	14	0.00	09	6.46	10	GLOR UNIF	MV 0 1
546	LOAD Z	17	14	0.00	20	6.46	22	GLOR UNIF	MV 0 1
547	LOAD X	17	14	6.46	57	6.46	62	GLOR UNIF	MV 0 1
548	LOAD Y	17	14	6.46	10	6.46	11	GLOR UNIF	MV 0 1
549	LOAD Z	17	14	6.46	22	6.46	24	GLOR UNIF	MV 0 1
550	LOAD X	17	14	12.93	62	6.46	68	GLOR UNIF	MV 0 1
551	LOAD Y	17	14	12.93	11	6.46	12	GLOR UNIF	MV 0 1
552	LOAD Z	17	14	12.93	24	6.46	26	GLOR UNIF	MV 0 1
553	LOAD X	17	14	19.59	68	6.46	74	GLOR UNIF	MV 0 1
554	LOAD Y	17	14	19.59	12	6.46	13	GLOR UNIF	MV 0 1
555	LOAD Z	17	14	19.59	26	6.46	28	GLOR UNIF	MV 0 1
556	LOAD X	16	9	0.00	58	6.44	63	GLOR UNIF	MV 0 1
557	LOAD Y	16	9	0.00	10	6.44	11	GLOR UNIF	MV 0 1
558	LOAD Z	16	9	0.00	22	6.44	24	GLOR UNIF	MV 0 1
559	LOAD X	16	9	6.44	63	6.44	66	GLOR UNIF	MV 0 1
560	LOAD Y	16	9	6.44	11	6.44	12	GLOR UNIF	MV 0 1
561	LOAD Z	16	9	6.44	24	6.44	25	GLOR UNIF	MV 0 1
562	LOAD X	16	9	12.86	66	6.48	70	GLOR UNIF	MV 0 1
563	LOAD Y	16	9	12.86	12	6.44	13	GLOR UNIF	MV 0 1
564	LOAD Z	16	9	12.86	25	6.44	27	GLOR UNIF	MV 0 1
565	LOAD X	17	16	0.00	51	10.00	55	GLOR UNIF	MV 0 1
566	LOAD Y	17	16	10.00	55	10.00	57	GLOR UNIF	MV 0 1
567	LOAD Z	17	16	20.00	57	10.00	58	GLOR UNIF	MV 0 1
568	LOAD X	18	15	0.00	58	10.00	59	GLOR UNIF	MV 0 1
569	LOAD Y	18	15	10.00	59	10.00	57	GLOR UNIF	MV 0 1
570	LOAD Z	18	15	20.00	57	10.00	55	GLOR UNIF	MV 0 1
571	LOAD X	17	18	0.00	51	10.00	55	GLOR UNIF	MV 0 1
572	LOAD Y	17	18	0.00	55	10.00	57	GLOR UNIF	MV 0 1
573	LOAD Z	17	18	20.00	57	10.00	58	GLOR UNIF	MV 0 1
574	LOAD X	16	15	0.00	58	10.00	59	GLOR UNIF	MV 0 1
575	LOAD Y	16	15	10.00	59	10.00	57	GLOR UNIF	MV 0 1
576	LOAD Z	16	15	20.00	57	10.00	55	GLOR UNIF	MV 0 1
577	LOAD X	21	17	0.00	61	5.90	68	GLOR UNIF	MV 0 1
578	LOAD Y	21	17	0.00	61	5.90	68	GLOR UNIF	MV 0 1
579	LOAD Z	21	17	5.90	68	5.90	76	GLOR UNIF	MV 0 1
580	LOAD X	21	17	5.90	68	5.90	76	GLOR UNIF	MV 0 1
581	LOAD Y	21	17	11.80	76	5.90	86	GLOR UNIF	MV 0 1
582	LOAD Z	21	17	11.80	76	5.90	86	GLOR UNIF	MV 0 1
583	LOAD X	21	17	17.70	86	5.90	97	GLOR UNIF	MV 0 1
584	LOAD Y	21	17	17.70	86	5.90	97	GLOR UNIF	MV 0 1
585	LOAD Z	21	17	23.60	97	5.90	110	GLOR UNIF	MV 0 1
586	LOAD X	21	17	23.60	7	5.90	110	GLOR UNIF	MV 0 1
587	LOAD Y	20	16	0.00	69	5.90	76	GLOR UNIF	MV 0 1
588	LOAD Z	20	16	0.00	68	5.90	76	GLOR UNIF	MV 0 1



STRAN INPUT DATA

45PILE BARR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8	
589	LOAD X	20	16	5.90	76	5.90	86	GLOR UNIF	MV 0 1
590	LOAD Y	20	16	5.90	76	5.90	86	GLOR UNIF	MV 0 1
591	LOAD X	20	16	11.80	86	5.90	97	GLOR UNIF	MV 0 1
592	LOAD Y	20	16	11.80	86	5.90	97	GLOR UNIF	MV 0 1
593	LOAD X	20	16	17.70	97	5.90	110	GLOR UNIF	MV 0 1
594	LOAD Y	20	16	17.70	97	5.90	110	GLOR UNIF	MV 0 1
595	LOAD X	20	16	23.60	110	5.90	125	GLOR UNIF	MV 0 1
596	LOAD Y	20	16	23.60	110	5.90	125	GLOR UNIF	MV 0 1
597	LOAD X	22	18	0.00	68	5.90	76	GLOR UNIF	MV 0 1
598	LOAD Y	22	18	0.00	68	5.90	76	GLOR UNIF	MV 0 1
599	LOAD X	22	18	5.90	76	5.90	86	GLOR UNIF	MV 0 1
600	LOAD Y	22	18	5.90	76	5.90	86	GLOR UNIF	MV 0 1
601	LOAD X	22	18	11.80	86	5.90	97	GLOR UNIF	MV 0 1
602	LOAD Y	22	18	11.80	86	5.90	97	GLOR UNIF	MV 0 1
603	LOAD X	22	18	17.70	97	5.90	110	GLOR UNIF	MV 0 1
604	LOAD Y	22	18	17.70	97	5.90	110	GLOR UNIF	MV 0 1
605	LOAD X	22	18	23.60	110	5.90	125	GLOR UNIF	MV 0 1
606	LOAD Y	22	18	23.60	110	5.90	125	GLOR UNIF	MV 0 1
607	LOAD X	19	15	0.00	68	5.90	75	GLOR UNIF	MV 0 1
608	LOAD Y	19	15	0.00	68	5.90	75	GLOR UNIF	MV 0 1
609	LOAD X	19	15	5.90	75	5.90	84	GLOR UNIF	MV 0 1
610	LOAD Y	19	15	5.90	75	5.90	84	GLOR UNIF	MV 0 1
611	LOAD X	19	15	11.80	84	5.90	94	GLOR UNIF	MV 0 1
612	LOAD Y	19	15	11.80	84	5.90	94	GLOR UNIF	MV 0 1
613	LOAD X	19	15	17.70	94	5.90	106	GLOR UNIF	MV 0 1
614	LOAD Y	19	15	17.70	94	5.90	106	GLOR UNIF	MV 0 1
615	LOAD X	19	15	23.60	106	5.90	118	GLOR UNIF	MV 0 1
616	LOAD Y	19	15	23.60	106	5.90	118	GLOR UNIF	MV 0 1
617	LOAD X	25	21	0.00	74	1.51	76	GLOR UNIF	MV 0 1
618	LOAD Y	25	21	0.00	74	1.51	76	GLOR UNIF	MV 0 1
619	LOAD Z	25	21	0.00	15	1.51	15	GLOR UNIF	MV 0 1
620	LOAD X	25	21	1.51	76	1.51	78	GLOR UNIF	MV 0 1
621	LOAD Y	25	21	1.51	76	1.51	78	GLOR UNIF	MV 0 1
622	LOAD Z	25	21	1.51	15	1.51	16	GLOR UNIF	MV 0 1
623	LOAD X	25	21	3.03	78	1.51	80	GLOR UNIF	MV 0 1
624	LOAD Y	25	21	3.03	78	1.51	80	GLOR UNIF	MV 0 1
625	LOAD Z	25	21	3.03	16	1.51	16	GLOR UNIF	MV 0 1
626	LOAD X	24	20	0.00	85	1.51	87	GLOR UNIF	MV 0 1
627	LOAD Y	24	20	0.00	85	1.51	87	GLOR UNIF	MV 0 1
628	LOAD X	24	20	1.51	87	1.51	89	GLOR UNIF	MV 0 1
629	LOAD Y	24	20	1.51	87	1.51	89	GLOR UNIF	MV 0 1
630	LOAD X	24	20	3.03	89	1.51	91	GLOR UNIF	MV 0 1
631	LOAD Y	24	20	3.03	89	1.51	91	GLOR UNIF	MV 0 1
632	LOAD X	26	22	0.00	85	1.51	87	GLOR UNIF	MV 0 1
633	LOAD Y	26	22	0.00	85	1.51	87	GLOR UNIF	MV 0 1
634	LOAD X	26	22	1.51	87	1.51	89	GLOR UNIF	MV 0 1
635	LOAD Y	26	22	1.51	87	1.51	89	GLOR UNIF	MV 0 1
636	LOAD X	26	22	3.03	89	1.51	91	GLOR UNIF	MV 0 1
637	LOAD Y	26	22	3.03	89	1.51	91	GLOR UNIF	MV 0 1



STRAN I UNIT DATA

PILE ACNR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8	
638	LOAD X	23	19	0.00	62	1.51	85	GLOR UNIF	MV 0 1
639	LOAD Y	23	19	0.00	82	1.51	85	GLOR UNIF	MV 0 1
640	LOAD Z	23	19	0.00	16	1.51	17	GLOR UNIF	MV 0 1
641	LOAD X	23	19	1.51	45	1.51	87	GLOR UNIF	MV 0 1
642	LOAD Y	23	19	1.51	85	1.51	87	GLOR UNIF	MV 0 1
643	LOAD Z	23	19	1.51	17	1.51	17	GLOR UNIF	MV 0 1
644	LOAD X	23	19	3.03	87	1.51	89	GLOR UNIF	MV 0 1
645	LOAD Y	23	19	3.03	87	1.51	89	GLOR UNIF	MV 0 1
646	LOAD Z	23	19	3.03	17	1.51	1A	GLOR UNIF	MV 0 1
647	LOAD Y	25	24	0.00	26	10.30	28	GLOR UNIF	MV 0 1
648	LOAD Y	25	24	10.30	26	10.30	29	GLOR UNIF	MV 0 1
649	LOAD Y	25	24	20.60	29	10.30	30	GLOR UNIF	MV 0 1
650	LOAD Y	26	23	0.00	30	10.30	30	GLOR UNIF	MV 0 1
651	LOAD Y	26	23	10.30	57	10.30	31	GLOR UNIF	MV 0 1
652	LOAD Y	26	23	20.60	50	10.30	29	GLOR UNIF	MV 0 1
653	LOAD X	25	26	0.00	28	10.30	28	GLOR UNIF	MV 0 1
654	LOAD X	25	26	10.30	28	10.30	29	GLOR UNIF	MV 0 1
655	LOAD X	25	26	20.60	29	10.30	30	GLOR UNIF	MV 0 1
656	LOAD X	24	23	0.00	30	10.30	30	GLOR UNIF	MV 0 1
657	LOAD X	24	23	10.30	50	10.30	30	GLOR UNIF	MV 0 1
658	LOAD X	24	23	20.60	50	10.30	29	GLOR UNIF	MV 0 1
659	LOAD X	26	27	0.00	34	7.28	34	GLOR UNIF	MV 0 1
660	LOAD Y	26	27	0.00	34	7.28	34	GLOR UNIF	MV 0 1
661	LOAD X	26	27	7.28	34	7.28	34	GLOR UNIF	MV 0 1
662	LOAD Y	26	27	7.28	34	7.28	34	GLOR UNIF	MV 0 1
663	LOAD X	26	27	14.57	34	7.28	34	GLOR UNIF	MV 0 1
664	LOAD Y	26	27	14.57	34	7.28	34	GLOR UNIF	MV 0 1
665	LOAD X	27	24	0.00	34	7.28	34	GLOR UNIF	MV 0 1
666	LOAD Y	27	24	0.00	34	7.28	34	GLOR UNIF	MV 0 1
667	LOAD X	27	24	7.28	34	7.28	34	GLOR UNIF	MV 0 1
668	LOAD Y	27	24	7.28	34	7.28	34	GLOR UNIF	MV 0 1
669	LOAD X	27	24	14.57	34	7.28	34	GLOR UNIF	MV 0 1
670	LOAD Y	27	24	14.57	34	7.28	34	GLOR UNIF	MV 0 1
671	LOAD X	30	25	0.00	46	7.57	51	GLOR UNIF	MV 0 1
672	LOAD Y	30	25	0.00	46	7.57	51	GLOR UNIF	MV 0 1
673	LOAD Z	30	25	0.00	09	7.57	10	GLOR UNIF	MV 0 1
674	LOAD X	30	25	7.57	51	7.57	57	GLOR UNIF	MV 0 1
675	LOAD Y	30	25	7.57	51	7.57	57	GLOR UNIF	MV 0 1
676	LOAD Z	30	25	7.57	10	7.57	11	GLOR UNIF	MV 0 1
677	LOAD X	30	25	15.15	57	7.57	64	GLOR UNIF	MV 0 1
678	LOAD Y	30	25	15.15	57	7.57	64	GLOR UNIF	MV 0 1
679	LOAD Z	30	25	15.15	11	7.57	13	GLOR UNIF	MV 0 1
680	LOAD X	30	25	22.72	64	7.57	74	GLOR UNIF	MV 0 1
681	LOAD Y	30	25	22.72	64	7.57	74	GLOR UNIF	MV 0 1
682	LOAD Z	30	25	22.72	13	7.57	15	GLOR UNIF	MV 0 1
683	LOAD X	29	24	0.00	54	7.57	60	GLOR UNIF	MV 0 1
684	LOAD Y	29	24	0.00	54	7.57	60	GLOR UNIF	MV 0 1
685	LOAD X	29	24	7.57	60	7.57	66	GLOR UNIF	MV 0 1
686	LOAD Y	29	24	7.57	60	7.57	66	GLOR UNIF	MV 0 1



STRAN I N U T DATA

4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8	
687	LOAD X	29	24	74				GLOR UNIF	MV 0 1
688	LOAD Y	29	24	74				GLOR UNIF	MV 0 1
689	LOAD X	29	24	85				GLOR UNIF	MV 0 1
690	LOAD Y	29	24	85				GLOR UNIF	MV 0 1
691	LOAD X	31	26	60				GLOR UNIF	MV 0 1
692	LOAD Y	31	26	60				GLOR UNIF	MV 0 1
693	LOAD X	31	26	66				GLOR UNIF	MV 0 1
694	LOAD Y	31	26	66				GLOR UNIF	MV 0 1
695	LOAD X	31	26	74				GLOR UNIF	MV 0 1
696	LOAD Y	31	26	74				GLOR UNIF	MV 0 1
697	LOAD X	31	26	85				GLOR UNIF	MV 0 1
698	LOAD Y	31	26	85				GLOR UNIF	MV 0 1
699	LOAD X	28	23	59				GLOR UNIF	MV 0 1
700	LOAD Y	28	23	59				GLOR UNIF	MV 0 1
701	LOAD Z	28	23	12				GLOR UNIF	MV 0 1
702	LOAD X	28	23	65				GLOR UNIF	MV 0 1
703	LOAD Y	28	23	65				GLOR UNIF	MV 0 1
704	LOAD Z	28	23	13				GLOR UNIF	MV 0 1
705	LOAD X	28	23	73				GLOR UNIF	MV 0 1
706	LOAD Y	28	23	73				GLOR UNIF	MV 0 1
707	LOAD Z	28	23	14				GLOR UNIF	MV 0 1
708	LOAD X	28	23	82				GLOR UNIF	MV 0 1
709	LOAD Y	28	23	82				GLOR UNIF	MV 0 1
710	LOAD Z	28	23	16				GLOR UNIF	MV 0 1
711	LOAD X	29	25	15				GLOR UNIF	MV 0 1
712	LOAD Y	29	25	15				GLOR UNIF	MV 0 1
713	LOAD Z	29	25	13				GLOR UNIF	MV 0 1
714	LOAD X	29	25	16				GLOR UNIF	MV 0 1
715	LOAD Y	29	25	16				GLOR UNIF	MV 0 1
716	LOAD Z	29	25	15				GLOR UNIF	MV 0 1
717	LOAD X	29	25	18				GLOR UNIF	MV 0 1
718	LOAD Y	29	25	18				GLOR UNIF	MV 0 1
719	LOAD Z	29	25	17				GLOR UNIF	MV 0 1
720	LOAD X	28	26	12				GLOR UNIF	MV 0 1
721	LOAD Y	28	26	29				GLOR UNIF	MV 0 1
722	LOAD Z	28	26	16				GLOR UNIF	MV 0 1
723	LOAD X	28	26	13				GLOR UNIF	MV 0 1
724	LOAD Y	28	26	32				GLOR UNIF	MV 0 1
725	LOAD Z	28	26	18				GLOR UNIF	MV 0 1
726	LOAD X	28	26	15				GLOR UNIF	MV 0 1
727	LOAD Y	28	26	36				GLOR UNIF	MV 0 1
728	LOAD Z	28	26	20				GLOR UNIF	MV 0 1
729	LOAD X	28	26	17				GLOR UNIF	MV 0 1
730	LOAD Y	28	26	40				GLOR UNIF	MV 0 1
731	LOAD Z	28	26	23				GLOR UNIF	MV 0 1
732	LOAD X	31	25	31				GLOR UNIF	MV 0 1
733	LOAD Y	31	25	15				GLOR UNIF	MV 0 1
734	LOAD Z	31	25	13				GLOR UNIF	MV 0 1
735	LOAD X	31	25	35				GLOR UNIF	MV 0 1



STRAN INQUIRY DATA

PILE ACORN STRUCTURE -- U.S. NAVY (36-IN. DIAPHRAGM PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8	
736	LOAD Y	31	25	15.12	15	15.12	16	GLOR UNIF	KV 0 1
737	LOAD Z	31	25	15.12	15	15.12	15	GLOR UNIF	KV 0 1
738	LOAD X	31	25	30.24	34	15.12	39	GLOR UNIF	KV 0 1
739	LOAD Y	31	25	30.24	16	15.12	18	GLOR UNIF	KV 0 1
740	LOAD Z	31	25	30.24	15	15.12	17	GLOR UNIF	KV 0 1
741	LOAD X	28	24	0.00	26	11.34	29	GLOR UNIF	KV 0 1
742	LOAD Y	28	24	0.00	11	11.34	12	GLOR UNIF	KV 0 1
743	LOAD Z	28	24	0.00	15	11.34	16	GLOR UNIF	KV 0 1
744	LOAD X	24	24	11.34	29	11.34	32	GLOR UNIF	KV 0 1
745	LOAD Y	28	24	11.34	12	11.34	13	GLOR UNIF	KV 0 1
746	LOAD Z	28	24	11.34	16	11.34	18	GLOR UNIF	KV 0 1
747	LOAD X	28	24	22.68	32	11.34	36	GLOR UNIF	KV 0 1
748	LOAD Y	28	24	22.68	13	11.34	15	GLOR UNIF	KV 0 1
749	LOAD Z	28	24	22.68	18	11.34	20	GLOR UNIF	KV 0 1
750	LOAD X	28	24	34.03	36	11.34	40	GLOR UNIF	KV 0 1
751	LOAD Y	28	24	34.03	15	11.34	17	GLOR UNIF	KV 0 1
752	LOAD Z	28	24	34.03	20	11.34	23	GLOR UNIF	KV 0 1
753	LOAD X	30	29	0.00	16	12.30	17	GLOR UNIF	KV 0 1
754	LOAD Y	30	29	12.30	17	12.30	18	GLOR UNIF	KV 0 1
755	LOAD Z	30	29	24.60	18	12.30	19	GLOR UNIF	KV 0 1
756	LOAD X	31	28	0.00	19	12.30	19	GLOR UNIF	KV 0 1
757	LOAD Y	31	28	12.30	19	12.30	19	GLOR UNIF	KV 0 1
758	LOAD Z	31	28	24.60	19	12.30	19	GLOR UNIF	KV 0 1
759	LOAD X	30	31	0.00	16	12.30	17	GLOR UNIF	KV 0 1
760	LOAD Y	30	31	12.30	17	12.30	18	GLOR UNIF	KV 0 1
761	LOAD Z	30	31	24.60	18	12.30	19	GLOR UNIF	KV 0 1
762	LOAD X	29	28	0.00	19	12.30	19	GLOR UNIF	KV 0 1
763	LOAD Y	29	28	12.30	19	12.30	19	GLOR UNIF	KV 0 1
764	LOAD Z	29	28	24.60	19	12.30	19	GLOR UNIF	KV 0 1
765	LOAD X	31	32	0.00	24	8.70	24	GLOR UNIF	KV 0 1
766	LOAD Y	31	32	0.00	24	8.70	24	GLOR UNIF	KV 0 1
767	LOAD Z	31	32	8.70	24	8.70	24	GLOR UNIF	KV 0 1
768	LOAD X	31	32	8.70	24	8.70	24	GLOR UNIF	KV 0 1
769	LOAD Y	31	32	17.39	24	8.70	24	GLOR UNIF	KV 0 1
770	LOAD Z	31	32	17.39	24	8.70	24	GLOR UNIF	KV 0 1
771	LOAD X	32	29	0.00	24	8.70	24	GLOR UNIF	KV 0 1
772	LOAD Y	32	29	0.00	24	8.70	24	GLOR UNIF	KV 0 1
773	LOAD Z	32	29	8.70	24	8.70	24	GLOR UNIF	KV 0 1
774	LOAD X	32	29	8.70	24	8.70	24	GLOR UNIF	KV 0 1
775	LOAD Y	32	29	17.39	24	8.70	24	GLOR UNIF	KV 0 1
776	LOAD Z	32	29	17.39	24	8.70	24	GLOR UNIF	KV 0 1
777	LOAD X	35	30	0.00	33	10.44	37	GLOR UNIF	KV 0 1
778	LOAD Y	35	30	0.00	33	10.44	37	GLOR UNIF	KV 0 1
779	LOAD Z	35	30	0.00	07	10.44	07	GLOR UNIF	KV 0 1
780	LOAD X	35	30	10.44	37	10.44	41	GLOR UNIF	KV 0 1
781	LOAD Y	35	30	10.44	37	10.44	41	GLOR UNIF	KV 0 1
782	LOAD Z	35	30	10.44	07	10.44	07	GLOR UNIF	KV 0 1
783	LOAD X	35	30	20.87	41	10.44	46	GLOR UNIF	KV 0 1
784	LOAD Y	35	30	20.87	41	10.44	46	GLOR UNIF	KV 0 1

STRAN IN T DATA

PILE ACNR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8	
765	LOAD Z	35	30	20.87	08	10.68		GLOR UNIF	WV 0 1
786	LOAD X	34	29	0.00	41	10.68		GLOR UNIF	WV 0 1
787	LOAD Y	34	29	0.00	41	10.44		GLOR UNIF	WV 0 1
788	LOAD X	34	29	10.44	44	10.44		GLOR UNIF	WV 0 1
789	LOAD Y	34	29	10.44	44	10.44		GLOR UNIF	WV 0 1
790	LOAD X	34	29	20.87	49	10.44		GLOR UNIF	WV 0 1
791	LOAD Y	34	29	20.87	49	10.44		GLOR UNIF	WV 0 1
792	LOAD X	36	31	0.00	41	10.44		GLOR UNIF	WV 0 1
793	LOAD Y	36	31	0.00	41	10.44		GLOR UNIF	WV 0 1
794	LOAD X	36	31	10.44	44	10.44		GLOR UNIF	WV 0 1
795	LOAD Y	36	31	10.44	44	10.44		GLOR UNIF	WV 0 1
796	LOAD X	36	31	20.87	49	10.44		GLOR UNIF	WV 0 1
797	LOAD Y	36	31	20.87	49	10.44		GLOR UNIF	WV 0 1
798	LOAD X	35	28	0.00	45	10.44		GLOR UNIF	WV 0 1
799	LOAD Y	35	28	0.00	45	10.44		GLOR UNIF	WV 0 1
800	LOAD Z	33	28	0.00	48	10.44		GLOR UNIF	WV 0 1
801	LOAD X	33	28	10.44	48	10.44		GLOR UNIF	WV 0 1
802	LOAD Y	33	28	10.44	48	10.44		GLOR UNIF	WV 0 1
803	LOAD Z	33	28	10.44	48	10.44		GLOR UNIF	WV 0 1
804	LOAD X	33	28	20.87	48	10.44		GLOR UNIF	WV 0 1
805	LOAD Y	33	28	20.87	48	10.44		GLOR UNIF	WV 0 1
806	LOAD Z	33	28	20.87	10	10.44		GLOR UNIF	WV 0 1
807	LOAD X	35	29	0.00	05	12.68		GLOR UNIF	WV 0 1
808	LOAD Y	35	29	0.00	15	12.68		GLOR UNIF	WV 0 1
809	LOAD Z	35	29	0.00	08	12.68		GLOR UNIF	WV 0 1
810	LOAD X	35	29	12.68	06	12.68		GLOR UNIF	WV 0 1
811	LOAD Y	35	29	12.68	16	12.68		GLOR UNIF	WV 0 1
812	LOAD Z	35	29	12.68	09	12.68		GLOR UNIF	WV 0 1
813	LOAD X	35	29	25.35	06	12.68		GLOR UNIF	WV 0 1
814	LOAD Y	35	29	25.35	18	12.68		GLOR UNIF	WV 0 1
815	LOAD Z	35	29	25.35	10	12.68		GLOR UNIF	WV 0 1
816	LOAD X	35	29	38.03	07	12.68		GLOR UNIF	WV 0 1
817	LOAD Y	35	29	38.03	20	12.68		GLOR UNIF	WV 0 1
818	LOAD Z	35	29	38.03	11	12.68		GLOR UNIF	WV 0 1
819	LOAD X	36	28	0.00	08	16.90		GLOR UNIF	WV 0 1
820	LOAD Y	36	28	0.00	19	16.90		GLOR UNIF	WV 0 1
821	LOAD Z	36	28	0.00	08	16.90		GLOR UNIF	WV 0 1
822	LOAD X	36	28	16.90	09	16.90		GLOR UNIF	WV 0 1
823	LOAD Y	36	28	16.90	21	16.90		GLOR UNIF	WV 0 1
824	LOAD Z	36	28	16.90	09	16.90		GLOR UNIF	WV 0 1
825	LOAD X	36	28	33.80	09	16.90		GLOR UNIF	WV 0 1
826	LOAD Y	36	28	33.80	23	16.90		GLOR UNIF	WV 0 1
827	LOAD Z	36	28	33.80	10	16.90		GLOR UNIF	WV 0 1
828	LOAD X	35	31	0.00	15	12.68		GLOR UNIF	WV 0 1
829	LOAD Y	35	31	0.00	05	12.68		GLOR UNIF	WV 0 1
830	LOAD Z	35	31	0.00	08	12.68		GLOR UNIF	WV 0 1
831	LOAD X	35	31	12.68	16	12.68		GLOR UNIF	WV 0 1
832	LOAD Y	35	31	12.68	06	12.68		GLOR UNIF	WV 0 1
833	LOAD Z	35	31	12.68	09	12.68		GLOR UNIF	WV 0 1

STRAN INPUT DATA

4-MILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
R34	LOAD X	35	31	18	12.68			MV 0 1
R35	LOAD Y	35	31	06	12.68			MV 0 1
R36	LOAD Z	35	31	10	12.68			MV 0 1
R37	LOAD X	35	31	20	12.68			MV 0 1
R38	LOAD Y	35	31	20	12.68			MV 0 1
R39	LOAD Z	35	31	11	12.68			MV 0 1
R40	LOAD X	34	28	19	16.90			MV 0 1
R41	LOAD Y	34	28	08	16.90			MV 0 1
R42	LOAD Z	34	28	08	16.90			MV 0 1
R43	LOAD X	34	28	21	16.90			MV 0 1
R44	LOAD Y	34	28	09	16.90			MV 0 1
R45	LOAD Z	34	28	09	16.90			MV 0 1
R46	LOAD X	34	28	23	16.90			MV 0 1
R47	LOAD Y	34	28	09	16.90			MV 0 1
R48	LOAD Z	34	28	10	16.90			MV 0 1
R49	LOAD Y	35	34	15	14.37			MV 0 1
R50	LOAD Y	35	34	17	14.37			MV 0 1
R51	LOAD Y	35	34	18	14.37			MV 0 1
R52	LOAD Y	36	33	18	14.37			MV 0 1
R53	LOAD Y	36	33	19	14.37			MV 0 1
R54	LOAD Y	36	33	19	14.37			MV 0 1
R55	LOAD X	35	36	15	14.37			MV 0 1
R56	LOAD X	35	36	17	14.37			MV 0 1
R57	LOAD X	35	36	18	14.37			MV 0 1
R58	LOAD X	34	33	19	14.37			MV 0 1
R59	LOAD X	34	33	19	14.37			MV 0 1
R60	LOAD X	34	33	28	14.37			MV 0 1
R61	LOAD X	36	37	20	10.16			MV 0 1
R62	LOAD X	36	37	20	10.16			MV 0 1
R63	LOAD X	35	36	20	10.16			MV 0 1
R64	LOAD Y	36	37	20	10.16			MV 0 1
R65	LOAD X	36	37	20	10.16			MV 0 1
R66	LOAD Y	36	37	20	10.16			MV 0 1
R67	LOAD X	37	34	20	10.16			MV 0 1
R68	LOAD Y	37	34	20	10.16			MV 0 1
R69	LOAD X	37	34	20	10.16			MV 0 1
R70	LOAD Y	37	34	20	10.16			MV 0 1
R71	LOAD X	37	34	20	10.16			MV 0 1
R72	LOAD Y	37	34	20	10.16			MV 0 1
R73	LOAD X	42	35	29	11.78			MV 0 1
R74	LOAD Y	42	35	29	11.78			MV 0 1
R75	LOAD Z	42	35	06	11.78			MV 0 1
R76	LOAD X	42	35	30	11.78			MV 0 1
R77	LOAD Y	42	35	30	11.78			MV 0 1
R78	LOAD Z	42	35	06	11.78			MV 0 1
R79	LOAD X	42	35	31	11.78			MV 0 1
R80	LOAD Y	42	35	31	11.78			MV 0 1
R81	LOAD Z	42	35	06	11.78			MV 0 1
R82	LOAD X	40	34	36	11.78			MV 0 1



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8-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO. 1 2 3 4 5 6 7 8

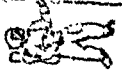
LINE NO.	1	2	3	4	5	6	7	8
883	LOAD Y	40	34	0.00	11.70	37	GLOR UNIF	MV 0 1
884	LOAD X	40	34	11.78	11.78	34	GLOR UNIF	MV 0 1
885	LOAD Y	40	34	11.78	11.78	38	GLOR UNIF	MV 0 1
886	LOAD X	40	34	23.57	11.74	41	GLOR UNIF	MV 0 1
887	LOAD Y	40	34	23.57	11.78	41	GLOR UNIF	MV 0 1
888	LOAD X	44	36	0.00	11.78	37	GLOR UNIF	MV 0 1
889	LOAD Y	44	36	0.00	11.78	37	GLOR UNIF	MV 0 1
890	LOAD X	44	36	11.78	11.78	34	GLOR UNIF	MV 0 1
891	LOAD Y	44	36	11.78	11.78	34	GLOR UNIF	MV 0 1
892	LOAD X	44	36	23.57	11.78	41	GLOR UNIF	MV 0 1
893	LOAD Y	44	36	23.57	11.78	41	GLOR UNIF	MV 0 1
894	LOAD X	38	33	0.00	11.78	34	GLOR UNIF	MV 0 1
895	LOAD Y	38	33	0.00	11.78	34	GLOR UNIF	MV 0 1
896	LOAD X	38	33	0.00	11.78	34	GLOR UNIF	MV 0 1
897	LOAD Y	38	33	11.78	11.78	38	GLOR UNIF	MV 0 1
898	LOAD Z	38	33	11.78	11.78	38	GLOR UNIF	MV 0 1
899	LOAD X	38	33	23.57	11.78	40	GLOR UNIF	MV 0 1
900	LOAD Y	38	33	23.57	11.78	40	GLOR UNIF	MV 0 1
901	LOAD Z	38	33	23.57	11.78	40	GLOR UNIF	MV 0 1
902	LOAD X	35	41	0.00	13.75	12	GLOR UNIF	MV 0 1
903	LOAD Y	35	41	0.00	13.75	12	GLOR UNIF	MV 0 1
904	LOAD Z	35	41	0.00	13.75	15	GLOR UNIF	MV 0 1
905	LOAD X	35	41	13.75	13.75	06	GLOR UNIF	MV 0 1
906	LOAD Y	35	41	13.75	13.75	11	GLOR UNIF	MV 0 1
907	LOAD Z	35	41	13.75	13.75	15	GLOR UNIF	MV 0 1
908	LOAD X	35	41	13.75	13.75	05	GLOR UNIF	MV 0 1
909	LOAD Y	35	41	27.50	13.75	11	GLOR UNIF	MV 0 1
910	LOAD Z	35	41	27.50	13.75	15	GLOR UNIF	MV 0 1
911	LOAD X	41	34	27.50	13.75	05	GLOR UNIF	MV 0 1
912	LOAD Y	41	34	0.00	13.75	11	GLOR UNIF	MV 0 1
913	LOAD Z	41	34	0.00	13.75	15	GLOR UNIF	MV 0 1
914	LOAD X	41	34	0.00	13.75	08	GLOR UNIF	MV 0 1
915	LOAD Y	41	34	13.75	13.75	11	GLOR UNIF	MV 0 1
916	LOAD Z	41	34	13.75	13.75	16	GLOR UNIF	MV 0 1
917	LOAD X	41	34	13.75	13.75	09	GLOR UNIF	MV 0 1
918	LOAD Y	41	34	27.50	13.75	13	GLOR UNIF	MV 0 1
919	LOAD Z	41	34	27.50	13.75	17	GLOR UNIF	MV 0 1
920	LOAD X	41	34	27.50	13.75	09	GLOR UNIF	MV 0 1
921	LOAD Y	36	45	0.00	13.75	12	GLOR UNIF	MV 0 1
922	LOAD Z	36	45	0.00	13.75	16	GLOR UNIF	MV 0 1
923	LOAD X	36	45	0.00	13.75	09	GLOR UNIF	MV 0 1
924	LOAD Y	36	45	13.75	13.75	11	GLOR UNIF	MV 0 1
925	LOAD Z	36	45	13.75	13.75	16	GLOR UNIF	MV 0 1
926	LOAD X	36	45	13.75	13.75	09	GLOR UNIF	MV 0 1
927	LOAD Y	36	45	27.50	13.75	11	GLOR UNIF	MV 0 1
928	LOAD Z	36	45	27.50	13.75	16	GLOR UNIF	MV 0 1
929	LOAD X	45	33	0.00	13.75	08	GLOR UNIF	MV 0 1
930	LOAD Y	45	33	0.00	13.75	13	GLOR UNIF	MV 0 1
931	LOAD Z	45	33	0.00	13.75	18	GLOR UNIF	MV 0 1

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STRAN INPUT DATA

4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
932	LOAD Z	45 33	0.00	06	13.75	06	GLOR UNIF	WV 0 1
933	LOAD X	45 33	13.75	13	13.75	13	GLOR UNIF	WV 0 1
934	LOAD Y	45 33	13.75	18	13.75	18	GLOR UNIF	WV 0 1
935	LOAD Z	45 33	13.75	06	13.75	06	GLOR UNIF	WV 0 1
936	LOAD X	45 33	27.50	13	13.75	14	GLOR UNIF	WV 0 1
937	LOAD Y	45 33	27.50	18	13.75	19	GLOR UNIF	WV 0 1
938	LOAD Z	45 33	27.50	06	13.75	07	GLOR UNIF	WV 0 1
939	LOAD X	35 43	0.00	16	13.75	15	GLOR UNIF	WV 0 1
940	LOAD Y	35 43	0.00	12	13.75	12	GLOR UNIF	WV 0 1
941	LOAD Z	35 43	0.00	06	13.75	06	GLOR UNIF	WV 0 1
942	LOAD X	35 43	13.75	15	13.75	15	GLOR UNIF	WV 0 1
943	LOAD Y	35 43	13.75	12	13.75	11	GLOR UNIF	WV 0 1
944	LOAD Z	35 43	13.75	06	13.75	05	GLOR UNIF	WV 0 1
945	LOAD X	35 43	27.50	15	13.75	15	GLOR UNIF	WV 0 1
946	LOAD Y	35 43	27.50	11	13.75	11	GLOR UNIF	WV 0 1
947	LOAD Z	35 43	27.50	05	13.75	05	GLOR UNIF	WV 0 1
948	LOAD X	43 36	0.00	14	13.75	15	GLOR UNIF	WV 0 1
949	LOAD Y	43 36	0.00	10	13.75	11	GLOR UNIF	WV 0 1
950	LOAD Z	43 36	0.00	08	13.75	08	GLOR UNIF	WV 0 1
951	LOAD X	43 36	13.75	15	13.75	16	GLOR UNIF	WV 0 1
952	LOAD Y	43 36	13.75	11	13.75	11	GLOR UNIF	WV 0 1
953	LOAD Z	43 36	13.75	08	13.75	09	GLOR UNIF	WV 0 1
954	LOAD X	43 36	27.50	16	13.75	17	GLOR UNIF	WV 0 1
955	LOAD Y	43 36	27.50	11	13.75	13	GLOR UNIF	WV 0 1
956	LOAD Z	43 36	27.50	09	13.75	09	GLOR UNIF	WV 0 1
957	LOAD X	34 39	0.00	17	13.75	16	GLOR UNIF	WV 0 1
958	LOAD Y	34 39	0.00	13	13.75	12	GLOR UNIF	WV 0 1
959	LOAD Z	34 39	0.00	09	13.75	09	GLOR UNIF	WV 0 1
960	LOAD X	34 39	13.75	16	13.75	16	GLOR UNIF	WV 0 1
961	LOAD Y	34 39	13.75	12	13.75	11	GLOR UNIF	WV 0 1
962	LOAD Z	34 39	13.75	09	13.75	09	GLOR UNIF	WV 0 1
963	LOAD X	34 39	27.50	16	13.75	16	GLOR UNIF	WV 0 1
964	LOAD Y	34 39	27.50	11	13.75	11	GLOR UNIF	WV 0 1
965	LOAD Z	34 39	27.50	09	13.75	08	GLOR UNIF	WV 0 1
966	LOAD X	39 33	0.00	17	13.75	18	GLOR UNIF	WV 0 1
967	LOAD Y	39 33	0.00	13	13.75	13	GLOR UNIF	WV 0 1
968	LOAD Z	39 33	0.00	06	13.75	06	GLOR UNIF	WV 0 1
969	LOAD X	39 33	13.75	18	13.75	18	GLOR UNIF	WV 0 1
970	LOAD Y	39 33	13.75	13	13.75	13	GLOR UNIF	WV 0 1
971	LOAD Z	39 33	13.75	06	13.75	06	GLOR UNIF	WV 0 1
972	LOAD X	39 33	27.50	18	13.75	19	GLOR UNIF	WV 0 1
973	LOAD Y	39 33	27.50	13	13.75	14	GLOR UNIF	WV 0 1
974	LOAD Z	39 33	27.50	06	13.75	07	GLOR UNIF	WV 0 1
975	LOAD X	42 41	0.00	13	8.35	14	GLOR UNIF	WV 0 1
976	LOAD Y	42 41	8.35	14	8.35	14	GLOR UNIF	WV 0 1
977	LOAD Z	42 41	16.70	14	8.35	15	GLOR UNIF	WV 0 1
978	LOAD X	41 40	0.00	15	8.35	15	GLOR UNIF	WV 0 1
979	LOAD Y	41 40	8.35	15	8.35	16	GLOR UNIF	WV 0 1
980	LOAD Z	41 40	16.70	16	8.35	16	GLOR UNIF	WV 0 1



STRAN I N T DATA

4-PILE ACRH STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
981	LOAD Y	44	45	16	8.35	16	GLOB UNIF	MV 0 1
982	LOAD Y	44	45	16	8.35	16	GLOB UNIF	MV 0 1
983	LOAD Y	44	45	16	8.35	16	GLOB UNIF	MV 0 1
984	LOAD Y	45	46	16	8.35	16	GLOB UNIF	MV 0 1
985	LOAD Y	45	46	16	8.35	16	GLOB UNIF	MV 0 1
986	LOAD Y	45	46	16	8.35	16	GLOB UNIF	MV 0 1
987	LOAD X	42	43	14	8.35	14	GLOB UNIF	MV 0 1
988	LOAD X	42	43	14	8.35	14	GLOB UNIF	MV 0 1
989	LOAD X	42	43	14	8.35	14	GLOB UNIF	MV 0 1
990	LOAD X	43	44	15	8.35	15	GLOB UNIF	MV 0 1
991	LOAD X	43	44	15	8.35	15	GLOB UNIF	MV 0 1
992	LOAD X	43	44	15	8.35	15	GLOB UNIF	MV 0 1
993	LOAD X	40	39	16	8.35	16	GLOB UNIF	MV 0 1
994	LOAD X	40	39	16	8.35	16	GLOB UNIF	MV 0 1
995	LOAD X	40	39	16	8.35	16	GLOB UNIF	MV 0 1
996	LOAD X	39	38	16	8.35	16	GLOB UNIF	MV 0 1
997	LOAD X	39	38	16	8.35	16	GLOB UNIF	MV 0 1
998	LOAD X	39	38	16	8.35	16	GLOB UNIF	MV 0 1
999	LOAD X	40	39	16	8.35	16	GLOB UNIF	MV 0 1
1000	LOAD Y	44	46	18	11.81	18	GLOB UNIF	MV 0 1
1001	LOAD Y	44	46	18	11.81	18	GLOB UNIF	MV 0 1
1002	LOAD Y	44	46	18	11.81	18	GLOB UNIF	MV 0 1
1003	LOAD X	44	46	18	11.81	18	GLOB UNIF	MV 0 1
1004	LOAD Y	44	46	18	11.81	18	GLOB UNIF	MV 0 1
1005	LOAD X	46	40	14	11.81	14	GLOB UNIF	MV 0 1
1006	LOAD X	46	40	14	11.81	14	GLOB UNIF	MV 0 1
1007	LOAD X	46	40	14	11.81	14	GLOB UNIF	MV 0 1
1008	LOAD Y	46	40	14	11.81	14	GLOB UNIF	MV 0 1
1009	LOAD X	46	40	14	11.81	14	GLOB UNIF	MV 0 1
1010	LOAD Y	46	40	14	11.81	14	GLOB UNIF	MV 0 1
1011	LOAD X	2	4	12	1.240	12	GLOB CONC	STAIRWAY
1012	LOAD X	5	4	12	2.588	12	GLOB CONC	STAIRWAY
1013	LOAD Y	5	4	12	6.533	12	GLOB CONC	STAIRWAY
1014	LOAD X	12	11	27	13.166	27	GLOB CONC	STAIRWAY
1015	LOAD Y	12	11	27	12.867	27	GLOB CONC	STAIRWAY
1016	LOAD X	17	16	12	25.880	12	GLOB CONC	STAIRWAY
1017	LOAD Y	17	16	12	6.747	12	GLOB CONC	STAIRWAY
1018	LOAD X	25	24	9	13.536	9	GLOB CONC	STAIRWAY
1019	LOAD Y	25	24	9	8.65	9	GLOB CONC	STAIRWAY
1020	LOAD X	25	24	0	15.56	0	GLOB CONC	BOATLAND
1021	LOAD Y	25	24	0	8.65	0	GLOB CONC	BOATLAND
1022	LOAD X	25	24	30	15.56	30	GLOB CONC	BOATLAND
1023	LOAD Y	25	24	30	-0.150	30	GLOB UNIF	ADD LEAD
1024	LOAD Z	6	3	7	-0.150	7	GLOB UNIF	ADD LEAD
1025	LOAD Z	7	2	4	0.075	4	GLOB UNIF	ADD LEAD
1026	LOAD Z	8	1	1	-0.075	1	GLOB UNIF	ADD LEAD
1027	LOAD Z	12	11	11	-0.113	11	GLOB UNIF	ADD LEAD
1028	LOAD Z	13	10	10	-0.113	10	GLOB UNIF	ADD LEAD

STRAN INPUT DATA

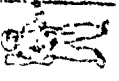
PAGE 22
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4-PILE ACPR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

LINE NO.	1	2	3	4	5	6	7	8
1030	LOADEN							
1031	LOAD Z	3						
1032	LOAD Z	7	2					
1033	LOAD Z	5	4					
1034	LOAD Z	8	1					
1035	LOAD Z	13	10					
1036	LOAD Z	12	11					
1037	LDCOMR							
1038	LDCOMR	4	100.	1	100.	2	100.	MAX TENS
1039	LDCOMR	5	100.	1	100.	2	100.	MAX COMP
1040	END							

GLOB UNIF LIVELOAD
 GLOB UNIF LIVELOAD
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 GLOB UNIF LIVELOAD
 GLOB UNIF LIVELOAD

MAX TENS
MAX COMP



STRAN - JOINT DEFLECTIONS AND ROTATIONS

PAGE 1
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LOAD CONDITION NO. 1 4-PILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

JOINT NUMBER /-----DEFLECTION IN INCHES-----/ /-----ROTATION IN RADIANS-----/ /-----MEMBERS-----/ Z

JOINT NUMBER	X	Y	Z	X	Y	Z	MEMBERS
1	5.09280	5.09228	5.09298	-.00007	.00050	.00065	.00005
2	5.15960	5.09312	5.16448	-.00024	.00005	.00045	.00045
3	5.17419	5.09402	5.17726	-.00036	.00008	.00033	.00033
4	5.23111	5.09442	5.08544	-.00030	.00066	.00150	.00150
5	5.12598	5.12586	5.09063	-.00040	.00040	.00002	.00002
6	5.17366	5.12447	5.09041	-.00051	.00041	.00074	.00074
7	5.16814	5.12387	5.12439	-.00043	.00075	.00015	.00015
8	5.09504	5.12249	5.08244	-.00065	.00050	.00149	.00149
9	4.98205	4.98049	4.98070	-.00049	.00049	.00002	.00002
10	5.09067	5.07270	5.15741	-.00038	.00007	.00038	.00038
11	5.01027	5.03346	5.04318	-.00044	.00054	.00154	.00154
12	5.04720	5.04697	5.09109	-.00053	.00053	.00001	.00001
13	5.08874	5.02860	5.08875	-.00046	.00075	.00031	.00031
14	5.03376	5.01023	5.08518	-.00054	.00054	.00153	.00153
15	4.79457	4.79453	4.79444	-.00052	.00052	.00061	.00061
16	4.79751	4.81620	4.79749	-.00077	.00265	.00135	.00135
17	4.81650	4.81664	4.91666	-.00288	.00288	.00000	.00000
18	4.81632	4.79746	4.79780	-.00265	.00278	.00135	.00135
19	3.04076	3.04071	3.18631	-.00467	.00467	.00001	.00001
20	3.04412	3.01530	3.07694	-.00470	.00470	.00101	.00101
21	2.07849	2.07848	2.07848	-.00454	.00454	.00000	.00000
22	3.03529	3.04809	3.07694	-.00462	.00470	.00018	.00018
23	2.81811	2.81810	2.6924	-.00375	.00374	.00001	.00001
24	2.82284	2.78633	2.7863	-.00361	.00359	.00040	.00040
25	2.76072	2.76092	1.9953	-.00349	.00349	.00000	.00000
26	2.78634	2.82283	2.7882	-.00359	.00361	.00040	.00040
27	2.79117	2.79115	2.10673	-.00318	.00318	.00000	.00000
28	2.13659	2.13659	2.64302	-.00135	.00135	.00000	.00000
29	2.17001	2.13662	2.13695	-.00120	.00120	.00095	.00095
30	2.13515	2.13516	3.9729	-.00132	.00132	.00000	.00000
31	2.13663	2.17501	2.13695	-.00126	.00126	.00090	.00090
32	2.13767	2.13767	2.0917	-.00111	.00111	.00000	.00000
33	1.44537	1.44537	2.87745	-.00197	.00197	.00060	.00060
34	1.44815	1.47693	2.12139	-.00194	.00194	.00155	.00155
35	1.44609	1.44609	6.4935	-.00215	.00215	.00000	.00000
36	1.47643	1.44615	1.47643	-.00194	.00195	.00155	.00155
37	1.45786	1.45786	2.75255	-.00131	.00131	.00000	.00000
38	5.0774	5.0774	1.06746	-.00223	.00223	.00000	.00000
39	5.0758	5.0758	5.0758	-.00187	.00229	.00031	.00031
40	4.9816	5.0193	5.12086	-.00233	.00206	.00199	.00199
41	5.0963	5.0963	3.0729	-.00238	.00186	.00057	.00057
42	5.0576	5.0577	3.9981	-.00241	.00241	.00000	.00000
43	5.0334	5.0963	3.6730	-.00186	.00235	.00037	.00037
44	5.0193	4.9816	4.9816	-.00206	.00233	.00198	.00198
45	5.5436	5.0963	5.5950	-.00229	.00187	.00031	.00031
46	5.0919	5.0919	3.7768	-.00138	.00138	.00000	.00000
47	4.80624	4.80614	4.80614	-.00016	.00016	.00000	.00000
48	2.81823	2.81840	4.44899	-.00375	.00374	.00001	.00001

STEAM - JOINT DEFLECTIONS AND ROTATIONS

LIAR CONDITION NO. 1

APPLIC ACNR STRUCTURE - U.S. NAVY (36-IN. DIAMETER PILING) - J. ATKINSON

JOINT NUMBER	DEFLECTION IN INCHES	X	Y	Z	ROTATION IN RADIALS	X	Y	Z	REMARKS
53	2.01919		2.01918	-.21518	-.00366	-.00366	-.00001	-.00001	
54	2.01267		2.07636	-.00361	-.00359	-.00359	-.00004	-.00004	
55	2.02454		2.07600	-.07511	-.00372	-.00366	-.00001	-.00001	
56	2.06181		2.06000	-.52649	-.00349	-.00349	-.00000	-.00000	
57	2.06129		2.06129	-.10947	-.00349	-.00349	-.00000	-.00000	
58	2.07644		2.01293	-.07816	-.00361	-.00361	-.00004	-.00004	
59	2.08253		2.02253	-.07511	-.00265	-.00265	-.00001	-.00001	
60	2.13531		2.13531	-.70769	-.00135	-.00135	-.00000	-.00000	
61	2.13531		2.13531	-.22247	-.00143	-.00143	-.00001	-.00001	
62	2.14764		2.11442	-.13690	-.00120	-.00120	-.00000	-.00000	
63	2.16251		2.15509	-.06501	-.00151	-.00151	-.00000	-.00000	
64	2.13395		2.13398	-.46060	-.00152	-.00137	-.00000	-.00000	
65	2.13360		2.13361	-.12664	-.00131	-.00131	-.00000	-.00000	
66	2.11465		2.14767	-.13489	-.00120	-.00120	-.00000	-.00000	
67	2.13510		2.10651	-.06501	-.00151	-.00151	-.00000	-.00000	
68	1.46540		1.46543	-.97213	-.00197	-.00197	-.00000	-.00000	
69	1.44930		1.44930	-.23688	-.00162	-.00162	-.00001	-.00001	
70	1.41054		1.43927	-.12119	-.00195	-.00194	-.00160	-.00160	
71	1.45009		1.47247	-.05919	-.00165	-.00173	-.00040	-.00040	
72	1.46717		1.46714	-.75252	-.00215	-.00215	-.00000	-.00000	
73	1.46977		1.46977	-.10721	-.00164	-.00164	-.00000	-.00000	
74	1.41058		1.41058	-.12119	-.00197	-.00197	-.00160	-.00160	
75	1.45609		1.45609	-.05919	-.00173	-.00165	-.00040	-.00040	
76	5.0440		5.0441	-1.17467	-.00223	-.00223	-.00000	-.00000	
77	5.0320		5.0324	-.50305	-.00415	-.00415	-.00001	-.00001	
78	4.9159		4.9159	-.11457	-.00232	-.00206	-.00184	-.00184	
79	4.9393		4.9760	-.04403	-.00434	-.00434	-.00015	-.00015	
80	5.0257		5.0257	-.95570	-.00241	-.00241	-.00000	-.00000	
81	5.0116		5.0116	-.23578	-.00421	-.00421	-.00000	-.00000	
82	4.9549		4.9549	-.11457	-.00206	-.00232	-.00184	-.00184	
83	4.9760		4.9393	-.04403	-.00435	-.00448	-.00015	-.00015	
101	0.00000		0.00000	0.00000	-.00531	-.00531	-.00032	-.00032	
102	0.00000		0.00000	0.00000	-.00497	-.00497	-.00000	-.00000	
103	0.00000		0.00000	0.00000	-.00493	-.00493	-.00001	-.00001	
104	0.00000		0.00000	0.00000	-.00531	-.00531	-.00032	-.00032	

STRAH - JOINT DEFLECTIONS AND ROTATIONS

4-LEAF A&M STRUCTURE - U.S. NAVY (36-IN. DIAMETER PILING) - J. ATKINSON

DEFLECTION IN INCHES / ROTATION IN RADIANS

JOINT NUMBER	X	Y	Z	X	Y	Z	REMARKS
1	.30658	1.02228	-.02499	-.00026	.00004	.00072	
2	.39519	1.02280	-.01379	-.00051	-.00063	.00075	
3	.50436	1.02332	-.01010	-.00008	-.00062	.00126	
4	.5619	1.02380	.00603	.00016	.00015	.00085	
5	.58726	.74926	.02550	.00018	.00012	.00205	
6	.50356	.74894	.00574	.00018	.00075	.00050	
7	.39531	.74843	-.01176	.00003	.00074	.00100	
8	.30708	.74792	-.01277	.00018	.00003	.00056	
9	.30040	.97437	-.02490	.00024	.00002	.00072	
10	.43440	.98309	-.01365	.00002	-.00030	.00108	
11	.56815	.99180	.00608	.00021	.00014	.00085	
12	.57486	.72029	.02360	.00015	.00013	.00200	
13	.43411	.71726	.00451	.00007	.00040	.00055	
14	.30142	.71424	.01265	.00017	.00004	.00055	
15	.29204	.90868	.02575	.00072	.00012	.00068	
16	.52905	.42051	.00673	.00079	.00043	.00070	
17	.53570	.67861	.02376	.00045	.00045	.00196	
18	.28859	.67229	.01203	.00044	.00014	.00053	
19	.20652	.50099	-.02114	.00369	.00019	.00049	
20	.28858	.40446	.00611	.00378	.00045	.00050	
21	.27493	.42547	.02184	.00347	.00052	.00112	
22	.20783	.42503	-.01099	.00051	.00020	.00011	
23	.19517	.46368	-.03001	.00067	.00016	.00044	
24	.26470	.45492	.00987	.00062	.00036	.00024	
25	.25540	.39452	.03087	.00039	.00042	.00100	
26	.19676	.39887	.01390	.00040	.00014	.00009	
27	.22956	.42604	.00057	.00000	.00001	.00010	
28	.10118	.34016	-.07457	.00024	.00012	.00025	
29	.19422	.35627	.01098	.00024	.00013	.00005	
30	.19041	.29951	.06049	.00019	.00019	.00046	
31	.14377	.29599	-.02474	.00024	.00012	.00065	
32	.16696	.31423	.00731	.00018	.00010	.00008	
33	.08857	.21503	.10042	.00037	.00014	.00011	
34	.12110	.22214	.02327	.00034	.00021	.00000	
35	.12338	.18534	.09053	.00029	.00021	.00019	
36	.08894	.18351	.03619	.00032	.00015	.00004	
37	.10013	.20196	-.00081	.00022	.00011	.00006	
38	.02378	.07040	.12138	.00033	.00014	.00007	
39	.03762	.07760	.04746	.00024	.00017	.00002	
40	.04018	.07045	.03036	.00035	.00018	.00000	
41	.04497	.07111	.06947	.00035	.00013	.00002	
42	.04125	.05248	.11206	.00030	.00020	.00011	
43	.02512	.05598	.03471	.00025	.00017	.00002	
44	.02276	.05192	.04274	.00030	.00015	.00003	
45	.02479	.05768	.08264	.00032	.00013	.00005	
46	.03212	.06185	.00734	.00024	.00013	.00002	
47	.40992	.79809	.00360	.00004	-.00001	.00072	
51	.18461	.47429	.04994	.00067	.00016	.00044	



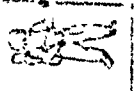
STRAN - JOINT DEFLECTIONS AND ROTATIONS

LOAD CONDITION NO. 2

PILE LCHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

JOINT NUMBER /-----DEFLECTION IN INCHES-----/ /-----ROTATION IN RADIANS-----/ /-----REMARKS-----/

JOINT NUMBER	X	Z	X	Z	Z
52	.19513	-.02382	-.00666	.00014	.00050
53	.25787	.01494	-.00036	.00036	.00029
54	.26501	.00705	-.00062	.00036	-.00027
55	.27449	.05034	-.00039	.00042	.00100
56	.25544	.02451	-.00037	.00043	.00112
57	.19450	-.02001	-.00064	.00018	.00010
58	.15646	-.01172	-.00043	.00018	.00018
59	.13558	-.00405	-.00026	.00012	.00023
60	.14120	-.02711	-.00033	.00008	.00053
61	.19310	.01355	-.00024	.00013	.00004
62	.19407	.00766	-.00022	.00014	.00021
63	.20140	.06960	-.00019	.00019	.00046
64	.19032	.02714	-.00013	.00026	.00111
65	.14318	-.03272	-.00024	.00012	.00002
66	.14372	-.01156	-.00025	.00015	.00007
67	.06589	-.11251	-.00057	.00014	.00610
68	.08905	.02974	-.00034	.00008	.00051
69	.12098	.02653	-.00034	.00021	-.00001
70	.12123	.00750	-.00025	.00014	.00021
71	.12812	.02257	-.00021	.00021	.00019
72	.12572	.02420	-.00014	.00027	.00112
73	.08793	-.06007	-.00032	.00015	.00004
74	.08892	.01192	-.00027	.00013	.00008
75	.02184	-.13273	-.00053	.00014	.00008
76	.02375	-.03658	-.00044	.00013	.00050
77	.04055	.03455	-.00035	.00018	.00001
78	.03900	.01033	-.00060	.00034	-.00027
79	.04365	.12397	-.00030	.00020	.00011
80	.04098	.03060	-.00033	.00044	.00112
81	.02220	-.04628	-.00050	.00015	.00002
82	.02267	.01344	-.00045	.00022	.00010
101	0.00000	0.00000	-.00051	.00024	.00011
102	0.00000	0.00000	-.00059	.00050	.00112
103	0.00000	0.00000	-.00074	.00015	.00049
104	0.00000	0.00000	-.00070	.00041	.00029



STRAN - JOINT DEFLECTIONS AND ROTATIONS

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U-PILE ACHR STRUCTURE - U.S. NAVY (36-IN. DIAMETER PILING) - J. ATKINSON

LOADS CONDITION NO. 3

JOINT NUMBER / DEFLECTION IN INCHES / ROTATION IN RADIANS / REMARKS

JOINT NUMBER	DEFLECTION IN INCHES	ROTATION IN RADIANS	REMARKS
1	.01020	.00012	.00000
2	.00905	.00059	.00001
3	.00735	.00057	.00001
4	.00586	.00009	.00000
5	.00567	.00018	.00000
6	.00734	.00115	.00002
7	.00906	.00117	.00001
8	.01004	.00023	.00000
9	.00825	.00001	.00000
10	.00772	.00001	.00000
11	.00725	.00001	.00000
12	.00678	.00002	.00000
13	.00772	.00001	.00000
14	.00845	.00002	.00000
15	.00671	.00000	.00000
16	.00552	.00002	.00000
17	.00606	.00004	.00000
18	.00639	.00000	.00000
19	.00442	.00001	.00000
20	.00462	.00000	.00000
21	.00270	.00001	.00000
22	.00333	.00002	.00000
23	.00383	.00001	.00000
24	.00386	.00000	.00000
25	.00332	.00001	.00000
26	.00355	.00002	.00000
27	.00350	.00001	.00000
28	.00185	.00000	.00000
29	.00163	.00000	.00000
30	.00197	.00001	.00000
31	.00189	.00000	.00000
32	.00194	.00000	.00000
33	.00068	.00000	.00000
34	.00068	.00000	.00000
35	.00046	.00000	.00000
36	.00049	.00000	.00000
37	.00057	.00000	.00000
38	.00057	.00000	.00000
39	.00059	.00000	.00000
40	.00050	.00000	.00000
41	.00073	.00000	.00000
42	.00048	.00000	.00000
43	.00058	.00000	.00000
44	.00049	.00000	.00000
45	.00071	.00000	.00000
46	.00052	.00000	.00000
47	.00022	.00000	.00000
51	.00385	.00001	.00000

STRAN - JOINT DEFLECTIONS AND ROTATIONS

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LOAD CONDITION NO. 3

4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

JOINT NUMBER / DEFLECTION IN INCHES / X / ROTATION IN RADIANS / Y / REMARKS / Z

JOINT NUMBER	DEFLECTION IN INCHES / X	DEFLECTION IN INCHES / Z	ROTATION IN RADIANS / Y	ROTATION IN RADIANS / Z	REMARKS
52	0.0364	0.0240	0.0001	0.0000	
53	0.0345	0.0170	0.0001	0.0000	
54	0.0393	0.0178	0.0001	0.0000	
55	0.0326	0.0466	0.0001	0.0000	
56	0.0324	0.0494	0.0001	0.0000	
57	0.0352	0.0308	0.0002	0.0000	
58	0.0359	0.0311	0.0002	0.0000	
59	0.0189	0.0110	0.0000	0.0070	
60	0.0187	0.0129	0.0000	0.0000	
61	0.0160	0.0121	0.0000	0.0000	
62	0.0163	0.0089	0.0000	0.0000	
63	0.0197	0.0129	0.0001	0.0001	
64	0.0199	0.0130	0.0000	0.0000	
65	0.0188	0.0094	0.0000	0.0000	
66	0.0191	0.0095	0.0000	0.0000	
67	0.0068	0.0044	0.0000	0.0000	
68	0.0056	0.0040	0.0000	0.0000	
69	0.0068	0.0035	0.0000	0.0000	
70	0.0067	0.0036	0.0000	0.0000	
71	0.0046	0.0031	0.0000	0.0000	
72	0.0046	0.0032	0.0000	0.0000	
73	0.0049	0.0045	0.0000	0.0000	
74	0.0050	0.0044	0.0000	0.0000	
75	0.0054	0.0036	0.0000	0.0000	
76	0.0057	0.0037	0.0000	0.0000	
77	0.0060	0.0029	0.0000	0.0000	
78	0.0060	0.0025	0.0001	0.0000	
79	0.0046	0.0024	0.0000	0.0000	
80	0.0043	0.0021	0.0001	0.0000	
81	0.0044	0.0040	0.0000	0.0000	
82	0.0044	0.0032	0.0000	0.0000	
101	0.0000	0.0000	0.0000	0.0000	
102	0.0000	0.0000	0.0001	0.0000	
103	0.0000	0.0000	0.0000	0.0000	
104	0.0000	0.0000	0.0001	0.0000	



STRAN - JOINT DEFLECTIONS AND ROTATIONS

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DATE 04/30/76

LEAD CONDITION NO. 4

REPORT FOR HYDROGRAPHIC U.S. NAVY (36-IN. DIAPHRAGM PILING) -- J. ATKINSON

JOINT NUMBER	DEFLECTION IN INCHES	ROTATION IN RADIANS	REMARKS
1	5.3993A	6.11450	.00083
2	5.55479	6.11592	.00029
3	5.67855	6.11734	.00044
4	5.71931	6.11876	.00046
5	5.72324	5.87532	.00058
6	5.67722	5.87381	.00070
7	5.55441	5.87231	.00039
8	5.40212	5.87081	.00082
9	5.29289	5.96630	.00072
10	5.52507	5.99578	.00031
11	5.57841	6.02526	.00065
12	5.62206	5.76726	.00068
13	5.52884	5.74586	.00045
14	5.33519	5.72847	.00071
15	5.08661	5.70320	.00324
16	5.32656	5.73671	.00356
17	5.35220	5.49505	.00334
18	5.10492	5.46974	.00310
19	5.25128	5.50571	.00547
20	3.33670	3.50970	.00508
21	3.25341	3.40395	.00501
22	3.22313	3.47311	.00513
23	3.01328	3.28178	.00482
24	3.08754	3.20125	.00423
25	3.01636	3.15744	.00388
26	2.98310	3.22170	.00402
27	3.02072	3.21719	.00318
28	2.27778	2.47675	.00163
29	2.36823	2.47290	.00144
30	2.32557	2.42567	.00151
31	2.28020	2.46600	.00144
32	2.30463	2.45190	.00129
33	1.53378	1.66080	.00234
34	1.56925	1.69907	.00229
35	1.56887	1.65143	.00204
36	1.56587	1.63167	.00226
37	1.56199	1.65082	.00151
38	5.3152	5.7814	.00256
39	1.00520	6.3195	.00211
40	5.3830	5.7238	.00268
41	5.9460	1.03446	.00273
42	5.5825	5.5825	.00261
43	6.0361	6.0361	.00211
44	5.5008	5.5008	.00236
45	5.7915	1.02519	.00261
46	5.4131	5.7104	.00162
47	5.21616	5.60424	.00020
51	3.00280	3.29277	.00442



STRAN - JOINT DEFLECTIONS AND ROTATIONS

UNITED CONDITION NO. 4

PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINS

JOINT NUMBER /-----DEFLECTION IN INCHES-----/ /-----ROTATION IN RADIANS-----/ /-----REMARKS-----/
 X Z X Z

52	3.01432	3.26313	-.00432	-.00380	.00051	
53	3.07071	3.22438	-.00323	-.00395	-.00073	
54	3.00955	3.24321	-.00434	-.00402	-.00026	
55	3.04037	3.13333	-.00368	-.00390	-.00100	
56	3.01673	3.15794	-.00386	-.00392	-.00112	
57	3.07094	3.20957	-.00402	-.00380	-.00053	
58	2.94687	3.22359	-.00408	-.00391	-.00009	
59	2.27059	2.48091	-.00162	-.00146	-.00025	
60	2.27651	2.47530	-.00176	-.00150	-.00053	
61	2.34074	2.44951	-.00140	-.00133	-.00094	
62	2.36258	2.47113	-.00161	-.00165	-.00024	
63	2.33534	2.41329	-.00151	-.00151	-.00044	
64	2.32342	2.42800	-.00148	-.00157	-.00111	
65	2.25763	2.44299	-.00144	-.00132	-.00192	
66	2.27263	2.46840	-.00176	-.00152	-.00038	
67	1.53220	1.66412	-.00234	-.00211	-.00011	
68	1.53794	1.66482	-.00196	-.00171	-.00053	
69	1.53152	1.66132	-.00229	-.00229	-.00215	
70	1.57137	1.70684	-.00191	-.00187	-.00016	
71	1.59524	1.64795	-.00204	-.00236	-.00019	
72	1.59349	1.65546	-.00180	-.00191	-.00111	
73	1.52727	1.59312	-.00226	-.00210	-.00164	
74	1.56734	1.63185	-.00200	-.00178	-.00032	
75	1.57625	1.67625	-.00256	-.00238	-.00008	
76	1.52708	1.57317	-.00480	-.00429	-.00051	
77	1.49194	1.52598	-.00268	-.00274	-.00163	
78	1.53373	1.56754	-.00507	-.00469	-.00042	
79	1.54622	1.55218	-.00271	-.00261	-.00011	
80	1.54215	1.55342	-.00054	-.00465	-.00111	
81	1.47775	1.50286	-.00236	-.00248	-.00186	
82	1.52022	1.54557	-.00480	-.00470	-.00026	
101	0.00000	0.00000	-.00576	-.00555	-.00043	
102	0.00000	0.00000	-.00535	-.00547	-.00111	
103	0.00000	0.00000	-.00567	-.00508	-.00050	
104	0.00000	0.00000	-.00600	-.00560	-.00061	

LOAD CONDITION NO. 5

STRAN - JOINT DEFLECTIONS AND ROTATIONS

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W-PILE ACRH STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

JOINT NUMBER	X	Y	Z	X	Y	Z	REMARKS
1	5.38918	6.10254	2.2627	0.0072	0.0086	0.0076	
2	5.54574	6.10507	2.26601	0.0029	0.0071	0.0120	
3	5.67120	6.10756	2.27003	0.0102	0.0085	0.0130	
4	5.71253	6.11011	2.27404	0.0071	0.0085	0.0254	
5	5.71757	5.86422	1.9263	0.0071	0.0051	0.0204	
6	5.66988	5.86051	1.9284	0.0185	0.0062	0.0028	
7	5.54539	5.85661	2.0206	0.0077	0.0054	0.0116	
8	5.39208	5.85311	2.1116	0.0060	0.0055	0.0185	
9	5.26423	5.84974	2.2008	0.0071	0.0050	0.0075	
10	5.1735	5.84714	2.2825	0.0051	0.0054	0.0147	
11	5.57116	6.01582	2.0065	0.0050	0.0064	0.0239	
12	5.61527	5.75609	1.9969	0.0066	0.0071	0.0200	
13	5.51713	5.75447	1.3744	0.0052	0.0075	0.0025	
14	5.32674	5.71896	1.3129	0.0069	0.0044	0.0184	
15	5.07988	5.49756	1.2150	0.0052	0.0065	0.0069	
16	5.2104	5.71950	2.0642	0.0054	0.0059	0.0205	
17	5.34615	5.48441	1.6642	0.0030	0.0031	0.0175	
18	5.09855	5.46301	1.0686	0.0031	0.0029	0.0169	
19	3.22486	3.52254	2.1580	0.0046	0.0053	0.0050	
20	3.54204	3.50779	0.8194	0.0054	0.0056	0.0044	
21	3.25071	3.40249	0.8911	0.0050	0.0050	0.0112	
22	3.21980	3.46310	1.0059	0.0051	0.0049	0.0079	
23	3.00945	3.27921	0.3640	0.0091	0.0036	0.0045	
24	3.08369	3.23006	0.8105	0.0042	0.0054	0.0069	
25	3.01304	3.15532	1.7526	0.0049	0.0059	0.0088	
26	2.97955	3.21865	1.0526	0.0040	0.0050	0.0049	
27	3.21477	3.21477	1.2105	0.0019	0.0021	0.0010	
28	2.27589	2.47546	0.7213	0.0162	0.0146	0.0025	
29	2.36259	2.47166	1.5743	0.0143	0.0133	0.0099	
30	2.32360	2.47439	4.0296	0.0150	0.0150	0.0066	
31	2.27851	2.46505	1.7933	0.0144	0.0131	0.0067	
32	2.30269	2.45962	2.2820	0.0129	0.0121	0.0008	
33	1.53306	1.65976	0.98747	0.0230	0.0211	0.0071	
34	1.56857	1.65472	1.0957	0.0229	0.0214	0.0055	
35	1.58900	1.65111	0.72536	0.0244	0.0236	0.0019	
36	1.56538	1.63122	1.7013	0.0226	0.0210	0.0159	
37	1.56142	1.65444	0.5946	0.0153	0.0142	0.0006	
38	5.3209	5.7850	1.19424	0.0256	0.0257	0.0007	
39	1.00579	0.5239	0.51758	0.0210	0.0240	0.0034	
40	5.3894	5.7267	1.0189	0.0268	0.0224	0.0199	
41	1.03473	0.5365	0.5365	0.0273	0.0198	0.0039	
42	5.4749	5.5851	0.93720	0.0271	0.0261	0.0011	
43	0.8896	0.6066	3.2034	0.0211	0.0255	0.0035	
44	5.2518	5.5048	1.7616	0.0236	0.0248	0.0021	
45	5.7986	1.02558	0.65319	0.0261	0.0199	0.0029	
46	5.0183	5.7136	0.39711	0.0161	0.0150	0.0002	
47	5.59793	5.59793	1.0579	0.0020	0.0019	0.0072	
51	2.99898	3.29020	0.50759	0.0441	0.0589	0.0044	



STRAN - JOINT DEFLECTIONS AND ROTATIONS

LOAD CONDITION NO. 5

4-PILE APC4 STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

J N O I N T N U M B E R	X	Y	Z	X	Y	Z	R E M A R K S
DEFLECTION IN INCHES				ROTATION IN RADIANS			
52	3.01046	3.28853	-2.0784	-0.0031	0.00379	0.0051	
53	3.00687	3.22225	-0.7393	-0.0023	0.00394	-0.0073	
54	3.08564	3.24106	-0.7684	-0.0035	0.00401	-0.0026	
55	3.03703	3.11124	-3.258	-0.0084	0.00391	0.0100	
56	3.01340	3.15489	-1.1943	-0.00387	0.00393	0.0112	
57	2.90742	3.20649	-1.1024	-0.00401	0.00380	0.0053	
58	2.58134	3.20448	-0.9308	-0.00407	0.00391	0.0009	
59	2.26962	2.47460	-0.0115	-0.0162	0.0146	0.0023	
60	2.27464	2.47401	-2.6325	-0.0174	0.0150	0.0053	
61	2.33913	2.44830	-1.683	-0.0145	0.0133	0.0094	
62	2.36095	2.46968	-0.6826	-0.0161	0.0164	0.0024	
63	2.33338	2.41200	-5.1505	-0.0150	0.0150	0.0046	
64	2.32193	2.42270	-4.117	-0.0145	0.0156	0.011	
65	2.25575	2.44205	-1.8237	-0.0144	0.0131	0.0042	
66	2.27092	2.49545	-0.8576	-0.0174	0.0152	0.0058	
67	1.53162	1.66367	-1.09410	-0.0234	0.0211	0.0011	
68	1.53720	1.66439	-2.7221	-0.0196	0.0176	0.0053	
69	1.53084	1.66097	-1.0607	-0.0220	0.0214	0.0161	
70	1.57065	1.70053	-3.9495	-0.0191	0.0186	0.0018	
71	1.59484	1.64762	-8.0048	-0.0244	0.0236	0.0019	
72	1.53303	1.65514	-1.6615	-0.0161	0.0190	0.0112	
73	1.52673	1.59267	-1.7382	-0.0226	0.0210	0.0164	
74	1.56689	1.63342	-0.7932	-0.0199	0.0178	0.0032	
75	5.2680	5.7661	-1.31664	-0.0258	0.0237	0.0004	
76	5.2757	5.7352	-3.4371	-0.0479	0.0424	0.0051	
77	4.9254	5.2626	-0.9156	-0.0268	0.0224	0.0183	
78	5.330	5.6780	-0.5906	-0.0308	0.0464	0.0042	
79	5.4668	5.5243	1.06468	-0.0271	0.0261	0.0011	
80	5.4257	5.5363	-2.6546	-0.0455	0.0466	0.0111	
81	4.7823	5.0326	-1.7341	-0.0234	0.0247	0.0186	
82	5.2067	5.4596	-0.6372	-0.0441	0.0470	0.0026	
101	0.00000	0.00000	0.00000	-0.0570	0.0556	0.0043	
102	0.00000	0.00000	0.00000	-0.0537	0.0548	0.011	
103	0.00000	0.00000	0.00000	-0.0567	0.0506	0.0050	
104	0.00000	0.00000	0.00000	-0.0601	0.0560	0.0061	



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STRAN - REACTION FORCES AND MOMENTS

LOAD CONDITION NO. 1

4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

JOINT NUMBER	FORCE IN KIPS	MOMENT IN IN-KIPS	M=V	M=Z	REMARKS
	F _{XY}	F _{XZ}	M _{XY}	M _{XZ}	
101	-101.8254	140.5507	0.0000	0.0000	
102	-220.3049	-1129.0108	0.0000	0.0000	
103	-245.7955	1360.1873	0.0000	0.0000	
104	-133.2224	150.9552	0.0000	0.0000	
TOTAL	-701.1529	533.0868	0.0000	0.0000	

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STRAIN - REACTION FORCES AND MOMENTS

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LOAD CONDITION NO. 4

4-PILE CAP STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

JOINT NUMBER	FORCE IN KIPS			MOMENT IN IN-KIPS			REMARKS
	F-X	F-Y	F-Z	M-X	M-Y	M-Z	
101	-99,960	-147,4612	206,3100	0.0000	0.0000	0.0000	
102	-203,6174	-243,2963	-1284,4427	0.0000	0.0000	0.0000	
103	-263,3254	-275,8648	1515,5840	0.0000	0.0000	0.0000	
104	-138,7502	-120,8236	105,7755	0.0000	0.0000	0.0000	
TOTAL	-745,8539	-787,4429	543,2268	0.0000	0.0000	0.0000	

STRAN - REACTION FORCES AND MOMENTS

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LIGHT CONDITION NO. 5
4-PILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

JOINT NUMBER	F-X	F-Y	F-Z	M-X	M-Y	M-Z	REMARKS
101	-98.3692	-149.6298	387.6494	0.0000	0.0000	0.0000	
102	-241.7259	-241.6743	1257.0076	0.0000	0.0000	0.0000	
103	-264.9674	-277.3668	1529.8103	0.0000	0.0000	0.0000	
104	-140.7914	-119.3719	124.2206	0.0000	0.0000	0.0000	
TOTAL	-745.8539	-787.4029	624.2268	0.0000	0.0000	0.0000	

STRAN GROUP SUMMARY REPORT

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MEMBER GROUP ID: W16
FILE ADDR STRUCTURE: U.S. NAVY (36-IN, DIAMETER PILING) -- J. ATKINSON

MEMBER GROUP ID	FIRST		SECOND		THIRD		MEMBER		U. CK		MEMBERS		NUMBER OF MEMBERS IN GROUP	
	NO.	MAX.	LD.	U. CK	NO.	MAX.	LD.	U. CK	NO.	MAX.	LD.	U. CK	GT	LT
W21	5	.91	5	.76	5	.72	5	4	5	.72	5	0	0	3
SSC	19	.90	5	.95	4	.90	5	20	16	.90	5	0	0	4
W17	7	.06	5	.02	5							0	0	2
W16	12	1.75	5	1.34	1	.67	5	11	10	.67	5	2	1	1
OR3	17	1.07	5	.99	4			17	11		4	1	0	0
HR3	17	1.75	4	.90	4	.84	4	17	18	.84	4	1	1	0
XR1	27	.99	4	.83	5	.71	4	17	47	.71	4	0	0	3
JL1	25	2.06	5	1.92	5	1.71	5	24	23	1.71	5	3	6	6
PI	19	1.15	5	1.15	5	1.10	4	50	64	1.10	4	0	4	2
MR1	25	1.07	4	.79	4	.77	4	25	26	.77	4	1	1	4
WRN	34	1.20	5	.79	5	.19	5	79	80	.19	5	0	0	32
DB1	31	1.31	5	1.20	5	.05	5	24	24	.05	5	0	3	0
XR2	32	.82	5	.74	5	.32	5	32	28	.32	5	0	0	4
DR2	35	.43	5	.78	5	.34	5	34	39	.77	5	0	0	0
HR2	39	.65	5	.62	5	.43	5	42	43	.60	5	0	0	4
XR3	37	.31	5	.30	5	.37	5	37	33	.29	5	0	0	8
P2	60	1.01	5	.92	4	.68	5	74	82	.68	5	0	1	0
P3	76	6.61	5	5.64	4	1.49	5	82	101	1.49	5	4	4	0

TOTAL MEMBERS 161

TOTAL MEMBERS 9

TOTAL MEMBERS 25

TOTAL MEMBERS 73

STRAN UNITY CHECK SUMMARY REPORT

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MEMBER NAME: J. J. ATKINSON

THREE HIGHEST UNITY CHECK S.G.T. 1.00

MEMBER GROUP ID

UNITY LD. CHECK CN. LD. CHECK CN.

MEMBER NUMBER	MEMBER GROUP ID	UNITY LD. CHECK CN.	UNITY LD. CHECK CN.	UNITY LD. CHECK CN.
12	11	173	5	72.11
70	104	2.41	4	5.88
60	102	5.64	5	5.01
23	21	2.06	4	1.86
23	19	1.92	4	1.75
17	16	1.75	4	1.11
28	23	1.71	4	1.56
02	101	1.49	4	1.15
14	9	1.38	4	1.32
31	24	1.31	4	1.11
50	24	1.21	4	1.10
20	25	1.20	4	1.11
19	52	1.15	4	1.04
52	60	1.15	4	1.04
78	104	1.15	5	1.03
56	60	1.10	4	.99
23	56	1.10	5	.99
7	14	1.07	4	.94
25	24	1.07	4	.72
24	20	1.05	4	.97
28	24	1.05	4	.88
26	22	1.04	4	.97
29	24	1.04	4	.93
31	26	1.01	4	.93
68	70	1.01	4	.92

STRAN MEMBER STRESS REPORT NO. 1

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4-PILE 4CHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

MEMBER NO.	GROUP ID	MAXIMUM COMBINED UNITY CK	UNITY CHECK COMPONENT VALUES			LOAD COND FROM	DIST FROM	FORCE		CONTROLLING MEMBER ACTIONS		MOMENT		COMBINED LD	
			AXIAL	Y-AXIS	Z-AXIS			FX	FY	IN-KIPS	IN-KIPS	IN-KIPS	IN-KIPS	UNITY CK	UNITY CK
2-	1 W21-01	.621	.037	.374	.110	5	10.0	-9.89	.94	885.90	-27.94	.261	3	.260	4
3-	1 W21-01	.185	.048	.044	.073	5	0.0	-3.79	-.00	103.80	-18.58	.169	4	.164	1
4-	1 SSC-01	.071	.007	.046	.014	5	0.0	-17.30	17.87	-908.60	568.50	.058	4	.044	1
5-	2 W21-01	.265	.037	.166	.061	5	0.0	-9.86	-.01	-393.43	-15.59	.152	3	.115	4
6-	2 W17-01	.162	.028	.001	.033	5	30.0	-.62	-.01	4.56	4.56	.062	4	.052	4
7-	3 W21-01	.724	.037	.274	.404	5	0.0	-9.86	-.94	658.92	103.47	.461	4	.284	3
8-	3 W17-01	.005	.005	.001	.019	5	30.0	1.25	.68	2.54	135.58	.023	4	.020	2
9-	4 W21-01	.076	.107	.031	.538	5	0.0	-5.94	.01	24.52	135.58	.635	4	.512	2
10-	4 SSC-01	.086	.007	.042	.036	5	0.0	-16.22	50.71	971.42	900.04	.064	4	.049	1
11-	6 W21-01	.914	.054	.567	.293	5	0.0	-14.51	.97	1341.76	273.85	.529	3	.389	4
12-	5 SSC-01	.088	.008	.080	.000	5	15.0	-19.52	67.82	-1341.89	75.49	.076	3	.052	4
13-	7 W21-01	.492	.054	.311	.126	5	0.0	-12.50	-.01	-736.07	32.12	.302	4	.191	4
14-	8 W21-01	.768	.054	.618	.092	5	10.0	-14.44	-.99	1462.37	23.45	.522	3	.244	4
15-	8 SSC-01	.096	.069	.047	.000	5	15.0	-20.57	-42.63	1842.38	104.87	.076	3	.061	4
16-	9 W18-01	.553	.304	.168	.553	5	15.0	-73.18	.45	403.46	-13.35	.869	4	.349	1
17-	9 W18-01	1.176	.850	.002	.523	1	0.0	-54.44	.00	4.85	-23.47	1.339	4	1.319	5
18-	9 SSC-01	.440	.667	.451	.321	5	0.0	-157.64	88.15	9939.02	-8383.44	.835	4	.716	1
19-	9 W23-01	.490	.473	.167	.320	4	33.1	160.88	345.11	366.97	2067.81	.990	5	.798	1
20-	9 W18-01	.810	.372	.179	.810	5	53.1	120.82	39.88	322.66	588.35	.810	4	.801	1
21-	10 W18-01	.072	.319	.103	.261	5	0.0	-73.16	-.42	323.87	60.89	.589	4	.320	1
22-	10 W18-01	.032	.001	.000	.030	5	30.0	28	.01	7.87	7.87	.032	4	.018	1
23-	11 W18-01	1.73	.991	.236	1.71	4	48.8	-93.45	380	540.22	327.89	72.111	4	1.840	2
24-	11 SSC-01	.866	.041	.459	.366	5	0.0	-97.40	-532.60	10343.33	-9251.51	.859	4	.712	1
25-	11 W23-01	.071	.420	.003	.548	5	0.0	145.72	12.53	46.19	632.22	.967	4	.938	1
26-	13 W18-01	.990	.204	.238	.448	5	0.0	-48.14	.43	549.48	-37.82	.419	4	.264	3
27-	12 SSC-01	.457	.021	.417	.419	5	0.0	-49.91	71.81	9950.95	-9952.59	.839	4	.720	1
28-	14 W18-01	.524	.204	.273	.047	5	15.0	-48.32	-.40	677.41	12.15	.345	4	.343	1
29-	14 W23-01	1.073	.401	.003	.669	5	0.0	139.03	-56.75	53.48	-771.37	1.071	4	.937	1
30-	14 SSC-01	.793	.043	.402	.348	5	0.0	-101.60	312.19	9236.21	-8588.84	.784	4	.711	1
31-	15 W23-01	.497	.125	.324	.448	4	30.0	-43.33	-10.79	574.77	-676.53	.896	5	.759	1
32-	15 W23-01	.958	.106	.240	.416	1	30.0	-28.14	5.34	455.93	600.34	.744	5	.742	4
33-	15 SSC-01	.054	.077	.512	.369	5	29.5	-170.11	149.43	11300.40	-9589.12	.953	4	.818	1
34-	15 W18-01	.670	.123	.546	.000	4	21.2	-60.00	-29.23	1080.09	32.55	.664	5	.562	1
35-	16 W23-01	1.747	.096	.090	1.562	4	0.0	-25.44	-5.44	442.61	1847.28	1.744	5	1.109	2
36-	16 SSC-01	.901	.015	.496	.390	5	29.5	-32.29	-120.90	11163.54	-9849.28	.893	4	.736	1
37-	16 W21-01	.508	.049	.083	.056	4	21.2	-43.30	-331.21	-69.20	-904.00	.547	5	.307	1
38-	16 W23-01	.845	.123	.194	.524	4	0.0	-32.72	-10.02	-434.27	-707.48	.642	5	.688	1
39-	17 W23-01	.952	.043	.452	.457	4	29.5	105.39	634.20	10740.73	-10845.75	.942	5	.819	1
40-	17 W21-01	.715	.108	.529	.076	4	0.0	-52.67	1.20	-1119.07	-829.90	.712	5	.572	1
41-	18 SSC-01	.822	.024	.430	.368	5	29.5	-52.78	1671.82	9852.83	2917.74	.812	4	.735	1
42-	17 W21-01	.305	.076	.000	.229	1	0.0	-36.81	271.26	-8.68	-453.48	.268	4	.267	5
43-	19 W21-01	1.919	.648	.949	.002	5	0.0	1297.33	317.09	-1216.47	-592.98	1.918	4	1.745	1
44-	20 W21-01	1.149	.506	.040	.149	5	4.5	-1492.24	73	-15873.57	917.34	1.145	4	1.039	1
45-	20 W21-01	1.050	.627	.005	1.019	5	0.0	36.33	2259.19	-13345.14	1.049	4	.965	1	
46-	20 W21-01	.730	.627	.001	.702	5	4.5	-79.32	14.64	619.57	17387.57	.724	4	.658	1
47-	21 W21-01	2.061	.910	1.142	.000	5	0.0	-1230.93	626.87	14933.29	-62.12	2.051	4	1.859	1
48-	20 W21-01	1.101	.452	.649	1.101	4	4.5	1333.74	-.04	16061.11	-178.93	1.096	5	.995	1
49-	22 W21-01	1.042	.089	.000	.950	4	0.0	118.78	-2167.07	67.81	12463.61	1.042	5	.965	1

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UNIT CHECK /-----/ CONTROLLING MEMBER ACTIONS /-----/ /NEXT TWO HIGH CASES- /

MEMBER NO.	GROUP IO	MAXIMUM COMBINED UNITY CK	COMPONENT VALUES			LOAD COND FROM DIST	FORCE FX KIPS	TORSION		MOMENT		MZ IN-KIPS	COMBINED LD UNITY CK	CN	LD	CN
			AXIAL	Y-AXIS	Z-AXIS			IN-KIPS	MX IN-KIPS	MY IN-KIPS	IN-KIPS					
24	58 P1-01	.716	.063	.000	.653	5	4.5	-15.85	-794.27	-16165.43	.709	4	.656	1		
25	23 H1-01	.790	.147	.574	.069	4	30.9	-1.40	907.31	-514.60	.789	5	.681	1		
26	23 H1-01	.702	.109	.408	.044	4	30.9	-11.91	810.88	353.36	.701	5	.681	1		
27	23 X1-01	.987	.207	.779	.001	4	21.8	-27.90	1540.84	66.45	.936	5	.904	1		
28	23 J1-01	1.712	.971	.730	.002	5	30.3	194.24	-9667.22	-509.57	1.711	4	1.556	1		
29	51 H1-01	.016	.015	.000	.001	4	2.8	.72	-7.10	-202.87	.015	5	.014	1		
30	24 H1-01	1.972	.288	.258	.526	4	0.0	-10.58	-671.02	658.71	1.071	5	.724	1		
31	24 H1-01	1.371	.155	.609	.217	4	0.0	-391.63	-1.44	789.03	.369	5	.299	1		
32	24 H1-01	1.050	.547	.336	.117	5	45.4	-175.74	-1534.69	789.01	1.049	4	.861	1		
33	24 J1-01	1.037	.225	.003	.810	5	30.3	450.71	591.40	-10599.20	1.035	4	.934	1		
34	53 H1-01	1.10	.001	.000	.000	4	2.8	-8.44	-7.10	-2386.05	.010	5	.008	1		
35	26 H1-01	.770	.233	.519	.018	4	30.0	-3.29	788.30	-140.65	.766	5	.725	1		
36	27 H1-01	.832	.230	.572	.030	5	0.0	-1.82	-1159.31	-267.52	.832	4	.759	1		
37	25 H1-01	1.202	.711	.265	.227	5	45.4	253.34	-1156.51	1063.81	1.195	4	1.106	1		
38	25 J1-01	1.206	.457	.749	.026	5	39.3	415.30	9786.74	132.03	1.200	4	1.096	1		
39	25 H1-01	1.306	.707	.238	.399	5	45.4	-158.96	-113.09	1572.22	1.300	4	1.106	1		
40	55 H1-01	.005	.005	.000	.000	4	2.8	.36	-7.10	162.22	.005	1	.003	5		
41	27 H1-01	.303	.153	.001	.150	4	21.8	375.02	-21.73	207.23	.301	5	.299	1		
42	26 H1-01	.912	.496	.278	.198	5	45.4	157.12	-1208.16	-869.74	.909	4	.881	1		
43	26 J1-01	1.008	.254	.000	.755	5	39.3	-753.20	35.47	5804.68	1.008	4	.914	1		
44	57 H1-01	.069	.001	.000	.000	4	2.8	7.72	-7.10	2182.80	.009	5	.008	1		
45	28 H1-01	.249	.012	.031	.206	4	38.9	3.43	128.06	-329.86	.249	5	.236	1		
46	28 H2-01	.133	.007	.026	.019	5	28.1	-4.70	118.02	344.85	.254	4	.236	1		
47	28 J1-01	.538	.372	.165	.197	5	50.7	84.28	310.90	56.22	.532	4	.423	1		
48	28 H2-01	.757	.426	.135	.197	5	50.7	20.55	2101.43	188.62	.756	4	.684	1		
49	28 H1-01	.076	.076	.000	.000	4	2.8	-11.87	525.88	683.91	.076	4	.686	1		
50	29 H1-01	.309	.106	.011	.192	4	0.0	.59	-7.10	168.29	.296	5	.295	1		
51	29 H2-01	.319	.118	.005	.106	5	0.0	2.98	71.15	502.21	.318	4	.285	1		
52	29 J1-01	.267	.111	.017	.178	5	31.3	-14.70	-77.05	1916.25	.266	4	.215	1		
53	29 H2-01	.547	.291	.104	.152	5	0.0	290.66	666.45	497.69	.545	4	.511	1		
54	31 H1-01	.353	.123	.010	.220	5	2.8	.06	412.57	2657.20	.352	4	.309	1		
55	32 H2-01	.091	.016	.039	.037	5	0.0	-4.89	70.93	-334.95	.091	4	.076	1		
56	33 J1-01	.598	.497	.101	.621	5	31.3	204.30	-1475.57	-27.28	.595	4	.532	1		
57	33 H1-01	.073	.073	.000	.000	5	0.0	.06	23.67	.06	.073	4	.037	1		
58	32 H2-01	.293	.117	.005	.171	5	26.1	14.11	-77.05	457.88	.292	4	.215	1		
59	31 H2-01	.547	.287	.103	.157	5	0.0	-11.64	413.18	-309.86	.546	4	.511	1		
60	31 J1-01	.230	.078	.021	.131	5	31.3	-340.04	728.58	-1836.00	.229	4	.215	1		
61	65 H1-01	.018	.007	.000	.011	4	2.8	-9.94	-7.10	-2811.60	.018	5	.018	1		
62	33 H2-01	.366	.139	.084	.173	5	43.1	-1.80	370.44	-532.01	.395	4	.367	1		
63	33 H2-01	.369	.116	.070	.163	5	43.1	-3.88	334.97	543.57	.368	4	.362	1		
64	33 H3-01	.288	.079	.208	.000	5	30.5	-7.96	656.01	24.52	.287	4	.267	1		
65	33 J1-01	.108	.006	.098	.000	5	0.0	29.38	-1281.26	-22.32	.103	4	.099	1		
66	33 H2-01	.738	.485	.033	.219	5	41.3	-17.82	230.57	-598.92	.737	4	.677	1		
67	33 H2-01	.694	.438	.029	.227	5	41.3	23.07	219.84	609.88	.692	4	.677	1		
68	67 H1-01	.054	.063	.000	.001	5	2.8	-0.52	-7.10	-106.95	.064	4	.059	1		

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MEMBER NO.	GROUP ID	MAXIMUM COMBINED		LOAD DIST		FORCE		TORSION		MEMBER ACTIONS		MOMENT		COMBINED LD	
		UNITY CK	CM	AXIAL	Y-AXIS	Z-AXIS	NO.	END(FT)	KIPS	IN-KIPS	MX	MY	IN-KIPS	MZ	UNITY CK
34	HR2-01	.269	.073	.006	.189	5	0.0	-31.49	7.02	-88.96	406.49	.268	4	.266	1
34	HR3-01	.307	.042	.012	.203	5	0.0	-55.17	-97.09	-159.85	658.43	.307	4	.290	1
34	HR2-01	.789	.580	.058	.131	5	0.0	-278.81	-70.55	-235.55	-355.75	.767	4	.684	1
40	JL1-01	.063	.405	.013	.046	5	0.0	5.03	250.47	-363.09	-680.05	.063	4	.060	1
41	HR2-01	.649	.405	.118	.086	5	41.3	264.24	-69.61	391.62	333.83	.647	4	.610	1
34	HR2-01	.011	.001	.000	.010	4	2.8	.55	-8.48	-7.10	-2681.27	.011	5	.011	1
35	HR2-01	.285	.071	.005	.209	5	0.0	-30.66	-12.16	-79.36	-533.65	.285	4	.266	1
35	HR3-01	.086	.055	.021	.010	5	0.0	-33.07	-2.00	-70.68	-54.56	.085	4	.068	1
35	HR2-01	.776	.533	.004	.219	5	0.0	-756.22	-14.31	-72.75	559.57	.774	4	.730	1
42	JL1-01	.029	.003	.026	.000	5	0.0	3.37	57.02	341.12	-.25	.028	4	.026	1
34	HR2-01	.782	.577	.003	.252	5	0.0	-253.43	26.39	-66.38	-569.01	.780	4	.730	1
35	HR2-01	.066	.066	.000	.000	5	2.8	-70.77	-.03	-7.10	-6.85	.065	4	.061	1
37	HR3-01	.297	.092	.014	.192	5	30.5	-55.28	92.09	-165.61	623.73	.297	4	.290	1
35	HR2-01	.643	.480	.117	.086	5	41.3	261.05	64.15	149.88	-333.80	.642	4	.610	1
44	JL1-01	.067	.004	.016	.042	5	0.0	-250.50	-249.74	-237.45	352.71	.062	4	.060	1
35	HR2-01	.708	.521	.058	.125	5	0.0	-142.98	-7.04	-339.75	948.59	.702	4	.684	1
36	HR2-01	.013	.003	.000	.010	4	2.8	3.35	9.01	-7.10	2547.29	.013	5	.011	1
38	HR2-01	.645	.241	.045	.309	5	25.0	-142.98	-7.04	-339.75	948.59	.645	4	.610	1
38	HR2-01	.619	.215	.038	.308	5	25.0	-126.68	5.83	313.84	976.42	.618	4	.610	1
46	HR3-01	.266	.077	.059	.000	5	35.4	-5.83	-4.24	814.95	14.14	.265	4	.258	1
46	HR2-01	.205	.202	.001	.200	5	2.8	-217.14	1.38	-7.10	391.32	.203	4	.187	1
46	HR2-01	.546	.267	.091	.189	5	25.0	158.38	17.91	403.30	580.40	.546	4	.510	1
41	HR2-01	.523	.287	.039	.197	5	0.0	-149.56	4.05	-206.94	-597.66	.522	4	.491	1
40	HR3-01	.284	.011	.019	.215	4	0.0	-6.30	-101.15	-207.74	705.61	.244	5	.240	1
40	HR2-01	.032	.004	.000	.028	1	2.8	-3.47	25.72	-7.10	7274.31	.031	5	.031	4
42	HR2-01	.594	.214	.027	.355	5	0.0	126.87	1.98	-250.36	928.17	.593	4	.579	1
42	HR2-01	.603	.212	.024	.366	5	0.0	126.11	-4.80	-245.85	-955.62	.602	4	.579	1
42	HR3-01	.060	.015	.044	.000	1	35.4	10.13	-.00	-139.84	-.00	.059	5	.059	4
42	HR2-01	.193	.193	.000	.000	5	0.0	208.37	.08	39.21	-.00	.192	4	.179	1
43	HR2-01	.512	.243	.047	.189	5	0.0	-147.80	-0.00	-264.17	578.11	.511	4	.491	1
45	HR2-01	.519	.244	.049	.186	5	25.0	145.01	-0.00	394.55	-571.00	.518	4	.510	1
46	HR3-01	.240	.099	.019	.212	1	35.4	-5.58	91.88	-210.01	695.86	.240	4	.240	5
46	HR2-01	.038	.008	.000	.030	4	2.8	-8.56	-27.09	-7.10	-7661.42	.037	5	.032	1
51	HR2-01	.016	.015	.000	.001	4	0.0	-16.48	-.00	7.10	-202.87	.015	5	.014	1
52	HR2-01	.148	.500	.640	.002	5	0.0	-1490.03	.44	-15873.37	917.34	1.145	4	1.038	1
53	HR2-01	.010	.000	.000	.009	4	0.0	-.09	.00	7.10	-2386.07	.010	5	.006	1
54	HR2-01	.752	.629	.001	.702	5	0.0	-79.37	9.27	619.57	17387.57	.725	4	.659	1
55	HR2-01	.005	.005	.000	.000	4	0.0	-5.21	.00	7.10	102.22	.005	1	.003	5
56	HR2-01	.102	.453	.649	.000	4	0.0	1334.51	.02	16061.11	-178.95	1.096	5	.995	1
57	HR2-01	.009	.001	.000	.008	4	0.0	-.86	-.00	7.10	2182.82	.009	5	.008	1
58	HR2-01	.721	.687	.000	.653	5	0.0	-183.58	-8.84	-294.27	-16165.43	.713	4	.659	1
59	HR2-01	.376	.176	.660	.301	4	0.0	31.55	.00	7.10	168.29	.076	5	.070	1
61	HR2-01	.731	.558	.173	.001	5	0.0	-1513.41	.44	4699.20	-296.37	.726	4	.653	1
61	HR2-01	.015	.005	.000	.010	1	0.0	4.92	.00	7.10	2657.31	.015	4	.015	5
62	HR2-01	.253	.034	.001	.218	5	0.0	-91.90	9.27	353.07	-5417.15	.246	4	.246	4
63	HR2-01	.073	.073	.000	.000	5	0.0	-78.83	.00	7.10	-15.77	.073	4	.067	1
64	HR2-01	.648	.452	.195	.000	4	0.0	1333.67	.02	-4837.31	14.72	.640	5	.580	1
65	HR2-01	.018	.007	.000	.011	4	0.0	7.48	-.00	7.10	-2811.61	.018	5	.015	1

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MEMBER NO.	GROUP ID	MAXIMUM COMBINED UNITS	DIST FROM	FORCE		TORSION		MOMENT		EMBER ACTIONS		Z-AXIS SHEAR		LD Y-AXIS SHEAR		LD WLY/RY	KLZ/HZ	NEXT HIGH UN, CK.	LD CN
				KIPS	FX	MY	MZ	IN-KIPS	IN-KIPS	IN-KIPS	IN-KIPS	UNITS	UNITS	UNITS	UNITS				
2	1	W21-01	5	10.0	-9.69	.94	885.90	-27.94	.095	5	.004	5	14.3	69.5	.261	3			
3	1	W21-01	5	0.0	-3.79	.00	103.88	-18.32	.019	5	.001	1	42.9	208.5	.169	4			
4	1	SSC-01	5	0.0	-17.50	17.87	-988.60	568.30	.008	5	.007	5	17.7	17.7	.058	4			
5	2	W17-01	5	0.0	-9.86	-.01	-393.43	-15.59	.008	4	.002	4	14.3	69.5	.152	3			
6	2	W17-01	5	30.0	-.62	-.01	4.56	-.210	.210	5	.000	5	50.0	283.0	.862	4			
7	3	W21-01	5	0.0	-9.86	-.96	658.92	103.47	.083	5	.009	5	14.3	69.5	.661	4			
8	3	W17-01	5	30.0	1.25	.02	.95	2.54	.210	6	.000	5	50.0	283.0	.023	4			
9	4	W21-01	5	0.0	-5.94	.01	771.52	135.68	.018	5	.014	5	47.9	208.5	.635	4			
10	4	SSC-01	5	0.0	-16.22	50.71	971.42	400.84	.011	5	.009	5	17.7	17.7	.084	4			
11	5	W21-01	5	0.0	-14.51	.97	1341.70	703.85	.019	3	.008	5	14.3	69.5	.529	3			
12	5	SSC-01	5	15.0	-19.52	62.82	-1301.69	754.49	.014	3	.011	5	17.7	17.7	.076	3			
13	7	W21-01	5	0.0	-18.50	-.01	-736.07	32.12	.006	4	.003	4	14.3	69.5	.302	3			
14	8	W21-01	5	10.0	-14.44	-.99	1462.37	23.45	.165	5	.003	1	18.1	69.5	.522	3			
15	8	SSC-01	5	15.0	-20.57	-42.03	1462.56	104.87	.013	5	.011	5	17.7	17.7	.076	3			
16	9	W18-01	5	15.0	-73.18	.45	403.96	-14.35	.048	5	.001	5	19.4	86.4	.469	4			
17	9	W18-01	5	0.0	-54.44	.00	4.05	-23.47	.010	1	.001	1	43.7	194.4	1.339	4			
18	9	SSC-01	5	0.0	-157.64	88.15	9949.02	-8383.44	.057	4	.053	4	16.5	16.5	.635	4			
19	9	DR3-01	8	33.1	164.08	-25.11	366.97	-467.81	.080	1	.061	1	61.0	61.0	.090	5			
20	9	DR3-01	8	33.1	124.82	58.46	348.35	348.35	.091	5	.066	5	61.0	61.0	.810	4			
21	10	W18-01	5	0.0	-73.18	-.46	253.27	60.89	.036	5	.003	5	19.4	86.4	.589	4			
22	10	W18-01	5	30.0	-63.48	.01	7.87	7.87	.129	4	.000	4	43.7	194.4	.032	4			
23	11	W18-01	5	30.0	-97.40	.00	560.22	307.49	.131	5	.094	2	43.7	194.4	72.111	4			
24	11	SSC-01	5	0.0	-534.90	10353.33	-9251.51	102.4	.102	4	.078	4	16.5	16.5	.859	4			
25	11	DR3-01	5	0.0	145.72	12.53	46.19	652.72	.091	1	.069	1	61.0	61.0	.967	4			
26	13	W18-01	5	0.0	-48.34	.43	589.88	-37.82	.070	5	.003	5	19.4	86.4	.419	4			
27	12	SSC-01	5	0.0	-49.91	71.81	5936.95	-9942.59	.060	5	.056	5	16.5	16.5	.839	4			
28	14	W18-01	5	15.0	-48.32	-.46	677.61	16.15	.076	5	.001	1	19.4	86.4	.355	4			
29	14	DR3-01	5	0.0	139.03	-56.75	53.48	-771.37	.122	4	.085	4	61.0	61.0	1.071	4			
30	14	SSC-01	5	0.0	-101.60	312.19	9236.21	-8588.84	.079	4	.065	4	16.5	16.5	.784	4			
31	15	W18-01	5	0.0	-43.33	-10.79	574.77	-676.53	.060	5	.053	5	68.5	68.5	.896	5			
32	15	DR3-01	1	30.0	-28.14	3.34	455.93	600.34	.046	1	.044	1	68.5	68.5	.748	5			
33	15	SSC-01	5	29.5	-170.11	149.43	11300.40	-9589.12	.069	5	.062	5	34.8	34.8	.953	4			
34	15	W18-01	4	21.2	-60.00	-29.23	1080.09	32.55	.036	4	.027	4	29.4	29.4	.669	5			
35	16	W18-01	4	0.0	-25.44	-5.44	-442.61	147.28	.119	5	.115	5	68.5	68.5	1.744	5			
36	16	SSC-01	5	29.5	-52.29	-1206.90	11163.54	-489.24	.165	4	.111	4	34.8	34.8	.893	4			
37	18	W18-01	4	21.2	-43.30	-331.21	-69.20	-902.00	.266	4	.14	4	29.4	29.4	.547	5			
38	18	DR3-01	5	0.0	-32.72	-10.02	-438.27	-707.88	.084	4	.047	4	68.5	68.5	.842	5			
39	17	SSC-01	4	29.5	105.39	636.20	10794.73	-1024.75	.135	5	.087	4	34.8	34.8	.982	5			
40	17	W18-01	4	0.0	-52.67	1.20	-1114.07	-426.90	.010	4	.018	4	29.4	29.4	.712	5			
41	18	SSC-01	5	29.5	-52.76	1071.62	9852.63	-9117.74	.147	4	.100	4	34.8	34.8	.812	4			
42	18	W18-01	5	0.0	-54.81	271.26	-8.68	243.48	.235	5	.120	5	29.4	29.4	.268	4			
43	19	W18-01	5	9.0	1297.33	312.09	-12416.47	-592.98	.176	4	.158	4	3.9	3.9	1.918	4			
44	52	W1-01	5	4.5	-1492.24	.73	-15873.57	917.34	.029	5	.028	5	4.4	4.4	1.145	4			
45	20	J11-01	5	0.0	36.33	2259.14	917.53	-13345.14	.379	5	.250	5	3.9	3.9	1.049	4			
46	54	W1-01	5	4.5	-79.52	14.84	619.57	17367.57	.115	1	.080	1	4.4	4.4	.724	4			
47	21	J11-01	5	0.0	-1230.93	626.87	14933.29	-62.12	.259	5	.223	5	3.9	3.9	2.051	4			
48	21	W18-01	4	4.5	1333.74	-.04	16081.11	-178.93	.040	4	.040	4	4.4	4.4	1.096	5			
49	72	J11-01	4	0.0	-118.78	-2167.07	67.61	12463.61	.360	4	.236	4	3.9	3.9	1.642	5			

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PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

MEMBER NO.	GROUP ID	MAXIMUM COMBINED LOAD UNITS CK	DIST FROM COND END (FT)	CONTROLLING MEMBER ACTIONS		MOMENT		SHEAR		LD Y-AXIS UNITS CK	LD X-AXIS UNITS CK	LD RY/RZ HIGH UN. CK.	NEXT HIGH UN. CK.
				FX	FY	MZ	MY	WZ	WX				
22	56 H1-01	.716	5	4.5	515.33	294.27	-16105.83	.115	1	.000	1	4.4	.709
24	23 H1-01	.790	4	30.9	41.22	903.31	-314.60	.027	1	.025	4	70.1	.789
26	23 H1-01	.702	4	30.9	50.06	810.88	553.16	.035	5	.029	5	70.1	.701
27	23 H1-01	.987	4	21.8	108.42	150.84	66.45	.041	4	.033	4	30.4	.986
28	23 J1-01	1.712	5	30.3	1301.33	9667.22	-509.57	.072	4	.050	4	26.0	1.711
23	51 H1-01	.016	4	2.8	14.43	4.72	-202.87	.001	4	.008	2	1.4	.015
25	24 H1-01	1.072	4	0.0	141.72	671.02	958.71	.112	4	.106	4	70.1	1.071
27	24 H1-01	.571	4	0.0	81.43	7.44	428.03	.318	5	.170	5	30.4	.369
28	24 H1-01	1.050	5	45.4	173.79	1330.69	789.01	.087	4	.057	4	59.3	1.049
28	24 H1-01	1.037	5	30.3	301.70	591.40	-10599.20	.140	5	.095	1	26.0	1.035
24	53 H1-01	.010	4	2.8	48.44	7.10	-2386.05	.001	4	.048	5	1.4	.010
25	26 H1-01	.770	4	30.9	104.57	708.38	-144.95	.024	1	.022	1	70.1	.766
25	27 H1-01	.832	5	0.0	121.17	-1159.31	-267.52	.020	5	.019	5	30.4	.832
29	25 H1-01	1.202	5	45.4	472.77	-1156.31	-1049.81	.114	5	.069	5	59.3	1.195
30	25 J1-01	1.206	5	30.3	613.03	978.74	132.03	.098	5	.075	5	26.0	1.200
31	25 H1-01	1.308	5	45.4	-470.11	-1113.09	1572.22	.094	1	.057	1	59.3	1.300
25	55 H1-01	.005	4	2.8	8.21	-7.10	102.22	.001	4	.004	5	1.4	.005
26	27 H1-01	.303	4	21.8	80.24	-21.73	297.73	.302	5	.140	5	30.4	.301
28	26 H1-01	.912	5	45.4	137.02	-1204.19	-49.74	.073	4	.045	4	59.3	.919
31	26 J1-01	1.008	5	30.3	359.94	35.07	9864.08	.140	1	.085	1	26.0	1.008
29	57 H1-01	.009	4	2.8	.86	7.72	2182.80	.001	4	.009	4	1.4	.009
29	28 H1-01	.249	4	36.9	5.36	128.06	-329.86	.017	1	.015	1	85.2	.249
31	28 H1-01	.256	5	36.9	-3.63	4.70	594.85	.018	5	.016	5	85.2	.256
32	28 H2-01	.133	5	26.1	11.54	314.09	36.22	.009	4	.007	4	32.5	.132
33	28 J1-01	.538	5	31.3	44.70	2151.43	184.92	.032	4	.027	4	26.9	.537
34	28 H2-01	.757	5	50.7	259.76	533.72	-645.89	.033	4	.027	4	74.0	.756
36	28 H2-01	.714	5	50.7	221.85	625.88	680.91	.031	1	.026	1	74.0	.712
28	59 H1-01	.076	4	2.8	48.55	-7.10	168.29	.001	1	.007	2	1.4	.076
30	29 H1-01	.309	1	0.0	48.41	71.15	294.82	.015	1	.014	1	85.2	.309
32	29 H2-01	.319	5	0.0	56.07	-77.45	502.21	.026	5	.022	5	32.3	.318
34	29 J1-01	.267	5	31.3	130.87	606.45	1916.25	.058	1	.034	1	26.9	.266
35	29 H2-01	.547	5	0.0	173.03	412.57	497.69	.020	1	.018	1	74.0	.545
29	61 H1-01	.015	1	2.8	4.92	9.39	2657.29	.001	5	.123	5	1.4	.015
30	31 H1-01	.353	9	0.0	56.07	-4.89	70.43	.017	4	.015	4	85.2	.352
30	32 H2-01	.091	5	0.0	4.60	-4.81	134.24	.007	5	.006	5	32.3	.091
35	30 J1-01	.594	5	31.3	621.30	-1475.57	-24.28	.038	5	.027	5	26.9	.595
30	63 H1-01	.075	5	0.0	78.83	.06	.00	.002	5	.001	2	1.4	.073
31	32 H2-01	.293	5	26.1	14.11	73.05	437.88	.024	5	.020	5	32.3	.292
35	31 H2-01	.587	5	0.0	170.67	413.18	-509.88	.024	5	.020	5	70.0	.586
36	31 J1-01	.230	5	31.3	-98.03	728.56	-1830.00	.058	5	.039	5	26.9	.229
31	65 H1-01	.018	4	2.8	-7.49	-7.10	-2111.60	.001	1	.115	5	1.4	.018
34	33 H2-01	.396	5	43.1	-59.74	370.44	-532.01	.015	5	.015	5	77.7	.395
36	33 H2-01	.569	5	43.1	-49.81	334.97	543.57	.017	4	.015	4	77.7	.568
37	33 H1-01	.288	5	30.5	-47.81	-7.96	24.32	.012	4	.010	4	34.1	.287
39	33 J1-01	.104	5	0.0	7.40	-1281.26	-22.32	.018	5	.016	5	30.4	.103
39	33 H2-01	.738	5	41.3	288.07	-17.82	-594.92	.028	1	.022	1	59.3	.737
45	33 H2-01	.694	5	41.3	259.79	219.84	609.88	.029	4	.022	4	59.3	.692
33	67 H1-01	.684	5	2.8	68.57	-7.10	-146.95	.001	5	.006	2	1.4	.684

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G-PILE ACHN STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

CONTROLLING MEMBER ACTIONS

MEMBER NO.	GROUP ID	MAXIMUM COMBINED LOAD FROM UNITY CK COND. END (FT)	DIST FROM FX	FORCE		TORSION		MOMENT		Z-AXIS SHEAR		LD Y-AXIS CN	LD XLY/HY CN	NEXT HIGH UN. CK.
				KIPS	IN-KIPS	IN-KIPS	IN-KIPS	IN-KIPS	IN-KIPS	UN. CK.	UN. CK.			
34	HR2-01	.269	5 0.0	-51.49	7.02	-88.96	0.16	480.49	.015	1	77.7	77.7	.266	4
35	HR2-01	.307	5 0.0	-55.17	-97.09	-159.45	.063	658.45	.040	5	34.1	34.1	.307	4
36	HR2-01	.769	5 0.0	-278.61	-70.33	-235.55	.053	355.75	.053	4	59.3	59.3	.767	4
40	JL1-01	.063	5 0.0	5.03	250.47	680.05	.041	50.4	.056	4	59.3	59.3	.063	4
41	HR2-01	.649	5 41.3	264.74	-69.61	393.83	.056	335.83	.035	4	59.3	59.3	.647	4
54	HR2-01	.011	4 2.8	.55	-9.48	-7681.27	.001	-7681.27	.110	5	1.4	1.4	.011	5
55	HR2-01	.285	5 0.0	-30.66	-12.16	-79.56	.019	533.65	.016	4	77.7	77.7	.285	4
55	HR2-01	.086	5 0.0	-33.07	-2.00	-79.68	.004	54.56	.004	5	34.1	34.1	.085	4
55	HR2-01	.776	5 0.0	-256.22	-14.31	-72.75	.024	559.57	.016	1	59.3	59.3	.774	4
35	JL1-01	.029	5 0.0	3.37	57.02	341.13	.025	30.4	.013	5	30.4	30.4	.028	4
42	HR2-01	.782	5 0.0	-253.45	26.39	-64.34	.028	549.01	.020	4	59.3	59.3	.780	4
35	HR2-01	.066	5 2.8	-76.77	-7.03	-7.10	.001	-8.85	.000	5	1.4	1.4	.065	4
56	HR2-01	.297	5 30.5	-55.28	92.09	-165.61	.060	625.73	.038	5	34.1	34.1	.297	4
57	HR2-01	.643	5 41.3	261.45	69.15	345.80	.055	335.80	.055	4	59.3	59.3	.642	4
44	JL1-01	.062	5 0.0	5.12	-245.74	348.55	.041	640.88	.026	1	30.4	30.4	.062	4
36	HR2-01	.704	5 0.0	-250.50	69.12	-245.74	.053	352.71	.032	4	59.3	59.3	.702	4
36	HR2-01	.013	4 2.8	3.35	9.01	-7.10	.001	2547.29	.104	5	1.4	1.4	.013	5
36	HR2-01	.635	5 25.0	-142.98	-7.04	359.75	.024	966.59	.022	5	42.7	42.7	.685	4
45	HR2-01	.619	5 25.0	-126.68	3.83	313.84	.023	976.42	.021	1	42.7	42.7	.618	4
46	HR2-01	.266	5 35.4	-3.83	-6.24	613.95	.012	14.14	.021	4	40.1	40.1	.265	4
36	HR2-01	.203	5 2.8	-217.14	1.58	7.10	.000	391.32	.016	2	1.4	1.4	.203	4
40	HR2-01	.546	5 25.0	-158.38	17.91	403.30	.024	580.40	.019	5	42.7	42.7	.546	4
41	HR2-01	.523	5 0.0	-149.56	4.05	-266.94	.017	597.66	.016	5	42.7	42.7	.522	4
46	HR2-01	.244	4 0.0	-6.36	-101.15	-207.78	.065	705.61	.041	5	40.1	40.1	.244	5
40	HR2-01	.032	1 2.8	3.87	25.72	-7.10	.001	7274.31	.019	5	1.4	1.4	.031	5
42	HR2-01	.594	5 0.0	126.07	-1.44	-257.56	.020	920.17	.020	1	42.7	42.7	.593	4
42	HR2-01	.603	5 0.0	126.11	-6.30	-245.85	.021	955.62	.021	4	42.7	42.7	.602	4
42	HR2-01	.060	1 35.4	10.13	-9.00	-159.64	.005	9.00	.005	1	40.1	40.1	.059	5
42	HR2-01	.193	5 0.0	208.37	.08	39.21	.002	39.21	.001	5	1.4	1.4	.192	4
43	HR2-01	.512	5 0.0	-147.40	-2.00	-2.00	.018	574.11	.018	5	42.7	42.7	.511	4
44	HR2-01	.519	5 25.0	145.01	-20.00	398.55	.075	571.00	.018	5	42.7	42.7	.518	4
44	HR2-01	.240	1 35.4	5.56	91.68	-210.61	.062	695.86	.039	5	40.1	40.1	.240	4
44	HR2-01	.036	4 2.8	-4.56	-27.09	-7.10	.001	7661.42	.014	5	1.4	1.4	.037	5
51	HR2-01	.016	4 0.0	-16.48	7.10	7.10	.001	202.67	.008	2	1.4	1.4	.015	5
52	HR2-01	1.148	5 0.0	-1490.03	.44	-15873.37	.029	917.54	.029	4	29.6	29.6	1.145	4
53	HR2-01	.010	4 0.0	-8.89	.00	7.10	.001	2486.87	.001	5	1.4	1.4	.010	5
54	HR2-01	.732	5 0.0	-79.37	9.27	612.57	.036	17387.57	.036	5	29.6	29.6	.725	4
55	HR2-01	.005	4 0.0	-5.21	.00	7.10	.001	102.22	.004	5	1.4	1.4	.005	4
56	HR2-01	1.102	4 0.0	1334.51	.02	16061.11	.030	1708.93	.030	5	29.6	29.6	1.096	5
57	HR2-01	.009	4 0.0	-8.86	.00	7.10	.001	2182.88	.009	4	1.4	1.4	.009	5
58	HR2-01	.721	5 0.0	-183.38	-8.84	-244.37	.030	6151.53	.030	5	29.6	29.6	.713	4
59	HR2-01	.076	4 0.0	81.55	.00	7.10	.001	168.29	.007	4	1.4	1.4	.076	5
60	HR2-01	.751	5 0.0	-1513.61	.44	4699.20	.012	296.37	.012	5	30.6	30.6	.726	4
61	HR2-01	.015	1 0.0	4.92	.00	7.10	.001	2657.31	.015	1	1.4	1.4	.015	4
62	HR2-01	.253	5 0.0	-91.90	9.27	353.07	.014	5417.35	.014	5	30.6	30.6	.286	4
63	HR2-01	.073	5 0.0	-78.83	.00	7.10	.001	15.77	.001	2	1.4	1.4	.073	4
64	HR2-01	.648	4 0.0	1331.67	.02	-4837.31	.012	14.72	.012	5	30.6	30.6	.640	5
65	HR2-01	.018	4 0.0	7.48	-9.00	-7.10	.001	2811.61	.015	4	1.4	1.4	.018	5



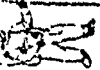
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4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

MEMBER NO.	GROUP ID	MAXIMUM COMBINED LOAD COND. END (FT)	DIST FROM FROM	FORCE		TORSION		MOMENT		Z-AXIS SHEAR		LD V-Axis		LD KLV/RY	KLZ/RZ	HIGH UN. CK.	LD CN
				FX KIPS	FY KIPS	IN-KIPS	IN-KIPS	IN-KIPS	IN-KIPS	UNITY CK	UNITY CK	LD CN	LD CN				
66	74 P1 -01	.281	5 0.0	519.51	59.84	59.28	5150.88	.015	5	.013	5	30.6	30.6			.273	4
67	68 WBN-01	.064	5 0.0	54.57	7.10	7.10	1657.95	.001	4	.006	2	1.4	1.4			.054	4
68	76 P2 -01	1.006	5 35.3	1533.30	.44	1657.57	-878.03	.021	5	.021	5	34.7	34.7			1.001	4
69	78 WBN-01	.011	4 0.0	2.53	.62	7.10	-2681.28	.001	1	.110	5	1.4	1.4			.011	5
70	78 P2 -01	.672	5 35.3	121.12	4.27	52.8.35	-1839.49	.023	5	.023	5	34.7	34.7			.665	4
71	72 WBN-01	.666	5 0.0	77.77	.00	7.10	-8.85	.001	1	.000	5	1.4	1.4			.665	4
72	80 P2 -01	.921	4 35.3	1294.23	.02	16.04	1.75	.020	5	.020	5	34.7	34.7			.916	5
73	74 WBN-01	.013	4 0.0	3.35	.00	7.10	2507.31	.001	5	.104	5	1.4	1.4			.013	5
74	62 P2 -01	.675	5 35.3	-225.38	-8.84	418.29	17577.33	.022	5	.022	5	34.7	34.7			.667	4
75	76 WBN-01	.203	5 0.0	217.18	.00	7.10	391.33	.001	1	.016	2	1.4	1.4			.202	4
76	103 P3 -01	6.615	5 0.0	1561.55	5.20	1657.57	-878.83	.953	5	.051	2	2.2	2.2			6.559	4
77	78 WBN-01	.032	1 0.0	3.87	.00	7.10	774.36	.001	5	.330	5	1.4	1.4			.031	5
78	104 P3 -01	1.148	1 0.0	318.59	-0.00	105.27	-1659.36	.018	2	1.061	5	2.2	2.2			1.107	5
79	80 WBN-01	.194	5 0.0	204.57	.00	7.10	22.36	.001	1	.001	5	1.4	1.4			.195	4
80	102 P3 -01	5.645	4 0.0	1328.04	-0.00	16.04	1.75	.930	5	.003	5	2.2	2.2			5.544	5
81	82 WBN-01	.038	4 0.0	328.56	.00	7.10	7081.47	.001	4	.314	5	1.4	1.4			.037	5
82	101 P3 -01	1.490	5 0.0	-226.55	-0.00	418.29	17577.33	.028	4	1.011	5	2.2	2.2			1.407	4

CONTROLLING MEMBER ACTIONS



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4-PILE ACMR STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. ATKINSON

MEMBER GROUP NO.	ID	MAXIMUM COMBINED UNITS	LOAD CK NO.	DIST FROM END(FT)	AXIAL STRESS KSI	BENDING STRESS Y KSI	Z KSI	SHEAR FORCE		KLZ/RV		HIGHEST LOAD		THIRD-HIGHEST UNITS	
								FV KIPS	FZ KIPS	KLZ/RV	RV	UNITY CHECK	COND	UNITY CHECK	COND
2	1	.521	5	10.0	-2.34	.38		10.31	14.3	69.5		.261	3	.260	4
8	1	.185	5	0.0	-1.58	.08		-2.11	42.9	208.5		.169	4	.164	1
9	1	.071	5	0.0	-1.36	4.17		9.97	17.7	17.7		.058	4	.044	1
3	2	.265	5	0.0	-3.59	-1.33		.24	14.3	69.5		.152	3	.115	4
7	2	.062	5	30.0	.01	.02		8.85	50.0	283.0		.062	4	.052	1
4	3	.724	5	0.0	-6.01	6.01		-9.04	14.3	69.5		.461	4	.264	3
6	3	.024	5	36.0	.11	.40		8.85	50.0	283.0		.023	4	.020	2
5	4	.678	5	0.0	-3.17	.68		-1.93	42.9	208.5		.635	4	.512	2
11	4	.086	5	0.0	-1.14	1.70		-9.06	17.7	17.7		.064	4	.049	1
5	6	.914	5	0.0	-8.90	12.24		-17.59	14.3	69.5		.529	3	.369	4
12	5	.088	5	15.0	-1.17	0.00		-5.09	17.7	17.7		.076	3	.052	4
6	7	.492	5	0.0	-8.90	4.72		-12.72	14.3	69.5		.302	3	.191	4
7	8	.764	5	10.0	-8.89	13.34		18.26	14.3	69.5		.522	3	.240	4
14	8	.096	5	15.0	-1.18	0.00		14.00	17.7	17.7		.076	3	.061	4
10	9	.553	5	15.0	-4.52	4.72		4.71	19.4	86.4		.469	4	.349	1
14	9	1.376	1	0.0	-3.56	.05		-4.8	43.7	194.4		1.339	4	1.319	5
15	9	.840	5	0.0	-1.40	-16.69		-52.08	16.5	16.5		.835	4	.716	1
16	9	.990	4	33.1	10.22	11.17		3.36	61.0	61.0		.990	5	.793	1
15	9	.610	4	33.1	8.03	9.48		3.08	61.0	61.0		.810	4	.801	1
13	10	.672	5	0.0	-4.52	2.58		-5.54	19.4	86.4		.589	4	.320	1
13	10	.032	5	30.0	.02	.01		6.93	43.7	194.4		.032	4	.018	1
12	11	1.75	215	30.0	-5.92	5.70		-12.69	43.7	194.4		1.711	4	1.840	2
16	11	.866	5	0.0	-9.08	-17.82		-45.12	16.5	16.5		.859	4	.712	1
17	11	.971	5	0.0	9.08	10.22		10.22	61.0	61.0		.957	4	.938	1
12	13	.590	5	0.0	-2.98	6.00		5.28	17.7	26.4		.819	4	.268	3
17	12	.857	5	0.0	-4.4	-18.05		-59.22	15.5	18.5		.839	4	.729	1
13	14	.524	5	15.0	-2.08	6.49		7.50	17.4	82.0		.555	4	.343	1
17	14	1.073	5	0.0	6.66	14.52		-10.59	61.0	61.0		1.071	4	.957	1
18	14	.793	5	0.0	-9.0	-16.19		-37.38	16.5	16.5		.784	4	.711	1
16	15	.897	4	30.0	-2.70	-16.67		9.48	68.5	68.5		.896	5	.759	1
16	15	.761	1	30.0	-1.75	-14.16		29.21	68.5	68.5		.744	5	.742	4
19	15	.958	5	29.5	-1.51	-19.02		41.55	34.8	34.8		.953	4	.818	1
47	15	.670	4	21.2	-2.46	-11.81		5.50	29.4	29.4		.669	5	.562	1
17	16	1.747	4	0.0	-1.56	-15.67		-29.89	61.0	61.0		1.704	5	1.100	2
20	16	.901	5	29.5	-1.29	-19.15		30.75	34.8	34.8		.893	4	.734	1
47	16	.508	4	21.2	-1.78	-9.91		11.24	29.4	29.4		.547	5	.307	1
17	18	.845	4	0.0	-2.04	-15.59		-9.25	68.5	68.5		.842	5	.688	1
21	17	.952	4	29.5	.93	19.64		42.27	34.8	34.8		.942	5	.819	1
17	17	.715	4	0.0	-2.16	-13.10		5.55	29.4	29.4		.712	5	.572	1
22	18	.822	5	29.5	-4.47	-17.23		30.09	34.8	34.8		.812	4	.735	1
18	47	.305	1	0.0	-1.51	-4.96		22.24	29.4	29.4		.268	4	.267	5
23	19	1.919	5	0.0	20.91	20.54		12.73	3.9	3.9		1.918	4	1.745	1
19	52	1.149	5	4.5	-10.90	-13.88		10.20	4.4	4.4		1.145	4	1.034	1
24	20	1.050	5	0.0	.59	22.10		106.54	3.9	3.9		1.049	4	.965	1
29	54	.730	5	4.5	-5.8	-15.18		11.22	4.4	4.4		.724	4	.656	1
25	21	2.061	5	0.0	-19.84	-24.68		166.85	3.9	3.9		2.051	4	1.859	1
21	56	1.101	4	4.5	9.77	14.02		203.90	4.4	4.4		1.096	4	.995	1
26	22	1.042	4	0.0	1.91	20.60		25.19	3.9	3.9		1.042	5	.965	1

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PIPE ARCH STRUCTURE -- U.S. NAVY (36-IN. DIAMETER PILING) -- J. AYKINSON

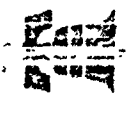
MEMBER NO.	GROUP ID	MAXIMUM COMBINED UNIT CK	LOAD COND NO.	DIST FROM END(FT)	AXIAL STRESS KSI	BENDING STRESS KSI	SHEAR STRESS KSI	FZ KIPS	KLY/RV KLZ/RZ	SECOND-HIGHEST LOAD COND		THIRD-HIGHEST LOAD COND	
										UNITY CHECK	COND	UNITY CHECK	COND
22	58 PI-01	.716	5	4.5	-1.33	-1.33	0.00	2.43	4.4	4.4	.709	4	.656
24	23 MB1-01	.790	4	30.9	4.12	13.08	0.00	4.91	70.1	70.1	.789	5	.681
26	23 MB1-01	.702	4	30.9	2.12	12.10	0.00	4.91	70.1	70.1	.701	5	.681
27	23 MB1-01	.987	4	21.8	4.87	12.85	0.00	6.41	50.4	50.4	.986	5	.906
28	23 JLI-01	1.712	5	30.3	28.97	16.80	0.00	-43.91	26.0	26.0	1.711	4	1.556
23	51 MBN-01	.016	4	2.6	2.33	5.66	0.00	-.48	1.4	1.4	.015	5	.014
25	24 MB1-01	1.072	4	0.0	6.21	16.93	0.00	3.07	70.1	70.1	1.071	5	.724
27	24 MB1-01	1.072	4	0.0	6.21	16.93	0.00	3.07	70.1	70.1	1.071	5	.724
28	24 MB1-01	1.050	5	45.4	13.14	4.68	0.00	-7.25	59.3	59.3	1.049	4	.881
29	24 JLI-01	1.037	5	30.3	4.86	17.58	0.00	-6.89	59.3	59.3	1.035	4	.934
24	53 MBN-01	.010	4	2.8	5.22	11.40	0.00	7.03	1.4	1.4	.010	5	.008
25	26 MB1-01	.770	4	38.9	5.22	11.40	0.00	4.76	70.1	70.1	.766	5	.725
25	27 MB1-01	.832	5	0.0	4.08	13.00	0.00	0.01	30.4	30.4	.832	4	.739
29	25 MB1-01	1.206	5	45.4	-12.43	-8.82	0.00	9.44	59.3	59.3	1.195	4	1.106
30	25 JLI-01	1.206	5	30.3	-9.88	-16.17	0.00	7.39	26.0	26.0	1.200	4	1.096
31	25 MB1-01	1.306	5	45.4	-12.36	-10.76	0.00	-10.76	59.3	59.3	1.300	4	1.106
25	55 MBN-01	.303	4	2.8	3.10	5.00	0.00	-.44	1.4	1.4	.301	5	.299
26	27 MB1-01	.912	5	45.4	-8.68	-3.26	0.00	6.01	30.4	30.4	.909	4	.841
31	26 JLI-01	1.008	5	30.3	5.48	16.36	0.00	7.6	26.0	26.0	1.008	4	.934
26	57 MBN-01	.009	4	2.8	0.02	0.00	0.00	-.42	1.4	1.4	.008	5	.008
29	28 MB1-01	.289	4	36.9	.25	5.12	0.00	1.24	85.2	85.2	.289	5	.236
31	28 MB1-01	.256	5	36.9	.17	-5.27	0.00	1.24	85.2	85.2	.256	4	.236
32	28 MB2-01	.133	5	26.1	-.13	-2.73	0.00	2.38	32.3	32.3	.132	4	.123
33	28 JLI-01	.538	5	31.3	8.03	3.59	0.00	-.43	26.9	26.9	.537	4	.492
34	28 MB2-01	.757	5	50.7	9.19	7.16	0.00	4.49	74.0	74.0	.756	4	.688
36	28 MB2-01	.714	5	50.7	8.07	7.35	0.00	4.49	74.0	74.0	.712	4	.668
28	59 MBN-01	.076	4	2.8	-1.03	-.00	0.00	-.13	1.4	1.4	.076	5	.070
30	29 MB1-01	.309	1	0.0	2.28	4.39	0.00	-1.02	85.2	85.2	.296	5	.295
32	29 MB2-01	.319	4	0.0	2.55	4.34	0.00	-.13	32.3	32.3	.318	4	.285
33	29 JLI-01	.267	5	31.3	-2.24	-3.35	0.00	3.95	26.9	26.9	.266	4	.215
35	29 MB2-01	.547	5	0.0	6.29	5.52	0.00	4.46	74.0	74.0	.545	4	.511
39	61 MBN-01	.015	1	2.8	-.10	-.00	0.00	-.40	1.4	1.4	.015	4	.015
30	31 MB1-01	.353	5	0.0	2.06	4.95	0.00	-1.01	85.2	85.2	.352	4	.309
30	32 MB2-01	.091	5	0.0	.35	1.63	0.00	-1.71	32.3	32.3	.091	4	.076
35	30 JLI-01	.598	5	31.3	-10.01	-2.44	0.00	5.04	26.9	26.9	.595	4	.532
30	63 MBN-01	.073	5	0.0	1.58	-.00	0.00	-1.12	1.4	1.4	.073	4	.067
31	32 MB2-01	.291	5	26.1	2.51	3.80	0.00	-.10	32.3	32.3	.292	4	.285
35	31 MB2-01	.547	5	0.0	6.21	5.61	0.00	4.45	74.0	74.0	.546	4	.511
36	31 JLI-01	.230	5	31.3	-1.58	-3.26	0.00	4.05	26.9	26.9	.229	4	.215
33	65 MBN-01	.018	4	2.8	-.15	-.00	0.00	-.39	1.4	1.4	.018	5	.015
34	33 MB2-01	.396	5	43.1	-2.17	-5.54	0.00	2.47	77.7	77.7	.395	4	.362
36	33 MB2-01	.369	5	43.1	-1.81	-5.46	0.00	2.38	77.7	77.7	.368	4	.362
37	33 MB3-01	.288	5	30.5	-1.56	-4.51	0.00	3.74	30.4	30.4	.287	4	.267
38	33 JLI-01	.104	5	0.0	.12	2.12	0.00	13.14	30.4	30.4	.103	4	.099
39	33 MB2-01	.738	5	41.3	10.28	5.45	0.00	3.05	59.3	59.3	.737	4	.677
35	33 MB2-01	.69	5	41.3	9.45	5.54	0.00	3.04	59.3	59.3	.692	4	.677
33	67 MBN-01	.064	5	2.8	1.37	-.00	0.00	-.66	1.4	1.4	.064	4	.059

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4-PILE ACPH STRUCTURE -- U.S. NAVY (36-IN. DIAPHYER PILING) -- J. ATKINSON

PILER NO.	GROUP ID	MAXIMUM CURBING UNIT	LOAD COND NO.	DIST FROM END(FT)	AXIAL STRESS KSI	BENDING STRESS Y KSI	Z KSI	SHEAR FORCE / FZ KIPS	KLY/RY KLZ/RZ	SECUND-HIGHEST		IMRD-HIGHEST	
										UNITY CHECK	LOAD COND	UNITY CHECK	LOAD COND
35	34 HR2-01	.269	5	0.0	-1.15	-4.23	0.00	4.63	77.7	77.7	.268	4	.266
36	34 HR3-01	.307	5	0.0	-1.40	-4.65	0.00	7.49	34.1	34.1	.307	4	.290
37	34 HR2-01	.749	5	0.0	-10.10	-3.65	0.00	3.72	59.3	59.3	.747	4	.684
38	34 JLI-01	.063	5	0.0	.08	1.27	0.00	-10.90	30.4	30.4	.063	4	.060
39	34 HR2-01	.649	5	41.3	9.61	4.40	0.00	3.65	59.3	59.3	.647	4	.610
40	34 HRN-01	.011	4	2.6	.01	-.00	-.22	79.00	1.4	1.4	.011	5	.011
41	34 HR2-01	.285	5	0.0	-1.12	-4.73	0.00	4.73	77.7	77.7	.285	4	.266
42	34 HR3-01	.046	5	0.0	-1.08	-.66	0.00	-.21	34.1	34.1	.046	4	.068
43	41 HR2-01	.776	5	0.0	-9.32	-4.82	0.00	4.83	59.3	59.3	.774	4	.730
44	35 JLI-01	.029	5	0.0	.05	.56	0.00	-.01	30.4	30.4	.028	4	.026
45	45 HR2-01	.782	5	0.0	-9.22	-5.07	0.00	4.69	59.3	59.3	.780	4	.730
46	71 HRN-01	.066	5	2.8	-1.42	-.00	-.00	.26	1.4	1.4	.065	4	.061
47	37 HR3-01	.297	5	30.5	-1.60	-4.63	0.00	7.25	34.1	34.1	.297	4	.290
48	36 HR2-01	.643	5	41.5	9.51	4.54	0.00	3.65	59.3	59.3	.642	4	.610
49	36 JLI-01	.062	5	0.0	.08	1.26	0.00	10.74	30.4	30.4	.062	4	.060
50	45 HR2-01	.704	5	0.0	-9.11	-3.64	0.00	5.71	59.3	59.3	.702	4	.684
51	73 HRN-01	.013	4	2.8	.07	-.00	.21	-75.05	1.4	1.4	.013	5	.011
52	38 HR2-01	.645	5	25.0	-5.20	-4.74	0.00	7.57	42.7	42.7	.645	4	.610
53	38 HR2-01	.610	5	25.0	-4.61	-6.76	0.00	7.59	42.7	42.7	.614	4	.610
54	38 HR3-01	.265	5	35.4	-1.12	-5.54	0.00	-.04	40.1	40.1	.265	4	.258
55	75 HRN-01	.203	5	2.6	-4.34	-.00	.03	-11.53	1.0	1.0	.203	4	.187
56	39 HR2-01	.546	5	25.0	5.76	6.04	0.00	-.95	42.7	42.7	.546	4	.510
57	40 HR2-01	.523	5	0.0	-5.44	-5.59	0.00	-1.10	42.7	42.7	.522	4	.491
58	40 HR3-01	.244	4	0.0	-.21	-5.04	0.00	7.26	40.1	40.1	.244	5	.240
59	77 HRN-01	.032	1	2.5	-.08	-.00	.61	-210.32	1.4	1.4	.031	5	.031
60	41 HR2-01	.594	5	0.0	4.62	8.22	0.00	7.12	42.7	42.7	.593	4	.579
61	43 HR2-01	.603	5	0.0	4.59	8.43	0.00	7.16	42.7	42.7	.602	4	.579
62	46 HR3-01	.060	1	35.4	.53	.96	0.00	.00	40.1	40.1	.059	5	.059
63	79 HRN-01	.193	5	0.0	4.17	.01	.00	4.88	34.1	34.1	.192	4	.179
64	44 HR2-01	.512	5	0.0	-5.43	-5.43	0.00	.54	42.7	42.7	.511	4	.491
65	45 HR2-01	.519	5	25.0	5.24	5.93	0.00	.91	42.7	42.7	.514	4	.510
66	46 HR3-01	.250	1	35.4	-.14	-4.99	0.00	-.71	40.1	40.1	.240	4	.240
67	81 HRN-01	.034	4	12.6	-.17	-.00	-.04	225.73	1.4	1.4	.037	5	.032
68	52 HRN-01	.016	4	0.0	-.33	-.00	-.02	55.99	1.4	1.4	.015	5	.014
69	80 PI-01	1.148	5	0.0	-10.92	-13.88	0.00	3.35	29.6	29.6	1.145	4	1.038
70	54 HRN-01	.010	4	0.0	-.02	-.00	-.20	-78.30	1.4	1.4	.010	5	.008
71	62 PI-01	.732	5	0.0	4.58	-15.16	0.00	62.72	29.6	29.6	.725	4	.659
72	56 HRN-01	.005	4	0.0	9.10	14.02	0.00	3.01	1.4	1.4	.005	1	.003
73	56 HRN-01	1.102	4	0.0	9.74	14.02	0.00	2.53	29.6	29.6	1.094	5	.995
74	56 HRN-01	.009	4	0.0	-.02	-.00	.18	64.31	1.4	1.4	.009	5	.008
75	66 PI-01	.721	5	0.0	-1.34	-14.11	0.00	50.57	29.6	29.6	.713	4	.659
76	60 HRN-01	.076	4	0.0	1.63	-.00	.01	4.96	1.4	1.4	.076	5	.070
77	68 PI-01	.731	5	0.0	-11.69	-8.51	0.00	-1.57	30.6	30.6	.726	4	.653
78	62 HRN-01	.015	1	0.0	.10	-.00	.22	75.29	1.4	1.4	.015	4	.015
79	70 PI-01	.253	5	0.0	-1.67	-4.74	0.00	-25.71	30.6	30.6	.246	4	.240
80	64 HRN-01	.073	5	0.0	-1.58	-.00	-.00	-.25	1.4	1.4	.073	4	.067
81	72 PI-01	.048	4	0.0	9.77	4.22	0.00	-.08	30.6	30.6	.040	5	.040
82	66 HRN-01	.018	4	0.0	.15	-.00	-.23	-82.64	1.4	1.4	.018	5	.015



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4-PILE ACHR STRUCTURE -- U.S. NAVY (36-IN. DIAPETER PILING) -- J. ATKINSON

MEMBER NO.	GROUP ID	MAXIMUM COMBINED UNITS	LOAD COND NO.	DIST FROM END(FT)	AXIAL STRESS		BENDING STRESS		FZ KIPS	FY KIPS	KLVRY KLZ/RZ	SECOND-HIGHEST LOAD COND		THIRD-HIGHEST LOAD COND	
					Y KSI	Z KSI	Y KSI	Z KSI				CHECK	UNITY	CHECK	UNITY
65	74 P1 -01	.261	5	0.0	-1.34	0.00	0.00	24.33	-5.48	30.6	30.6	.275	4	.240	1
67	68 W8N-01	.064	5	0.0	-1.37	0.00	0.01	-4.53	-.42	1.4	1.4	.064	4	.059	1
69	76 P2 -01	1.006	5	15.3	-12.34	-12.34	0.00	2.74	48.84	34.7	34.7	1.001	4	.916	1
70	70 W8N-01	.011	4	0.0	0.01	0.02	0.02	-79.00	-.42	1.4	1.4	.011	5	.011	1
71	72 W8N-01	.066	5	35.3	-1.79	0.00	0.00	53.07	1.68	34.7	34.7	.065	4	.619	1
72	80 P2 -01	.421	4	35.3	7.34	11.93	0.00	-.37	-44.60	34.7	34.7	.914	5	.459	1
73	74 W8N-01	.013	4	0.0	0.07	0.06	0.21	75.05	-.42	1.4	1.4	.015	5	.011	1
74	82 P2 -01	.675	5	15.3	-1.34	-13.04	0.04	-50.90	3.03	34.7	34.7	.667	4	.619	1
75	78 W8N-01	.203	5	0.0	0.34	0.00	0.03	11.53	-.42	1.4	1.4	.202	4	.187	1
76	103 P3 -01	6.615	5	0.0	-129.05	12.28	-0.65	-8.77	-165.27	2.2	2.2	6.559	4	5.879	1
77	78 W8N-01	.032	1	0.0	0.08	0.00	0.61	214.32	-.42	1.4	1.4	.031	5	.031	4
78	104 P3 -01	1.144	1	0.0	-13.36	0.00	0.00	-160.20	-1.55	2.2	2.2	1.147	5	1.056	4
79	80 W8N-01	.194	5	0.0	0.17	0.00	0.00	.66	-.42	1.4	1.4	.193	4	.160	1
80	102 P3 -01	5.645	4	0.0	109.07	11.63	-2.03	-2.57	159.96	2.2	2.2	5.584	5	4.997	1
81	82 W8N-01	.038	4	0.0	0.17	0.00	0.04	-225.73	-.42	1.4	1.4	.037	5	.032	1
82	101 P3 -01	1.400	5	0.0	-18.82	0.31	13.05	175.36	-4.46	2.2	2.2	1.407	4	1.148	1

SYRAN

13.25.28.88C				
13.25.29.910P				
13.25.26158*	6 440	25,010	17104	2459,
13.25.29371	35	2		
13.25.29.CUST.				
13.25.294FL	61040	25.710		
	SERVICE UNITS*	182.2		
13.25.30.	JOB COSTS	45.55		
13.25.303PL	12288	25.032		5
13.25.30.FXIT.				
13.25.303JTD	2368	25.033		
13.25.30.*PL*	*CPU SEC, *DISC PRUS* DISC ACC			
13.25.30.*P.F.	*PRUS*P.F, ACC *TAPE PRUS* TAPE ACC			